

Stormwater Management Report

Jersey Shore University Medical Center Parking Garage, Perioperative Expansion – East, & Critical Care Tower – West

Block 1201 Lots 1, 2, 4, & 5 1945 Corlies Avenue & 81 Davis Avenue Township of Neptune Monmouth County, NJ 07753

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Prepared for: **Jersey Shore University Medical Center** 1945 State Highway Route 33 Neptune, NJ 07753

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1. Project Summary

The subject site is located at 1945 Corlies Avenue, 19 Davis Avenue, & 81 Davis Avenue, Neptune Township, Monmouth County, New Jersey, also known as Block 1201 Lots 1, 2, 4, & 5 per a certified list of property owners provided by the Township's tax assessor (State Coord. (X)- 620519.45, (Y)- 501487.52). The subject properties are approximately 34.59± acres and have frontage to State Highway Route 33 to the South and Davis Avenue to the East, municipal and commercial buildings to the East, residential properties to the West, and school buildings and recreational fields to the North.

The existing site consists of the Jersey Shore University Medical Center campus which is made up of various medical buildings, associated parking and utility structures, and maintained landscaping. The site contains approximately 26.1± acres of impervious cover total. Based on a review of aerial imagery, the current improvements appear to have been completed sometime between 2017 and 2018 with the construction of Hope Tower on Lot 4. There is 15' wide drainage easement for a municipal storm culvert which enters the site from the West and ultimately discharges to Route 33 at the South of the campus.

An environmental review was conducted on the hospital campus by Dewberry Engineers, Inc., which identified no regulated environmental areas on the campus. There are no streams located within the immediate area of the hospital campus. Based on FEMA's National Flood Hazard Layer Viewer (FIRM panel 34025C0333G, 6/15/2022), JSUMC campus and adjacent parcels are not located within the 100-year floodplain. Runoff is directed to the hospital campus storm sewer system and directed offsite.

The applicant is seeking Preliminary and Final Site Plan Approval for a new parking garage, Central Utility Plant (CUP) expansion, and 4-story perioperative expansion (Phase 2). In general, the existing site features related to the medical office building in Lot 2 are proposed to be removed, including the building, surface parking, lighting, and select utilities. In addition, the perioperative expansion will be constructed above the existing loading dock on Lot 1 and maintain select utilities including existing drainage network. In addition, the applicant is seeking a Preliminary Site Plan Approval to construct a new 11-story critical care tower in Lot 1 and intends to submit a Final Site Plan Application in the future (Phase 3). In general, the existing site features related to the existing hospital wing "Rosa" and surrounding features are proposed to be removed, including the building, surface parking, lighting, and select utilities.

The stormwater management approach for this project allows for both the East and West additions to be analyzed independently of each other. The goal of this report is to show compliances of both Phase 2 and 3 with the Township's ordinance on Stormwater Management through hydrologic and hydrographic comparisons of the existing and full-buildout conditions of the site. Using this method, there will be a reduction to the peak stormwater flow and volume discharging from the site due to each addition at any point in time during the duration of the storm

As part of the proposed project, the applicant proposes a series of temporary enabling projects (Phase 1) to prepare the campus for these improvements. For the East Addition, a temporary loading dock and modified drop off area is proposed as the perioperative expansion (East Addition) is constructed.. At the conclusion of Phase 2 of this project, the temporary loading dock will be demolished and restored to its original, predevelopment, condition. Finally, within Neptune City parcel Block 115 Lot 2, the existing parking lot is proposed to be reconfigured to allow additional parking spaces for overflow hospital parking and will decrease the existing impervious coverage on this parcel while maintaining the existing drainage patterns. A formal application to the Borough of Neptune City will be submitted for the proposed improvements.

The proposed improvements are consistent with the permitted uses of the Neptune Township Civic Zone district. The proposed improvements are located within the limits of the existing disturbed/maintained areas. Access to the hospital campus will continue to be provided via Davis Ave and NJ Route 33 (Corlies Ave). This project, as



currently proposed, will result in a reduction in the overall and vehicular impervious cover and maintain existing drainage patterns. Where practical, portions of previously disturbed/maintained land areas within these study areas will be enhanced by planting dense native, non-invasive herbaceous and woody species. Refer to the associated Preliminary and Final Major Site Plan submitted in support of this application for additional information.

All elevations herein are based on the North American Vertical Datum of 1988 (NAVD 88) unless otherwise noted. For this site, to convert from NAVD88 to NGVD29, add approximately 1.177 feet. This property is located within the State Metropolitan Planning Area (PA-1).

2. East Addition and Garage (Phase 2) Design Overview

This section includes an analysis of the proposed development's effects on local stormwater drainage patterns as well as an overview of the stormwater BMP green infrastructure designs for the East Addition for JSUMC.. Dewberry has prepared this report in accordance with the requirements of the New Jersey Department of Environmental Protection (NJDEP) N.J.A.C. 7:8 for Stormwater Management, the NJDEP Stormwater Best Management Practices (BMP) Manual, the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) National Engineering Handbook Part 630 for Hydrology (NEH630), and the Township of Neptune's ordinance § LDO – 528 Stormwater Management. Drainage maps have been prepared herein and a grading plan has been developed for the proposed site improvements with consideration to match the existing drainage patterns to the maximum extent practical. The existing conditions are based upon available survey and historic aerial information. Refer to the associated site plans for more details on this project.

Two points of analysis (POA) were utilized for the proposed addition, which are designated as DA-1 for the eastern portion of the proposed east addition, and DA-2 for the western portion of the proposed east addition.

Drainage area DA-1 consists primarily of vehicular surface area, it is generally flat and slopes south. Runoff is collected through a series of drainage catch basins located throughout the site and ultimately conveyed via underground storm piping to a 3'x10' box culvert that exits the site towards Route 33 to the South. Refer to the aerial and site figures attachments for additional information.

Drainage area DA-2 consists primarily of vehicular surface area and generally slopes from North to South with a POS in Davis Avenue where the runoff flows along the gutter line. Refer to the aerial and site figures attachments for additional information

The proposed development will result in the disturbance of more than one acre of land since February 2, 2004; therefore, this project is classified as a "major development" as defined in N.J.A.C. 7:8-1.2. Per N.J.A.C. 7:8-5, stormwater management measures for major developments shall be designed to address the design and performance standards for erosion control, groundwater recharge, stormwater runoff quantity control, and stormwater runoff quality treatment. The motor vehicle surface areas associated with these improvements will decrease by 0.66± acres and the overall impervious surface areas for the site will decrease by 0.27± acres, as compared from existing to proposed conditions.

This project addresses these through the following:

- Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction (proposed) runoff hydrographs for the 2-, 10-, and 100-year storm events do not exceed, at any point in time, the pre-construction (existing) runoff hydrographs for the same storm events [N.J.A.C. 7:8-5.6(b)1];
- Stormwater runoff quality standards are not applicable when the major development increases motor vehicle surface by less than a one-quarter acre [N.J.A.C. 7:8-5.5(a)];



- Stormwater recharge requirements do not apply to projects within the "urban redevelopment area" [N.J.A.C. 7:8-5.4(b)2;
- Design and performance standards for erosion control per the Soil Erosion and Sediment Control Act [N.J.S.A. 4:24-39 et seq.].

2.1 Existing Conditions

2.1.1 Land Cover

The proposed drainage area is currently developed and actively maintained. It mainly consists of a 1-story medical office building and a loading dock area for the main hospital campus along with associated vehicular parking area, driveways, pedestrian paths, and landscaped areas. Under existing conditions, there are 6.40± acres of impervious cover, including 4.48± acres of vehicular impervious cover. Additionally, there are 1.69± acres of pervious, vegetated cover. The vegetated cover primarily consists of maintained lawn and landscape areas. Refer to DA-01A East Existing Drainage Plan in Appendix IV for additional information.

2.1.2 Hydrologic Conditions

The existing conditions were analyzed as two separate drainage areas. The drainage area "EDA-1" is located along the eastern portion of the east addition and drainage area and "EDA-2" is located along the western portion of the east addition:

- EDA-1: This area is approximately 6.99± acres which includes 5.59± acres of impervious surface with a weighted CN of 98.00 and 1.40± acres of pervious surface with weighted CN of 54.35. The stormwater runoff in this drainage area primarily consists of impervious vehicular surface coverage which is collected into storm drains and discharged from the site by an existing 3'x10' box culvert. For current storm events, a time of concentration of 0.11 hours has been calculated for impervious surfaces and 0.20 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.11 hours has been calculated for impervious surfaces and 0.19 hours for pervious surfaces.
- EDA-2: This area is approximately 1.10± acres which includes 0.81± acres of impervious surface with a weighted CN of 98.00 and 0.29± acres of pervious surface with weighted CN of 50.62. The stormwater runoff in this drainage area is overland flows that flow from the north of the medical office building towards the south to Davis Ave to the POA at the edge of the site. For the current storm events, a time of concentration of 0.05 hours has been calculated for impervious surfaces and 0.09 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.05 hours has been calculated for impervious surfaces and 0.08 hours for pervious surfaces.

2.1.3 Upland Drainage

Based on topographic information and aerial photography, there are portions of the surrounding area that convey stormwater runoff through the analyzed drainage areas. In addition to the onsite runoff from the hospital campus, the municipal culvert conveys runoff from the surrounding residential and school properties to the North and West of the campus. These offsite drainage areas are outside the area of analysis for this report and are not proposed to be impacted due to the addition.

2.2 Proposed Conditions

2.2.1 Land Cover

The East Addition proposes a new parking garage, CUP expansion, and a 4-story perioperative expansion to the hospital building. The proposed improvements will include associated driveways, pedestrian paths, landscaped areas. Under proposed conditions there will be 6.12± acres of impervious surface, including 3.81± acres of vehicular impervious surface, and 1.97± acres of pervious, vegetated cover. The proposed improvements will result in an overall reduction of impervious surfaces by 0.28± acres. It also includes a reduction of motor vehicle surface by 0.67± acres compared to existing conditions.



The proposed stormwater management design serves to match the existing drainage patterns to the maximum extent practical when compared to existing conditions in accordance with N.J.A.C 7:8. As part of the proposed site improvements, low impact development was considered to mitigate the rate of stormwater runoff flows and volume. Additionally, the proposed design maintains the existing stormwater management devices that are onsite by matching the area flowing into the underground detention basin and matching or eliminating the vehicular surface area conveyed to onsite water quality structures. Refer to the DA-02A East Proposed Drainage Area Plan in Appendix IV for land cover identification, time of concentration (ToC) flow paths, and the POA locations.

2.2.2 Hydrologic Conditions

Similar to the existing conditions, the proposed conditions were analyzed as two separate drainage areas. The drainage area "PDA-1" is in the eastern portion of the east addition drainage area and "PDA-2" is located in the western portion of the east addition drainage area:

- PDA-1: This area is approximately 7.00± acres which includes 5.58± acres of impervious surface with a weighted CN of 98.00 and 1.42± acres of pervious surface with weighted CN of 48.03. For the POA, the stormwater runoff in this drainage area will be collected into storm drains and discharged from the site by the existing 3'x10' box culvert. For current storm events, a time of concentration of 0.11 hours has been calculated for impervious surfaces and 0.19 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.11 hours has been calculated for impervious surfaces and 0.18 hours for pervious surfaces.
- PDA-2: This area is approximately 1.09± acres which includes 0.54± acres of impervious surface with a weighted CN of 98.00 and 0.55± acres of pervious surface with weighted CN of 50.09. The stormwater runoff in this drainage area is overland flows that flow from the north of the medical office building towards the south to Davis Ave to the POA at the edge of the site. For the current storm events, a time of concentration of 0.07 hours has been calculated for impervious surfaces and 0.10 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.07 hours has been calculated for impervious surfaces and 0.09 hours for pervious surfaces

2.2.3 Offsite Drainage

The proposed stormwater management approach has been designed to minimize the impact to the drainage characteristics of the subject property, receiving municipal system and the neighboring region. The project as proposed will have no adverse impact on the existing offsite drainage systems through the reduction of post construction stormwater runoff rates and peak volumes compared to the existing conditions for all the design storms. In addition, the proposed improvements will reduce the amount of motor vehicle surfaces which will improve water quality discharging from the site while also maintaining the existing water quality devices within the project limits.

2.3 Design Methodology

2.3.1 Calculation Methods

Hydrologic scenarios are modeled via Bentley PondPack computer software utilizing NRCS (Unit Hydrograph) methodology. The 2-, 10-, and 100-year storm events are based upon NOAA 24-hour rainfall frequency data for Neptune, New Jersey utilizing the NOAA Region D rainfall distribution and these values were adjusted based on the requirements of N.J.A.C. 7:8-5.7(c) and (d) as shown in Table 2.1 below. Runoff hydrographs have been generated using the SCS Dimensionless Unit Hydrograph, and pervious and impervious catchment areas have been modeled separately [N.J.A.C. 7:8-5.7(a)4.].



Table 2.1 Design Storm Precipitation Depths

DESIGN STORM PRECIPITATION DEPTHS						
STORM EVENT DEPTH CURRENT ADJUSTMENT FUTURE ADJUSTM (24 HOUR) (NOAA ATLAS 14) (PER N.J.A.C 7:8 TABLE 5-5) (PER N.J.A.C. 7:8 TABLE						
2 year	3.48"	3.48" x 1.00 = 3.48 "	3.48" x 1.19 = 4.14"			
10 year	5.39"	5.39" x 1.01 = 5.44 "	5.39" x 1.19 = 6.41"			
100 year	9.22"	9.22" x 1.02 = 9.40 "	9.22" x 1.26 = 11.62"			

2.3.2 Hydrologic Soil Groups (HSG) and Curve Numbers (CN)

Soil classifications for use in establishing runoff curve numbers (CN) for each of the drainage areas have been determined based on data provided by the NRCS Web Soil Survey:

Table 2.2: On-site Hydrologic Soil Groups

ON-SITE HYDROLOGIC SOIL RATINGS					
SOIL TYPE MAP UNIT NAME HSG RATING					
KkhB	Klej loamy sand-Urban land complex, 0 to 5 percent slopes	A/D			
UdauB	Udorthents-Urban land complex, 0 to 8 percent slopes	D			

Soil boring test at the project site found groundwater at a depth of 7 to 10 feet. These results were compared to historical geotechnical reports conducted on the site during the months of January to April, which corroborated the soil findings and found the SHWT at depths of 8 to 12 feet. Per these reports, the seasonal highwater table, SHWT, has consistently been found to be greater than 24 inches below the ground surface. Per the NJDEP Stormwater BMP Manual Chapter 12 "Soil Testing Criteria", dual hydrologic soil group, HSG, classes are classified as HSG "D" soil when the SHWT is within 24 inches of the ground surface. Thus, as part of this site development, the KkhB soil was modeled as HSG 'A' soil.

Runoff CN values for developing the unit hydrographs, per NEH 630 Chapter 9, are as follows:

Table 2.3: CN Values

CN VALUES					
GROUND COVERAGE CONDITION	CN VALUE				
Buildings/Paved Areas – All Soils	98				
Gravel – Good Conditions – A Soils	76				
Gravel – Good Conditions – D Soils	91				
Brush – Good Conditions – A Soils	30				
Brush – Good Conditions – D Soils	73				
Open Space – Good Conditions – A Soils	39				
Open Space – Good Conditions – D Soils	80				



The existing and proposed gravel surfaces within the drainage area are made up of uncompacted landscape stones in non-vehicular areas. These surfaces are either behind an elevated curb or bollards preventing vehicular traffic from affecting these areas.

2.3.3 Stormwater Conveyance

Manning's Equation was used to study the proposed pipe capacities and discharges based on the current 25-year storm event for the site. Inlet drainage areas for overland flow runoff have been identified for the new stormwater drainage system located along Davis Ave to the south of the proposed garage. These sub-catchment areas include the total contributory area and the impervious coverage percentage. Refer to Appendix II for SewerGEMS FlexTables and Pipe Profiles.

Channel Surface	Manning's 'n'
Reinforced Concrete	0.013

The proposed subsurface stormwater conveyance structures are reinforced concrete pipes (RCP) that will convey runoff from the site and buildings to existing stormwater infrastructure. The existing municipal culvert discharges into the system located in Route 33 to the south will not be modified nor will receive additional flow from the project.

Furthermore, the proposed design maintains the existing stormwater management devices that are onsite by matching the area flowing into the underground detention basin and matching or eliminating the vehicular surface area conveyed to onsite water quality structures. The stormwater conveyance model compares the existing, surveyed, drainage network to the proposed drainage area of the loading dock to demonstrated continued capacity of the system.

EAST ADDITION DA-1 OXYGEN TANKS STORMWATER CONVEYANCE							
	PIPE DESCRIPTION DRAINAGE AREA (ACRES) IMPERVIOUS FLOW (CFS) CAPACITY						
EXISTING	27LF 24" RCP @1.5%	0.79	61.7	3.72	27.88		
PROPOSED	27LF 24" RCP @1.5%	0.79	65.0	4.25	27.88		

EAST ADDITION DA-1 LOADING DOCK STORMWATER CONVEYANCE						
PIPE DESCRIPTION DRAINAGE AREA (ACRES) IMPERVIOUS FLOW (CFS) CAPACITY						
EXISTING	36LF 42" RCP @1.0%	10.27	89.3	53.25	99.20	
PROPOSED	36LF 42" RCP @1.0%	10.01	90.1	53.66	99.20	

2.3.4 Time of Concentrations

Time of concentrations (ToC) to the POS have been calculated for all pervious and impervious catchment areas under existing and proposed conditions. Equations and assumptions are in accordance with NEH 630 Chapter 15 and the BMP Manual Chapter 5. The maximum Manning's Roughness Coefficient (n) for sheet flow is 0.40 and the maximum sheet flow length is 100 feet [BMP Chapter 5].



Sheet Flow Surface	Manning's 'n'
Asphalt/Rooftops/Gravel	0.011
Grass	0.15
Dense Grass	0.24

Refer to Appendix II for ToC calculations. Refer to Appendix IV for the Existing Drainage Area Map and Proposed Drainage Area Map that includes the hydraulic flow paths and groundcover types.

2.3.5 Water Quantity Requirements

Hydrological evaluations for existing and proposed conditions have been calculated for the NOAA 2-, 10-, and 100-year 24-hour storm events for Neptune, NJ utilizing the 'Current' and 'Future' precipitation adjustment factors [N.J.A.C. 7:8-5.7(c) and (d), respectively]. See Table 2.1 for precipitation values.

The east addition's stormwater quantity control standards have been addressed by demonstrating through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction (proposed) runoff hydrographs for the 2-, 10-, and 100-year storm events, as defined and determined pursuant to N.J.A.C. 7:8-5.7(c) and (d), respectively, do not exceed, at any point in time, the pre-construction (existing) runoff hydrographs for the same storm events [N.J.A.C. 7:8-5.6(b)1.].

Runoff quantity requirements are achieved through land cover management – restoring previously developed areas with native landscaping material, matching existing drainage patterns and reducing the overall impervious coverage of the site. Refer to Appendix II for the complete PondPack modeling summaries, runoff hydrographs in graphical and numerical form, and master network summary for the East Addition.

Table 2.4 East Peak Runoff Rates and Volumes to POS-1 - Current Storm

	EAST PEAK RUNOFF RATES AND VOLUMES TO POS-1 - CURRENT STORM						
	Existing Proposed Runoff Runoff Rate (CFS) (CFS) Net Runoff Runoff (AC-FT) Resisting Proposed Volume Volume (AC-FT)						
2 year	10.42	10.40	-0.02	1.497	1.474	-0.023	
10 year	16.78	16.45	-0.33	2.475	2.425	-0.050	
100 year	30.45	29.85	-0.60	4.548	4.452	-0.096	



Table 2.5 East Peak Runoff Rates and Volumes to POS-1 - Future Storm

EAST PEAK RUNOFF RATES AND VOLUMES TO POS-1 -FUTURE STORM						
	Existing Runoff Rate Rate (CFS) Residue (CFS) Res					
2 year	12.48	12.44	-0.04	1.820	1.792	-0.028
10 year	20.11	19.70	-0.41	2.973	2.917	-0.056
100 year	38.42	37.68	-0.74	5.741	5.636	-0.105

Table 2.6 East Peak Runoff Rates and Volumes to POS-2 - Current Storm

EAST PEAK RUNOFF RATES AND VOLUMES TO POS-2 - CURRENT STORM							
	Existing Runoff Rate (CFS)	Proposed Runoff Rate (CFS)	Net Runoff Rate (CFS)	Existing Volume (AC-FT)	Proposed Volume (AC-FT)	Net Runoff Volume (AC-FT)	
2 year	1.74	1.13	-0.61	0.216	0.150	-0.066	
10 year	2.82	1.90	-0.92	0.360	0.265	-0.095	
100 year	5.27	4.01	-1.26	0.672	0.537	-0.135	

Table 2.7 East Peak Runoff Rates and Volumes to POS-2 - Future Storm

EAST PEAK RUNOFF RATES AND VOLUMES TO POS-2 - FUTURE STORM							
	Existing Runoff Rate (CFS)	Proposed Runoff Rate (CFS)	Net Runoff Rate (CFS)	Existing Volume (AC-FT)	Proposed Volume (AC-FT)	Net Runoff Volume (AC-FT)	
2 year	2.08	1.35	-0.73	0.264	0.187	-0.077	
10 year	3.41	2.40	-1.01	0.434	0.328	-0.106	
100 year	6.74	5.36	-1.38	0.853	0.702	-0.151	

2.3.6 Runoff Quality and Groundwater Recharge Requirements

Per N.J.A.C 7:8-5.4, groundwater recharge standards do not apply to projects within the "urban redevelopment area". The proposed project site was previously developed and is located within the State Metropolitan Planning Area (PA-1); therefore, the proposed development does not include any proposed groundwater recharge methods. Furthermore, per N.J.A.C. 7:8-5.4(b).1.ii, this project meets the standards for groundwater recharge by reducing the stormwater runoff volume from pre-construction to post-construction for the future 2-year storm event.



Per N.J.A.C 7:8-5.5(a), stormwater runoff quality standards are applicable when projects propose an increase of 0.25 acres or more of regulated motor vehicle surface. The proposed development will result in a decrease of motor vehicle surface by 0.65± acres. Furthermore, previously approved existing onsite water quality devices will be maintained and reset as necessary to maintain their current flows. The areas draining to these existing water quality units will be maintained to the relocated units thereby maintaining the same level of treatment as the approved condition. When you consider the reduction in vehicle surface and maintenance of the existing water quality devices, the project will result in a net improvement in water quality.

2.3.7 Nonstructural Stormwater Management Strategies

Since this project is a major development, it has been designed to incorporate nonstructural stormwater management strategies [NJAC 7:8-2.4(g); BMP Chapter 2]. The following nonstructural strategies for major developments have been considered in the design:

- 1. Protect areas that provide water quality benefits or areas particularly susceptible to erosion and sediment loss;
- 2. Minimize impervious surfaces and break up or disconnect the flow of runoff over impervious surfaces;
- 3. Maximize the protection of natural drainage features and vegetation;
- 4. Minimize the decrease in the "time of concentration" from pre-construction (existing) to post-construction (proposed):
- 5. Minimize land disturbance including clearing and grading;
- 6. Minimize soil compaction;
- 7. Provide low maintenance landscaping that encourages retention and planting of native vegetation and minimizes the use of lawns, fertilizers, and pesticides;

The major goals for this redevelopment have been the reduction of the impervious (overall and vehicular) area from existing to proposed conditions and the restoration of actively disturbed lawn areas to its native, non-invasive vegetated cover. Additionally, other pervious surfaces within the site will be landscaped with shade trees, evergreens, shrubs, and groundcover.

3. West Addition Design (Phase 3) Overview

This section includes a preliminary analysis of the proposed development's effects on local stormwater drainage patterns as well as an overview of the stormwater BMP green infrastructure designs for the West Addition for JSUMC. Dewberry has prepared this report in accordance with the requirements of the New Jersey Department of Environmental Protection (NJDEP) N.J.A.C. 7:8 for Stormwater Management, the NJDEP Stormwater Best Management Practices (BMP) Manual, the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) National Engineering Handbook Part 630 for Hydrology (NEH630), and the Township of Neptune's ordinance § LDO – 528 Stormwater Management. Drainage maps have been prepared herein and a grading plan has been developed for the proposed site improvements with consideration to match the existing drainage patterns to the maximum extent practical. The existing conditions are based upon available survey and historic aerial information. Refer to the associated site plans for more details on this project.

Two points of analysis (POA) were utilized for the proposed addition, which are designated as DA-1 for the southern portion of the proposed West Addition, and DA-2 for the northern portion of the proposed West Addition.

Drainage area DA-1 consists primarily of vehicular surface area, it is generally flat and slopes south. Runoff is collected through a series of drainage catch basins located throughout the site and ultimately conveyed via underground storm piping towards Route 33 to the South. Refer to the aerial and site figures attachments for additional information.

Drainage area DA-2 consists primarily of vehicular surface area and building coverage, it is generally flat and slopes east. Runoff is collected through a series of drainage catch basins located throughout the site and



ultimately conveyed via underground storm piping to an 18" RCP that runs underneath the main hospital building towards the loading dock at the east of the campus. Except for a small overhang area of the proposed West Addition (1,800± SF/0.04± acres), runoff from the proposed addition will be collected and discharged to the proposed collection system. The small amount of runoff from the overhang will drain to the gravel landscaped area below the overhang. This will allow this runoff to sheet flow over the landscape stone to the proposed landscaped area and ultimately be collected by area inlet within the planted area.

The proposed development will result in the disturbance of more than one acre of land since February 2, 2004; therefore, this project is classified as a "major development" as defined in N.J.A.C. 7:8-1.2. Per N.J.A.C. 7:8-5, stormwater management measures for major developments shall be designed to address the design and performance standards for erosion control, groundwater recharge, stormwater runoff quantity control, and stormwater runoff quality treatment. The motor vehicle surface areas associated with these improvements will decrease by 0.78± acres and the overall impervious surface areas for the site will decrease by 0.29± acres, as compared from existing to proposed conditions.

This project addresses these through the following:

- Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction (proposed) runoff hydrographs for the 2-, 10-, and 100-year storm events do not exceed, at any point in time, the pre-construction (existing) runoff hydrographs for the same storm events [N.J.A.C. 7:8-5.6(b)1];
- Stormwater runoff quality standards are not applicable when the major development increases motor vehicle surface by less than a one-quarter acre [N.J.A.C. 7:8-5.5(a)];
- Stormwater recharge requirements do not apply to projects within the "urban redevelopment area" [N.J.A.C. 7:8-5.4(b)2;
- Design and performance standards for erosion control per the Soil Erosion and Sediment Control Act [N.J.S.A. 4:24-39 et seq.].

3.1 Existing Conditions

3.1.1 Land Cover

The proposed West Addition project area is currently developed and mainly consists of the "Rosa" hospital building and associated vehicular parking surface, driveways, pedestrian paths, and landscaped areas. Under existing conditions, there are 3.27± acres of impervious cover, including 2.44± acres of vehicular impervious cover. Additionally, there are 1.47± acres of pervious, vegetated cover. The vegetated cover consists of maintained lawn and landscape areas. Refer to DA-01B West Existing Site Drainage Map for additional information.

3.1.2 Hydrologic Conditions

The existing conditions were analyzed as two separate drainage areas. The drainage area "EDA-1" is located along the southern portion of the side and drainage area "EDA-2" is located along the northern portion of the site:

- EDA-1: This area is approximately 3.22± acres which includes 2.17± acres of impervious surface with a weighted CN of 98.00 and 0.88± acres of pervious surface with weighted CN of 66.20. The stormwater runoff in this drainage area are primarily overland flows which is collected into storm drains and discharged from the site towards Route 33. For current storm events, a time of concentration of 0.08 hours has been calculated for impervious surfaces and 0.13 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.08 hours has been calculated for impervious surfaces and 0.12 hours for pervious surfaces.
- **EDA-2**: This area is approximately 1.51± acres which includes 0.93± acres of impervious surface with a weighted CN of 98.00 and 0.59± acres of pervious surface with weighted CN of 69.99. The stormwater



runoff in this drainage area primarily consists of impervious vehicular and building cover which is collected into storm drains and discharged towards the east of the site by an existing 18" reinforced concrete pipe. For current storm events, a time of concentration of 0.05 hours has been calculated for impervious surfaces and 0.18 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.05 hours has been calculated for impervious surfaces and 0.17 hours for pervious surfaces.

3.1.3 Upland Drainage

Based on topographic information and aerial photography, there are portions of the surrounding area that convey stormwater runoff through the analyzed drainage areas. In addition to the onsite runoff from the hospital campus, the municipal culvert conveys runoff from the surrounding residential and school properties to the North and West of the campus. These offsite drainage areas are outside the area of analysis for this report and are not proposed to be impacted due to the addition.

3.2 Proposed Conditions

3.2.1 Land Cover

The West Addition preliminarily proposes a new 11-story critical care tower. The proposed improvements will include associated driveways, surface parking, and pedestrian paths. Under proposed conditions there will be 2.98± acres of impervious surface, including 1.66± acres of vehicular impervious surface, and 1.75± acres of pervious, vegetated cover. The proposed improvements will result in an overall reduction of impervious surfaces by 0.29± acres. It also includes a reduction of motor vehicle surface by 0.78± acres compared to existing conditions.

The proposed stormwater management design serves to match the existing drainage patterns to the maximum extent practical when compared to existing conditions in accordance with N.J.A.C 7:8. As part of the proposed site improvements, low impact development was considered to mitigate the rate of stormwater runoff flows and volume. Additionally, the proposed design maintains the existing stormwater management devices that are onsite by matching the area flowing into the underground detention basin and matching or eliminating the vehicular surface area conveyed to onsite water quality structures. Refer to the DA-02B West Proposed Drainage Area Plan in Appendix IV for land cover identification, time of concentration (ToC) flow paths, and the POA locations.

3.2.2 Hydrologic Conditions

Similar to the existing conditions, the proposed conditions were analyzed as two separate drainage areas. The drainage area "PDA-1" is located along the southern portion of the studied drainage area "PDA-2" is located along the northern portion of the studied drainage area:

- PDA-1: This area is approximately 3.25± acres which includes 2.06± acres of impervious surface with a weighted CN of 98.00 and 1.19± acres of pervious surface with weighted CN of 65.12. For the POA, the stormwater runoff in this drainage area will be collected into storm drains and conveyed south towards Route 33. For current storm events, a time of concentration of 0.08 hours has been calculated for impervious surfaces and 0.13 hours for pervious surfaces. For the projected future storm events, a time of concentration of 0.08 hours has been calculated for impervious surfaces and 0.12 hours for pervious surfaces.
- PDA-2: This area is approximately 1.48± acres which includes 0.92± acres of impervious surface with a weighted CN of 98.00 and 0.56± acres of pervious surface with weighted CN of 70.96. The stormwater runoff in this drainage area primarily consists of impervious vehicular and building cover which is collected into storm drains and discharged towards the east of the site by an existing 18" reinforced concrete pipe. For current storm events, a time of concentration of 0.05 hours has been calculated for impervious surfaces and 0.17 hours for pervious surfaces. For the projected future storm events, a time of



concentration of 0.05 hours has been calculated for impervious surfaces and 0.16 hours for pervious surfaces.

3.2.3 Offsite Drainage

The proposed stormwater management approach has been designed to minimize the impact to the drainage characteristics of the subject property, receiving municipal system and the neighboring region. The project as proposed will have no adverse impact on the existing offsite drainage systems through the reduction of post construction stormwater runoff rates and peak volumes compared to the existing conditions for all the design storms. In addition, the proposed improvements will reduce the amount of motor vehicle surfaces which will improve water quality discharging from the site while also maintaining the existing water quality devices within the project limits.

3.3 Design Methodology

3.3.1 Calculation Methods

Hydrologic scenarios are modeled via Bentley PondPack computer software utilizing NRCS (Unit Hydrograph) methodology. The 2-, 10-, and 100-year storm events are based upon NOAA 24-hour rainfall frequency data for Neptune, NJ utilizing the NOAA Region D rainfall distribution and these values were adjusted based on the requirements of N.J.A.C. 7:8-5.7(c) and (d) as shown in Table 3.1. Runoff hydrographs have been generated using the SCS Dimensionless Unit Hydrograph, and pervious and impervious catchment areas have been modeled separately [N.J.A.C. 7:8-5.7(a)4.].

Table 3.1 Design Storm Precipitation Depths

DESIGN STORM PRECIPITATION DEPTHS							
STORM EVENT (24 HOUR)	DEPTH (NOAA ATLAS 14)	CURRENT ADJUSTMENT (PER N.J.A.C 7:8 TABLE 5-5)	FUTURE ADJUSTMENT (PER N.J.A.C. 7:8 TABLE 5-6)				
2 year	3.48"	3.48" x 1.00 = 3.48"	3.48" x 1.19 = 4.14 "				
10 year	5.39"	5.39" x 1.01 = 5.44 "	5.39" x 1.19 = 6.41 "				
100 year	9.22"	9.22" x 1.02 = 9.40"	9.22" x 1.26 = 11.62 "				

3.3.2 Hydrologic Soil Groups (HSG) and Curve Numbers (CN)

Soil classifications for use in establishing runoff curve numbers (CN) for each of the drainage areas have been determined based on data provided by the NRCS Web Soil Survey:

Table 3.2: On-site Hydrologic Soil Groups

ON-SITE HYDROLOGIC SOIL RATINGS					
SOIL TYPE	MAP UNIT NAME	HSG RATING			
KkhB	Klej loamy sand-Urban land complex, 0 to 5 percent slopes	A/D			
UdauB	Udorthents-Urban land complex, 0 to 8 percent slopes	D			

Soil boring test at the project site found groundwater at a depth of 7 to 10 feet. These results were compared to historical geotechnical reports conducted on the site during the months of January to April, which corroborated the soil findings and found the SHWT at depths of 8 to 12 feet. Per these reports, the seasonal highwater table, SHWT, has consistently been found to be greater than 24 inches below the ground surface. Per the NJDEP



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Stormwater BMP Manual Chapter 12 "Soil Testing Criteria", dual hydrologic soil group, HSG, classes are classified as HSG "D" soil when the SHWT is within 24 inches of the ground surface. Thus, as part of this site development, the KkhB soil was modeled as HSG 'A' soil.

Runoff CN values for developing unit hydrographs, per NEH 630 Chapter 9, are as follows:

Table 3.3: CN Values

CN VALUES					
GROUND COVERAGE CONDITION	CN VALUE				
Buildings/Paved Areas – All Soils	98				
Gravel – Good Conditions – A Soils	76				
Gravel – Good Conditions – D Soils	91				
Brush – Good Conditions – A Soils	30				
Brush – Good Conditions – D Soils	73				
Open Space – Good Conditions – A Soils	39				
Open Space – Good Conditions – D Soils	80				

The existing and proposed gravel surfaces within the drainage area are made up of uncompacted landscape stones in non-vehicular areas. These surfaces are either behind an elevated curb or bollards preventing vehicular traffic from affecting these areas.

3.3.3 Stormwater Conveyance

Manning's Equation was used to study the proposed pipe capacities and discharges based on the current 25-year storm event for the site. Preliminary drainage areas for overland flow runoff have been identified for the proposed stormwater drainage system, these catchment areas include the total contributory area and the impervious coverage percentage. Refer to Appendix III for SewerGEMS FlexTables and Pipe Profiles.

Channel Surface	<u>Manning's 'n'</u>
Reinforced Concrete	0.013

The proposed subsurface stormwater conveyance structures are reinforced concrete pipes (RCP) that will convey runoff from the site and buildings to existing stormwater infrastructure. As part of the proposed West Addition, the existing municipal culvert will not be modified nor receive additional flow from the project.

Furthermore, the proposed design maintains the existing stormwater management devices that are onsite by matching the area flowing into the underground detention basin and matching or eliminating the vehicular surface area conveyed to onsite water quality structures. The stormwater conveyance model compares the existing, surveyed, drainage network to the preliminary, proposed drainage area to demonstrated continued capacity of the system.



WEST ADDITION DA-1 30" RCP STORMWATER CONVEYANCE						
	PIPE DESCRIPTION DRAINAGE AREA (ACRES) % IMPERVIOUS FLOW (CFS) CAPACITY					
EXISTING	124LF 30" RCP @ 0.7%	4.31	72.7	21.95	33.96	
PROPOSED	124LF 30" RCP @ 0.7%	4.56	63.4	21.55	33.96	

WEST ADDITION DA-2 18" RCP STORMWATER CONVEYANCE						
	PIPE DESCRIPTION DRAINAGE AREA (ACRES) % IMPERVIOUS FLOW (CFS) CAPACIT					
EXISTING	263LF 18" RCP @ 1.0%	1.36	61.6	5.50	10.52	
PROPOSED	263LF 18" RCP @ 1.0%	1.45	62.2	6.05	10.52	

3.3.4 Time of Concentrations

Time of concentrations (ToC) to the POS have been calculated for all pervious and impervious catchment areas under existing and proposed conditions. Equations and assumptions are in accordance with NEH 630 Chapter 15 and the BMP Manual Chapter 5. The maximum Manning's Roughness Coefficient (n) for sheet flow is 0.40 and the maximum sheet flow length is 100 feet [BMP Chapter 5].

Sheet Flow Surface	<u>Manning's 'n'</u>
Asphalt/Rooftops	0.011
Grass	0.15
Dense Grass	0.24

Refer to Appendix III for ToC calculations. Refer to Appendix IV for the Existing Drainage Area Map and Proposed Drainage Area Map that includes the hydraulic flow paths and groundcover types.

3.3.5 Water Quantity Requirements

Hydrological evaluations for existing and proposed conditions have been calculated for the NOAA 2-, 10-, and 100-year 24-hour storm events for Parsippany utilizing the 'Current' and 'Future' precipitation adjustment factors [N.J.A.C. 7:8-5.7(c) and (d), respectively]. See Table 3.1 for precipitation values.

The site's stormwater quantity control standards have been addressed by demonstrating through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction (proposed) runoff hydrographs for the 2-, 10-, and 100-year storm events, as defined and determined pursuant to N.J.A.C. 7:8-5.7(c) and (d), respectively, do not exceed, at any point in time, the pre-construction (existing) runoff hydrographs for the same storm events [N.J.A.C. 7:8-5.6(b)1.].

Runoff quantity requirements are achieved through land cover management – restoring previously developed areas with native landscaping material, matching existing drainage patterns and reducing the overall impervious



coverage of the site. Refer to Appendix III for the complete PondPack modeling summaries, runoff hydrographs in graphical and numerical form, and master network summary.

Table 3.4 Peak Runoff Rates and Volumes to POS-1 - Current Storm

PEAK RUNOFF RATES AND VOLUMES TO POS-1 - CURRENT STORM						
	Existing Proposed Net Runoff Existing Proposed Net Runo Runoff Rate Rate (CFS) (CFS) (AC-FT) (AC-FT)					
2 year	5.02	4.51	-0.51	0.667	0.611	-0.056
10 year	8.40	7.77	-0.63	1.123	1.053	-0.070
100 year	15.56	14.84	-0.72	2.092	2.009	-0.083

Table 3.5 Peak Runoff Rates and Volumes to POS-1 - Future Storm

PEAK RUNOFF RATES AND VOLUMES TO POS-1 -FUTURE STORM						
	Existing Proposed Net Runoff Existing Proposed Net Runoff Runoff Rate (CFS) (CFS) (AC-FT) (AC-FT)					
2 year	6.14	5.61	-0.53	0.818	0.756	-0.062
10 year	10.13	9.53	-0.60	1.357	1.281	-0.076
100 year	19.66	19.10	-0.56	2.647	2.562	-0.085

Table 3.6 Peak Runoff Rates and Volumes to POS-2 - Current Storm

PEAK RUNOFF RATES AND VOLUMES TO POS-2 - CURRENT STORM						
	Existing Proposed Runoff Runoff Rate (CFS) (CFS) Net Runoff Volume Volume (AC-FT) (AC-FT)					
2 year	2.19	2.19	-0.00	0.291	0.288	-0.003
10 year	3.74	3.72	-0.02	0.503	0.497	-0.006
100 year	7.01	6.97	-0.04	0.959	0.943	-0.016

Table 3.7 Peak Runoff Rates and Volumes to POS-2 - Future Storm

PEAK RUNOFF RATES AND VOLUMES TO POS-2 - FUTURE STORM								
	Proposed Volume (AC-FT)	Net Runoff Volume (AC-FT)						
2 year	2.72	2.71	-0.01	0.360	0.356	-0.004		
10 year	4.56	4.54	-0.02	0.612	0.603	-0.009		
100 year	8.97	8.90	-0.07	1.221	1.198	-0.023		

3.3.6 Runoff Quality and Groundwater Recharge Requirements

Per N.J.A.C 7:8-5.4, groundwater recharge standards do not apply to projects within the "urban redevelopment area". The proposed project site was previously developed and is located within the State Metropolitan Planning Area (PA-1); therefore, the proposed development does not include any proposed groundwater recharge methods. Furthermore, per N.J.A.C. 7:8-5.4(b).1.ii, this project meets the standards for groundwater recharge by reducing the stormwater runoff volume from pre-construction to post-construction for the future 2-year storm event

Per N.J.A.C 7:8-5.5(a), stormwater runoff quality standards are applicable when projects propose an increase of 0.25 acres or more of motor vehicle surface. The proposed development will result in a decrease of motor vehicle surface by 0.78± acres. Furthermore, previously approved existing onsite water quality devices will be maintained and reset as necessary to maintain their current flows. The areas draining to these existing water quality units will be maintained to the relocated units thereby maintaining the same level of treatment as the approved condition. When you consider the reduction in vehicle surface and maintenance of the existing water quality devices, the project will result in a net improvement in water quality.

3.3.7 Nonstructural Stormwater Management Strategies

Since this project is a major development, it has been designed to incorporate nonstructural stormwater management strategies [NJAC 7:8-2.4(g); BMP Chapter 2]. The following nonstructural strategies for major developments have been considered in the design:

- 1. Protect areas that provide water quality benefits or areas particularly susceptible to erosion and sediment loss:
- 2. Minimize impervious surfaces and break up or disconnect the flow of runoff over impervious surfaces;
- 3. Maximize the protection of natural drainage features and vegetation;
- 4. Minimize the decrease in the "time of concentration" from pre-construction (existing) to post-construction (proposed);
- 5. Minimize land disturbance including clearing and grading;
- 6. Minimize soil compaction;
- 7. Provide low maintenance landscaping that encourages retention and planting of native vegetation and minimizes the use of lawns, fertilizers, and pesticides;

The major goals for this redevelopment have been the reduction of the impervious (overall and vehicular) area from existing to proposed conditions and the restoration of actively disturbed lawn areas to its native, non-invasive



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vegetated cover. The existing onsite wooded areas shall remain untouched and other pervious surfaces within the site will be landscaped with shade trees, evergreens, shrubs, and groundcover.

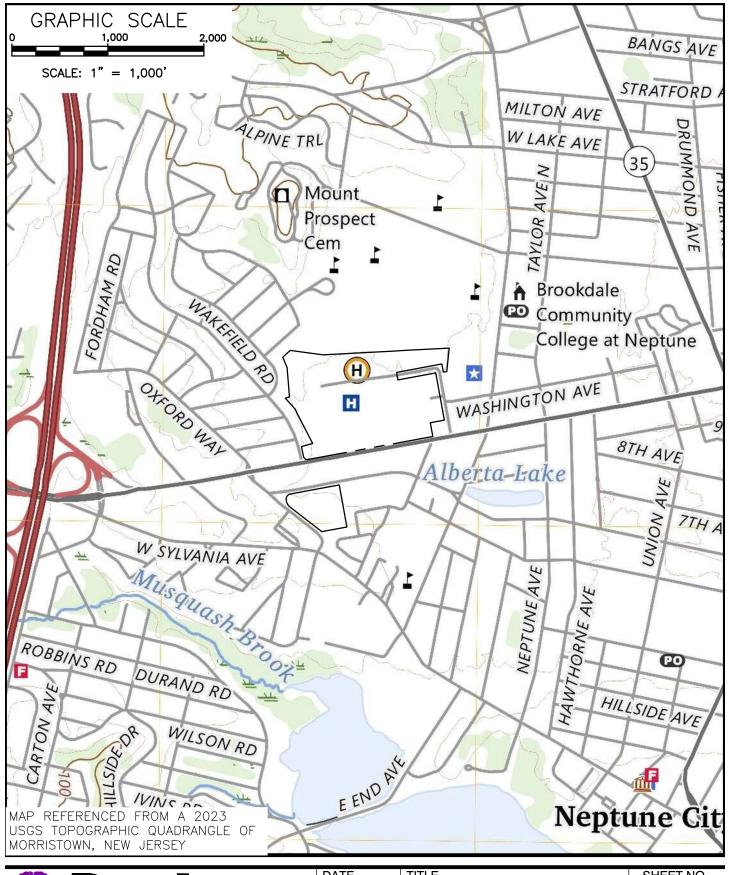
4. Conclusion

As described above, the stormwater management and conveyance system are designed in accordance with applicable state regulations and requirements. The proposed development has been designed with provisions for the safe and efficient control of stormwater runoff in a manner that will maintain the existing drainage patterns and not have adverse effect on adjacent roadways, properties, or natural habitats. Stormwater runoff quantity requirements, as set forth by the NJDEP, have been satisfied in this design, and stormwater runoff quality and groundwater recharge requirements are not applicable. This project aims to reduce the impact on land by reducing impervious areas and avoiding the need for structural stormwater detention measures.



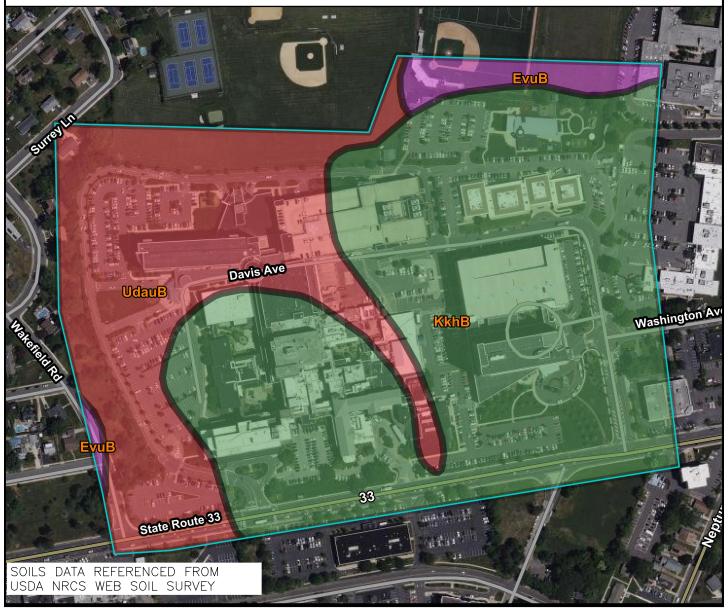
APPENDIX I:

SK-1 USGS Location Map SK-2 Site Soil Map SK-3 Site Aerial Map SK-4 State Planning Area SK-5 Hydrologic Unit Code 14 SK-6 NOAA Atlas 14 Site Precipitation

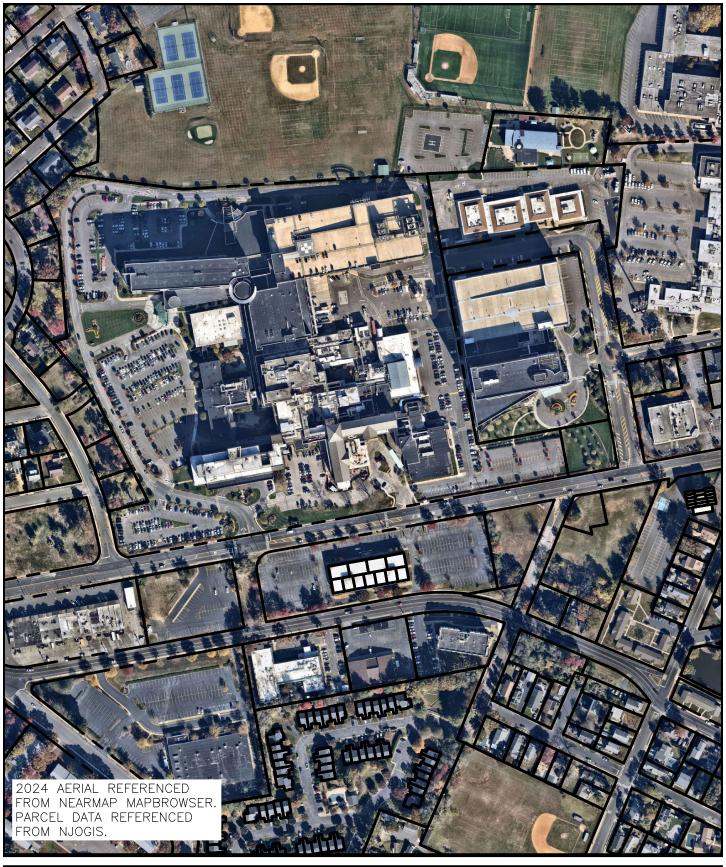


Dewberry	DATE	USGS SITE LOCATION MAP	SHEET NO.
	PROJ. NO.	PROJECT	SK-01
	50182713	JERSEY SHORE UNIVERSITY MEDICAL CENTER	

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
EvuB	Evesboro-Urban land complex, 0 to 5 percent slopes	2.1	3.8%
KkhB	Klej loamy sand-Urban land complex, 0 to 5 percent slopes	34.0	63.1%
UdauB	Udorthents-Urban land complex, 0 to 8 percent slopes	17.8	33.1%
Totals for Area of Interest		53.8	100.0%

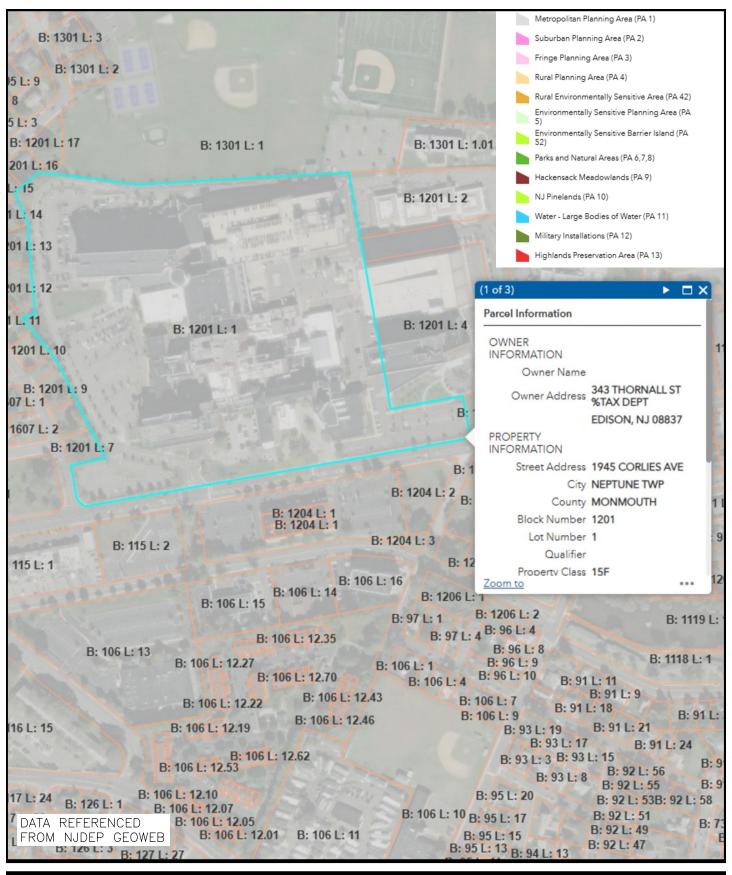


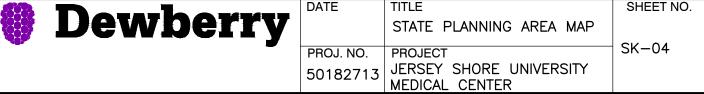
Dewberry	DATE	TITLE SITE SOIL SURVEY	SHEET NO.
	PROJ. NO. 50182713	PROJECT JERSEY SHORE UNIVERSITY MEDICAL CENTER	SK-02

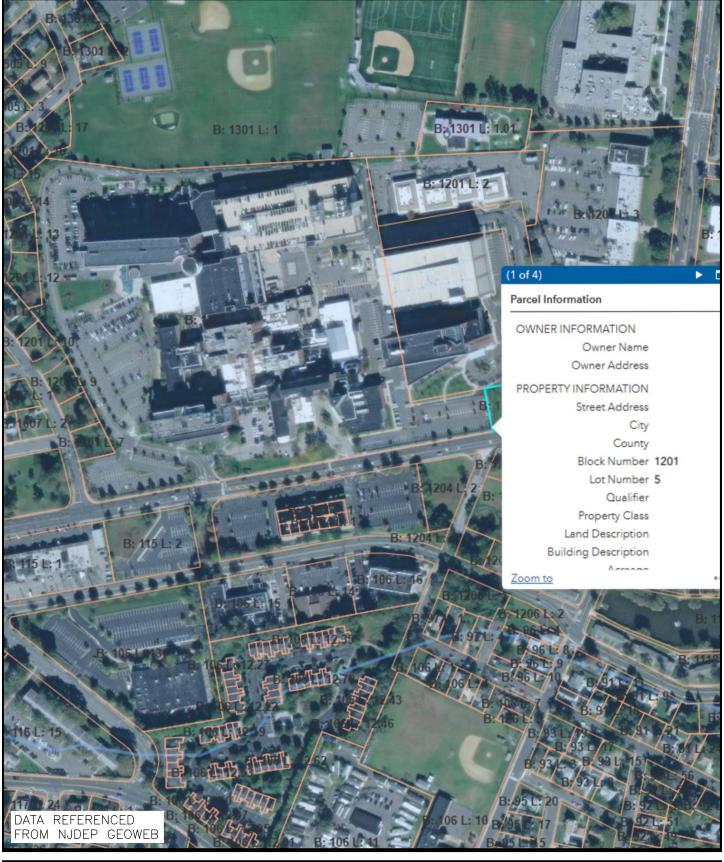




DATE	TITLE	SHEET NO.
	SITE AERIAL MAP	
		CV 07
PROJ. NO.	PROJECT	SK-03
50182713	JERSEY SHORE UNIVERSITY	
30102713	MEDICAL CENTER	









DATE	TITLE SITE HYDROLOGIC UNIT CODE 14
PROJ. NO.	PROJECT
50182713	JERSEY SHORE UNIVERSITY MEDICAL CENTER

SHEET NO.

SK-05



NOAA Atlas 14, Volume 2, Version 3 Location name: Neptune, New Jersey, USA* Latitude: 40.209°, Longitude: -74.041° Elevation: 38 ft**



* source: ESRI Maps ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PD	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									
Duration	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.341 (0.307-0.379)	0.408 (0.368-0.453)	0.483 (0.435-0.537)	0.539 (0.484-0.598)	0.608 (0.544-0.675)	0.658 (0.585-0.729)	0.708 (0.626-0.787)	0.754 (0.663-0.841)	0.813 (0.707-0.911)	0.859 (0.740-0.967)
10-min	0.545 (0.491-0.605)	0.652 (0.588-0.724)	0.774 (0.697-0.860)	0.862 (0.775-0.957)	0.969 (0.866-1.08)	1.05 (0.931-1.16)	1.12 (0.995-1.25)	1.20 (1.05-1.33)	1.29 (1.12-1.44)	1.35 (1.16-1.52)
15-min	0.681 (0.614-0.756)	0.819 (0.740-0.910)	0.979 (0.882-1.09)	1.09 (0.980-1.21)	1.23 (1.10-1.36)	1.33 (1.18-1.47)	1.42 (1.26-1.58)	1.51 (1.33-1.68)	1.62 (1.41-1.82)	1.70 (1.46-1.91)
30-min	0.934 (0.841-1.04)	1.13 (1.02-1.26)	1.39 (1.25-1.54)	1.58 (1.42-1.75)	1.82 (1.63-2.02)	2.00 (1.78-2.22)	2.18 (1.93-2.42)	2.35 (2.06-2.62)	2.58 (2.24-2.89)	2.75 (2.37-3.10)
60-min	1.16 (1.05-1.29)	1.42 (1.28-1.58)	1.78 (1.61-1.98)	2.06 (1.85-2.28)	2.42 (2.17-2.69)	2.71 (2.41-3.00)	3.00 (2.65-3.34)	3.29 (2.90-3.67)	3.70 (3.21-4.14)	4.01 (3.46-4.52)
2-hr	1.44 (1.29-1.60)	1.76 (1.58-1.96)	2.22 (2.00-2.48)	2.58 (2.31-2.88)	3.09 (2.74-3.43)	3.49 (3.09-3.88)	3.91 (3.44-4.36)	4.35 (3.80-4.86)	4.97 (4.28-5.58)	5.47 (4.67-6.16)
3-hr	1.59 (1.43-1.78)	1.94 (1.76-2.17)	2.47 (2.22-2.76)	2.88 (2.58-3.21)	3.45 (3.07-3.84)	3.92 (3.47-4.37)	4.41 (3.87-4.91)	4.92 (4.28-5.50)	5.65 (4.84-6.34)	6.24 (5.29-7.04)
6-hr	2.02 (1.82-2.27)	2.47 (2.21-2.76)	3.12 (2.79-3.48)	3.65 (3.25-4.06)	4.41 (3.89-4.91)	5.04 (4.42-5.61)	5.72 (4.96-6.37)	6.45 (5.54-7.20)	7.50 (6.35-8.42)	8.38 (7.00-9.44)
12-hr	2.46 (2.21-2.76)	2.99 (2.68-3.35)	3.81 (3.40-4.26)	4.49 (4.00-5.01)	5.50 (4.85-6.13)	6.38 (5.58-7.10)	7.33 (6.33-8.16)	8.38 (7.14-9.36)	9.95 (8.32-11.1)	11.3 (9.29-12.7)
24-hr	2.86 (2.62-3.14)	3.48 (3.19-3.82)	4.50 (4.13-4.94)	5.39 (4.92-5.90)	6.73 (6.10-7.33)	7.90 (7.10-8.59)	9.22 (8.22-10.0)	10.7 (9.44-11.6)	13.0 (11.2-14.1)	15.0 (12.8-16.3)





DATE	TITLE NOAA ATLAS 14 SITE PRECIPITATION
PROJ. NO.	PROJECT
50182713	JERSEY SHORE UNIVERSITY MEDICAL CENTER

SHEET NO.

SK-06

APPENDIX II:

East Addition Pondpack: Master Summary Report
East Addition PondPack: Rainfall Report
East Addition PondPack: Unit Hydrograph
East Addition PondPack: Routing Diagrams
East Addition Pondpack: Graphical & Numerical Hydrograph Comparisons
East Addition Time of Concentration Calculations
East Addition SewerGEMS: FlexTables

East Addition SewerGEMS: Pipe Profile

JSUMC East Addition: PondPack Report

Title	JSUMC East Addition
Engineer	
Company	Dewberry Engineers Inc
Date	12/3/2024

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	Unit Hydrograph (Hydrograph Table), 2 years (Current 2 year)	155
	Unit Hydrograph Summary, 10 years (Current 10 year)	156
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Subsection: User Notifications

User Mounications						
Message Id	7					
Scenario	Current 2 year					
Element Type	Catchment					
Element Id	33					
Label	EDA1 - Imp - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.5 % is greater than 1.5 %.					
3	Computed peak flow= 18.10 ft ³ /s Interp. peak flow= 10.42 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 2 year					
Element Type	Catchment					
Element Id	35					
Label	EDA2 - Imp - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 2.91 ft³/s Interp. peak flow= 1.74 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 2 year					
Element Type	Catchment					
Element Id	37					
Label	PDA1 - Imp - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.5 % is greater than 1.5 %. Computed peak flow= 18.07 ft³/s Interp. peak flow= 10.40 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 2 year					
Element Type	Catchment					
Element Id	39					
Label	PDA2 - Imp - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 1.89 ft ³ /s Interp. peak flow= 1.13 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 2 year					
Element Type	Catchment					
Element Id	38					
Label	PDA2 - Per - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 1.8 % is greater than 1.5 %. Computed peak flow= 0.02 ft ³ /s Interp. peak flow= 0.02 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: User Notifications

User Notifications	
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	33
Label	EDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 28.48 ft ³ /s Interp. peak flow= 16.42 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	32
Label	EDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 12.3 % is greater than 1.5 %. Computed peak flow= 1.31 ft ³ /s Interp. peak flow= 1.15 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	35
Label	EDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2% is greater than 1.5% . Computed peak flow= $4.58 \text{ ft}^3/\text{s}$ Interp. peak flow= $2.74 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	34
Label	EDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 49.9 % is greater than 1.5 %. Computed peak flow= $0.26 \text{ ft}^3/\text{s}$ Interp. peak flow= $0.13 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	37
Label	PDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 28.42 ft³/s Interp. peak flow= 16.39 ft³/s. Output increment for this catchment may be too large.
Source	Warning

Subsection: User Notifications

User Nothications						
Message Id	7					
Scenario	Current 10 year					
Element Type	Catchment					
Element Id	36					
Label	PDA1 - Per - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 7.4 % is greater than 1.5 %.					
J	Computed peak flow= $0.67 \text{ ft}^3/\text{s}$ Interp. peak flow= $0.62 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 10 year					
Element Type	Catchment					
Element Id	39					
Label	PDA2 - Imp - Cur					
Time	· (N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 2.97 ft ³ /s Interp. peak flow= 1.78 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 10 year					
Element Type	Catchment					
Element Id	38					
Label	PDA2 - Per - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 45.5 % is greater than 1.5 %. Computed peak flow= 0.45 ft ³ /s Interp. peak flow= 0.25 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 100 year					
Element Type	Catchment					
Element Id	33					
Label	EDA1 - Imp - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 49.36 ft ³ /s Interp. peak flow= 28.50 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Current 100 year					
Element Type	Catchment					
Element Id	32					
Label	EDA1 - Per - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 16.9 % is greater than 1.5 %. Computed peak flow= 4.79 ft ³ /s Interp. peak flow= 3.98 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: User Notifications

Mossago Id	7
Message Id	
Scenario	Current 100 year
Element Type	Catchment 35
Element Id	
Label	EDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 7.94 ft ³ /s Interp. peak flow= 4.75 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	34
Label	EDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 53.0 % is greater than 1.5 %. Computed peak flow= $1.11 \text{ft}^3/\text{s}$ Interp. peak flow= $0.52 \text{ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	37
Label	PDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 49.27 ft ³ /s Interp. peak flow= 28.44 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	36
Label	PDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 18.1 % is greater than 1.5 %. Computed peak flow= 3.73 ft ³ /s Interp. peak flow= 3.05 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	39
Label	PDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 5.15 ft ³ /s Interp. peak flow= 3.08 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning

Subsection: User Notifications

User Nothications						
Message Id	7					
Scenario	Current 100 year					
Element Type	Catchment					
Element Id	38					
Label	PDA2 - Per - Cur					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 53.9 % is greater than 1.5 %.					
J	Computed peak flow= 2.01 ft³/s Interp. peak flow= 0.92 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	43					
Label	EDA1 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.4 % is greater than 1.5 %. Computed peak flow= 21.60 ft ³ /s Interp. peak flow= 12.44 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	41					
Label	EDA1 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 6.9 % is greater than 1.5 %. Computed peak flow= $0.47 \text{ ft}^3/\text{s}$ Interp. peak flow= $0.44 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	47					
Label	EDA2 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 3.48 ft ³ /s Interp. peak flow= 2.08 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	45					
Label	EDA2 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 34.5 % is greater than 1.5 %. Computed peak flow= 0.07 ft ³ /s Interp. peak flow= 0.04 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: User Notifications

Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	42					
Label Time	PDA1 - Imp - Fut					
	(N/A) The difference between calculated peak flow and interpolated peak flow 42.4.0% is greater than 1.5.0%					
Message	The difference between calculated peak flow and interpolated peak flow 42.4 % is greater than 1.5 %. Computed peak flow= 21.60 ft ³ /s Interp. peak flow= 12.44 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	46					
Label	PDA2 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 2.25 ft³/s Interp. peak flow= 1.35 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 2 year					
Element Type	Catchment					
Element Id	44					
Label	PDA2 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 25.5 % is greater than 1.5 %. Computed peak flow= 0.11 ft ³ /s Interp. peak flow= 0.08 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 10 year					
Element Type	Catchment					
Element Id	43					
Label	EDA1 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 33.60 ft³/s Interp. peak flow= 19.38 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 10 year					
Element Type	Catchment					
Element Id	41					
Label	EDA1 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 17.1 % is greater than 1.5 %. Computed peak flow= 2.10 ft ³ /s Interp. peak flow= 1.74 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: User Notifications

User Notifications	
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	47
Label	EDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 5.40 ft³/s Interp. peak flow= 3.23 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	45
Label	EDA2 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 56.9 % is greater than 1.5 %. Computed peak flow= 0.45 ft ³ /s Interp. peak flow= 0.20 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	42
Label	PDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 33.60 ft ³ /s Interp. peak flow= 19.38 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	40
Label	PDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 15.5 % is greater than 1.5 %. Computed peak flow= 1.21 ft ³ /s Interp. peak flow= 1.02 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	46
Label	PDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 3.51 ft³/s Interp. peak flow= 2.10 ft³/s. Output increment for this catchment may be too large.
Source	Warning

Subsection: User Notifications

Mossage Id	7					
Message Id						
Scenario	Future 10 year Catchment					
Element Type						
Element Id	44 PDA2 - Por - Fut					
Label	PDA2 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 53.2 % is greater than 1.5 %. Computed peak flow= 0.80 ft ³ /s Interp. peak flow= 0.37 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	43					
Label	EDA1 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 61.05 ft ³ /s Interp. peak flow= 35.25 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	41					
Label	EDA1 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 21.4 % is greater than 1.5 %. Computed peak flow= 7.22 ft³/s Interp. peak flow= 5.67 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	47					
Label	EDA2 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 9.81 ft ³ /s Interp. peak flow= 5.88 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	45					
Label	EDA2 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 49.7 % is greater than 1.5 %. Computed peak flow= 1.72 ft ³ /s Interp. peak flow= 0.86 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: User Notifications

Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	42					
Label PDA1 - Imp - Fut						
Time (N/A)						
Message	The difference between calculated peak flow and interpolated peak flow 42.3 % is greater than 1.5 %. Computed peak flow= 61.05 ft³/s Interp. peak flow= 35.25 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	40					
Label	PDA1 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 24.0 % is greater than 1.5 %. Computed peak flow= 5.79 ft ³ /s Interp. peak flow= 4.40 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	46					
Label	PDA2 - Imp - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 6.37 ft³/s Interp. peak flow= 3.81 ft³/s. Output increment for this catchment may be too large.					
Source	Warning					
Message Id	7					
Scenario	Future 100 year					
Element Type	Catchment					
Element Id	44					
Label	PDA2 - Per - Fut					
Time	(N/A)					
Message	The difference between calculated peak flow and interpolated peak flow 50.6 % is greater than 1.5 %. Computed peak flow= 3.14 ft ³ /s Interp. peak flow= 1.55 ft ³ /s. Output increment for this catchment may be too large.					
Source	Warning					

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
EDA1 - Per - Cur	Current 2 year	2	0.037	12.25	0.16
EDA1 - Per - Cur	Current 10 year	10	0.134	12.25	1.15
EDA1 - Per - Cur	Current 100 year	100	0.425	12.25	3.98
EDA1 - Imp - Cur	Current 2 year	2	1.460	12.00	10.42
EDA1 - Imp - Cur	Current 10 year	10	2.341	12.00	16.42
EDA1 - Imp - Cur	Current 100 year	100	4.123	12.00	28.50
EDA2 - Per - Cur	Current 2 year	2	0.005	12.50	0.01
EDA2 - Per - Cur	Current 10 year	10	0.021	12.25	0.13
EDA2 - Per - Cur	Current 100 year	100	0.074	12.00	0.52
EDA2 - Imp - Cur	Current 2 year	2	0.211	12.00	1.74
EDA2 - Imp - Cur	Current 10 year	10	0.339	12.00	2.74
EDA2 - Imp - Cur	Current 100 year	100	0.597	12.00	4.75
PDA1 - Per - Cur	Current 2 year	2	0.017	13.00	0.03
PDA1 - Per - Cur	Current 10 year	10	0.088	12.25	0.62
PDA1 - Per - Cur	Current 100 year	100	0.336	12.25	3.05
PDA1 - Imp - Cur	Current 2 year	2	1.457	12.00	10.40
PDA1 - Imp - Cur	Current 10 year	10	2.336	12.00	16.39
PDA1 - Imp - Cur	Current 100 year	100	4.116	12.00	28.44
PDA2 - Per - Cur	Current 2 year	2	0.009	12.50	0.02
PDA2 - Per - Cur	Current 10 year	10	0.039	12.25	0.25
PDA2 - Per - Cur	Current 100 year	100	0.138	12.25	0.92
PDA2 - Imp - Cur	Current 2 year	2	0.141	12.00	1.13
PDA2 - Imp - Cur	Current 10 year	10	0.226	12.00	1.78
PDA2 - Imp - Cur	Current 100 year	100	0.398	12.00	3.08
PDA1 - Per - Fut	Future 2 year	2	0.032	12.50	0.09
PDA1 - Per - Fut	Future 10 year	10	0.130	12.25	1.02
PDA1 - Per - Fut	Future 100 year	100	0.490	12.25	4.40
EDA1 - Per - Fut	Future 2 year	2	0.064	12.25	0.44
EDA1 - Per - Fut	Future 10 year	10	0.196	12.25	1.74
EDA1 - Per - Fut	Future 100 year	100	0.618	12.25	5.67
PDA1 - Imp - Fut	Future 2 year	2	1.756	12.00	12.44
PDA1 - Imp - Fut	Future 10 year	10	2.777	12.00	19.38
PDA1 - Imp - Fut	Future 100 year	100	5.123	12.00	35.25
EDA1 - Imp - Fut	Future 2 year	2	1.756	12.00	12.44
EDA1 - Imp - Fut	Future 10 year	10	2.777	12.00	19.38
EDA1 - Imp - Fut	Future 100 year	100	5.123	12.00	35.25
PDA2 - Per - Fut	Future 2 year	2	0.017	12.25	0.08
PDA2 - Per - Fut	Future 10 year	10	0.060	12.25	0.37
PDA2 - Per - Fut	Future 100 year	100	0.207	12.00	1.55
EDA2 - Per - Fut	Future 2 year	2	0.009	12.25	0.04
EDA2 - Per - Fut	Future 10 year	10	0.032	12.25	0.20
EDA2 - Per - Fut	Future 100 year	100	0.111	12.00	0.86
PDA2 - Imp - Fut	Future 2 year	2	0.170	12.00	1.35
PDA2 - Imp - Fut	Future 10 year	10	0.268	12.00	2.10
PDA2 - Imp - Fut	Future 100 year	100	0.495	12.00	3.81
EDA2 - Imp - Fut	Future 2 year	2	0.254	12.00	2.08

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
EDA2 - Imp - Fut	Future 10 year	10	0.402	12.00	3.23
EDA2 - Imp - Fut	Future 100 year	100	0.742	12.00	5.88

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
Pre - POS1	Current 2 year	2	1.497	12.00	10.42
Pre - POS1	Future 2 year	2	1.820	12.00	12.48
Pre - POS1	Current 10 year	10	2.475	12.00	16.78
Pre - POS1	Future 10 year	10	2.973	12.00	20.11
Pre - POS1	Current 100 year	100	4.548	12.00	30.45
Pre - POS1	Future 100 year	100	5.741	12.00	38.42
Post - POS1	Current 2 year	2	1.474	12.00	10.40
Post - POS1	Future 2 year	2	1.788	12.00	12.44
Post - POS1	Current 10 year	10	2.425	12.00	16.45
Post - POS1	Future 10 year	10	2.907	12.00	19.65
Post - POS1	Current 100 year	100	4.452	12.00	29.85
Post - POS1	Future 100 year	100	5.613	12.00	37.61
Pre - POS2	Current 2 year	2	0.216	12.00	1.74
Pre - POS2	Future 2 year	2	0.264	12.00	2.08
Pre - POS2	Current 10 year	10	0.360	12.00	2.82
Pre - POS2	Future 10 year	10	0.434	12.00	3.41
Pre - POS2	Current 100 year	100	0.672	12.00	5.27
Pre - POS2	Future 100 year	100	0.853	12.00	6.74
Post - POS2	Current 2 year	2	0.150	12.00	1.13
Post - POS2	Future 2 year	2	0.187	12.00	1.35
Post - POS2	Current 10 year	10	0.265	12.00	1.90
Post - POS2	Future 10 year	10	0.328	12.00	2.40
Post - POS2	Current 100 year	100	0.537	12.00	4.01
Post - POS2	Future 100 year	100	0.702	12.00	5.36

Subsection: Time-Depth Curve Return Event: 10 years Label: Current Storm Storm Event: Current 10

Scenario: Current 10 year

Time-Depth Curve: Current 10	
Label	Current 10
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	10 years

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.01	0.02	0.02
0.50	0.03	0.04	0.04	0.05	0.06
1.00	0.06	0.07	0.08	0.08	0.09
1.50	0.10	0.10	0.11	0.12	0.12
2.00	0.13	0.14	0.14	0.15	0.16
2.50	0.17	0.17	0.18	0.19	0.20
3.00	0.20	0.21	0.22	0.23	0.24
3.50	0.24	0.25	0.26	0.27	0.28
4.00	0.29	0.29	0.30	0.31	0.32
4.50	0.33	0.34	0.35	0.35	0.36
5.00	0.37	0.38	0.39	0.40	0.41
5.50	0.42	0.43	0.44	0.45	0.45
6.00	0.46	0.47	0.48	0.49	0.50
6.50	0.52	0.53	0.54	0.55	0.56
7.00	0.57	0.59	0.60	0.61	0.62
7.50	0.64	0.65	0.66	0.68	0.69
8.00	0.71	0.72	0.74	0.75	0.77
8.50	0.78	0.80	0.81	0.83	0.85
9.00	0.86	0.88	0.90	0.92	0.94
9.50	0.96	0.98	1.00	1.03	1.05
10.00	1.08	1.10	1.13	1.16	1.19
10.50	1.22	1.25	1.29	1.33	1.37
11.00	1.41	1.47	1.52	1.58	1.65
11.50	1.72	1.82	1.93	2.07	2.27
12.00	2.61	3.17	3.37	3.51	3.62
12.50	3.72	3.79	3.86	3.92	3.97
13.00	4.03	4.07	4.11	4.15	4.19
13.50	4.22	4.25	4.28	4.31	4.34
14.00	4.36	4.39	4.41	4.44	4.46
14.50	4.48	4.50	4.52	4.54	4.56
15.00	4.58	4.59	4.61	4.63	4.64
15.50	4.66	4.67	4.69	4.70	4.72
16.00	4.73	4.75	4.76	4.78	4.79
16.50	4.80	4.82	4.83	4.84	4.85
17.00	4.87	4.88	4.89	4.90	4.91
17.50	4.92	4.94	4.95	4.96	4.97
18.00	4.98	4.99	4.99	5.00	5.01

Subsection: Time-Depth Curve Return Event: 10 years

Label: Current Storm Storm Event: Current 10 Scenario: Current 10 year

	Time	Depth	Depth	Depth	Depth	Depth
	(hours)	(in)	(in)	(in)	(in)	(in)
	18.50	5.02	5.03	5.04	5.05	5.06
	19.00	5.07	5.08	5.09	5.09	5.10
	19.50	5.11	5.12	5.13	5.14	5.15
ı	20.00	5.15	5.16	5.17	5.18	5.19
ı	20.50	5.20	5.20	5.21	5.22	5.23
	21.00	5.24	5.24	5.25	5.26	5.27
	21.50	5.27	5.28	5.29	5.30	5.30
ı	22.00	5.31	5.32	5.32	5.33	5.34
	22.50	5.34	5.35	5.36	5.36	5.37
	23.00	5.38	5.38	5.39	5.40	5.40
	23.50	5.41	5.42	5.42	5.43	5.43
ı	24.00	5.44	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 100 years Label: Current Storm Storm Event: Current 100

Scenario: Current 100 year

Time-Depth Curve: Current 100	
Label	Current 100
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	100 years

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.02	0.03	0.04
0.50	0.05	0.06	0.07	0.09	0.10
1.00	0.11	0.12	0.13	0.14	0.15
1.50	0.17	0.18	0.19	0.20	0.21
2.00	0.23	0.24	0.25	0.26	0.28
2.50	0.29	0.30	0.31	0.33	0.34
3.00	0.35	0.37	0.38	0.39	0.41
3.50	0.42	0.44	0.45	0.46	0.48
4.00	0.49	0.51	0.52	0.54	0.55
4.50	0.57	0.58	0.60	0.61	0.63
5.00	0.64	0.66	0.67	0.69	0.71
5.50	0.72	0.74	0.75	0.77	0.79
6.00	0.80	0.82	0.84	0.85	0.87
6.50	0.89	0.91	0.93	0.95	0.97
7.00	0.99	1.01	1.03	1.05	1.08
7.50	1.10	1.12	1.15	1.17	1.19
8.00	1.22	1.25	1.27	1.30	1.32
8.50	1.35	1.38	1.41	1.43	1.46
9.00	1.49	1.52	1.55	1.59	1.62
9.50	1.66	1.70	1.74	1.78	1.82
10.00	1.86	1.91	1.96	2.00	2.05
10.50	2.11	2.16	2.22	2.29	2.37
11.00	2.44	2.53	2.63	2.74	2.85
11.50	2.98	3.15	3.33	3.57	3.92
12.00	4.50	5.48	5.83	6.07	6.25
12.50	6.42	6.55	6.66	6.77	6.87
13.00	6.96	7.03	7.11	7.18	7.24
13.50	7.29	7.35	7.40	7.44	7.49
14.00	7.54	7.58	7.62	7.66	7.70
14.50	7.74	7.78	7.81	7.85	7.88
15.00	7.91	7.94	7.97	7.99	8.02
15.50	8.05	8.08	8.10	8.13	8.15
16.00	8.18	8.21	8.23	8.25	8.28
16.50	8.30	8.32	8.35	8.37	8.39
17.00	8.41	8.43	8.45	8.47	8.49
17.50	8.51	8.53	8.55	8.56	8.58
18.00	8.60	8.61	8.63	8.65	8.66

Subsection: Time-Depth Curve Return Event: 100 years

Label: Current Storm Storm Event: Current 100 Scenario: Current 100 year

		•			
Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
18.50	8.68	8.69	8.71	8.73	8.74
19.00	8.76	8.77	8.79	8.80	8.82
19.50	8.83	8.85	8.86	8.88	8.89
20.00	8.91	8.92	8.94	8.95	8.96
20.50	8.98	8.99	9.01	9.02	9.03
21.00	9.05	9.06	9.07	9.09	9.10
21.50	9.11	9.12	9.14	9.15	9.16
22.00	9.17	9.19	9.20	9.21	9.22
22.50	9.23	9.25	9.26	9.27	9.28
23.00	9.29	9.30	9.31	9.33	9.34
23.50	9.35	9.36	9.37	9.38	9.39
24.00	9.40	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 2 years
Label: Current Storm Storm Event: Current 2 Year

Scenario: Current 2 year

Time-Depth Curve: Current 2 Year				
Label	Current 2 Year			
Start Time	0.00 hours			
Increment	0.10 hours			
End Time	24.00 hours			
Return Event	2 years			

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.00	0.01	0.01	0.02
0.50	0.02	0.02	0.03	0.03	0.04
1.00	0.04	0.04	0.05	0.05	0.06
1.50	0.06	0.07	0.07	0.07	0.08
2.00	0.08	0.09	0.09	0.10	0.10
2.50	0.11	0.11	0.12	0.12	0.13
3.00	0.13	0.14	0.14	0.15	0.15
3.50	0.16	0.16	0.17	0.17	0.18
4.00	0.18	0.19	0.19	0.20	0.20
4.50	0.21	0.22	0.22	0.23	0.23
5.00	0.24	0.24	0.25	0.26	0.26
5.50	0.27	0.27	0.28	0.28	0.29
6.00	0.30	0.30	0.31	0.32	0.32
6.50	0.33	0.34	0.34	0.35	0.36
7.00	0.37	0.37	0.38	0.39	0.40
7.50	0.41	0.42	0.42	0.43	0.44
8.00	0.45	0.46	0.47	0.48	0.49
8.50	0.50	0.51	0.52	0.53	0.54
9.00	0.55	0.56	0.58	0.59	0.60
9.50	0.61	0.63	0.64	0.66	0.67
10.00	0.69	0.71	0.72	0.74	0.76
10.50	0.78	0.80	0.82	0.85	0.88
11.00	0.90	0.94	0.97	1.01	1.06
11.50	1.10	1.17	1.23	1.32	1.45
12.00	1.67	2.03	2.16	2.25	2.31
12.50	2.38	2.42	2.47	2.51	2.54
13.00	2.58	2.60	2.63	2.66	2.68
13.50	2.70	2.72	2.74	2.76	2.77
14.00	2.79	2.81	2.82	2.84	2.85
14.50	2.87	2.88	2.89	2.90	2.92
15.00	2.93	2.94	2.95	2.96	2.97
15.50	2.98	2.99	3.00	3.01	3.02
16.00	3.03	3.04	3.05	3.06	3.06
16.50	3.07	3.08	3.09	3.10	3.11
17.00	3.11	3.12	3.13	3.14	3.14
17.50	3.15	3.16	3.16	3.17	3.18
18.00	3.18	3.19	3.20	3.20	3.21

Subsection: Time-Depth Curve Return Event: 2 years

Label: Current Storm Storm Event: Current 2 Year Scenario: Current 2 year

	ime ours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
	18.50	3.21	3.22	3.22	3.23	3,24
	19.00	3.24	3.25	3.25	3.26	3.26
	19.50	3.27	3.28	3.28	3.29	3.29
	20.00	3.30	3.30	3.31	3.31	3.32
	20.50	3.32	3.33	3.33	3.34	3.34
	21.00	3.35	3.35	3.36	3.36	3.37
	21.50	3.37	3.38	3.38	3.39	3.39
	22.00	3.40	3.40	3.41	3.41	3.41
	22.50	3.42	3.42	3.43	3.43	3.44
	23.00	3.44	3.44	3.45	3.45	3.46
	23.50	3.46	3.46	3.47	3.47	3.48
	24.00	3.48	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 10 years Label: Future Storm Storm Event: Future 10

Scenario: Future 10 year

Time-Depth Curve: Future 10	
Label	Future 10
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	10 years

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.00	0.00	0.01	0.01	0.02	0.03
0.50	0.04	0.04	0.05	0.06	0.07
1.00	0.07	0.08	0.09	0.10	0.10
1.50	0.11	0.12	0.13	0.14	0.15
2.00	0.15	0.16	0.17	0.18	0.19
2.50	0.20	0.21	0.21	0.22	0.23
3.00	0.24	0.25	0.26	0.27	0.28
3.50	0.29	0.30	0.31	0.32	0.33
4.00	0.34	0.35	0.36	0.37	0.38
4.50	0.39	0.40	0.41	0.42	0.43
5.00	0.44	0.45	0.46	0.47	0.48
5.50	0.49	0.50	0.51	0.52	0.54
6.00	0.55	0.56	0.57	0.58	0.60
6.50	0.61	0.62	0.63	0.65	0.66
7.00	0.68	0.69	0.70	0.72	0.73
7.50	0.75	0.77	0.78	0.80	0.81
8.00	0.83	0.85	0.87	0.88	0.90
8.50	0.92	0.94	0.96	0.98	1.00
9.00	1.02	1.04	1.06	1.08	1.11
9.50	1.13	1.16	1.18	1.21	1.24
10.00	1.27	1.30	1.33	1.37	1.40
10.50	1.44	1.47	1.52	1.56	1.61
11.00	1.67	1.73	1.79	1.87	1.95
11.50	2.03	2.15	2.27	2.44	2.67
12.00	3.07	3.74	3.97	4.14	4.26
12.50	4.38	4.46	4.54	4.62	4.68
13.00	4.74	4.80	4.85	4.89	4.94
13.50	4.97	5.01	5.04	5.08	5.11
14.00	5.14	5.17	5.20	5.23	5.25
14.50	5.28	5.30	5.33	5.35	5.37
15.00	5.39	5.41	5.43	5.45	5.47
15.50	5.49	5.51	5.53	5.54	5.56
16.00	5.58	5.60	5.61	5.63	5.64
16.50	5.66	5.68	5.69	5.71	5.72
17.00	5.73	5.75	5.76	5.78	5.79
17.50	5.80	5.81	5.83	5.84	5.85
18.00	5.86	5.87	5.89	5.90	5.91

Subsection: Time-Depth Curve Return Event: 10 years Label: Future Storm Storm Event: Future 10

Scenario: Future 10 year

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
, ,	, ,	` '	` '	` '	` '
18.50	5.92	5.93	5.94	5.95	5.96
19.00	5.97	5.98	5.99	6.00	6.01
19.50	6.02	6.03	6.04	6.05	6.06
20.00	6.07	6.08	6.09	6.10	6.11
20.50	6.12	6.13	6.14	6.15	6.16
21.00	6.17	6.18	6.19	6.20	6.20
21.50	6.21	6.22	6.23	6.24	6.25
22.00	6.26	6.26	6.27	6.28	6.29
22.50	6.30	6.31	6.31	6.32	6.33
23.00	6.34	6.34	6.35	6.36	6.37
23.50	6.37	6.38	6.39	6.40	6.40
24.00	6.41	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 100 years Label: Future Storm Storm Event: Future 100

Label: Future Storm Scenario: Future 100 year

Time-Depth Curve: Future 100	
Label	Future 100
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	100 years

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.03	0.04	0.05
0.50	0.06	0.08	0.09	0.11	0.12
1.00	0.13	0.15	0.16	0.18	0.19
1.50	0.20	0.22	0.23	0.25	0.26
2.00	0.28	0.29	0.31	0.33	0.34
2.50	0.36	0.37	0.39	0.40	0.42
3.00	0.44	0.45	0.47	0.49	0.50
3.50	0.52	0.54	0.56	0.57	0.59
4.00	0.61	0.63	0.65	0.66	0.68
4.50	0.70	0.72	0.74	0.76	0.78
5.00	0.79	0.81	0.83	0.85	0.87
5.50	0.89	0.91	0.93	0.95	0.97
6.00	0.99	1.01	1.03	1.06	1.08
6.50	1.10	1.13	1.15	1.17	1.20
7.00	1.22	1.25	1.28	1.30	1.33
7.50	1.36	1.39	1.42	1.45	1.48
8.00	1.51	1.54	1.57	1.60	1.64
8.50	1.67	1.70	1.74	1.77	1.81
9.00	1.84	1.88	1.92	1.96	2.01
9.50	2.05	2.10	2.15	2.20	2.25
10.00	2.30	2.36	2.42	2.48	2.54
10.50	2.60	2.67	2.75	2.83	2.92
11.00	3.02	3.13	3.25	3.38	3.53
11.50	3.68	3.89	4.12	4.42	4.84
12.00	5.57	6.78	7.20	7.50	7.73
12.50	7.94	8.09	8.24	8.37	8.49
13.00	8.60	8.70	8.79	8.87	8.95
13.50	9.02	9.08	9.14	9.20	9.26
14.00	9.32	9.37	9.42	9.47	9.52
14.50	9.57	9.61	9.66	9.70	9.74
15.00	9.78	9.81	9.85	9.88	9.92
15.50	9.95	9.98	10.02	10.05	10.08
16.00	10.11	10.14	10.17	10.20	10.23
16.50	10.26	10.29	10.32	10.34	10.37
17.00	10.40	10.42	10.45	10.47	10.49
17.50	10.52	10.54	10.56	10.59	10.61
18.00	10.63	10.65	10.67	10.69	10.71

Subsection: Time-Depth Curve Return Event: 100 years

Label: Future Storm

Storm Event: Future 100
Scenario: Future 100 year

		•			
Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
18.50	10.73	10.75	10.77	10.79	10.81
19.00	10.83	10.84	10.86	10.88	10.90
19.50	10.92	10.94	10.96	10.97	10.99
20.00	11.01	11.03	11.05	11.06	11.08
20.50	11.10	11.12	11.13	11.15	11.17
21.00	11.18	11.20	11.22	11.23	11.25
21.50	11.26	11.28	11.29	11.31	11.33
22.00	11.34	11.36	11.37	11.39	11.40
22.50	11.42	11.43	11.44	11.46	11.47
23.00	11.49	11.50	11.51	11.53	11.54
23.50	11.56	11.57	11.58	11.59	11.61
24.00	11.62	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 2 years Label: Future Storm Storm Event: Future 2

Scenario: Future 2 year

Time-Depth Curve: Future 2	
Label	Future 2
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	2 years

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.00	0.01	0.01	0.02
0.50	0.02	0.03	0.03	0.04	0.04
1.00	0.05	0.05	0.06	0.06	0.07
1.50	0.07	0.08	0.08	0.09	0.09
2.00	0.10	0.10	0.11	0.12	0.12
2.50	0.13	0.13	0.14	0.14	0.15
3.00	0.16	0.16	0.17	0.17	0.18
3.50	0.19	0.19	0.20	0.20	0.21
4.00	0.22	0.22	0.23	0.24	0.24
4.50	0.25	0.26	0.26	0.27	0.28
5.00	0.28	0.29	0.30	0.30	0.31
5.50	0.32	0.32	0.33	0.34	0.35
6.00	0.35	0.36	0.37	0.38	0.38
6.50	0.39	0.40	0.41	0.42	0.43
7.00	0.44	0.45	0.45	0.46	0.47
7.50	0.48	0.49	0.50	0.52	0.53
8.00	0.54	0.55	0.56	0.57	0.58
8.50	0.59	0.61	0.62	0.63	0.64
9.00	0.66	0.67	0.68	0.70	0.71
9.50	0.73	0.75	0.76	0.78	0.80
10.00	0.82	0.84	0.86	0.88	0.90
10.50	0.93	0.95	0.98	1.01	1.04
11.00	1.08	1.12	1.16	1.21	1.26
11.50	1.31	1.39	1.47	1.57	1.72
12.00	1.98	2.42	2.57	2.67	2.75
12.50	2.83	2.88	2.93	2.98	3.02
13.00	3.06	3.10	3.13	3.16	3.19
13.50	3.21	3.24	3.26	3.28	3.30
14.00	3.32	3.34	3.36	3.38	3.39
14.50	3.41	3.43	3.44	3.46	3.47
15.00	3.48	3.50	3.51	3.52	3.53
15.50	3.55	3.56	3.57	3.58	3.59
16.00	3.60	3.61	3.62	3.64	3.65
16.50	3.66	3.67	3.68	3.69	3.69
17.00	3.70	3.71	3.72	3.73	3.74
17.50	3.75	3.76	3.76	3.77	3.78
18.00	3.79	3.79	3.80	3.81	3.82

Subsection: Time-Depth Curve Return Event: 2 years Label: Future Storm Storm Event: Future 2

Scenario: Future 2 year

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
18.50	3.82	3.83	3.84	3.84	3.85
19.00	3.86	3.86	3.87	3.88	3.88
19.50	3.89	3.90	3.90	3.91	3.92
20.00	3.92	3.93	3.94	3.94	3.95
20.50	3.95	3.96	3.97	3.97	3.98
21.00	3.98	3.99	4.00	4.00	4.01
21.50	4.01	4.02	4.02	4.03	4.04
22.00	4.04	4.05	4.05	4.06	4.06
22.50	4.07	4.07	4.08	4.08	4.09
23.00	4.09	4.10	4.10	4.11	4.11
23.50	4.12	4.12	4.13	4.13	4.14
24.00	4.14	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method (Computational Notes) Definition of Terms

At Total area (acres): At = Ai+Ap
Ai Impervious area (acres)
Ap Pervious area (acres)

CNi Runoff curve number for impervious area
CNp Runoff curve number for pervious area
fLoss f loss constant infiltration (depth/time)
gKs Saturated Hydraulic Conductivity (depth/time)

Md Volumetric Moisture Deficit
Psi Capillary Suction (length)

hK Horton Infiltration Decay Rate (time^-1) fo Initial Infiltration Rate (depth/time)

fc Ultimate(capacity)Infiltration Rate (depth/time)

Ia Initial Abstraction (length)

dt Computational increment (duration of unit excess rainfall)

Default dt is smallest value of 0.1333Tc, rtm, and th (Smallest dt is then adjusted to match up with Tp)

UDdt User specified override computational main time increment

(only used if UDdt is => .1333Tc)

D(t) Point on distribution curve (fraction of P) for time step t K 2/(1 + (Tr/Tp)): default K = 0.75: (for Tr/Tp = 1.67)

Ks Hydrograph shape factor = Unit Conversions * K: = ((1hr/3600sec) *

(1ft/12in) * ((5280ft)**2/sq.mi)) * K Default Ks = 645.333 * 0.75 = 484

Lag time from center of excess runoff (dt) to Tp: Lag = 0.6Tc

P Total precipitation depth, inches
Pa(t) Accumulated rainfall at time step t
Pi(t) Incremental rainfall at time step t

qp Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi. = (Ks * A * Q) /

Tp (where Q = 1in. runoff, A=sq.mi.)

Qu(t) Unit hydrograph ordinate (cfs) at time step t Q(t) Final hydrograph ordinate (cfs) at time step t

Rai(t) Accumulated runoff (inches) at time step t for impervious area
Rap(t) Accumulated runoff (inches) at time step t for pervious area
Rii(t) Incremental runoff (inches) at time step t for impervious area
Rip(t) Incremental runoff (inches) at time step t for pervious area

R(t) Incremental weighted total runoff (inches)

Rtm Time increment for rainfall table

Si S for impervious area: Si = (1000/CNi) - 10Sp S for pervious area: Sp = (1000/CNp) - 10

t Time step (row) number
Tc Time of concentration

Tb Time (hrs) of entire unit hydrograph: Tb = Tp + TrTp Time (hrs) to peak of a unit hydrograph: Tp = (dt/2) + LagTr Time (hrs) of receding limb of unit hydrograph: Tr = ratio of Tp

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method Computational Notes Precipitation

Column (1) Time for time step t

Column (2) D(t) = Point on distribution curve for time step t Column (3) <math>Pi(t) = Pa(t) - Pa(t-1): Col.(4) - Preceding Col.(4)

Column (4) $Pa(t) = D(t) \times P$: $Col.(2) \times P$

Pervious Area Runoff (using SCS Runoff CN Method)

Column (5) Rap(t) = Accumulated pervious runoff for time step t

If $(Pa(t) is \le 0.2Sp)$ then use: Rap(t) = 0.0

If (Pa(t) is > 0.2Sp) then use:

Rap(t) = (Col.(4)-0.2Sp)**2 / (Col.(4)+0.8Sp)

Column (6) Rip(t) = Incremental pervious runoff for time step t

Rip(t) = Rap(t) - Rap(t-1)

Rip(t) = Col.(5) for current row - Col.(5) for preceding row.

Impervious Area Runoff

Column (7 & 8)... Did not specify to use impervious areas.

Incremental Weighted Runoff

Column (9) $R(t) = (Ap/At) \times Rip(t) + (Ai/At) \times Rii(t)$ $R(t) = (Ap/At) \times Col.(6) + (Ai/At) \times Col.(8)$

SCS Unit Hydrograph Method

Column (10) Q(t) is computed with the SCS unit hydrograph method

using R(t) and Qu(t).

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA1 - Imp - Cur
Storm Event: Current 2 Year
Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.590 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	18.10 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	0.20
Interpolated Output)	12.00 hours
Flow (Peak Interpolated	10.42 ft ³ /s
Output)	10.12.10 / 5
Drainage Area	
	00.05
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention	
(Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth	3,25 in
(Pervious)	
Runoff Volume (Pervious)	1.512 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	1.460 ac-ft
- Claime	11.150 de 16
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.11 hours
Computational Time	0.04 have
Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft³/s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA1 - Imp - Cur Storm Event: Current 2 Year Scenario: Current 2 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 2 years

Label: EDA1 - Imp - Cur Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.01	0.04	0.06	0.07
2.25	0.09	0.11	0.13	0.14	0.15
3.50	0.17	0.18	0.19	0.20	0.21
4.75	0.22	0.24	0.25	0.26	0.26
6.00	0.27	0.29	0.32	0.34	0.36
7.25	0.39	0.41	0.43	0.46	0.48
8.50	0.50	0.53	0.55	0.62	0.70
9.75	0.77	0.85	0.93	1.01	1.25
11.00	1.54	1.99	2.50	3.92	10.42
12.25	7.89	3.63	2.41	1.90	1.51
13.50	1.22	1.03	0.95	0.88	0.80
14.75	0.72	0.65	0.59	0.57	0.55
16.00	0.53	0.51	0.48	0.46	0.44
17.25	0.42	0.40	0.38	0.35	0.34
18.50	0.33	0.33	0.32	0.32	0.31
19.75	0.31	0.30	0.30	0.29	0.29
21.00	0.28	0.27	0.27	0.26	0.26
22.25	0.25	0.25	0.24	0.24	0.23
23.50	0.23	0.22	0.21	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA1 - Imp - Cur Scenario: Current 10 year Return Event: 10 years Storm Event: Current 10

10 year	
Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.11 hours
(Composite)	F F00
Area (User Defined)	5.590 acres
Computational Time	
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	28.48 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 110015
Flow (Peak Interpolated	16.42 ft ³ /s
Output)	·
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention	0.20 in
(Pervious)	0.20 in
Maximum Retention	0.04 in
(Pervious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth	F 20 :
(Pervious)	5.20 in
Runoff Volume (Pervious)	2.424 ac-ft
Hydrograph Volume (Area und	er Hydrograph curve)
Volume	2.341 ac-ft
SCS Unit Hydrograph Paramet	ters
Time of Concentration	0.11 hours
(Composite)	U.11 NOURS
Computational Time	0.01 hours
Increment	
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours
· · ·	

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Imp - Cur Storm Event: Current 10 Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.05	0.09	0.13
1.75	0.17	0.20	0.23	0.25	0.27
3.00	0.29	0.31	0.33	0.35	0.37
4.25	0.38	0.40	0.41	0.43	0.44
5.50	0.45	0.47	0.48	0.51	0.54
6.75	0.58	0.62	0.65	0.69	0.72
8.00	0.76	0.79	0.83	0.87	0.90
9.25	1.01	1.13	1.25	1.38	1.49
10.50	1.62	2.01	2.46	3.17	3.96
11.75	6.21	16.42	12.40	5.70	3.78
13.00	2.98	2.36	1.91	1.62	1.49
14.25	1.38	1.25	1.13	1.01	0.93
15.50	0.90	0.86	0.83	0.79	0.76
16.75	0.73	0.69	0.66	0.62	0.59
18.00	0.56	0.53	0.52	0.51	0.51
19.25	0.50	0.49	0.48	0.47	0.46
20.50	0.46	0.45	0.44	0.43	0.42
21.75	0.41	0.41	0.40	0.39	0.38
23.00	0.37	0.36	0.35	0.35	0.34

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA1 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100		
Return Event	100 years		
Duration	24.00 hours		
Depth	9.40 in		
Time of Concentration	0.11 hours		
(Composite)	0.11 Hours		
Area (User Defined)	5.590 acres		
Computational Time Increment	0.01 hours		
Time to Peak (Computed)	12.11 hours		
Flow (Peak, Computed)	49.36 ft ³ /s		
Output Increment	0.25 hours		
Time to Flow (Peak	12.00 hours		
Interpolated Output)	12.00 110015		
Flow (Peak Interpolated	28.50 ft ³ /s		
Output)			
Drainage Area			
SCS CN (Composite)	98.000		
Area (User Defined)	5.590 acres		
Maximum Retention (Pervious)	0.20 in		
Maximum Retention (Pervious, 20 percent)	0.04 in		
(1 civious, 20 percent)			
Cumulative Runoff			
Cumulative Runoff Depth (Pervious)	9.16 in		
Runoff Volume (Pervious)	4.267 ac-ft		
Hydrograph Volume (Area under	r Hydrograph curve)		
Volume	4.123 ac-ft		
SCS Unit Hydrograph Paramete	re		
- 3C3 Offic Hydrograph Paramete	15		
Time of Concentration (Composite)	0.11 hours		
Computational Time	0.01 hours		
Increment	0101 110010		
Unit Hydrograph Shape Factor	483.432		
K Factor	0.749		
Receding/Rising, Tr/Tp	1.670		
Unit peak, qp	57.58 ft ³ /s		
Unit peak time, Tp	0.07 hours		
Unit receding limb, Tr	0.29 hours		

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA1 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: EDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.02	0.14	0.25	0.32
1.50	0.39	0.44	0.49	0.53	0.56
2.75	0.60	0.63	0.65	0.68	0.70
4.00	0.73	0.75	0.77	0.79	0.81
5.25	0.83	0.85	0.87	0.88	0.94
6.50	1.00	1.06	1.12	1.18	1.24
7.75	1.30	1.36	1.42	1.48	1.54
9.00	1.60	1.79	2.00	2.20	2.42
10.25	2.62	2.84	3.51	4.29	5.53
11.50	6.90	10.80	28.50	21.48	9.87
12.75	6.54	5.15	4.09	3.31	2.80
14.00	2.59	2.38	2.17	1.96	1.75
15.25	1.61	1.55	1.49	1.43	1.37
16.50	1.31	1.26	1.20	1.14	1.08
17.75	1.02	0.96	0.92	0.90	0.89
19.00	0.88	0.86	0.85	0.83	0.82
20.25	0.80	0.79	0.77	0.76	0.74
21.50	0.73	0.71	0.70	0.69	0.67
22.75	0.66	0.64	0.62	0.61	0.60
24.00	0.58	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA1 - Imp - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2			
Return Event	2 years			
Duration	24.00 hours			
Depth	4.14 in			
Time of Concentration (Composite)	0.11 hours			
Area (User Defined)	5.590 acres			
Computational Time Increment	0.01 hours			
Time to Peak (Computed)	12.11 hours			
Flow (Peak, Computed)	21.60 ft ³ /s			
Output Increment	0.25 hours			
Time to Flow (Peak Interpolated Output)	12.00 hours			
Flow (Peak Interpolated Output)	12.44 ft³/s			
Drainage Area				
SCS CN (Composite)	98.000			
Area (User Defined)	5.590 acres			
Maximum Retention (Pervious)	0.20 in			
Maximum Retention (Pervious, 20 percent)	0.04 in			
Cumulative Runoff				
Cumulative Runoff Depth (Pervious)	3.90 in			
Runoff Volume (Pervious)	1.819 ac-ft			
Hydrograph Volume (Area under	Hydrograph curvo)			
Hydrograph Volume (Area under Hydrograph curve)				
Volume	1.756 ac-ft			
SCS Unit Hydrograph Parameters	S			
Time of Concentration (Composite)	0.11 hours			
Computational Time Increment	0.01 hours			
Unit Hydrograph Shape Factor	483.432			
K Factor	0.749			
Receding/Rising, Tr/Tp	1.670			
Unit peak, qp	57.58 ft ³ /s			
Unit peak time, Tp	0.07 hours			
Unit receding limb, Tr	0.29 hours			

Subsection: Unit Hydrograph Summary

Return Event: 2 years Label: EDA1 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: EDA1 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.11 hours (Composite) Area (User Defined) 5.590 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.01	0.04	0.06	0.09
2.00	0.11	0.14	0.15	0.17	0.19
3.25	0.21	0.22	0.24	0.25	0.26
4.50	0.28	0.29	0.30	0.31	0.32
5.75	0.33	0.34	0.37	0.39	0.42
7.00	0.45	0.48	0.50	0.53	0.56
8.25	0.59	0.62	0.64	0.67	0.75
9.50	0.84	0.93	1.03	1.12	1.22
10.75	1.51	1.85	2.39	2.99	4.69
12.00	12.44	9.41	4.33	2.87	2.26
13.25	1.80	1.45	1.23	1.14	1.05
14.50	0.95	0.86	0.77	0.71	0.68
15.75	0.66	0.63	0.60	0.58	0.55
17.00	0.53	0.50	0.47	0.45	0.42
18.25	0.40	0.40	0.39	0.38	0.38
19.50	0.37	0.37	0.36	0.35	0.35
20.75	0.34	0.33	0.33	0.32	0.31
22.00	0.31	0.30	0.29	0.29	0.28
23.25	0.27	0.27	0.26	0.26	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

Storm Event	Future 10	
Return Event	10 years	
Duration	24.00 hours	
Depth	6.41 in	
Time of Concentration	0.11 hours	
(Composite)	0.11 Hours	
Area (User Defined)	5.590 acres	
		_
Computational Time Increment	0.01 hours	
Time to Peak (Computed)	12.11 hours	
Flow (Peak, Computed)	33.60 ft ³ /s	
Output Increment	0.25 hours	
Time to Flow (Peak	12.00 hours	
Interpolated Output)	12.0000.0	
Flow (Peak Interpolated Output)	19.38 ft ³ /s	
Drainage Area		
SCS CN (Composite)	98.000	
Area (User Defined)	5.590 acres	
Maximum Retention	0.20:-	
(Pervious)	0.20 in	
Maximum Retention	0.04 in	
(Pervious, 20 percent)		
Cumulative Runoff		
Cumulative Runoff Depth (Pervious)	6.17 in	
Runoff Volume (Pervious)	2.875 ac-ft	
		_
Hydrograph Volume (Area under F	lydrograph curve)	
Volume	2.777 ac-ft	
CCC Unit Undragraph Darameters		
SCS Unit Hydrograph Parameters		
Time of Concentration (Composite)	0.11 hours	
Computational Time Increment	0.01 hours	
Unit Hydrograph Shape Factor	483.432	
K Factor	0.749	
Receding/Rising, Tr/Tp	1.670	
Unit peak, qp	57.58 ft ³ /s	
Unit peak time, Tp	0.07 hours	
Unit receding limb, Tr	0.29 hours	
5 ,		

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA1 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.11 hours (Composite) Area (User Defined) 5.590 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.03	0.09	0.15	0.19
1.75	0.23	0.27	0.30	0.33	0.35
3.00	0.37	0.40	0.42	0.44	0.46
4.25	0.47	0.49	0.50	0.52	0.54
5.50	0.55	0.56	0.58	0.61	0.66
6.75	0.70	0.74	0.78	0.82	0.87
8.00	0.91	0.95	0.99	1.03	1.08
9.25	1.20	1.35	1.49	1.63	1.77
10.50	1.92	2.38	2.91	3.75	4.69
11.75	7.33	19.38	14.63	6.72	4.46
13.00	3.51	2.79	2.25	1.91	1.76
14.25	1.62	1.48	1.34	1.19	1.10
15.50	1.06	1.02	0.98	0.93	0.89
16.75	0.86	0.81	0.77	0.73	0.70
18.00	0.65	0.63	0.62	0.61	0.60
19.25	0.59	0.58	0.57	0.56	0.55
20.50	0.54	0.53	0.52	0.51	0.50
21.75	0.49	0.48	0.47	0.46	0.45
23.00	0.44	0.43	0.42	0.41	0.40

Subsection: Unit Hydrograph Summary

Label: EDA1 - Imp - Fut

Return Event: 100 years

Storm Event: Future 100

Label: EDA1 - Imp - Fut Scenario: Future 100 year

100 year	
Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.590 acres
0 1 1 7	
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	61.05 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	35.25 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	5.301 ac-ft
Hydrograph Volume (Area under	Hvdrograph curve)
Volume	5.123 ac-ft
SCS Unit Hydrograph Parameters	3
Time of Concentration (Composite)	0.11 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours
- ,	

Subsection: Unit Hydrograph Summary Return Event: 100 years
Label: EDA1 - Imp - Fut Storm Event: Future 100

Label: EDA1 - Imp - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.37 hours

5.590 acres

Subsection: Unit Hydrograph (Hydrograph Table)

Area (User Defined)

Label: EDA1 - Imp - Fut Scenario: Future 100 year

Storm Event Future 100
Return Event 100 years
Duration 24.00 hours
Depth 11.62 in
Time of Concentration (Composite) 0.11 hours

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.08	0.25	0.38	0.47
1.50	0.55	0.61	0.66	0.71	0.74
2.75	0.78	0.81	0.84	0.88	0.90
4.00	0.93	0.95	0.97	1.00	1.02
5.25	1.04	1.07	1.09	1.11	1.17
6.50	1.25	1.32	1.40	1.47	1.55
7.75	1.62	1.70	1.77	1.84	1.92
9.00	1.99	2.22	2.48	2.74	3.00
10.25	3.25	3.52	4.35	5.31	6.85
11.50	8.55	13.36	35.25	26.57	12.21
12.75	8.09	6.37	5.06	4.09	3.46
14.00	3.20	2.94	2.68	2.43	2.16
15.25	1.99	1.92	1.85	1.77	1.70
16.50	1.62	1.55	1.48	1.41	1.33
17.75	1.26	1.19	1.14	1.12	1.10
19.00	1.08	1.07	1.05	1.03	1.01
20.25	0.99	0.97	0.96	0.94	0.92
21.50	0.90	0.88	0.87	0.85	0.83
22.75	0.81	0.79	0.77	0.76	0.74
24.00	0.72	(N/A)	(N/A)	(N/A)	(N/A)

Return Event: 100 years

Storm Event: Future 100

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA1 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.20 hours
(Composite)	0.20 110013
Area (User Defined)	1.400 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.27 hours
Flow (Peak, Computed)	0.16 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.2E hours
Interpolated Output)	12.25 hours
Flow (Peak Interpolated	0.16 ft ³ /s
Output)	-
Drainage Area	
Drainage Area	
SCS CN (Composite)	54.350
Area (User Defined)	1.400 acres
Maximum Retention	8.40 in
(Pervious) Maximum Retention	
(Pervious, 20 percent)	1.68 in
Cumulative Runoff	
Cumulative Runoff Depth	0.32 in
(Pervious)	
Runoff Volume (Pervious)	0.037 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.037 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.20 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	7.93 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.53 hours
J ··· ·- / ··	

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA1 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.67 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA1 - Per - Cur

Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.20 hours
Area (User Defined)	1.400 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.16	0.13	0.10	0.09
13.25	0.08	0.07	0.06	0.05	0.05
14.50	0.05	0.04	0.04	0.04	0.04
15.75	0.04	0.03	0.03	0.03	0.03
17.00	0.03	0.03	0.03	0.03	0.03
18.25	0.02	0.02	0.02	0.02	0.02
19.50	0.02	0.02	0.02	0.02	0.02
20.75	0.02	0.02	0.02	0.02	0.02
22.00	0.02	0.02	0.02	0.02	0.02
23.25	0.02	0.02	0.02	0.02	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Per - Cur Storm Event: Current 10 Scenario: Current 10 year

•	
Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.20 havea
(Composite)	0.20 hours
Area (User Defined)	1.400 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.19 hours
Flow (Peak, Computed)	1.31 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.23 110015
Flow (Peak Interpolated	1.15 ft ³ /s
Output)	
Drainage Area	
SCS CN (Composite)	54.350
Area (User Defined)	1.400 acres
Maximum Retention (Pervious)	8.40 in
Maximum Retention	
(Pervious, 20 percent)	1.68 in
Cumulative Runoff	
Cumulative Runoff Depth	1.16 in
(Pervious)	
Runoff Volume (Pervious)	0.136 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.134 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.20 hours
Computational Time	0.03 hours
Increment	0.03 Hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	7.93 ft³/s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.53 hours

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Per - Cur Storm Event: Current 10 Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.67 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.20 hours
Area (User Defined)	1.400 acres

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
11.50	0.00	0.05	0.36	1.15	0.55
12.75	0.37	0.30	0.25	0.21	0.17
14.00	0.16	0.15	0.14	0.13	0.12
15.25	0.11	0.10	0.10	0.10	0.09
16.50	0.09	0.09	0.08	0.08	0.08
17.75	0.07	0.07	0.07	0.06	0.06
19.00	0.06	0.06	0.06	0.06	0.06
20.25	0.06	0.06	0.06	0.06	0.05
21.50	0.05	0.05	0.05	0.05	0.05
22.75	0.05	0.05	0.05	0.05	0.05
24.00	0.04	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA1 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.20 hours
(Composite)	
Area (User Defined)	1.400 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.19 hours
Flow (Peak, Computed)	4.79 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.20
Flow (Peak Interpolated Output)	3.98 ft ³ /s
Drainage Area	
SCS CN (Composite)	54.350
Area (User Defined)	1.400 acres
Maximum Retention	8.40 in
(Pervious)	0. 10 111
Maximum Retention (Pervious, 20 percent)	1.68 in
0 1 1: 5 "	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.70 in
Runoff Volume (Pervious)	0.431 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.425 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.20 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	7.93 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.53 hours
3 -,	

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA1 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.67 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: EDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.20 hours
Area (User Defined)	1.400 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.50	0.00	0.00	0.02	0.03	0.05
10.75	0.09	0.14	0.23	0.36	0.68
12.00	1.95	3.98	1.68	1.08	0.85
13.25	0.68	0.56	0.47	0.43	0.40
14.50	0.37	0.34	0.30	0.28	0.27
15.75	0.26	0.25	0.24	0.23	0.22
17.00	0.21	0.20	0.19	0.18	0.17
18.25	0.16	0.16	0.16	0.16	0.15
19.50	0.15	0.15	0.15	0.14	0.14
20.75	0.14	0.14	0.13	0.13	0.13
22.00	0.13	0.12	0.12	0.12	0.12
23.25	0.11	0.11	0.11	0.11	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA1 - Per - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration	0.19 hours
(Composite)	0.15 110015
Area (User Defined)	1.400 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.21 hours
Flow (Peak, Computed)	0.47 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.25 110013
Flow (Peak Interpolated Output)	0.44 ft ³ /s
Drainage Area	
SCS CN (Composite)	54.350
Area (User Defined)	1.400 acres
Maximum Retention (Pervious)	8.40 in
Maximum Retention	1.68 in
(Pervious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth	0.56 in
(Pervious) Runoff Volume (Pervious)	0.065 ac-ft
Tanon volume (rervious)	0.005 uc 10
Hydrograph Volume (Area under H	ydrograph curve)
Volume	0.064 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration	0.10 5
(Composite)	0.19 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape	
Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.35 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.51 hours

Subsection: Unit Hydrograph Summary

Label: EDA1 - Per - Fut Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Return Event: 2 years

Storm Event: Future 2

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: EDA1 - Per - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.19 hours (Composite) Area (User Defined) 1.400 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.75	0.00	0.04	0.44	0.25	0.18
13.00	0.15	0.13	0.11	0.09	0.09
14.25	0.08	0.08	0.07	0.06	0.06
15.50	0.06	0.06	0.05	0.05	0.05
16.75	0.05	0.05	0.04	0.04	0.04
18.00	0.04	0.04	0.04	0.04	0.04
19.25	0.04	0.03	0.03	0.03	0.03
20.50	0.03	0.03	0.03	0.03	0.03
21.75	0.03	0.03	0.03	0.03	0.03
23.00	0.03	0.03	0.03	0.03	0.03

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

Future 10
10 years
24.00 hours
6.41 in
0.19 hours
1.400 acres
0.03 hours
12.19 hours
2.10 ft ³ /s
0.25 hours
12.25 hours
TELES HOULS
1.74 ft ³ /s
54.350
1.400 acres
8.40 in
1.68 in
1.70 in
0.199 ac-ft
Hydrograph curve)
0.196 ac-ft
0.19 hours
0.03 hours
483.432
0.749
1.670
8.35 ft ³ /s
0.13 hours
0.51 hours

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA1 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA1 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.19 hours (Composite) Area (User Defined) 1.400 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.00	0.00	0.01	0.06	0.18	0.73
12.25	1.74	0.79	0.53	0.43	0.34
13.50	0.29	0.24	0.22	0.21	0.19
14.75	0.18	0.16	0.15	0.14	0.14
16.00	0.13	0.13	0.12	0.12	0.11
17.25	0.11	0.10	0.10	0.09	0.09
18.50	0.09	0.09	0.08	0.08	0.08
19.75	0.08	0.08	0.08	0.08	0.08
21.00	0.07	0.07	0.07	0.07	0.07
22.25	0.07	0.07	0.07	0.06	0.06
23.50	0.06	0.06	0.06	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA1 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.19 hours
Area (User Defined)	1.400 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.16 hours
Flow (Peak, Computed)	7.22 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.23 110013
Flow (Peak Interpolated Output)	5.67 ft³/s
Drainage Area	
SCS CN (Composite)	54.350
Area (User Defined)	1.400 acres
Maximum Retention (Pervious)	8.40 in
Maximum Retention (Pervious, 20 percent)	1.68 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.39 in
Runoff Volume (Pervious)	0.629 ac-ft
Hydrograph Volume (Area under	Hvdrograph curve)
Volume	0.618 ac-ft
voiume	0.010 aC-10
SCS Unit Hydrograph Parameter	rs
Time of Concentration (Composite)	0.19 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.35 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.51 hours

Subsection: Unit Hydrograph Summary

Label: EDA1 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: EDA1 - Per - Fut Scenario: Future 100 year Return Event: 100 years Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.19 hours
Area (User Defined)	1.400 acres

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
8.50	0.00	0.00	0.01	0.03	0.04
9.75	0.06	0.09	0.12	0.15	0.21
11.00	0.30	0.44	0.65	1.15	3.17
12.25	5.67	2.33	1.49	1.18	0.94
13.50	0.77	0.64	0.59	0.55	0.50
14.75	0.46	0.41	0.38	0.36	0.35
16.00	0.34	0.32	0.31	0.30	0.28
17.25	0.27	0.26	0.24	0.23	0.22
18.50	0.22	0.21	0.21	0.21	0.20
19.75	0.20	0.20	0.19	0.19	0.19
21.00	0.18	0.18	0.18	0.17	0.17
22.25	0.17	0.16	0.16	0.16	0.15
23.50	0.15	0.15	0.14	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA2 - Imp - Cur
Storm Event: Current 2 Year
Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.05 hours
(Composite)	0.03 110013
Area (User Defined)	0.810 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	2.91 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 havea
Interpolated Output)	12.00 hours
Flow (Peak Interpolated	1.74 ft ³ /s
Output)	11, 110 / 0
Duein and Ana-	
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.810 acres
Maximum Retention	0.20 in
(Pervious)	
Maximum Retention (Pervious, 20 percent)	0.04 in
(* 5. 115 55) 25 (5. 15 15)	
Cumulative Runoff	
Cumulative Runoff Depth	3,25 in
(Pervious)	J.25 III
Runoff Volume (Pervious)	0.219 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.211 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	18.36 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
- '	

Subsection: Unit Hydrograph Summary

Return Event: 2 years

Label: EDA2 - Imp - Cur

Storm Event: Current 2 Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 2 years

Label: EDA2 - Imp - Cur Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.00	0.01	0.01	0.01
2.25	0.01	0.02	0.02	0.02	0.02
3.50	0.02	0.03	0.03	0.03	0.03
4.75	0.03	0.03	0.04	0.04	0.04
6.00	0.04	0.04	0.05	0.05	0.05
7.25	0.06	0.06	0.06	0.07	0.07
8.50	0.07	0.08	0.08	0.09	0.10
9.75	0.11	0.12	0.14	0.15	0.19
11.00	0.23	0.31	0.37	0.68	1.74
12.25	0.81	0.51	0.33	0.27	0.21
13.50	0.17	0.15	0.14	0.12	0.11
14.75	0.10	0.09	0.09	0.08	0.08
16.00	0.08	0.07	0.07	0.07	0.06
17.25	0.06	0.06	0.05	0.05	0.05
18.50	0.05	0.05	0.05	0.05	0.05
19.75	0.04	0.04	0.04	0.04	0.04
21.00	0.04	0.04	0.04	0.04	0.04
22.25	0.04	0.04	0.03	0.03	0.03
23.50	0.03	0.03	0.03	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Cur Scenario: Current 10 year Return Event: 10 years Storm Event: Current 10

Storm Event	Current 10	
Return Event	10 years	
Duration	24.00 hours	
Depth	5.44 in	
Time of Concentration (Composite)	0.05 hours	
Area (User Defined)	0.810 acres	
Computational Time Increment	0.01 hours	
Time to Peak (Computed)	12.10 hours	
Flow (Peak, Computed)	4.58 ft ³ /s	
Output Increment	0.25 hours	
Time to Flow (Peak Interpolated Output)	12.00 hours	
Flow (Peak Interpolated Output)	2.74 ft³/s	
Drainage Area		
SCS CN (Composite)	98.000	
Area (User Defined)	0.810 acres	
Maximum Retention	0.010 ucres	
(Pervious)	0.20 in	
Maximum Retention (Pervious, 20 percent)	0.04 in	
Cumulative Runoff		
Cumulative Runoff Depth (Pervious)	5.20 in	
Runoff Volume (Pervious)	0.351 ac-ft	
· · ·		
Runoff Volume (Pervious)		
Runoff Volume (Pervious) Hydrograph Volume (Area under	Hydrograph curve) 0.339 ac-ft	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration	Hydrograph curve) 0.339 ac-ft	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters	Hydrograph curve) 0.339 ac-ft	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration (Composite) Computational Time	Hydrograph curve) 0.339 ac-ft s 0.05 hours	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape	Hydrograph curve) 0.339 ac-ft s 0.05 hours 0.01 hours	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor	Hydrograph curve) 0.339 ac-ft 0.05 hours 0.01 hours 483.432	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor	0.339 ac-ft 0.05 hours 0.01 hours 483.432 0.749	
Runoff Volume (Pervious) Hydrograph Volume (Area under Volume SCS Unit Hydrograph Parameters Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp	0.339 ac-ft 0.05 hours 0.01 hours 483.432 0.749 1.670	

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Cur Scenario: Current 10 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.17 hours

Return Event: 10 years

Storm Event: Current 10

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.01	0.01	0.02
1.75	0.03	0.03	0.03	0.04	0.04
3.00	0.04	0.05	0.05	0.05	0.05
4.25	0.06	0.06	0.06	0.06	0.06
5.50	0.07	0.07	0.07	0.08	0.08
6.75	0.09	0.09	0.10	0.10	0.11
8.00	0.11	0.12	0.12	0.13	0.13
9.25	0.15	0.17	0.19	0.20	0.22
10.50	0.24	0.31	0.36	0.49	0.59
11.75	1.07	2.74	1.27	0.80	0.52
13.00	0.42	0.33	0.27	0.23	0.21
14.25	0.20	0.18	0.16	0.14	0.13
15.50	0.13	0.12	0.12	0.11	0.11
16.75	0.10	0.10	0.09	0.09	0.08
18.00	0.08	0.08	0.08	0.07	0.07
19.25	0.07	0.07	0.07	0.07	0.07
20.50	0.07	0.06	0.06	0.06	0.06
21.75	0.06	0.06	0.06	0.06	0.05
23.00	0.05	0.05	0.05	0.05	0.05

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA2 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

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Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.05 hours
(Composite)	0.03 110413
Area (User Defined)	0.810 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	7.94 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00
Flow (Peak Interpolated Output)	4.75 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.810 acres
Maximum Retention	0.20 in
(Pervious)	
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	0.618 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.597 ac-ft
SCS Unit Hydrograph Parameter	·s
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	18.36 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
5 -,	

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA2 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: EDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.02	0.04	0.05
1.50	0.06	0.07	0.07	0.08	0.08
2.75	0.09	0.09	0.10	0.10	0.10
4.00	0.11	0.11	0.11	0.12	0.12
5.25	0.12	0.12	0.13	0.13	0.14
6.50	0.15	0.16	0.16	0.17	0.18
7.75	0.19	0.20	0.21	0.22	0.23
9.00	0.23	0.27	0.29	0.33	0.35
10.25	0.39	0.42	0.54	0.64	0.85
11.50	1.03	1.86	4.75	2.20	1.39
12.75	0.89	0.72	0.56	0.46	0.40
14.00	0.37	0.34	0.31	0.28	0.25
15.25	0.23	0.22	0.21	0.21	0.20
16.50	0.19	0.18	0.17	0.16	0.16
17.75	0.15	0.14	0.13	0.13	0.13
19.00	0.13	0.12	0.12	0.12	0.12
20.25	0.12	0.11	0.11	0.11	0.11
21.50	0.11	0.10	0.10	0.10	0.10
22.75	0.09	0.09	0.09	0.09	0.09
24.00	0.08	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

•				
Storm Event	Future 2			
Return Event	2 years			
Duration	24.00 hours			
Depth	4.14 in			
Time of Concentration	0.05 hours			
(Composite)				
Area (User Defined)	0.810 acres			
Computational Time Increment	0.01 hours			
Time to Peak (Computed)	12.10 hours			
Flow (Peak, Computed)	3.48 ft ³ /s			
Output Increment	0.25 hours			
Time to Flow (Peak	12.00 hours			
Interpolated Output)				
Flow (Peak Interpolated Output)	2.08 ft ³ /s			
Drainage Area				
SCS CN (Composite)	98.000			
Area (User Defined)	0.810 acres			
Maximum Retention	0.20 in			
(Pervious) Maximum Retention				
(Pervious, 20 percent)	0.04 in			
Cumulative Runoff				
Cumulative Runoff Depth (Pervious)	3.90 in			
Runoff Volume (Pervious)	0.264 ac-ft			
Hydrograph Volume (Area under Hydrograph curve)				
Volume	0.254 ac-ft			
SCS Unit Hydrograph Parameters				
Time of Concentration				
(Composite)	0.05 hours			
Computational Time Increment	0.01 hours			
Unit Hydrograph Shape Factor	483.432			
K Factor	0.749			
Receding/Rising, Tr/Tp	1.670			
Unit peak, qp	18.36 ft ³ /s			
Unit peak time, Tp	0.03 hours			
Unit receding limb, Tr	0.13 hours			

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Fut Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.17 hours

Return Event: 2 years

Storm Event: Future 2

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: EDA2 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.05 hours (Composite) Area (User Defined) 0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.00	0.01	0.01	0.01
2.00	0.02	0.02	0.02	0.03	0.03
3.25	0.03	0.03	0.03	0.04	0.04
4.50	0.04	0.04	0.04	0.05	0.05
5.75	0.05	0.05	0.05	0.06	0.06
7.00	0.07	0.07	0.07	0.08	0.08
8.25	0.09	0.09	0.09	0.10	0.11
9.50	0.12	0.14	0.15	0.17	0.18
10.75	0.23	0.27	0.37	0.44	0.81
12.00	2.08	0.97	0.61	0.39	0.32
13.25	0.25	0.20	0.17	0.16	0.15
14.50	0.14	0.12	0.11	0.10	0.10
15.75	0.09	0.09	0.09	0.08	0.08
17.00	0.08	0.07	0.07	0.06	0.06
18.25	0.06	0.06	0.06	0.06	0.05
19.50	0.05	0.05	0.05	0.05	0.05
20.75	0.05	0.05	0.05	0.05	0.05
22.00	0.04	0.04	0.04	0.04	0.04
23.25	0.04	0.04	0.04	0.04	(N/A)

Subsection: Unit Hydrograph Summary
Label: EDA2 - Imp - Fut
Return Event: 10 years
Storm Event: Future 10

Label: EDA2 - Imp - Fut Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration	0.05 hours
(Composite)	
Area (User Defined)	0.810 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	5.40 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output) Flow (Peak Interpolated	
Output)	3.23 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.810 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth	6.17 in
(Pervious)	0 417 as f r
Runoff Volume (Pervious)	0.417 ac-ft
Hydrograph Volume (Area under H	lydrograph curve)
Volume	0.402 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	18.36 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
- ,	

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA2 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA2 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.05 hours (Composite) Area (User Defined) 0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.01	0.01	0.02	0.03
1.75	0.03	0.04	0.04	0.05	0.05
3.00	0.05	0.06	0.06	0.06	0.07
4.25	0.07	0.07	0.07	0.08	0.08
5.50	0.08	0.08	0.08	0.09	0.10
6.75	0.10	0.11	0.11	0.12	0.13
8.00	0.13	0.14	0.14	0.15	0.16
9.25	0.18	0.20	0.22	0.24	0.26
10.50	0.28	0.36	0.43	0.58	0.70
11.75	1.27	3.23	1.50	0.95	0.61
13.00	0.49	0.38	0.32	0.27	0.25
14.25	0.23	0.21	0.19	0.17	0.16
15.50	0.15	0.15	0.14	0.13	0.13
16.75	0.12	0.12	0.11	0.11	0.10
18.00	0.09	0.09	0.09	0.09	0.09
19.25	0.08	0.08	0.08	0.08	0.08
20.50	0.08	0.08	0.07	0.07	0.07
21.75	0.07	0.07	0.07	0.07	0.06
23.00	0.06	0.06	0.06	0.06	0.06

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Fut

Return Event: 100 years

Storm Event: Future 100

Label: EDA2 - Imp - Fut Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration	0.05 hours
(Composite)	0.010
Area (User Defined)	0.810 acres
Constalling LTime	
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	9.81 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.0000.0
Flow (Peak Interpolated Output)	5.88 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.810 acres
Maximum Retention	0.20 in
(Pervious)	0.20 111
Maximum Retention (Pervious, 20 percent)	0.04 in
(Ferricas) to percently	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	0.768 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.742 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape	483.432
Factor K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	18.36 ft ³ /s
Unit peak, qp	0.03 hours
Unit receding limb, Tr	0.13 hours
one receasing similar, in	0.13 HOUIS

Subsection: Unit Hydrograph Summary

Label: EDA2 - Imp - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.17 hours

Return Event: 100 years

Storm Event: Future 100

Subsection: Unit Hydrograph (Hydrograph Table)

Label: EDA2 - Imp - Fut Scenario: Future 100 year Storm Event: Future 100

Return Event: 100 years

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.810 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.02	0.04	0.06	0.07
1.50	0.08	0.09	0.10	0.10	0.11
2.75	0.11	0.12	0.12	0.13	0.13
4.00	0.13	0.14	0.14	0.15	0.15
5.25	0.15	0.16	0.16	0.16	0.17
6.50	0.18	0.19	0.20	0.22	0.23
7.75	0.24	0.25	0.26	0.27	0.28
9.00	0.29	0.33	0.36	0.41	0.44
10.25	0.48	0.52	0.67	0.79	1.05
11.50	1.27	2.31	5.88	2.72	1.72
12.75	1.11	0.89	0.70	0.57	0.49
14.00	0.46	0.42	0.38	0.34	0.31
15.25	0.29	0.28	0.26	0.26	0.24
16.50	0.23	0.22	0.21	0.20	0.19
17.75	0.18	0.17	0.16	0.16	0.16
19.00	0.16	0.15	0.15	0.15	0.15
20.25	0.14	0.14	0.14	0.14	0.13
21.50	0.13	0.13	0.13	0.12	0.12
22.75	0.12	0.11	0.11	0.11	0.11
24.00	0.10	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA2 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.09 hours
(Composite)	
Area (User Defined)	0.290 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.52 hours
Flow (Peak, Computed)	0.01 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	0.20
Interpolated Output)	12.50 hours
Flow (Peak Interpolated	0.01.03/0
Output)	0.01 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention	9.76 in
(Pervious)	
Maximum Retention (Pervious, 20 percent)	1.95 in
(1 civious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth	0.21 in
(Pervious)	0.21 111
Runoff Volume (Pervious)	0.005 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.005 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	3.65 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours
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Subsection: Unit Hydrograph Summary Return Event: 2 years Storm Event: Current 2 Year

Label: EDA2 - Per - Cur Scenario: Current 2 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.30 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 2 years

Label: EDA2 - Per - Cur Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event Current 2 Year
Return Event 2 years
Duration 24.00 hours
Depth 3.48 in

Time of Concentration (Composite)

Area (User Defined)

0.09 hours
0.290 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.01	0.01	0.01	0.01
13.25	0.01	0.01	0.01	0.01	0.01
14.50	0.01	0.01	0.01	0.01	0.01
15.75	0.01	0.01	0.01	0.00	0.00
17.00	0.00	0.00	0.00	0.00	0.00
18.25	0.00	0.00	0.00	0.00	0.00
19.50	0.00	0.00	0.00	0.00	0.00
20.75	0.00	0.00	0.00	0.00	0.00
22.00	0.00	0.00	0.00	0.00	0.00
23.25	0.00	0.00	0.00	0.00	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Cur

Return Event: 10 years

Storm Event: Current 10

Label: EDA2 - Per - Cur Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.290 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	0.26 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.13 ft³/s
Drainage Area	
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention (Pervious)	9.76 in
Maximum Retention (Pervious, 20 percent)	1.95 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.92 in
Runoff Volume (Pervious)	0.022 ac-ft
Hydrograph Volume (Area under	r Hydrograph curvo)
Volume	0.021 ac-ft
SCS Unit Hydrograph Paramete	rs
Time of Concentration (Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	3.65 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Cur Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.30 hours

Return Event: 10 years

Storm Event: Current 10

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.290 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.75	0.00	0.08	0.13	0.08	0.06
13.00	0.05	0.04	0.03	0.03	0.03
14.25	0.03	0.02	0.02	0.02	0.02
15.50	0.02	0.02	0.02	0.02	0.02
16.75	0.02	0.01	0.01	0.01	0.01
18.00	0.01	0.01	0.01	0.01	0.01
19.25	0.01	0.01	0.01	0.01	0.01
20.50	0.01	0.01	0.01	0.01	0.01
21.75	0.01	0.01	0.01	0.01	0.01
23.00	0.01	0.01	0.01	0.01	0.01

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA2 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.290 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	1.11 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	0.52 ft³/s
Drainage Area	
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention	0.76 in
(Pervious)	9.76 in
Maximum Retention (Pervious, 20 percent)	1.95 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.23 in
Runoff Volume (Pervious)	0.078 ac-ft
Hydrograph Volume (Area und	er Hydrograph curve)
Volume	0.074 ac-ft
SCS Unit Hydrograph Paramet	ers
Time of Concentration (Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	3.65 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: EDA2 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.30 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: EDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.290 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.25	0.00	0.00	0.01	0.02	0.04
11.50	0.06	0.13	0.52	0.47	0.26
12.75	0.18	0.15	0.12	0.10	0.08
14.00	0.08	0.07	0.07	0.06	0.06
15.25	0.05	0.05	0.05	0.05	0.04
16.50	0.04	0.04	0.04	0.04	0.04
17.75	0.03	0.03	0.03	0.03	0.03
19.00	0.03	0.03	0.03	0.03	0.03
20.25	0.03	0.03	0.03	0.03	0.03
21.50	0.03	0.02	0.02	0.02	0.02
22.75	0.02	0.02	0.02	0.02	0.02
24.00	0.02	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

,	
Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration	0.08 hours
(Composite)	
Area (User Defined)	0.290 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	0.07 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	11120 110010
Flow (Peak Interpolated Output)	0.04 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention	0.76 :
(Pervious)	9.76 in
Maximum Retention (Pervious, 20 percent)	1.95 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.40 in
Runoff Volume (Pervious)	0.010 ac-ft
Hydrograph Volume (Area under H	ydrograph curve)
Volume	0.009 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration	
(Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	4.11 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Fut

Return Event: 2 years

Storm Event: Future 2

Label: EDA2 - Per - Fut Scenario: Future 2 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: EDA2 - Per - Fut Scenario: Future 2 year

Storm Event: Future 2

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	0.290 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.04	0.03	0.03	0.02
13.25	0.02	0.02	0.01	0.01	0.01
14.50	0.01	0.01	0.01	0.01	0.01
15.75	0.01	0.01	0.01	0.01	0.01
17.00	0.01	0.01	0.01	0.01	0.01
18.25	0.01	0.01	0.01	0.01	0.01
19.50	0.01	0.01	0.01	0.01	0.01
20.75	0.01	0.01	0.01	0.01	0.01
22.00	0.01	0.01	0.00	0.00	0.00
23.25	0.00	0.00	0.00	0.00	(N/A)

Return Event: 2 years

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: EDA2 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration	
(Composite)	0.08 hours
Area (User Defined)	0.290 acres
Computational Time	0.01 hours
Increment	0.01 110015
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	0.45 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12123 110013
Flow (Peak Interpolated	0.20 ft ³ /s
Output)	,
Drainage Area	
	F0 630
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention (Pervious)	9.76 in
Maximum Retention	
(Pervious, 20 percent)	1.95 in
Cumulative Runoff	
Cumulative Runoff Depth	1 40 :
(Pervious)	1.40 in
Runoff Volume (Pervious)	0.034 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.032 ac-ft
000 Heit Heder werd December	
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.08 hours
Computational Time	0.01 hours
Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	4.11 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
3 - 1	

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Fut Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.27 hours

Return Event: 10 years

Storm Event: Future 10

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: EDA2 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.08 hours (Composite) Area (User Defined) 0.290 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.25	0.00	0.00	0.02	0.18	0.20
12.50	0.12	0.08	0.07	0.06	0.05
13.75	0.04	0.04	0.04	0.03	0.03
15.00	0.03	0.03	0.03	0.02	0.02
16.25	0.02	0.02	0.02	0.02	0.02
17.50	0.02	0.02	0.02	0.02	0.02
18.75	0.02	0.02	0.02	0.02	0.01
20.00	0.01	0.01	0.01	0.01	0.01
21.25	0.01	0.01	0.01	0.01	0.01
22.50	0.01	0.01	0.01	0.01	0.01
23.75	0.01	0.01	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Label: EDA2 - Per - Fut Scenario: Future 100 year

.00 year	
Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	0.290 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	1.72 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	0.86 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.620
Area (User Defined)	0.290 acres
Maximum Retention (Pervious)	9.76 in
Maximum Retention (Pervious, 20 percent)	1.95 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.81 in
Runoff Volume (Pervious)	0.116 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.111 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	4.11 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours

Subsection: Unit Hydrograph Summary

Label: EDA2 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: EDA2 - Per - Fut Scenario: Future 100 year Return Event: 100 years Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	0.290 acres

rime on feet represents time for mot value in cash rown					***
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.25	0.00	0.00	0.01	0.01	0.02
10.50	0.02	0.03	0.05	0.08	0.12
11.75	0.24	0.86	0.66	0.38	0.26
13.00	0.21	0.17	0.14	0.12	0.11
14.25	0.10	0.09	0.09	0.08	0.07
15.50	0.07	0.07	0.06	0.06	0.06
16.75	0.06	0.05	0.05	0.05	0.05
18.00	0.04	0.04	0.04	0.04	0.04
19.25	0.04	0.04	0.04	0.04	0.04
20.50	0.04	0.04	0.04	0.04	0.03
21.75	0.03	0.03	0.03	0.03	0.03
23.00	0.03	0.03	0.03	0.03	0.03

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA1 - Imp - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year	
Return Event	2 years	
Duration	24.00 hours	
Depth	3.48 in	
Time of Concentration	0.11 hours	
(Composite)		
Area (User Defined)	5.580 acres	
Constabling LTm.		
Computational Time Increment	0.01 hours	
Time to Peak (Computed)	12.11 hours	
Flow (Peak, Computed)	18.07 ft ³ /s	
Output Increment	0.25 hours	
Time to Flow (Peak	12.00 haves	
Interpolated Output)	12.00 hours	
Flow (Peak Interpolated	10.40 ft ³ /s	
Output)	201.10 10 70	
Drainage Area		
SCS CN (Composite)	98.000	
Area (User Defined)	5.580 acres	
Maximum Retention		
(Pervious)	0.20 in	
Maximum Retention	0.04 in	
Maximum Retention (Pervious, 20 percent)	0.04 in	
	0.04 in	
(Pervious, 20 percent) Cumulative Runoff		
(Pervious, 20 percent)	0.04 in 3.25 in	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth		
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious)	3.25 in 1.510 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und	3.25 in 1.510 ac-ft der Hydrograph curve)	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious)	3.25 in 1.510 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours	
Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours 0.01 hours	
Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours 0.01 hours 483.432	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours 0.01 hours 483.432 0.749	
Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp Unit peak, qp	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours 0.01 hours 483.432 0.749 1.670	
(Pervious, 20 percent) Cumulative Runoff Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp	3.25 in 1.510 ac-ft der Hydrograph curve) 1.457 ac-ft eters 0.11 hours 0.01 hours 483.432 0.749 1.670 57.48 ft³/s	

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA1 - Imp - Cur Storm Event: Current 2 Year Scenario: Current 2 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 2 years

Label: PDA1 - Imp - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.580 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.01	0.04	0.06	0.07
2.25	0.09	0.11	0.13	0.14	0.15
3.50	0.17	0.18	0.19	0.20	0.21
4.75	0.22	0.24	0.25	0.26	0.26
6.00	0.27	0.29	0.32	0.34	0.36
7.25	0.39	0.41	0.43	0.46	0.48
8.50	0.50	0.53	0.55	0.62	0.70
9.75	0.77	0.85	0.93	1.01	1.25
11.00	1.53	1.99	2.49	3.92	10.40
12.25	7.88	3.63	2.41	1.90	1.50
13.50	1.22	1.03	0.95	0.88	0.80
14.75	0.72	0.64	0.59	0.57	0.55
16.00	0.53	0.51	0.48	0.46	0.44
17.25	0.42	0.40	0.38	0.35	0.34
18.50	0.33	0.33	0.32	0.32	0.31
19.75	0.31	0.30	0.30	0.29	0.28
21.00	0.28	0.27	0.27	0.26	0.26
22.25	0.25	0.25	0.24	0.24	0.23
23.50	0.23	0.22	0.21	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA1 - Imp - Cur Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.580 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	28.42 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 110013
Flow (Peak Interpolated	16.39 ft ³ /s
Output)	
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.580 acres
Maximum Retention	0.20:-
(Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.20 in
Runoff Volume (Pervious)	2.419 ac-ft
Hydrograph Volume (Area under H	lydrograph curve)
Volume	2.336 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration	
(Composite)	0.11 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.48 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours

Return Event: 10 years

Storm Event: Current 10

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA1 - Imp - Cur Storm Event: Current 10 Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.580 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.05	0.09	0.13
1.75	0.17	0.20	0.23	0.25	0.27
3.00	0.29	0.31	0.33	0.35	0.37
4.25	0.38	0.40	0.41	0.43	0.44
5.50	0.45	0.46	0.48	0.51	0.54
6.75	0.58	0.62	0.65	0.69	0.72
8.00	0.76	0.79	0.83	0.87	0.90
9.25	1.01	1.13	1.25	1.37	1.49
10.50	1.62	2.00	2.45	3.17	3.96
11.75	6.20	16.39	12.38	5.69	3.77
13.00	2.97	2.36	1.91	1.62	1.49
14.25	1.37	1.25	1.13	1.01	0.93
15.50	0.89	0.86	0.83	0.79	0.76
16.75	0.73	0.69	0.66	0.62	0.59
18.00	0.55	0.53	0.52	0.51	0.51
19.25	0.50	0.49	0.48	0.47	0.46
20.50	0.45	0.45	0.44	0.43	0.42
21.75	0.41	0.40	0.40	0.39	0.38
23.00	0.37	0.36	0.35	0.34	0.34

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA1 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

100 year	
Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.580 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	49.27 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 110013
Flow (Peak Interpolated Output)	28.44 ft ³ /s
Оигриг)	
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.580 acres
Maximum Retention	0.20 in
(Pervious)	0.20 111
Maximum Retention (Pervious, 20 percent)	0.04 in
(rervious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth	9.16 in
(Pervious)	
Runoff Volume (Pervious)	4.259 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	4.116 ac-ft
SCS Unit Hydrograph Parameters	3
Time of Concentration	0.11 hours
(Composite)	0.1100.0
Computational Time Increment	0.01 hours
Unit Hydrograph Shape	
Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.48 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA1 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: PDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.580 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.02	0.14	0.25	0.32
1.50	0.39	0.44	0.49	0.53	0.56
2.75	0.60	0.62	0.65	0.68	0.70
4.00	0.73	0.75	0.77	0.79	0.81
5.25	0.83	0.85	0.86	0.88	0.94
6.50	0.99	1.05	1.12	1.18	1.24
7.75	1.30	1.36	1.42	1.48	1.54
9.00	1.60	1.78	2.00	2.20	2.41
10.25	2.62	2.83	3.51	4.28	5.52
11.50	6.89	10.78	28.44	21.44	9.85
12.75	6.53	5.15	4.08	3.30	2.80
14.00	2.58	2.38	2.16	1.96	1.75
15.25	1.61	1.55	1.49	1.43	1.37
16.50	1.31	1.25	1.19	1.13	1.08
17.75	1.02	0.96	0.92	0.90	0.89
19.00	0.87	0.86	0.85	0.83	0.82
20.25	0.80	0.79	0.77	0.76	0.74
21.50	0.73	0.71	0.70	0.69	0.67
22.75	0.65	0.64	0.62	0.61	0.60
24.00	0.58	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA1 - Imp - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

,	
Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.590 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	21.60 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	
Flow (Peak Interpolated Output)	12.44 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention	0.20 in
(Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.90 in
Runoff Volume (Pervious)	1.819 ac-ft
Hydrograph Volume (Area under H	ydrograph curve)
Volume	1.756 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.11 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft³/s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours

Subsection: Unit Hydrograph Summary

Return Event: 2 years Label: PDA1 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: PDA1 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.11 hours (Composite) Area (User Defined) 5.590 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.01	0.04	0.06	0.09
2.00	0.11	0.14	0.15	0.17	0.19
3.25	0.21	0.22	0.24	0.25	0.26
4.50	0.28	0.29	0.30	0.31	0.32
5.75	0.33	0.34	0.37	0.39	0.42
7.00	0.45	0.48	0.50	0.53	0.56
8.25	0.59	0.62	0.64	0.67	0.75
9.50	0.84	0.93	1.03	1.12	1.22
10.75	1.51	1.85	2.39	2.99	4.69
12.00	12.44	9.41	4.33	2.87	2.26
13.25	1.80	1.45	1.23	1.14	1.05
14.50	0.95	0.86	0.77	0.71	0.68
15.75	0.66	0.63	0.60	0.58	0.55
17.00	0.53	0.50	0.47	0.45	0.42
18.25	0.40	0.40	0.39	0.38	0.38
19.50	0.37	0.37	0.36	0.35	0.35
20.75	0.34	0.33	0.33	0.32	0.31
22.00	0.31	0.30	0.29	0.29	0.28
23.25	0.27	0.27	0.26	0.26	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA1 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration	0.11 hours
(Composite)	
Area (User Defined)	5.590 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	33.60 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00
Flow (Peak Interpolated Output)	19.38 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention	0.20 in
(Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.17 in
Runoff Volume (Pervious)	2.875 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	2.777 ac-ft
SCS Unit Hydrograph Parameter	
Time of Concentration (Composite)	0.11 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours
5 ,	

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA1 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.37 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.03	0.09	0.15	0.19
1.75	0.23	0.27	0.30	0.33	0.35
3.00	0.37	0.40	0.42	0.44	0.46
4.25	0.47	0.49	0.50	0.52	0.54
5.50	0.55	0.56	0.58	0.61	0.66
6.75	0.70	0.74	0.78	0.82	0.87
8.00	0.91	0.95	0.99	1.03	1.08
9.25	1.20	1.35	1.49	1.63	1.77
10.50	1.92	2.38	2.91	3.75	4.69
11.75	7.33	19.38	14.63	6.72	4.46
13.00	3.51	2.79	2.25	1.91	1.76
14.25	1.62	1.48	1.34	1.19	1.10
15.50	1.06	1.02	0.98	0.93	0.89
16.75	0.86	0.81	0.77	0.73	0.70
18.00	0.65	0.63	0.62	0.61	0.60
19.25	0.59	0.58	0.57	0.56	0.55
20.50	0.54	0.53	0.52	0.51	0.50
21.75	0.49	0.48	0.47	0.46	0.45
23.00	0.44	0.43	0.42	0.41	0.40

Subsection: Unit Hydrograph Summary

Label: PDA1 - Imp - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA1 - Imp - Fut Scenario: Future 100 year

oo year	
Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	61.05 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	35.25 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	5.590 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	5.301 ac-ft
Hydrograph Volume (Area und	ler Hydrograph curve)
Volume	5.123 ac-ft
SCS Unit Hydrograph Parame	ters
Time of Concentration (Composite)	0.11 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	57.58 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.29 hours
' ' '	

Subsection: Unit Hydrograph Summary

Label: PDA1 - Imp - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters

Total unit time, Tb 0.37 hours

Return Event: 100 years

Storm Event: Future 100

Subsection: Unit Hydrograph (Hydrograph Table)

Label: PDA1 - Imp - Fut Scenario: Future 100 year Return Event: 100 years Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.11 hours
Area (User Defined)	5.590 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.08	0.25	0.38	0.47
1.50	0.55	0.61	0.66	0.71	0.74
2.75	0.78	0.81	0.84	0.88	0.90
4.00	0.93	0.95	0.97	1.00	1.02
5.25	1.04	1.07	1.09	1.11	1.17
6.50	1.25	1.32	1.40	1.47	1.55
7.75	1.62	1.70	1.77	1.84	1.92
9.00	1.99	2.22	2.48	2.74	3.00
10.25	3.25	3.52	4.35	5.31	6.85
11.50	8.55	13.36	35.25	26.57	12.21
12.75	8.09	6.37	5.06	4.09	3.46
14.00	3.20	2.94	2.68	2.43	2.16
15.25	1.99	1.92	1.85	1.77	1.70
16.50	1.62	1.55	1.48	1.41	1.33
17.75	1.26	1.19	1.14	1.12	1.10
19.00	1.08	1.07	1.05	1.03	1.01
20.25	0.99	0.97	0.96	0.94	0.92
21.50	0.90	0.88	0.87	0.85	0.83
22.75	0.81	0.79	0.77	0.76	0.74
24.00	0.72	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA1 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.19 hours
(Composite)	0.13 110013
Area (User Defined)	1.420 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	13.00 hours
Flow (Peak, Computed)	0.03 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	
Interpolated Output)	13.00 hours
Flow (Peak Interpolated	0.03 ft ³ /s
Output)	0.03 10 73
Duning and Aug	
Drainage Area	
SCS CN (Composite)	47.960
Area (User Defined)	1.420 acres
Maximum Retention	10.85 in
(Pervious) Maximum Retention	
(Pervious, 20 percent)	2.17 in
Cumulative Runoff	
Cumulative Runoff Depth	0.14 in
(Pervious)	0.17 111
Runoff Volume (Pervious)	0.017 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.017 ac-ft
Volume	0.017 ac-10
SCS Unit Hydrograph Paramet	ers
Time of Concentration	0.10.5
(Composite)	0.19 hours
Computational Time	0.03 hours
Increment	2.00 110010
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.47 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.51 hours

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Per - Cur Storm Event: Current 2 Year

Label: PDA1 - Per - Cur Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA1 - Per - Cur

Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.19 hours
Area (User Defined)	1.420 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.25	0.00	0.02	0.03	0.03	0.03
13.50	0.03	0.03	0.03	0.02	0.02
14.75	0.02	0.02	0.02	0.02	0.02
16.00	0.02	0.02	0.02	0.02	0.02
17.25	0.02	0.02	0.02	0.01	0.01
18.50	0.01	0.01	0.01	0.01	0.01
19.75	0.01	0.01	0.01	0.01	0.01
21.00	0.01	0.01	0.01	0.01	0.01
22.25	0.01	0.01	0.01	0.01	0.01
23.50	0.01	0.01	0.01	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA1 - Per - Cur Storm Event: Current 10 Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.19 hours
(Composite)	
Area (User Defined)	1.420 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.21 hours
Flow (Peak, Computed)	0.67 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.62 ft ³ /s
Drainage Area	
SCS CN (Composite)	47.960
Area (User Defined)	1.420 acres
Maximum Retention (Pervious)	10.85 in
Maximum Retention (Pervious, 20 percent)	2.17 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.76 in
Runoff Volume (Pervious)	0.090 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.088 ac-ft
CCC Unit Under work Danier	
SCS Unit Hydrograph Parameter	S
Time of Concentration (Composite)	0.19 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.47 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.51 hours

Subsection: Unit Hydrograph Summary

Label: PDA1 - Per - Cur Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Return Event: 10 years

Storm Event: Current 10

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.19 hours
Area (User Defined)	1.420 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.75	0.00	0.06	0.62	0.35	0.25
13.00	0.21	0.17	0.15	0.13	0.12
14.25	0.11	0.10	0.10	0.09	0.08
15.50	0.08	0.08	0.07	0.07	0.07
16.75	0.07	0.06	0.06	0.06	0.06
18.00	0.05	0.05	0.05	0.05	0.05
19.25	0.05	0.05	0.05	0.05	0.05
20.50	0.05	0.04	0.04	0.04	0.04
21.75	0.04	0.04	0.04	0.04	0.04
23.00	0.04	0.04	0.04	0.04	0.04

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA1 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.19 hours
(Composite)	
Area (User Defined)	1.420 acres
Computational Time Increment	0.03 hours
Time to Peak (Computed)	12.19 hours
Flow (Peak, Computed)	3.73 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.23 110010
Flow (Peak Interpolated Output)	3.05 ft ³ /s
Drainage Area	
SCS CN (Composite)	47.960
Area (User Defined)	1.420 acres
Maximum Retention	10.85 in
(Pervious)	20.00
Maximum Retention (Pervious, 20 percent)	2.17 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.89 in
Runoff Volume (Pervious)	0.342 ac-ft
Hydrograph Volume (Area under	r Hydrograph curve)
Volume	0.336 ac-ft
SCS Unit Hydrograph Paramete	rs
Time of Concentration (Composite)	0.19 hours
Computational Time Increment	0.03 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.47 ft ³ /s
Unit peak time, Tp	0.13 hours
Unit receding limb, Tr	0.51 hours
- •	

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA1 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.63 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: PDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.19 hours
Area (User Defined)	1.420 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.50	0.00	0.00	0.03	0.09	0.18
11.75	0.40	1.41	3.05	1.34	0.89
13.00	0.71	0.57	0.48	0.40	0.37
14.25	0.34	0.32	0.29	0.26	0.24
15.50	0.23	0.22	0.21	0.21	0.20
16.75	0.19	0.18	0.18	0.17	0.16
18.00	0.15	0.14	0.14	0.14	0.14
19.25	0.14	0.13	0.13	0.13	0.13
20.50	0.13	0.12	0.12	0.12	0.12
21.75	0.11	0.11	0.11	0.11	0.11
23.00	0.10	0.10	0.10	0.10	0.10

Subsection: Unit Hydrograph Summary

Label: PDA1 - Per - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	1.410 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.55 hours
Flow (Peak, Computed)	0.09 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.50 hours
Interpolated Output)	
Flow (Peak Interpolated Output)	0.09 ft ³ /s
Drainage Area	
	4T 00 -
SCS CN (Composite)	47.080
Area (User Defined)	1.410 acres
Maximum Retention (Pervious)	11.24 in
Maximum Retention (Pervious, 20 percent)	2.25 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.27 in
Runoff Volume (Pervious)	0.032 ac-ft
The decree of Maria (A	- Ulaska and Daniel
Hydrograph Volume (Area under	
Volume	0.032 ac-ft
SCS Unit Hydrograph Parameter	rs
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.88 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours

Subsection: Unit Hydrograph Summary

Return Event: 2 years Label: PDA1 - Per - Fut Storm Event: Future 2 Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: PDA1 - Per - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.18 hours (Composite) Area (User Defined) 1.410 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.07	0.09	0.08	0.07
13.25	0.06	0.06	0.05	0.05	0.05
14.50	0.04	0.04	0.04	0.03	0.03
15.75	0.03	0.03	0.03	0.03	0.03
17.00	0.03	0.03	0.03	0.03	0.02
18.25	0.02	0.02	0.02	0.02	0.02
19.50	0.02	0.02	0.02	0.02	0.02
20.75	0.02	0.02	0.02	0.02	0.02
22.00	0.02	0.02	0.02	0.02	0.02
23.25	0.02	0.02	0.02	0.02	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA1 - Per - Fut

Return Event: 10 years

Storm Event: Future 10

Label: PDA1 - Per - Fut Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	1.410 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	1.21 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.02 ft³/s
Drainage Area	
SCS CN (Composite)	47.080
Area (User Defined)	1.410 acres
Maximum Retention (Pervious)	11.24 in
Maximum Retention (Pervious, 20 percent)	2.25 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.12 in
Runoff Volume (Pervious)	0.132 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.130 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.88 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA1 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA1 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.18 hours (Composite) Area (User Defined) 1.410 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.50	0.00	0.00	0.26	1.02	0.52
12.75	0.36	0.30	0.25	0.21	0.18
14.00	0.16	0.15	0.14	0.13	0.12
15.25	0.11	0.11	0.10	0.10	0.10
16.50	0.09	0.09	0.09	0.08	0.08
17.75	0.08	0.07	0.07	0.07	0.07
19.00	0.07	0.07	0.06	0.06	0.06
20.25	0.06	0.06	0.06	0.06	0.06
21.50	0.06	0.06	0.06	0.05	0.05
22.75	0.05	0.05	0.05	0.05	0.05
24.00	0.05	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA1 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA1 - Per - Fut Scenario: Future 100 year

100 year	
Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	1.410 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	5.79 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.25 110015
Flow (Peak Interpolated Output)	4.40 ft ³ /s
Drainage Area	
SCS CN (Composite)	47.080
Area (User Defined)	1.410 acres
Maximum Retention	
(Pervious)	11.24 in
Maximum Retention (Pervious, 20 percent)	2.25 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	4.26 in
Runoff Volume (Pervious)	0.501 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.490 ac-ft
SCS Unit Hydrograph Parameter	s
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.88 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours

Subsection: Unit Hydrograph Summary

Label: PDA1 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA1 - Per - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: PDA1 - Per - Fut Scenario: Future 100 year Return Event: 100 years Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	1.410 acres

inite on fert represents time for mot value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.00	0.00	0.02	0.04	0.08	0.14
11.25	0.24	0.39	0.76	2.36	4.40
12.50	1.88	1.24	1.00	0.80	0.66
13.75	0.55	0.51	0.47	0.44	0.40
15.00	0.36	0.33	0.32	0.30	0.29
16.25	0.28	0.27	0.26	0.25	0.24
17.50	0.23	0.22	0.20	0.19	0.19
18.75	0.19	0.19	0.18	0.18	0.18
20.00	0.17	0.17	0.17	0.17	0.16
21.25	0.16	0.16	0.15	0.15	0.15
22.50	0.15	0.14	0.14	0.14	0.13
23.75	0.13	0.13	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA2 - Imp - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year	
Return Event	2 years	
Duration	24.00 hours	
Depth	3.48 in	
Time of Concentration (Composite)	0.07 hours	
Area (User Defined)	0.540 acres	
Computational Time Increment	0.01 hours	
Time to Peak (Computed)	12.11 hours	
Flow (Peak, Computed)	1.89 ft ³ /s	
Output Increment	0.25 hours	
Time to Flow (Peak Interpolated Output)	12.00 hours	
Flow (Peak Interpolated Output)	1.13 ft³/s	
Drainage Area		
SCS CN (Composite)	98.000	
Area (User Defined)	0.540 acres	
Maximum Retention	0.540 acres	
(Pervious)	0.20 in	
Maximum Retention (Pervious, 20 percent)	0.04 in	
Cumulative Runoff		
Cumulative Runoff Cumulative Runoff Depth (Pervious)	3.25 in	
Cumulative Runoff Depth	3.25 in 0.146 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious)	0.146 ac-ft	
Cumulative Runoff Depth (Pervious)	0.146 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parametrime of Concentration (Composite) Computational Time	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft ters 0.07 hours	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft ters 0.07 hours 0.01 hours	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft eters 0.07 hours 0.01 hours 483.432	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parametrime of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft eters 0.07 hours 0.01 hours 483.432 0.749	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parame Time of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft eters 0.07 hours 0.01 hours 483.432 0.749 1.670	
Cumulative Runoff Depth (Pervious) Runoff Volume (Pervious) Hydrograph Volume (Area und Volume SCS Unit Hydrograph Parametrime of Concentration (Composite) Computational Time Increment Unit Hydrograph Shape Factor K Factor Receding/Rising, Tr/Tp Unit peak, qp	0.146 ac-ft der Hydrograph curve) 0.141 ac-ft eters 0.07 hours 0.01 hours 483.432 0.749 1.670 8.74 ft³/s	

Subsection: Unit Hydrograph Summary

Return Event: 2 years

Label: PDA2 - Imp - Cur

Storm Event: Current 2 Year

Label: PDA2 - Imp - Cur Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 2 years

Label: PDA2 - Imp - Cur Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.07 hours
Area (User Defined)	0.540 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.00	0.00	0.01	0.01
2.25	0.01	0.01	0.01	0.01	0.02
3.50	0.02	0.02	0.02	0.02	0.02
4.75	0.02	0.02	0.02	0.02	0.03
6.00	0.03	0.03	0.03	0.03	0.04
7.25	0.04	0.04	0.04	0.04	0.05
8.50	0.05	0.05	0.05	0.06	0.07
9.75	0.08	0.08	0.09	0.10	0.13
11.00	0.15	0.20	0.25	0.42	1.13
12.25	0.61	0.34	0.22	0.18	0.14
13.50	0.11	0.10	0.09	0.08	0.08
14.75	0.07	0.06	0.06	0.05	0.05
16.00	0.05	0.05	0.05	0.04	0.04
17.25	0.04	0.04	0.04	0.03	0.03
18.50	0.03	0.03	0.03	0.03	0.03
19.75	0.03	0.03	0.03	0.03	0.03
21.00	0.03	0.03	0.03	0.03	0.02
22.25	0.02	0.02	0.02	0.02	0.02
23.50	0.02	0.02	0.02	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA2 - Imp - Cur Scenario: Current 10 year Return Event: 10 years Storm Event: Current 10

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.07 hours
(Composite)	
Area (User Defined)	0.540 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	2.97 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	0.25 110015
Interpolated Output)	12.00 hours
Flow (Peak Interpolated	1 70 ft3/a
Output)	1.78 ft ³ /s
Drainage Area	
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.540 acres
Maximum Retention	0.20 in
(Pervious) Maximum Retention	
(Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth	5.20 in
(Pervious)	J.20 III
Runoff Volume (Pervious)	0.234 ac-ft
Hydrograph Volume (Area und	er Hydrograph curve)
Volume	0.226 ac-ft
SCS Unit Hydrograph Paramet	ters
Time of Concentration	0.07.
(Composite)	0.07 hours
Computational Time	0.01 hours
Increment	0.01 Hours
Unit Hydrograph Shape	483.432
Factor	
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.74 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.19 hours

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Imp - Cur Storm Event: Current 10 Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.07 hours
Area (User Defined)	0.540 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.01	0.01	0.01	0.02
2.00	0.02	0.02	0.02	0.03	0.03
3.25	0.03	0.03	0.03	0.04	0.04
4.50	0.04	0.04	0.04	0.04	0.04
5.75	0.05	0.05	0.05	0.05	0.06
7.00	0.06	0.06	0.07	0.07	0.07
8.25	0.08	0.08	0.08	0.09	0.10
9.50	0.11	0.12	0.13	0.15	0.16
10.75	0.20	0.24	0.32	0.39	0.66
12.00	1.78	0.95	0.54	0.35	0.28
13.25	0.22	0.18	0.15	0.14	0.13
14.50	0.12	0.11	0.10	0.09	0.09
15.75	0.08	0.08	0.08	0.07	0.07
17.00	0.07	0.06	0.06	0.06	0.05
18.25	0.05	0.05	0.05	0.05	0.05
19.50	0.05	0.05	0.05	0.04	0.04
20.75	0.04	0.04	0.04	0.04	0.04
22.00	0.04	0.04	0.04	0.04	0.04
23.25	0.03	0.03	0.03	0.03	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA2 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.07 hours
(Composite)	0.07 Hours
Area (User Defined)	0.540 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	5.15 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 110015
Flow (Peak Interpolated	3.08 ft ³ /s
Output)	,
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.540 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
(1 civious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	0.412 ac-ft
Hydrograph Volume (Area under	· Hydrograph curve)
Volume	0.398 ac-ft
SCS Unit Hydrograph Parameter	
	· S
Time of Concentration (Composite)	0.07 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape	
Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.74 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.19 hours

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA2 - Imp - Cur Storm Event: Current 100 Scenario: Current 100 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: PDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.07 hours
Area (User Defined)	0.540 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.00	0.02	0.02	0.03
1.50	0.04	0.04	0.05	0.05	0.05
2.75	0.06	0.06	0.06	0.07	0.07
4.00	0.07	0.07	0.07	0.08	0.08
5.25	0.08	0.08	0.08	0.09	0.09
6.50	0.10	0.10	0.11	0.11	0.12
7.75	0.13	0.13	0.14	0.14	0.15
9.00	0.16	0.18	0.20	0.22	0.24
10.25	0.26	0.28	0.35	0.42	0.55
11.50	0.68	1.15	3.08	1.65	0.93
12.75	0.61	0.48	0.38	0.31	0.27
14.00	0.25	0.23	0.21	0.19	0.17
15.25	0.15	0.15	0.14	0.14	0.13
16.50	0.13	0.12	0.11	0.11	0.10
17.75	0.10	0.09	0.09	0.09	0.09
19.00	0.08	0.08	0.08	0.08	0.08
20.25	0.08	0.08	0.07	0.07	0.07
21.50	0.07	0.07	0.07	0.07	0.06
22.75	0.06	0.06	0.06	0.06	0.06
24.00	0.06	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA2 - Imp - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

·				
Storm Event	Future 2			
Return Event	2 years			
Duration	24.00 hours			
Depth	4.14 in			
Time of Concentration	0.07 hours			
(Composite)	0.07 Hours			
Area (User Defined)	0.540 acres			
Computational Time Increment	0.01 hours			
Time to Peak (Computed)	12.11 hours			
Flow (Peak, Computed)	2.25 ft ³ /s			
Output Increment	0.25 hours			
Time to Flow (Peak	12.00 hours			
Interpolated Output)	12.00 110013			
Flow (Peak Interpolated	1.35 ft ³ /s			
Output)	·			
Drainage Area				
	98.000			
SCS CN (Composite)				
Area (User Defined) Maximum Retention	0.540 acres			
(Pervious)	0.20 in			
Maximum Retention	0.04 :-			
(Pervious, 20 percent)	0.04 in			
Cumulative Runoff				
Cumulative Runoff Depth	3.90 in			
(Pervious)	0 176 as f t			
Runoff Volume (Pervious)	0.176 ac-ft			
Hydrograph Volume (Area under Hydrograph curve)				
Volume	0.170 ac-ft			
SCS Unit Hydrograph Parameters				
Time of Concentration (Composite)	0.07 hours			
Computational Time	0.01 hours			
Increment	0.01 hours			
Unit Hydrograph Shape Factor	483.432			
K Factor	0.749			
Receding/Rising, Tr/Tp	1.670			
Unit peak, qp	8.74 ft ³ /s			
Unit peak time, Tp	0.05 hours			
Unit receding limb, Tr	0.19 hours			

Subsection: Unit Hydrograph Summary

Return Event: 2 years Label: PDA2 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: PDA2 - Imp - Fut Storm Event: Future 2 Scenario: Future 2 year

> Storm Event Future 2 Return Event 2 years 24.00 hours Duration Depth 4.14 in Time of Concentration 0.07 hours (Composite) Area (User Defined) 0.540 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.00	0.01	0.01	0.01
2.25	0.01	0.02	0.02	0.02	0.02
3.50	0.02	0.02	0.02	0.03	0.03
4.75	0.03	0.03	0.03	0.03	0.03
6.00	0.03	0.04	0.04	0.04	0.04
7.25	0.05	0.05	0.05	0.05	0.06
8.50	0.06	0.06	0.07	0.07	0.08
9.75	0.09	0.10	0.11	0.12	0.15
11.00	0.18	0.24	0.30	0.50	1.35
12.25	0.72	0.41	0.27	0.21	0.17
13.50	0.14	0.12	0.11	0.10	0.09
14.75	0.08	0.07	0.07	0.07	0.06
16.00	0.06	0.06	0.06	0.05	0.05
17.25	0.05	0.05	0.04	0.04	0.04
18.50	0.04	0.04	0.04	0.04	0.04
19.75	0.04	0.03	0.03	0.03	0.03
21.00	0.03	0.03	0.03	0.03	0.03
22.25	0.03	0.03	0.03	0.03	0.03
23.50	0.03	0.03	0.02	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration	0.07 hours
(Composite)	
Area (User Defined)	0.540 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	3.51 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	
Interpolated Output)	12.00 hours
Flow (Peak Interpolated	2.10 ft ³ /s
Output)	2.10 10/3
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.540 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention	
(Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth	6.17 in
(Pervious)	
Runoff Volume (Pervious)	0.278 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.268 ac-ft
SCS Unit Hydrograph Paramet	ers
Time of Concentration	0.07 hours
(Composite)	0.07 Hours
Computational Time	0.01 hours
Increment	
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.74 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.19 hours
5 - ,	

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> SCS Unit Hydrograph Parameters Total unit time, Tb 0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA2 - Imp - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.07 hours (Composite) Area (User Defined) 0.540 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.01	0.01	0.02
1.75	0.02	0.03	0.03	0.03	0.03
3.00	0.04	0.04	0.04	0.04	0.04
4.25	0.05	0.05	0.05	0.05	0.05
5.50	0.05	0.05	0.06	0.06	0.06
6.75	0.07	0.07	0.08	0.08	0.08
8.00	0.09	0.09	0.10	0.10	0.10
9.25	0.12	0.13	0.15	0.16	0.17
10.50	0.19	0.24	0.29	0.38	0.46
11.75	0.78	2.10	1.12	0.63	0.42
13.00	0.33	0.26	0.21	0.18	0.17
14.25	0.15	0.14	0.13	0.11	0.11
15.50	0.10	0.10	0.09	0.09	0.09
16.75	0.08	0.08	0.07	0.07	0.07
18.00	0.06	0.06	0.06	0.06	0.06
19.25	0.06	0.06	0.05	0.05	0.05
20.50	0.05	0.05	0.05	0.05	0.05
21.75	0.05	0.05	0.04	0.04	0.04
23.00	0.04	0.04	0.04	0.04	0.04

Subsection: Unit Hydrograph Summary

Label: PDA2 - Imp - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA2 - Imp - Fut Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.07 hours
Area (User Defined)	0.540 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	6.37 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	3.81 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.540 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	0.512 ac-ft
Hydrograph Volume (Area und	er Hydrograph curve)
Volume	0.495 ac-ft
SCS Unit Hydrograph Paramet	ers
Time of Concentration (Composite)	0.07 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	8.74 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.19 hours

Subsection: Unit Hydrograph Summary

Label: PDA2 - Imp - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA2 - Imp - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.23 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: PDA2 - Imp - Fut Scenario: Future 100 year Return Event: 100 years Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.07 hours
Area (User Defined)	0.540 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.03	0.04	0.05
1.50	0.05	0.06	0.06	0.07	0.07
2.75	0.08	0.08	0.08	0.08	0.09
4.00	0.09	0.09	0.09	0.10	0.10
5.25	0.10	0.10	0.11	0.11	0.11
6.50	0.12	0.13	0.14	0.14	0.15
7.75	0.16	0.16	0.17	0.18	0.19
9.00	0.19	0.22	0.24	0.27	0.29
10.25	0.32	0.34	0.43	0.52	0.69
11.50	0.84	1.42	3.81	2.04	1.15
12.75	0.75	0.60	0.47	0.39	0.33
14.00	0.31	0.28	0.26	0.23	0.21
15.25	0.19	0.18	0.18	0.17	0.16
16.50	0.16	0.15	0.14	0.13	0.13
17.75	0.12	0.11	0.11	0.11	0.11
19.00	0.10	0.10	0.10	0.10	0.10
20.25	0.10	0.09	0.09	0.09	0.09
21.50	0.09	0.09	0.08	0.08	0.08
22.75	0.08	0.08	0.07	0.07	0.07
24.00	0.07	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA2 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

Storm Event	Current 2 Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.10 hours
(Composite)	
Area (User Defined)	0.550 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.52 hours
Flow (Peak, Computed)	0.02 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12 F0 haves
Interpolated Output)	12.50 hours
Flow (Peak Interpolated	0.02 ft ³ /s
Output)	0102 10 70
Duein and Ana	
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention	9.96 in
(Pervious)	
Maximum Retention (Pervious, 20 percent)	1.99 in
(12.112.23, 22.42.13)	
Cumulative Runoff	
Cumulative Runoff Depth	0.19 in
(Pervious)	0.15 111
Runoff Volume (Pervious)	0.009 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.009 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.10 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.23 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.27 hours

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA2 - Per - Cur Storm Event: Current 2 Year Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.33 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA2 - Per - Cur

Storm Event: Current 2 Year

Scenario: Current 2 year

Storm Event	Current 2 Year	
Return Event	2 years	
Duration	24.00 hours	
Depth	3.48 in	
Time of Concentration (Composite)	0.10 hours	
Area (User Defined)	0.550 acres	

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.02	0.02	0.02	0.02
13.25	0.02	0.01	0.01	0.01	0.01
14.50	0.01	0.01	0.01	0.01	0.01
15.75	0.01	0.01	0.01	0.01	0.01
17.00	0.01	0.01	0.01	0.01	0.01
18.25	0.01	0.01	0.01	0.01	0.01
19.50	0.01	0.01	0.01	0.01	0.01
20.75	0.01	0.01	0.01	0.01	0.01
22.00	0.01	0.01	0.01	0.01	0.01
23.25	0.01	0.01	0.01	0.01	(N/A)

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Per - Cur Storm Event: Current 10 Scenario: Current 10 year

- ,	
Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration	0.10 hours
(Composite)	0.10 Hours
Area (User Defined)	0.550 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	0.45 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	12.25 Hours
Flow (Peak Interpolated	0.25 ft ³ /s
Output)	,
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention (Pervious)	9.96 in
Maximum Retention	
(Pervious, 20 percent)	1.99 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.89 in
Runoff Volume (Pervious)	0.041 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.039 ac-ft
SCS Unit Hydrograph Parameters	
505 Unit Hydrograph Parameters	S
Time of Concentration (Composite)	0.10 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.23 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.27 hours
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Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Per - Cur Storm Event: Current 10 Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.33 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.10 hours
Area (User Defined)	0.550 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.75	0.00	0.12	0.25	0.15	0.11
13.00	0.09	0.07	0.06	0.05	0.05
14.25	0.05	0.04	0.04	0.04	0.03
15.50	0.03	0.03	0.03	0.03	0.03
16.75	0.03	0.03	0.03	0.02	0.02
18.00	0.02	0.02	0.02	0.02	0.02
19.25	0.02	0.02	0.02	0.02	0.02
20.50	0.02	0.02	0.02	0.02	0.02
21.75	0.02	0.02	0.02	0.02	0.02
23.00	0.02	0.02	0.02	0.02	0.01

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA2 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration	0.10 hours
(Composite)	
Area (User Defined)	0.550 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	2.01 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	
Interpolated Output)	12.25 hours
Flow (Peak Interpolated	0.92 ft ³ /s
Output)	0.52 10 75
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention (Pervious)	9.96 in
Maximum Retention	4.00 :
(Pervious, 20 percent)	1.99 in
Cumulative Runoff	
Cumulative Runoff Depth	3.16 in
(Pervious)	0.14F ac ft
Runoff Volume (Pervious)	0.145 ac-ft
Hydrograph Volume (Area unde	r Hydrograph curve)
Volume	0.138 ac-ft
SCS Unit Hydrograph Paramete	rs
Time of Concentration	0.10 hours
(Composite)	0.10 110013
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.23 ft ³ /s
Unit peak time, Tp	0.07 hours
Unit receding limb, Tr	0.27 hours

Subsection: Unit Hydrograph Summary

Return Event: 100 years Label: PDA2 - Per - Cur Storm Event: Current 100 Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.33 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years Label: PDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.10 hours
Area (User Defined)	0.550 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.25	0.00	0.00	0.01	0.03	0.06
11.50	0.11	0.24	0.92	0.92	0.49
12.75	0.34	0.28	0.22	0.18	0.16
14.00	0.15	0.14	0.13	0.12	0.10
15.25	0.10	0.09	0.09	0.09	0.08
16.50	0.08	0.08	0.07	0.07	0.07
17.75	0.06	0.06	0.06	0.06	0.06
19.00	0.06	0.05	0.05	0.05	0.05
20.25	0.05	0.05	0.05	0.05	0.05
21.50	0.05	0.05	0.05	0.04	0.04
22.75	0.04	0.04	0.04	0.04	0.04
24.00	0.04	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA2 - Per - Fut

Return Event: 2 years

Storm Event: Future 2

Scenario: Future 2 year

,	
Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration	0.09 hours
(Composite)	
Area (User Defined)	0.550 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.14 hours
Flow (Peak, Computed)	0.11 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	
Flow (Peak Interpolated Output)	0.08 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention	9.96 in
(Pervious)	3.30 III
Maximum Retention (Pervious, 20 percent)	1.99 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.38 in
Runoff Volume (Pervious)	0.017 ac-ft
Hydrograph Volume (Area under H	ydrograph curve)
Volume	0.017 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.92 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours
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Subsection: Unit Hydrograph Summary

Return Event: 2 years Label: PDA2 - Per - Fut Storm Event: Future 2 Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.30 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years Label: PDA2 - Per - Fut Storm Event: Future 2 Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.550 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
12.00	0.00	0.08	0.06	0.05	0.04
13.25	0.03	0.03	0.03	0.02	0.02
14.50	0.02	0.02	0.02	0.02	0.02
15.75	0.02	0.02	0.02	0.02	0.01
17.00	0.01	0.01	0.01	0.01	0.01
18.25	0.01	0.01	0.01	0.01	0.01
19.50	0.01	0.01	0.01	0.01	0.01
20.75	0.01	0.01	0.01	0.01	0.01
22.00	0.01	0.01	0.01	0.01	0.01
23.25	0.01	0.01	0.01	0.01	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years
Label: PDA2 - Per - Fut Storm Event: Future 10

Label: PDA2 - Per - Fut Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration	0.09 hours
(Composite)	
Area (User Defined)	0.550 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	0.80 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.25 hours
Interpolated Output)	
Flow (Peak Interpolated Output)	0.37 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention	9.96 in
(Pervious)	3.30 III
Maximum Retention (Pervious, 20 percent)	1.99 in
(1 ci vious, 20 percent)	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.36 in
Runoff Volume (Pervious)	0.062 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	0.060 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration	
(Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.92 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours
- ·	

Subsection: Unit Hydrograph Summary

Return Event: 10 years Label: PDA2 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.30 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years Label: PDA2 - Per - Fut Storm Event: Future 10 Scenario: Future 10 year

> Storm Event Future 10 Return Event 10 years 24.00 hours Duration Depth 6.41 in Time of Concentration 0.09 hours (Composite) Area (User Defined) 0.550 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.50	0.00	0.04	0.30	0.37	0.22
12.75	0.16	0.13	0.11	0.09	0.08
14.00	0.07	0.07	0.06	0.06	0.05
15.25	0.05	0.05	0.05	0.04	0.04
16.50	0.04	0.04	0.04	0.04	0.03
17.75	0.03	0.03	0.03	0.03	0.03
19.00	0.03	0.03	0.03	0.03	0.03
20.25	0.03	0.03	0.03	0.03	0.03
21.50	0.02	0.02	0.02	0.02	0.02
22.75	0.02	0.02	0.02	0.02	0.02
24.00	0.02	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary

Label: PDA2 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA2 - Per - Fut Scenario: Future 100 year

100 year	
Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration	0.09 hours
(Composite)	
Area (User Defined)	0.550 acres
Communicational Times	
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.12 hours
Flow (Peak, Computed)	3.14 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 110013
Flow (Peak Interpolated Output)	1.55 ft ³ /s
Drainage Area	
SCS CN (Composite)	50.090
Area (User Defined)	0.550 acres
Maximum Retention (Pervious)	9.96 in
Maximum Retention (Pervious, 20 percent)	1.99 in
Cumulative Runoff	
Cumulative Runoff Depth	4.72 :
(Pervious)	4.73 in
Runoff Volume (Pervious)	0.217 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.207 ac-ft
SCS Unit Hydrograph Parameters	<u> </u>
Time of Concentration	
(Composite)	0.09 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Unit peak, qp	6.92 ft ³ /s
Unit peak time, Tp	0.06 hours
Unit receding limb, Tr	0.24 hours

Subsection: Unit Hydrograph Summary

Label: PDA2 - Per - Fut

Return Event: 100 years

Storm Event: Future 100

Label: PDA2 - Per - Fut Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Total unit time, Tb	0.30 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Label: PDA2 - Per - Fut Scenario: Future 100 year

ear Storm Event: Future 100

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.09 hours
Area (User Defined)	0.550 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.25	0.00	0.00	0.01	0.02	0.03
10.50	0.04	0.06	0.09	0.14	0.22
11.75	0.43	1.55	1.30	0.71	0.49
13.00	0.39	0.32	0.26	0.22	0.21
14.25	0.19	0.18	0.16	0.14	0.13
15.50	0.13	0.13	0.12	0.12	0.11
16.75	0.11	0.10	0.10	0.09	0.09
18.00	0.08	0.08	0.08	0.08	0.08
19.25	0.08	0.07	0.07	0.07	0.07
20.50	0.07	0.07	0.07	0.07	0.06
21.75	0.06	0.06	0.06	0.06	0.06
23.00	0.06	0.06	0.05	0.05	0.05

Return Event: 100 years

Subsection: Addition Summary Return Event: 2 years

Label: Post - POS1 Storm Event: Current 2 Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA1 - Imp - Cur

<Catchment to Outflow Node> PDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	1.457	12.00	10.40
Flow (From)	PDA1 - Per - Cur	0.017	13.00	0.03
Flow (In)	Post - POS1	1.474	12.00	10.40

Subsection: Addition Summary

Label: Post - POS1

Return Event: 2 years

Storm Event: Future 2

Label: Post - POS1 Scenario: Future 2 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA1 - Imp - Fut

<Catchment to Outflow Node> PDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	1.756	12.00	12.44
Flow (From)	PDA1 - Per - Fut	0.032	12.50	0.09
Flow (In)	Post - POS1	1.788	12.00	12.44

Subsection: Addition Summary

Label: Post - POS1

Return Event: 10 years

Storm Event: Current 10

Label: Post - POS1 Scenario: Current 10 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA1 - Imp - Cur

<Catchment to Outflow Node> PDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	2.336	12.00	16.39
Flow (From)	PDA1 - Per - Cur	0.088	12.25	0.62
Flow (In)	Post - POS1	2.425	12.00	16.45

Subsection: Addition Summary

Label: Post - POS1

Return Event: 10 years

Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA1 - Imp - Fut

<Catchment to Outflow Node> PDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	2.777	12.00	19.38
Flow (From)	PDA1 - Per - Fut	0.130	12.25	1.02
Flow (In)	Post - POS1	2.907	12.00	19.65

Subsection: Addition Summary Return Event: 100 years Storm Event: Current 100

Label: Post - POS1

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node <Catchment to Outflow Node> PDA1 - Imp - Cur <Catchment to Outflow Node> PDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	4.116	12.00	28.44
Flow (From)	PDA1 - Per - Cur	0.336	12.25	3.05
Flow (In)	Post - POS1	4.452	12.00	29.85

Subsection: Addition Summary

Label: Post - POS1

Return Event: 100 years

Storm Event: Future 100

Label: Post - POS1 Scenario: Future 100 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA1 - Imp - Fut

<Catchment to Outflow Node> PDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	5.123	12.00	35.25
Flow (From)	PDA1 - Per - Fut	0.490	12.25	4.40
Flow (In)	Post - POS1	5.613	12.00	37.61

Subsection: Addition Summary Return Event: 2 years

Label: Post - POS2 Storm Event: Current 2 Year Scenario: Current 2 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Cur

<Catchment to Outflow Node> PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.141	12.00	1.13
Flow (From)	PDA2 - Per - Cur	0.009	12.50	0.02
Flow (In)	Post - POS2	0.150	12.00	1.13

Subsection: Addition Summary

Label: Post - POS2

Return Event: 2 years

Storm Event: Future 2

Label: Post - POS2 Scenario: Future 2 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Fut

<Catchment to Outflow Node> PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.170	12.00	1.35
Flow (From)	PDA2 - Per - Fut	0.017	12.25	0.08
Flow (In)	Post - POS2	0.187	12.00	1.35

Subsection: Addition Summary Return Event: 10 years
Label: Post - POS2 Storm Event: Current 10

Label: Post - POS2 Scenario: Current 10 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Cur

<Catchment to Outflow Node> PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.226	12.00	1.78
Flow (From)	PDA2 - Per - Cur	0.039	12.25	0.25
Flow (In)	Post - POS2	0.265	12.00	1.90

Subsection: Addition Summary

Label: Post - POS2

Return Event: 10 years

Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Fut

<Catchment to Outflow Node> PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.268	12.00	2.10
Flow (From)	PDA2 - Per - Fut	0.060	12.25	0.37
Flow (In)	Post - POS2	0.328	12.00	2.40

Subsection: Addition Summary Return Event: 100 years

Label: Post - POS2 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Cur

<Catchment to Outflow Node> PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.398	12.00	3.08
Flow (From)	PDA2 - Per - Cur	0.138	12.25	0.92
Flow (In)	Post - POS2	0.537	12.00	4.01

Subsection: Addition Summary Return Event: 100 years
Label: Post - POS2 Storm Event: Future 100

Label: Post - POS2 Scenario: Future 100 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> PDA2 - Imp - Fut

<Catchment to Outflow Node> PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.495	12.00	3.81
Flow (From)	PDA2 - Per - Fut	0.207	12.00	1.55
Flow (In)	Post - POS2	0.702	12.00	5.36

Subsection: Addition Summary Return Event: 2 years

Label: Pre - POS1 Storm Event: Current 2 Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA1 - Imp - Cur

<Catchment to Outflow Node> EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	1.460	12.00	10.42
Flow (From)	EDA1 - Per - Cur	0.037	12.25	0.16
Flow (In)	Pre - POS1	1.497	12.00	10.42

Subsection: Addition Summary

Label: Pre - POS1

Return Event: 2 years

Storm Event: Future 2

Label: Pre - POS1 Scenario: Future 2 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA1 - Imp - Fut

<Catchment to Outflow Node> EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	1.756	12.00	12.44
Flow (From)	EDA1 - Per - Fut	0.064	12.25	0.44
Flow (In)	Pre - POS1	1.820	12.00	12.48

Subsection: Addition Summary Return Event: 10 years

Label: Pre - POS1 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA1 - Imp - Cur

<Catchment to Outflow Node> EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	2.341	12.00	16.42
Flow (From)	EDA1 - Per - Cur	0.134	12.25	1.15
Flow (In)	Pre - POS1	2.475	12.00	16.78

Subsection: Addition Summary

Label: Pre - POS1

Return Event: 10 years

Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA1 - Imp - Fut

<Catchment to Outflow Node> EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	2.777	12.00	19.38
Flow (From)	EDA1 - Per - Fut	0.196	12.25	1.74
Flow (In)	Pre - POS1	2.973	12.00	20.11

Subsection: Addition Summary Return Event: 100 years

Label: Pre - POS1 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA1 - Imp - Cur

<Catchment to Outflow Node> EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	4.123	12.00	28.50
Flow (From)	EDA1 - Per - Cur	0.425	12.25	3.98
Flow (In)	Pre - POS1	4.548	12.00	30.45

Subsection: Addition Summary Return Event: 100 years Storm Event: Future 100

Label: Pre - POS1

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link Upstream Node <Catchment to Outflow Node> EDA1 - Imp - Fut <Catchment to Outflow Node> EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	5.123	12.00	35.25
Flow (From)	EDA1 - Per - Fut	0.618	12.25	5.67
Flow (In)	Pre - POS1	5.741	12.00	38.42

Subsection: Addition Summary Return Event: 2 years

Label: Pre - POS2 Storm Event: Current 2 Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA2 - Imp - Cur

<Catchment to Outflow Node> EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.211	12.00	1.74
Flow (From)	EDA2 - Per - Cur	0.005	12.50	0.01
Flow (In)	Pre - POS2	0.216	12.00	1.74

Subsection: Addition Summary

Return Event: 2 years

Label: Pre - POS2

Storm Event: Future 2

Label: Pre - POS2 Scenario: Future 2 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA2 - Imp - Fut

<Catchment to Outflow Node> EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.254	12.00	2.08
Flow (From)	EDA2 - Per - Fut	0.009	12.25	0.04
Flow (In)	Pre - POS2	0.264	12.00	2.08

Subsection: Addition Summary Return Event: 10 years

Label: Pre - POS2 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA2 - Imp - Cur

<Catchment to Outflow Node> EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.339	12.00	2.74
Flow (From)	EDA2 - Per - Cur	0.021	12.25	0.13
Flow (In)	Pre - POS2	0.360	12.00	2.82

Subsection: Addition Summary

Label: Pre - POS2

Return Event: 10 years

Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA2 - Imp - Fut

<Catchment to Outflow Node> EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.402	12.00	3.23
Flow (From)	EDA2 - Per - Fut	0.032	12.25	0.20
Flow (In)	Pre - POS2	0.434	12.00	3.41

Subsection: Addition Summary Return Event: 100 years

Label: Pre - POS2 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node

<Catchment to Outflow Node> EDA2 - Imp - Cur

<Catchment to Outflow Node> EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.597	12.00	4.75
Flow (From)	EDA2 - Per - Cur	0.074	12.00	0.52
Flow (In)	Pre - POS2	0.672	12.00	5.27

Subsection: Addition Summary Return Event: 100 years Storm Event: Future 100

Label: Pre - POS2

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link Upstream Node <Catchment to Outflow Node> EDA2 - Imp - Fut <Catchment to Outflow Node> EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.742	12.00	5.88
Flow (From)	EDA2 - Per - Fut	0.111	12.00	0.86
Flow (In)	Pre - POS2	0.853	12.00	6.74

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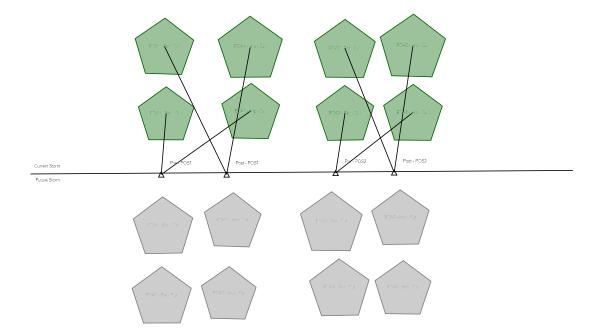
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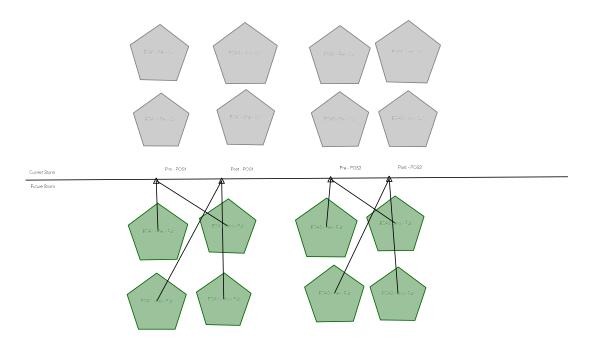
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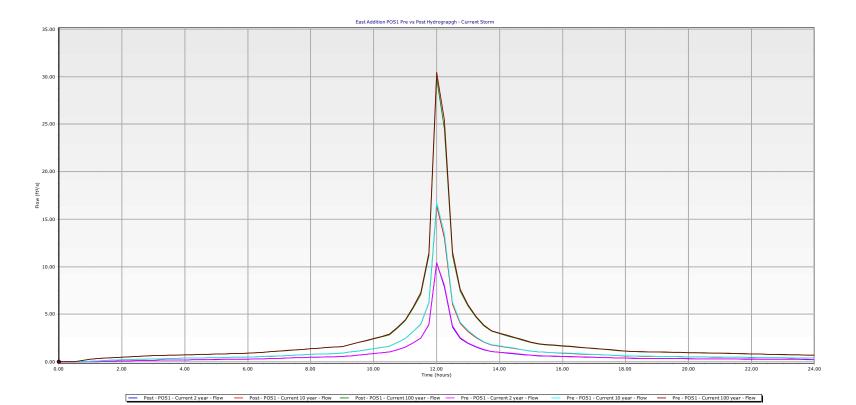
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Scenario: Current 2 year



Scenario: Future 2 year

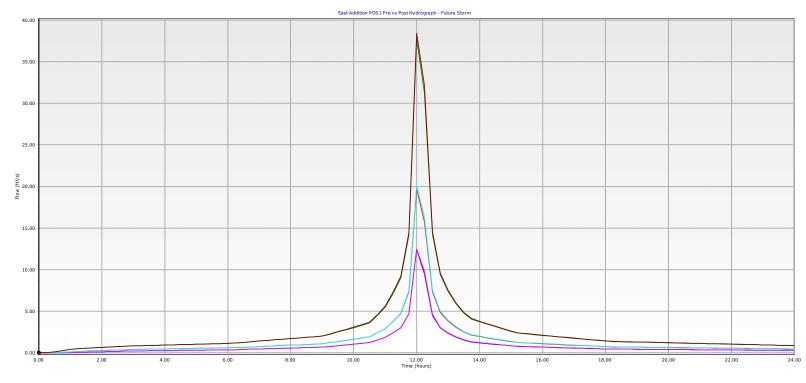




East Addition Current Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

	2	2 Year Storm Event 10-Year Storm Event 100-Year Storm Event							ent
Time	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.00
0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.14	0.14	0.00
1.00	0.00	0.00	0.00	0.05	0.05	0.00	0.25	0.25	0.00
1.25	0.01	0.01	0.00	0.09	0.09	0.00	0.32	0.32	0.00
1.50	0.04	0.04	0.00	0.13	0.13	0.00	0.39	0.39	0.00
1.75	0.06	0.06	0.00	0.17	0.17	0.00	0.44	0.44	0.00
2.00	0.07	0.07	0.00	0.20	0.20	0.00	0.49	0.49	0.00
2.25	0.09	0.09	0.00	0.23	0.23	0.00	0.53	0.53	0.00
2.50	0.11	0.11	0.00	0.25	0.25	0.00	0.56	0.56	0.00
2.75	0.13	0.13	0.00	0.27	0.27	0.00	0.60	0.60	0.00
3.00	0.14	0.14	0.00	0.29	0.29	0.00	0.63	0.62	0.01
3.25	0.15	0.15	0.00	0.31	0.31	0.00	0.65	0.65	0.00
3.50	0.17	0.17	0.00	0.33	0.33	0.00	0.68	0.68	0.00
3.75	0.18	0.18	0.00	0.35	0.35	0.00	0.70	0.70	0.00
4.00	0.19	0.19	0.00	0.37	0.37	0.00	0.73	0.73	0.00
4.25	0.20	0.20	0.00	0.38	0.38	0.00	0.75	0.75	0.00
4.50	0.21	0.21	0.00	0.40	0.40	0.00	0.77	0.77	0.00
4.75	0.22	0.22	0.00	0.41	0.41	0.00	0.79	0.79	0.00
5.00	0.24	0.24	0.00	0.43	0.43	0.00	0.81	0.81	0.00
5.25	0.25	0.25	0.00	0.44	0.44	0.00	0.83	0.83	0.00
5.50	0.26	0.26	0.00	0.45	0.45	0.00	0.85	0.85	0.00
5.75	0.26	0.26	0.00	0.47	0.46	0.01	0.87	0.86	0.01
6.00	0.27	0.27	0.00	0.48	0.48	0.00	0.88	0.88	0.00
6.25	0.29	0.29	0.00	0.51	0.51	0.00	0.94	0.94	0.00
6.50	0.32	0.32	0.00	0.54	0.54	0.00	1.00	0.99	0.01
6.75	0.34	0.34	0.00	0.58	0.58	0.00	1.06	1.05	0.01
7.00	0.36	0.36	0.00	0.62	0.62	0.00	1.12	1.12	0.00
7.25	0.39	0.39	0.00	0.65	0.65	0.00	1.18	1.18	0.00
7.50	0.41	0.41	0.00	0.69	0.69	0.00	1.24	1.24	0.00
7.75	0.43	0.43	0.00	0.72	0.72	0.00	1.30	1.30	0.00
8.00	0.46	0.46	0.00	0.76	0.76	0.00	1.36	1.36	0.00
8.25	0.48	0.48	0.00	0.79	0.79	0.00	1.42	1.42	0.00
8.50 8.75	0.50	0.50	0.00	0.83	0.83	0.00	1.48	1.48	0.00
9.00	0.53 0.55	0.53 0.55	0.00	0.87 0.90	0.87	0.00	1.54 1.60	1.54	0.00
9.25	0.53	0.62	0.00	1.01	0.90 1.01	0.00	1.79	1.60 1.78	0.00
9.50	0.82	0.62	0.00	1.13	1.01	0.00	2.00	2.00	0.00
9.75	0.70	0.70	0.00	1.15	1.15	0.00	2.00	2.20	0.00
10.00	0.85	0.85	0.00	1.38	1.37	0.01	2.44	2.41	0.03
10.25	0.93	0.93	0.00	1.49	1.49	0.00	2.66	2.62	0.04
10.50	1.01	1.01	0.00	1.62	1.62	0.00	2.89	2.83	0.06
10.75	1.25	1.25	0.00	2.01	2.00	0.01	3.60	3.51	0.09
11.00	1.54	1.53	0.01	2.46	2.45	0.01	4.43	4.31	0.12
11.25	1.99	1.99	0.00	3.17	3.17	0.00	5.76	5.61	0.15
11.50	2.50	2.49	0.01	3.96	3.96	0.00	7.27	7.07	0.20
11.75	3.92	3.92	0.00	6.26	6.20	0.06	11.47	11.18	0.29
12.00	10.42	10.40	0.02	16.78	16.46	0.32	30.45	29.86	0.59
12.25	8.06	7.88	0.18	13.55	13.01	0.54	25.46	24.50	0.96
12.50	3.76	3.65	0.11	6.26	6.04	0.22	11.55	11.20	0.35
12.75	2.51	2.43	0.08	4.15	4.03	0.12	7.62	7.42	0.20
13.00	1.99	1.93	0.06	3.28	3.19	0.09	6.01	5.86	0.15
13.25	1.58	1.54	0.04	2.61	2.53	0.08	4.77	4.66	0.11
13.50	1.28	1.24	0.04	2.12	2.06	0.06	3.87	3.78	0.09

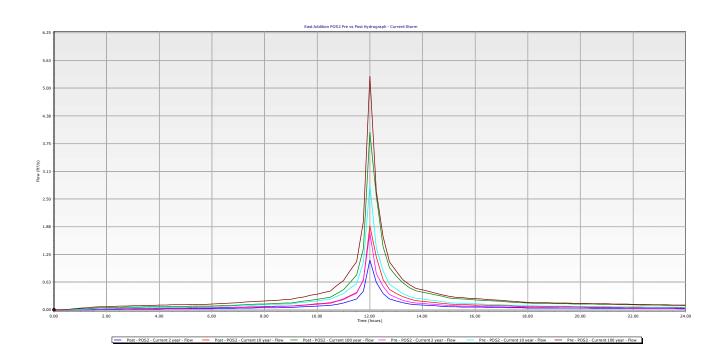
13.75	1.09	1.06	0.03	1.79	1.74	0.05	3.27	3.19	0.08
14.00	1.01	0.98	0.03	1.66	1.61	0.05	3.02	2.95	0.07
14.25	0.93	0.90	0.03	1.53	1.48	0.05	2.78	2.72	0.06
14.50	0.85	0.82	0.03	1.39	1.35	0.04	2.54	2.48	0.06
14.75	0.77	0.74	0.03	1.26	1.23	0.03	2.30	2.25	0.05
15.00	0.69	0.67	0.02	1.13	1.10	0.03	2.05	2.01	0.04
15.25	0.63	0.61	0.02	1.04	1.01	0.03	1.88	1.84	0.04
15.50	0.61	0.59	0.02	1.00	0.97	0.03	1.82	1.78	0.04
15.75	0.59	0.57	0.02	0.96	0.94	0.02	1.75	1.71	0.04
16.00	0.56	0.55	0.01	0.92	0.90	0.02	1.68	1.64	0.04
16.25	0.54	0.52	0.02	0.89	0.86	0.03	1.61	1.58	0.03
16.50	0.52	0.50	0.02	0.85	0.83	0.02	1.54	1.51	0.03
16.75	0.50	0.48	0.02	0.81	0.79	0.02	1.48	1.45	0.03
17.00	0.47	0.46	0.01	0.77	0.75	0.02	1.41	1.38	0.03
17.25	0.45	0.44	0.01	0.74	0.72	0.02	1.34	1.31	0.03
17.50	0.43	0.41	0.02	0.70	0.68	0.02	1.27	1.24	0.03
17.75	0.40	0.39	0.01	0.66	0.64	0.02	1.20	1.18	0.02
18.00	0.38	0.37	0.01	0.62	0.61	0.01	1.13	1.11	0.02
18.25	0.36	0.35	0.01	0.60	0.58	0.02	1.08	1.06	0.02
18.50	0.36	0.35	0.01	0.59	0.57	0.02	1.06	1.04	0.02
18.75	0.35	0.34	0.01	0.58	0.56	0.02	1.05	1.03	0.02
19.00	0.35	0.34	0.01	0.57	0.55	0.02	1.03	1.01	0.02
19.25	0.34	0.33	0.01	0.56	0.55	0.01	1.02	1.00	0.02
19.50	0.34	0.33	0.01	0.55	0.54	0.01	1.00	0.98	0.02
19.75	0.33	0.32	0.01	0.54	0.53	0.01	0.98	0.96	0.02
20.00	0.32	0.32	0.00	0.53	0.52	0.01	0.96	0.95	0.01
20.25	0.32	0.31	0.01	0.52	0.51	0.01	0.95	0.93	0.02
20.50	0.31	0.30	0.01	0.51	0.50	0.01	0.93	0.91	0.02
20.75	0.31	0.30	0.01	0.50	0.49	0.01	0.91	0.90	0.01
21.00	0.30	0.29	0.01	0.49	0.48	0.01	0.90	0.88	0.02
21.25	0.30	0.29	0.01	0.49	0.47	0.02	0.88	0.86	0.02
21.50	0.29	0.28	0.01	0.48	0.46	0.02	0.86	0.84	0.02
21.75	0.28	0.28	0.00	0.47	0.45	0.02	0.84	0.83	0.01
22.00	0.28	0.27	0.01	0.46	0.45	0.01	0.83	0.81	0.02
22.25	0.27	0.27	0.00	0.45	0.44	0.01	0.81	0.80	0.01
22.50	0.27	0.26	0.01	0.44	0.43	0.01	0.79	0.78	0.01
22.75	0.26	0.25	0.01	0.43	0.42	0.01	0.78	0.76	0.02
23.00	0.25	0.25	0.00	0.42	0.41	0.01	0.76	0.74	0.02
23.25	0.25	0.24	0.01	0.41	0.40	0.01	0.74	0.73	0.01
23.50	0.24	0.24	0.00	0.40	0.39	0.01	0.72	0.71	0.01
23.75	0.24	0.23	0.01	0.39	0.38	0.01	0.71	0.69	0.02
24.00	0.23	0.23	0.00	0.38	0.37	0.01	0.69	0.68	0.01



— Post - POS1 - Future 2 year - Flow — Post - POS1 - Future 10 year - Flow — Post - POS1 - Future 10 year - Flow — Pre - POS1 - Future 2 year - Flow — Pre - POS1 - Future 10 year - Flow — Pre - Fl

				st Addition Fu	•				
	2	Year Storm Even		DROGRAPH FLO	OW RATE COME D-Year Storm Eve			0-Year Storm Eve	nt
Time	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.08	0.08	0.00
0.75	0.00	0.00	0.00	0.03	0.03	0.00	0.25	0.25	0.00
1.00	0.01	0.01	0.00	0.09	0.09	0.00	0.38	0.38	0.00
1.25	0.04	0.04	0.00	0.15	0.15	0.00	0.47	0.47	0.00
1.50	0.06	0.06	0.00	0.19	0.19	0.00	0.55	0.55	0.00
1.75	0.09	0.09	0.00	0.23	0.23	0.00	0.61	0.61	0.00
2.00	0.11	0.11	0.00	0.27	0.27	0.00	0.66	0.66	0.00
2.25	0.14	0.14	0.00	0.30	0.30	0.00	0.71	0.71	0.00
2.50	0.15	0.15	0.00	0.33	0.33	0.00	0.74	0.74	0.00
2.75	0.17	0.17	0.00	0.35	0.35	0.00	0.78	0.78	0.00
3.00	0.19	0.19	0.00	0.37	0.37	0.00	0.81	0.81	0.00
3.25	0.21	0.21	0.00	0.40	0.40	0.00	0.84	0.84	0.00
3.50	0.22	0.22	0.00	0.42	0.42	0.00	0.88	0.88	0.00
3.75	0.24	0.24	0.00	0.44	0.44	0.00	0.90	0.90	0.00
4.00	0.25	0.25	0.00	0.46	0.46	0.00	0.93	0.93	0.00
4.25	0.26	0.26	0.00	0.47	0.47	0.00	0.95	0.95	0.00
4.50	0.28	0.28	0.00	0.49	0.49	0.00	0.97	0.97	0.00
4.75	0.29	0.29	0.00	0.50	0.50	0.00	1.00	1.00	0.00
5.00	0.30	0.30	0.00	0.52	0.52	0.00	1.02	1.02	0.00
5.25	0.31	0.31	0.00	0.54	0.54	0.00	1.04	1.04	0.00
5.50	0.32	0.32	0.00	0.55	0.55	0.00	1.07	1.07	0.00
5.75	0.33	0.33	0.00	0.56	0.56	0.00	1.09	1.09	0.00
6.00	0.34	0.34	0.00	0.58	0.58	0.00	1.11	1.11	0.00
6.25	0.37	0.37	0.00	0.61	0.61	0.00	1.17	1.17	0.00
6.50	0.39	0.39	0.00	0.66 0.70	0.66 0.70	0.00	1.25	1.25	0.00
6.75 7.00			0.00	0.70	0.70		1.32 1.40	1.32	
7.00	0.45 0.48	0.45 0.48	0.00	0.74	0.74	0.00	1.40	1.40 1.47	0.00
7.50	0.48	0.50	0.00	0.78	0.78	0.00	1.55	1.55	0.00
7.75	0.53	0.53	0.00	0.82	0.87	0.00	1.62	1.62	0.00
8.00	0.56	0.56	0.00	0.91	0.87	0.00	1.70	1.70	0.00
8.25	0.59	0.59	0.00	0.95	0.95	0.00	1.77	1.77	0.00
8.50	0.62	0.62	0.00	0.99	0.99	0.00	1.84	1.84	0.00
8.75	0.64	0.64	0.00	1.03	1.03	0.00	1.92	1.92	0.00
9.00	0.67	0.67	0.00	1.08	1.08	0.00	2.00	1.99	0.01
9.25	0.75	0.75	0.00	1.20	1.20	0.00	2.24	2.22	0.02
9.50	0.84	0.84	0.00	1.35	1.35	0.00	2.52	2.48	0.04
9.75	0.93	0.93	0.00	1.49	1.49	0.00	2.80	2.74	0.06
10.00	1.03	1.03	0.00	1.63	1.63	0.00	3.09	3.01	0.08
10.25	1.12	1.12	0.00	1.77	1.77	0.00	3.37	3.28	0.09
10.50	1.22	1.22	0.00	1.92	1.92	0.00	3.67	3.57	0.10
10.75	1.51	1.51	0.00	2.38	2.38	0.00	4.56	4.45	0.11
11.00	1.85	1.85	0.00	2.91	2.91	0.00	5.61	5.47	0.14
11.25	2.39	2.39	0.00	3.76	3.75	0.01	7.30	7.11	0.19
11.50	2.99	2.99	0.00	4.74	4.69	0.05	9.20	8.97	0.23
11.75	4.69	4.69	0.00	7.51	7.35	0.16	14.51	14.16	0.35
12.00	12.48	12.44	0.04	20.11	19.70	0.41	38.42	37.68	0.74
12.25	9.86	9.52	0.34	16.37	15.77	0.60	32.24	31.28	0.96
12.50	4.58	4.44	0.14	7.51	7.29	0.22	14.54	14.21	0.33
12.75	3.05	2.97	0.08	4.98	4.85	0.13	9.58	9.39	0.19
13.00	2.42	2.35	0.07	3.94	3.83	0.11	7.56	7.41	0.15
13.25	1.92	1.87	0.05	3.13	3.05	0.08	5.99	5.88	0.11
13.50	1.56	1.52	0.04	2.54	2.47	0.07	4.86	4.77	0.09
13.75	1.32	1.29	0.03	2.15	2.09	0.06	4.10	4.03	0.07

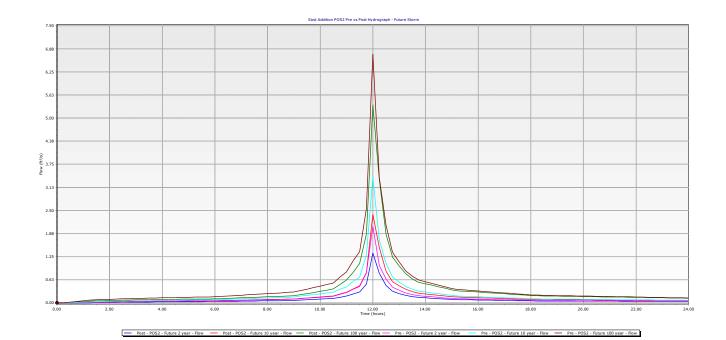
14.00	1.22	1.19	0.03	1.98	1.94	0.04	3.79	3.72	0.07
14.25	1.13	1.10	0.03	1.83	1.79	0.04	3.49	3.43	0.06
14.50	1.03	1.00	0.03	1.67	1.63	0.04	3.18	3.13	0.05
14.75	0.93	0.91	0.02	1.51	1.48	0.03	2.88	2.84	0.04
15.00	0.83	0.81	0.02	1.35	1.32	0.03	2.58	2.53	0.05
15.25	0.77	0.74	0.03	1.24	1.21	0.03	2.36	2.33	0.03
15.50	0.74	0.72	0.02	1.20	1.17	0.03	2.28	2.24	0.04
15.75	0.71	0.69	0.02	1.15	1.13	0.02	2.20	2.16	0.04
16.00	0.68	0.66	0.02	1.11	1.08	0.03	2.11	2.07	0.04
16.25	0.65	0.64	0.01	1.06	1.04	0.02	2.02	1.99	0.03
16.50	0.63	0.61	0.02	1.02	0.99	0.03	1.93	1.90	0.03
16.75	0.60	0.58	0.02	0.97	0.95	0.02	1.85	1.82	0.03
17.00	0.57	0.56	0.01	0.93	0.91	0.02	1.76	1.74	0.02
17.25	0.54	0.53	0.01	0.88	0.86	0.02	1.68	1.65	0.03
17.50	0.52	0.50	0.02	0.84	0.82	0.02	1.59	1.57	0.02
17.75	0.49	0.48	0.01	0.79	0.77	0.02	1.51	1.48	0.03
18.00	0.46	0.45	0.01	0.75	0.73	0.02	1.42	1.40	0.02
18.25	0.44	0.43	0.01	0.72	0.70	0.02	1.36	1.34	0.02
18.50	0.43	0.42	0.01	0.70	0.69	0.01	1.33	1.31	0.02
18.75	0.43	0.42	0.01	0.69	0.68	0.01	1.31	1.29	0.02
19.00	0.42	0.41	0.01	0.68	0.67	0.01	1.29	1.27	0.02
19.25	0.41	0.40	0.01	0.67	0.66	0.01	1.27	1.25	0.02
19.50	0.41	0.40	0.01	0.66	0.64	0.02	1.25	1.23	0.02
19.75	0.40	0.39	0.01	0.65	0.64	0.01	1.23	1.21	0.02
20.00	0.39	0.38	0.01	0.64	0.62	0.02	1.21	1.19	0.02
20.25	0.39	0.38	0.01	0.63	0.61	0.02	1.19	1.17	0.02
20.50	0.38	0.37	0.01	0.61	0.60	0.01	1.16	1.15	0.01
20.75	0.37	0.36	0.01	0.60	0.59	0.01	1.14	1.13	0.01
21.00	0.37	0.36	0.01	0.59	0.58	0.01	1.12	1.11	0.01
21.25	0.36	0.35	0.01	0.58	0.57	0.01	1.10	1.09	0.01
21.50	0.35	0.34	0.01	0.57	0.56	0.01	1.08	1.06	0.02
21.75	0.34	0.34	0.00	0.56	0.55	0.01	1.06	1.04	0.02
22.00	0.34	0.33	0.01	0.55	0.54	0.01	1.04	1.02	0.02
22.25	0.33	0.32	0.01	0.54	0.52	0.02	1.02	1.00	0.02
22.50	0.32	0.31	0.01	0.52	0.51	0.01	0.99	0.98	0.01
22.75	0.32	0.31	0.01	0.51	0.50	0.01	0.97	0.96	0.01
23.00	0.31	0.30	0.01	0.50	0.49	0.01	0.95	0.93	0.02
23.25	0.30	0.29	0.01	0.49	0.48	0.01	0.93	0.91	0.02
23.50	0.30	0.29	0.01	0.48	0.47	0.01	0.91	0.89	0.02
23.75	0.29	0.28	0.01	0.47	0.46	0.01	0.88	0.87	0.01
24.00	0.28	0.27	0.01	0.46	0.45	0.01	0.86	0.85	0.01



East Addition Current Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

	2	2 Year Storm Event 10-Year Storm Event 100-Year Storm Event							ent
Time	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.00
0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.14	0.14	0.00
1.00	0.00	0.00	0.00	0.05	0.05	0.00	0.25	0.25	0.00
1.25	0.01	0.01	0.00	0.09	0.09	0.00	0.32	0.32	0.00
1.50	0.04	0.04	0.00	0.13	0.13	0.00	0.39	0.39	0.00
1.75	0.06	0.06	0.00	0.17	0.17	0.00	0.44	0.44	0.00
2.00	0.07	0.07	0.00	0.20	0.20	0.00	0.49	0.49	0.00
2.25	0.09	0.09	0.00	0.23	0.23	0.00	0.53	0.53	0.00
2.50	0.11	0.11	0.00	0.25	0.25	0.00	0.56	0.56	0.00
2.75	0.13	0.13	0.00	0.27	0.27	0.00	0.60	0.60	0.00
3.00	0.14	0.14	0.00	0.29	0.29	0.00	0.63	0.62	0.01
3.25	0.15	0.15	0.00	0.31	0.31	0.00	0.65	0.65	0.00
3.50	0.17	0.17	0.00	0.33	0.33	0.00	0.68	0.68	0.00
3.75	0.18	0.18	0.00	0.35	0.35	0.00	0.70	0.70	0.00
4.00	0.19	0.19	0.00	0.37	0.37	0.00	0.73	0.73	0.00
4.25	0.20	0.20	0.00	0.38	0.38	0.00	0.75	0.75	0.00
4.50	0.21	0.21	0.00	0.40	0.40	0.00	0.77	0.77	0.00
4.75	0.22	0.22	0.00	0.41	0.41	0.00	0.79	0.79	0.00
5.00	0.24	0.24	0.00	0.43	0.43	0.00	0.81	0.81	0.00
5.25	0.25	0.25	0.00	0.44	0.44	0.00	0.83	0.83	0.00
5.50	0.26	0.26	0.00	0.45	0.45	0.00	0.85	0.85	0.00
5.75	0.26	0.26	0.00	0.47	0.46	0.01	0.87	0.86	0.01
6.00	0.27	0.27	0.00	0.48	0.48	0.00	0.88	0.88	0.00
6.25	0.29	0.29	0.00	0.51	0.51	0.00	0.94	0.94	0.00
6.50	0.32	0.32	0.00	0.54	0.54	0.00	1.00	0.99	0.01
6.75	0.34	0.34	0.00	0.58	0.58	0.00	1.06	1.05	0.01
7.00	0.36	0.36	0.00	0.62	0.62	0.00	1.12	1.12	0.00
7.25	0.39	0.39	0.00	0.65	0.65	0.00	1.18	1.18	0.00
7.50	0.41	0.41	0.00	0.69	0.69	0.00	1.24	1.24	0.00
7.75	0.43	0.43	0.00	0.72	0.72	0.00	1.30	1.30	0.00
8.00	0.46	0.46	0.00	0.76	0.76	0.00	1.36	1.36	0.00
8.25	0.48	0.48	0.00	0.79	0.79	0.00	1.42	1.42	0.00
8.50 8.75	0.50	0.50	0.00	0.83	0.83	0.00	1.48	1.48	0.00
9.00	0.53 0.55	0.53 0.55	0.00	0.87 0.90	0.87	0.00	1.54 1.60	1.54	0.00
9.25	0.53	0.62	0.00	1.01	0.90 1.01	0.00	1.79	1.60 1.78	0.00
9.50	0.82	0.62	0.00	1.13	1.13	0.00	2.00	2.00	0.00
9.75	0.70	0.70	0.00	1.15	1.15	0.00	2.00	2.20	0.00
10.00	0.85	0.85	0.00	1.38	1.37	0.01	2.44	2.41	0.03
10.25	0.93	0.93	0.00	1.49	1.49	0.00	2.66	2.62	0.04
10.50	1.01	1.01	0.00	1.62	1.62	0.00	2.89	2.83	0.06
10.75	1.25	1.25	0.00	2.01	2.00	0.01	3.60	3.51	0.09
11.00	1.54	1.53	0.01	2.46	2.45	0.01	4.43	4.31	0.12
11.25	1.99	1.99	0.00	3.17	3.17	0.00	5.76	5.61	0.15
11.50	2.50	2.49	0.01	3.96	3.96	0.00	7.27	7.07	0.20
11.75	3.92	3.92	0.00	6.26	6.20	0.06	11.47	11.18	0.29
12.00	10.42	10.40	0.02	16.78	16.46	0.32	30.45	29.86	0.59
12.25	8.06	7.88	0.18	13.55	13.01	0.54	25.46	24.50	0.96
12.50	3.76	3.65	0.11	6.26	6.04	0.22	11.55	11.20	0.35
12.75	2.51	2.43	0.08	4.15	4.03	0.12	7.62	7.42	0.20
13.00	1.99	1.93	0.06	3.28	3.19	0.09	6.01	5.86	0.15
13.25	1.58	1.54	0.04	2.61	2.53	0.08	4.77	4.66	0.11
13.50	1.28	1.24	0.04	2.12	2.06	0.06	3.87	3.78	0.09

13.75	1.09	1.06	0.03	1.79	1.74	0.05	3.27	3.19	0.08
14.00	1.01	0.98	0.03	1.66	1.61	0.05	3.02	2.95	0.07
14.25	0.93	0.90	0.03	1.53	1.48	0.05	2.78	2.72	0.06
14.50	0.85	0.82	0.03	1.39	1.35	0.04	2.54	2.48	0.06
14.75	0.77	0.74	0.03	1.26	1.23	0.03	2.30	2.25	0.05
15.00	0.69	0.67	0.02	1.13	1.10	0.03	2.05	2.01	0.04
15.25	0.63	0.61	0.02	1.04	1.01	0.03	1.88	1.84	0.04
15.50	0.61	0.59	0.02	1.00	0.97	0.03	1.82	1.78	0.04
15.75	0.59	0.57	0.02	0.96	0.94	0.02	1.75	1.71	0.04
16.00	0.56	0.55	0.01	0.92	0.90	0.02	1.68	1.64	0.04
16.25	0.54	0.52	0.02	0.89	0.86	0.03	1.61	1.58	0.03
16.50	0.52	0.50	0.02	0.85	0.83	0.02	1.54	1.51	0.03
16.75	0.50	0.48	0.02	0.81	0.79	0.02	1.48	1.45	0.03
17.00	0.47	0.46	0.01	0.77	0.75	0.02	1.41	1.38	0.03
17.25	0.45	0.44	0.01	0.74	0.72	0.02	1.34	1.31	0.03
17.50	0.43	0.41	0.02	0.70	0.68	0.02	1.27	1.24	0.03
17.75	0.40	0.39	0.01	0.66	0.64	0.02	1.20	1.18	0.02
18.00	0.38	0.37	0.01	0.62	0.61	0.01	1.13	1.11	0.02
18.25	0.36	0.35	0.01	0.60	0.58	0.02	1.08	1.06	0.02
18.50	0.36	0.35	0.01	0.59	0.57	0.02	1.06	1.04	0.02
18.75	0.35	0.34	0.01	0.58	0.56	0.02	1.05	1.03	0.02
19.00	0.35	0.34	0.01	0.57	0.55	0.02	1.03	1.01	0.02
19.25	0.34	0.33	0.01	0.56	0.55	0.01	1.02	1.00	0.02
19.50	0.34	0.33	0.01	0.55	0.54	0.01	1.00	0.98	0.02
19.75	0.33	0.32	0.01	0.54	0.53	0.01	0.98	0.96	0.02
20.00	0.32	0.32	0.00	0.53	0.52	0.01	0.96	0.95	0.01
20.25	0.32	0.31	0.01	0.52	0.51	0.01	0.95	0.93	0.02
20.50	0.31	0.30	0.01	0.51	0.50	0.01	0.93	0.91	0.02
20.75	0.31	0.30	0.01	0.50	0.49	0.01	0.91	0.90	0.01
21.00	0.30	0.29	0.01	0.49	0.48	0.01	0.90	0.88	0.02
21.25	0.30	0.29	0.01	0.49	0.47	0.02	0.88	0.86	0.02
21.50	0.29	0.28	0.01	0.48	0.46	0.02	0.86	0.84	0.02
21.75	0.28	0.28	0.00	0.47	0.45	0.02	0.84	0.83	0.01
22.00	0.28	0.27	0.01	0.46	0.45	0.01	0.83	0.81	0.02
22.25	0.27	0.27	0.00	0.45	0.44	0.01	0.81	0.80	0.01
22.50	0.27	0.26	0.01	0.44	0.43	0.01	0.79	0.78	0.01
22.75	0.26	0.25	0.01	0.43	0.42	0.01	0.78	0.76	0.02
23.00	0.25	0.25	0.00	0.42	0.41	0.01	0.76	0.74	0.02
23.25	0.25	0.24	0.01	0.41	0.40	0.01	0.74	0.73	0.01
23.50	0.24	0.24	0.00	0.40	0.39	0.01	0.72	0.71	0.01
23.75	0.24	0.23	0.01	0.39	0.38	0.01	0.71	0.69	0.02
24.00	0.23	0.23	0.00	0.38	0.37	0.01	0.69	0.68	0.01



				st Addition Fu	•				
	2	Year Storm Even		DROGRAPH FLO	OW RATE COME D-Year Storm Eve			0-Year Storm Eve	nt
Time	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction	Pre-Dev	Post-Dev	Reduction
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.08	0.08	0.00
0.75	0.00	0.00	0.00	0.03	0.03	0.00	0.25	0.25	0.00
1.00	0.01	0.01	0.00	0.09	0.09	0.00	0.38	0.38	0.00
1.25	0.04	0.04	0.00	0.15	0.15	0.00	0.47	0.47	0.00
1.50	0.06	0.06	0.00	0.19	0.19	0.00	0.55	0.55	0.00
1.75	0.09	0.09	0.00	0.23	0.23	0.00	0.61	0.61	0.00
2.00	0.11	0.11	0.00	0.27	0.27	0.00	0.66	0.66	0.00
2.25	0.14	0.14	0.00	0.30	0.30	0.00	0.71	0.71	0.00
2.50	0.15	0.15	0.00	0.33	0.33	0.00	0.74	0.74	0.00
2.75	0.17	0.17	0.00	0.35	0.35	0.00	0.78	0.78	0.00
3.00	0.19	0.19	0.00	0.37	0.37	0.00	0.81	0.81	0.00
3.25	0.21	0.21	0.00	0.40	0.40	0.00	0.84	0.84	0.00
3.50	0.22	0.22	0.00	0.42	0.42	0.00	0.88	0.88	0.00
3.75	0.24	0.24	0.00	0.44	0.44	0.00	0.90	0.90	0.00
4.00	0.25	0.25	0.00	0.46	0.46	0.00	0.93	0.93	0.00
4.25	0.26	0.26	0.00	0.47	0.47	0.00	0.95	0.95	0.00
4.50	0.28	0.28	0.00	0.49	0.49	0.00	0.97	0.97	0.00
4.75	0.29	0.29	0.00	0.50	0.50	0.00	1.00	1.00	0.00
5.00	0.30	0.30	0.00	0.52	0.52	0.00	1.02	1.02	0.00
5.25	0.31	0.31	0.00	0.54	0.54	0.00	1.04	1.04	0.00
5.50	0.32	0.32	0.00	0.55	0.55	0.00	1.07	1.07	0.00
5.75	0.33	0.33	0.00	0.56	0.56	0.00	1.09	1.09	0.00
6.00	0.34	0.34	0.00	0.58	0.58	0.00	1.11	1.11	0.00
6.25	0.37	0.37	0.00	0.61	0.61	0.00	1.17	1.17	0.00
6.50	0.39	0.39	0.00	0.66 0.70	0.66 0.70	0.00	1.25	1.25	0.00
6.75 7.00			0.00	0.70	0.70		1.32 1.40	1.32	
7.00	0.45 0.48	0.45 0.48	0.00	0.74	0.74	0.00	1.40	1.40 1.47	0.00
7.50	0.48	0.50	0.00	0.78	0.78	0.00	1.55	1.55	0.00
7.75	0.53	0.53	0.00	0.82	0.87	0.00	1.62	1.62	0.00
8.00	0.56	0.56	0.00	0.91	0.87	0.00	1.70	1.70	0.00
8.25	0.59	0.59	0.00	0.95	0.95	0.00	1.77	1.77	0.00
8.50	0.62	0.62	0.00	0.99	0.99	0.00	1.84	1.84	0.00
8.75	0.64	0.64	0.00	1.03	1.03	0.00	1.92	1.92	0.00
9.00	0.67	0.67	0.00	1.08	1.08	0.00	2.00	1.99	0.01
9.25	0.75	0.75	0.00	1.20	1.20	0.00	2.24	2.22	0.02
9.50	0.84	0.84	0.00	1.35	1.35	0.00	2.52	2.48	0.04
9.75	0.93	0.93	0.00	1.49	1.49	0.00	2.80	2.74	0.06
10.00	1.03	1.03	0.00	1.63	1.63	0.00	3.09	3.01	0.08
10.25	1.12	1.12	0.00	1.77	1.77	0.00	3.37	3.28	0.09
10.50	1.22	1.22	0.00	1.92	1.92	0.00	3.67	3.57	0.10
10.75	1.51	1.51	0.00	2.38	2.38	0.00	4.56	4.45	0.11
11.00	1.85	1.85	0.00	2.91	2.91	0.00	5.61	5.47	0.14
11.25	2.39	2.39	0.00	3.76	3.75	0.01	7.30	7.11	0.19
11.50	2.99	2.99	0.00	4.74	4.69	0.05	9.20	8.97	0.23
11.75	4.69	4.69	0.00	7.51	7.35	0.16	14.51	14.16	0.35
12.00	12.48	12.44	0.04	20.11	19.70	0.41	38.42	37.68	0.74
12.25	9.86	9.52	0.34	16.37	15.77	0.60	32.24	31.28	0.96
12.50	4.58	4.44	0.14	7.51	7.29	0.22	14.54	14.21	0.33
12.75	3.05	2.97	0.08	4.98	4.85	0.13	9.58	9.39	0.19
13.00	2.42	2.35	0.07	3.94	3.83	0.11	7.56	7.41	0.15
13.25	1.92	1.87	0.05	3.13	3.05	0.08	5.99	5.88	0.11
13.50	1.56	1.52	0.04	2.54	2.47	0.07	4.86	4.77	0.09
13.75	1.32	1.29	0.03	2.15	2.09	0.06	4.10	4.03	0.07

14.00	1.22	1.19	0.03	1.98	1.94	0.04	3.79	3.72	0.07
14.25	1.13	1.10	0.03	1.83	1.79	0.04	3.49	3.43	0.06
14.50	1.03	1.00	0.03	1.67	1.63	0.04	3.18	3.13	0.05
14.75	0.93	0.91	0.02	1.51	1.48	0.03	2.88	2.84	0.04
15.00	0.83	0.81	0.02	1.35	1.32	0.03	2.58	2.53	0.05
15.25	0.77	0.74	0.03	1.24	1.21	0.03	2.36	2.33	0.03
15.50	0.74	0.72	0.02	1.20	1.17	0.03	2.28	2.24	0.04
15.75	0.71	0.69	0.02	1.15	1.13	0.02	2.20	2.16	0.04
16.00	0.68	0.66	0.02	1.11	1.08	0.03	2.11	2.07	0.04
16.25	0.65	0.64	0.01	1.06	1.04	0.02	2.02	1.99	0.03
16.50	0.63	0.61	0.02	1.02	0.99	0.03	1.93	1.90	0.03
16.75	0.60	0.58	0.02	0.97	0.95	0.02	1.85	1.82	0.03
17.00	0.57	0.56	0.01	0.93	0.91	0.02	1.76	1.74	0.02
17.25	0.54	0.53	0.01	0.88	0.86	0.02	1.68	1.65	0.03
17.50	0.52	0.50	0.02	0.84	0.82	0.02	1.59	1.57	0.02
17.75	0.49	0.48	0.01	0.79	0.77	0.02	1.51	1.48	0.03
18.00	0.46	0.45	0.01	0.75	0.73	0.02	1.42	1.40	0.02
18.25	0.44	0.43	0.01	0.72	0.70	0.02	1.36	1.34	0.02
18.50	0.43	0.42	0.01	0.70	0.69	0.01	1.33	1.31	0.02
18.75	0.43	0.42	0.01	0.69	0.68	0.01	1.31	1.29	0.02
19.00	0.42	0.41	0.01	0.68	0.67	0.01	1.29	1.27	0.02
19.25	0.41	0.40	0.01	0.67	0.66	0.01	1.27	1.25	0.02
19.50	0.41	0.40	0.01	0.66	0.64	0.02	1.25	1.23	0.02
19.75	0.40	0.39	0.01	0.65	0.64	0.01	1.23	1.21	0.02
20.00	0.39	0.38	0.01	0.64	0.62	0.02	1.21	1.19	0.02
20.25	0.39	0.38	0.01	0.63	0.61	0.02	1.19	1.17	0.02
20.50	0.38	0.37	0.01	0.61	0.60	0.01	1.16	1.15	0.01
20.75	0.37	0.36	0.01	0.60	0.59	0.01	1.14	1.13	0.01
21.00	0.37	0.36	0.01	0.59	0.58	0.01	1.12	1.11	0.01
21.25	0.36	0.35	0.01	0.58	0.57	0.01	1.10	1.09	0.01
21.50	0.35	0.34	0.01	0.57	0.56	0.01	1.08	1.06	0.02
21.75	0.34	0.34	0.00	0.56	0.55	0.01	1.06	1.04	0.02
22.00	0.34	0.33	0.01	0.55	0.54	0.01	1.04	1.02	0.02
22.25	0.33	0.32	0.01	0.54	0.52	0.02	1.02	1.00	0.02
22.50	0.32	0.31	0.01	0.52	0.51	0.01	0.99	0.98	0.01
22.75	0.32	0.31	0.01	0.51	0.50	0.01	0.97	0.96	0.01
23.00	0.31	0.30	0.01	0.50	0.49	0.01	0.95	0.93	0.02
23.25	0.30	0.29	0.01	0.49	0.48	0.01	0.93	0.91	0.02
23.50	0.30	0.29	0.01	0.48	0.47	0.01	0.91	0.89	0.02
23.75	0.29	0.28	0.01	0.47	0.46	0.01	0.88	0.87	0.01
24.00	0.28	0.27	0.01	0.46	0.45	0.01	0.86	0.85	0.01

Project	JSMC East Addition		By	SPT	Date		-		
Location	Township of Neptune, I	Monmouth County, NJ	_Checked	MI	Date		-		
Circle One:	Present Developed								
Circle One:	T_c T_t	through subarea	Existing	DA-1, Imp	ervious, C	urrent Sto	orm		
NOTES: Space for worksh		ts per flow type can be used for ea	ach						
Include	e a map, schematic, or des	cription of flow segments.							
Sheet flow (Appl	icable to T _c Only)		Segment ID	1					
Surface description	cription (table 3-1)			Asphalt					
2. Manning's ro	ughness coeff., n (Table 15	5-1)		0.011					
3. Flow Length,	L		ft	100					
4. Two-yr 24-hr	rainfall, P ₂		in	3.48					
5. Land slope, s	3		ft/ft	0.008					
6. T _t =0.0	007(nL) ^{0.8}	Compute T _t	hr	0.028	+		+	=	0.028
ŀ	2 s								
Shallow concen	trated flow		Segment ID	2					
7. Surface desc	cription (paved or unpaved)			Paved					
8. Flow length,	L		ft	305					
9. Watercourse	slope, s		ft/ft	0.017					
10. Average velo	ocity, V (Figure 15-4)		ft/s	2.60			_		
11. T _t =	L	Compute T _t	hr	0.033	+		+	=	0.033
	3600 V								
Channel flow			Segment ID	3	4	5	6		
12. Cross section	nal flow area, a		ft ²	1.22718	1.76715	4.90874	7.068583		
13. Wetted perim	neter, p _w		ft	3.92699	4.71239	7.85398	9.424778		
14. Hydraulic rad	lius, r	$r = \frac{a}{P_W}$ Compute r	ft	0.31	0.38	0.63	0.75		
15. Channel slop	oe, s		ft/ft	0.012	0.015	0.016	0.005		
16. Manning's ro	ughness coeff., n			0.013	0.013	0.013	0.013		
=	4 402/3 - 1/2						İ		

Compute V

Compute T_t

18. Flow length, L $T_t = \frac{}{36}$

7.35

376

0.014

ft/s

5.68

81

0.004

10.60

31

0.001

6.69

12

0.000

			Segment ID	7		
12. Cross sectional flow area, a			ft ²	9.62113		
13. Wetted perimeter, p _w			ft	10.9956		
	r= <u>a</u>					
14. Hydraulic radius, r	p_{w}	Compute r	ft	0.88		
15. Channel slope, s			ft/ft	0.003		
16. Manning's roughness coeff., n				0.013		
$V = $ 1.49 $r^{2/3} s^{1/2}$						
17. v –n		Compute V	ft/s	5.74		
18. Flow length, L			ft	612		

Compute T_t

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

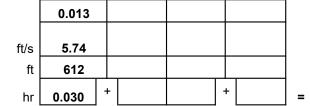
hr **0.11**

Project JSMC East Addition		Ву	SPT	Date				
Location Township of Neptune,	Monmouth County,	Checked	MI	Date				
Circle One: Present Developed								
Circle One: T_c T_t thro	ough subarea	Exis	ting DA-1, I	mpervious	s, Future S	Storm		
NOTES: Space for as many as two segments worksheet.	s per flow type can be	used for ea	ach					
Include a map, schematic, or desc	ription of flow segmer	nts.						
Sheet flow (Applicable to T _c Only)	Se	egment ID	1					
1. Surface description (table 3-1)			Asphalt					
2. Manning's roughness coeff., n (Table 15	-1)		0.011					
3. Flow Length, L		ft	100					
4. Two-yr 24-hr rainfall, P ₂		in	4.14					
5. Land slope, s		ft/ft	0.008				ı	
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$	Compute T _t	hr	0.026	+		+	=	0.026
Shallow concentrated flow	Se	egment ID	2					
7. Surface description (paved or unpaved)			Paved					
8. Flow length, L		ft	305					
9. Watercourse slope, s		ft/ft	0.017					
10. Average velocity, V (Figure 15-4)		ft/s	2.6				ı	
11. $T_t = \frac{L}{3600 \text{ V}}$	Compute T _t	hr	0.033	+		+	=	0.033
Channel flow	Se	egment ID	3	4	5	6		
12. Cross sectional flow area, a		ft ²	1.227185	1.76715	4.90874	7.06858		
13. Wetted perimeter, p _w		ft	3.926991	4.71239	7.85398	9.42478		
14. Hydraulic radius, r	: a Pw Compute r	ft	0.31	0.38	0.63	0.75		
15. Channel slope, s		ft/ft	0.012	0.015	0.016	0.005		
16. Manning's roughness coeff., n			0.013	0.013	0.013	0.013		
$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$	Compute V	ft/s	E 60	7 25	10.60	6 60		
18. Flow length, L	Compute v	ft	5.68 81	7.35 376	31	6.69 12		
$T_{t} = \frac{L}{3600 \text{ V}}$	Compute T _t			-		+ 0.000	=	0.019
19. 3000 V	Compute 1 _t	hr	0.004	0.014	0.001	0.000	_	0.019
	Se	egment ID	7					
12. Cross sectional flow area, a		ft ²	9.621128					
13. Wetted perimeter, p _w	а	ft	10.99557					
r = 14. Hydraulic radius, r	p _w Compute r	ft	0.88					
15. Channel slope, s	·	ft/ft	0.003					

16. Manning's roughness coeff., n

	V/ =	1.49 $r^{2/3} s^{1/2}$
17.	v –	n

Compute V



18. Flow length, L

19.
$$T_t = \frac{L}{3600 \text{ V}}$$

Compute T_t

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

hr **0.11**

0.030

Project	JSMC East Addition		By		SPT	Date				
Location	Township of Neptune, N	Monmouth County, NJ	Chec	ked	MI	Date				
Circle One:	Present Developed									
Circle One:	T_c T_t	through subarea		Existing	DA-1, Pe	rvious, Cu	rrent Storr	<u>n</u>		
NOTES: Space for worksh	or as many as two segment neet.	s per flow type can be ι	ised for each							
Include	e a map, schematic, or desc	cription of flow segment	S.							
Sheet flow (Appl	icable to T _c Only)		5	Segment ID	1					
					Short					
Surface desc	cription (table 3-1)				Grass					
2. Manning's ro	ughness coeff., n (Table 15	i-1)			0.150					
3. Flow Length,	L			ft	63					
4. Two-yr 24-hr	rainfall, P ₂			in	3.48					
5. Land slope,	S			ft/ft	0.012					
6. T _t =0.0	$\frac{0.07(nL)^{0.8}}{2^{0.5}s^{0.4}}$	Cor	mpute T _t	hr	0.133	+			=	0.133
Shallow concen	trated flow		5	Segment ID	2					
7. Surface desc	cription (paved or unpaved)				Paved					
8. Flow length,	L			ft	20					
9. Watercourse	slope, s			ft/ft	0.017					
10. Average velo	ocity, V (Figure 15-4)			ft/s	2.60					
11. T _t =	<u>L</u>	Cor	mpute T _t	hr	0.002	+			= [0.002
	3600 V									
Channel flow			S	Segment ID	3	4	5	6		
12. Cross section	nal flow area, a			ft ²	1.76715	3.14159	4.90874	20.5		
13. Wetted perin	neter, p _w			ft	4.71239	6.28319	7.85398	16.68		
14. Hydraulic rac	lius, r	$r = \frac{a}{P_W}$ Co	mpute r	ft	0.38	0.50	0.63	1.23		
15. Channel slop	oe, s			ft/ft	0.010	0.015	0.005	0.003		
16. Manning's ro	ughness coeff., n				0.013	0.013	0.013	0.013		
•	1 49 r ^{2/3} s ^{1/2}									

Compute V

Compute T_t

18. Flow length, L $T_t = \frac{}{36}$

5.90

140

0.007

ft/s

8.69

139

0.004

5.92

27

0.001

7.55

576

12	Cross	sectional	flow	area	а
1 – .	01033	300tional	IIOVV	aica,	а

13. Wetted perimeter, p_w

14. Hydraulic radius, r

15. Channel slope, s

16. Manning's roughness coeff., n

 $V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$ 17.

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

Compute V

Compute r

Segment ID	7	8	
ft ²	32	30	
ft	24	26	
ft	1.33	1.15	
ft/ft	0.003	0.003	
	0.013	0.013	
ft/s	7.60	6.91	
ft	160	708	
hr	0.006	+ 0.028	+

0.034

0.20

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

EDA1-Per - 2/2

Proje	ect JSMC East Addition	on		Ву	SPT	Date				
Loca	tion Township of Nept	une, Monmo	uth County, I	Checked	MI	Date				
Circle	e One: Present Develope	ed								
Circle	e One: T _c T _t	through sub	oarea	Exis	sting DA-1,	Pervious,	Future St	orm		
NOT	ES: Space for as many as two segn worksheet.	nents per flov	w type can be	used for ea	ach					
	Include a map, schematic, or	description o	f flow segmen	nts.						
Shee	et flow (Applicable to T _c Only)		Se	egment ID	1					
1.	Surface description (table 3-1)				Short Grass					
2.	Manning's roughness coeff., n (Tabl	e 15-1)			0.15					
3.	Flow Length, L			ft	63					
4.	Two-yr 24-hr rainfall, P ₂			in	4.14					
5.	Land slope, s			ft/ft	0.012				ı	
6.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{2}^{0.5}s^{0.4}}$		Compute T _t	hr	0.122	+	-	+	=	0.122
Chal	llaw as passificated flow		C.	amant ID	2					
	<u>llow concentrated flow</u> Surface description (paved or unpay	(od)	36	egment ID	2 Payed					
	Surface description (paved or unpay Flow length, L	reu)		ft	Paved 20					
	Watercourse slope, s			ft/ft	0.017					
	Average velocity, V (Figure 15-4)			ft/s	2.6					
			Compute T _t	hr	0.002	+	-	+	=	0.002
11.	$T_{t} = \frac{L}{3600 \text{ V}}$		Compute 1	ייין	0.002				_	0.002
Chai	nnel flow		Se	egment ID	3	4	5	6		
	Cross sectional flow area, a			ft ²	1.767146	3.14159	4.90874	20.5		
13. \	Wetted perimeter, p _w			ft	4.712389	6.28319	7.85398	16.68		
14.	Hydraulic radius, r	$r = \frac{a}{-p_w}$	Compute r	ft	0.38	0.50	0.63	1.23		
15.	Channel slope, s			ft/ft	0.010	0.015	0.005	0.003		
16.	Manning's roughness coeff., n				0.013	0.013	0.013	0.013		
4-	$V = $ 1.49 $r^{2/3} s^{1/2}$	_		6.7						
17.	n		Compute V	ft/s	5.90	8.69	5.92	7.55		
	Flow length, L			ft	140	139	27	576		
19.	$T_{t} = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.007	0.004	0.001	0.021	=	0.033
			Se	egment ID	7	8				
12.	Cross sectional flow area, a		00	ft ²	32	30				
	Wetted perimeter, p _w			ft	24	26				
		r = <u>a</u>								
	Hydraulic radius, r	p_{w}	Compute r	ft	1.33	1.15				
15	Channel slope s			ft/ft	0.003	0.003				

16. Manning's roughness coeff., n

	\/ =	$1.49 r^{2/3} s^{1/2}$
17.	v –	n

Compute V

	0.013		0.013		
ft/s	7.60		6.91	\perp	
ft	160		708		
hr	0.006	+	0.028	+	

18. Flow length, L

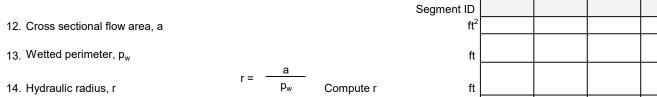
19.
$$T_t = \frac{L}{3600 \text{ V}}$$

Compute T_t

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

hr **0.19**

Project	JSMC East Addition		Ву	SPT	Date			
Location	Township of Neptune, N	Monmouth County, NJ	Checked	MI	Date			
Circle One:	Present Developed		_					
Circle One:	T_c T_t	through subarea	Existing	DA-2, Imp	ervious, C	urrent Storm		
NOTES: Space fo	,	s per flow type can be used for e	ach					
Include	e a map, schematic, or des	cription of flow segments.						
Sheet flow (Appl	icable to T _c Only)		Segment ID	1				
Surface desc	ription (table 3-1)			Asphalt				
2. Manning's ro	ughness coeff., n (Table 15	i-1)		0.011				
3. Flow Length,	L		ft	100				
4. Two-yr 24-hr	rainfall, P ₂		in	3.48				
5. Land slope, s	3		ft/ft	0.009				
6. $T_t = \frac{0.0}{F}$	007(nL) ^{0.8}	Compute T_t	hr	0.027	+	+	=	0.027
·	2 5					1		
Shallow concent	trated flow		Segment ID	2				
7. Surface desc	ription (paved or unpaved)			Paved				
8. Flow length, I	L		ft	241				
9. Watercourse	•		ft/ft	0.013				
10. Average velo	city, V (Figure 15-4)		ft/s	2.41				
11. T _t =	<u>L</u> 3600 V	Compute T _t	hr	0.028	+	+	=	0.028
Channel flow			Segment ID					
12. Cross section	nal flow area, a		ft ²					
13. Wetted perim	•		ft					
14. Hydraulic rad		$r = \frac{a}{P_W}$ Compute r	ft					
15. Channel slop		·	ft/ft					
16. Manning's ro	ughness coeff., n							
	1.49 r ^{2/3} s ^{1/2}	_						
		Compute V	ft/s					
18. Flow length, I			ft	I	1			
19. $T_t = \frac{1}{3}$	L 3600 V	Compute T _t	hr	-	+	+	=	0.000



- 14. Hydraulic radius, r15. Channel slope, sPw Compute
- 16. Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V
- 18. Flow length, L

 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t ft

 hr

 +

 =

 0.000

 hr

 0.05

ft/ft

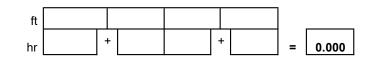
ft/s

Project JSMC East Addition	on	Ву	SPT	Date _			
Location Township of Nept	une, Monmouth County, NJ	Checked	MI	Date _			
Circle One: Present Develope	 ed	<u></u>				_	
Circle One: T_c T_t	through subarea	Exist	ting DA-2, I	mpervious,	Future Storm	_	
NOTES: Space for as many as two segr	nents per flow type can be use	d for each				_	
worksheet.							
Include a map, schematic, or	description of flow segments.	_				_	
Sheet flow (Applicable to T _c Only)		Segment ID	1			4	
1. Surface description (table 3-1)			Asphalt				
2. Manning's roughness coeff., n (Tab	le 15-1)		0.011				
3. Flow Length, L		ft	100				
4. Two-yr 24-hr rainfall, P ₂		in	4.14				
5. Land slope, s		ft/ft	0.009			<u> </u>	
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$	Compute T _t	hr	0.024	+	+	=	0.024
$P_2^{0.5}$ s ^{0.4}		•		<u> </u>			
Shallow concentrated flow		Segment ID	2				
7. Surface description (paved or unpar	ved)		Paved				
8. Flow length, L		ft	241]	
9. Watercourse slope, s		ft/ft	0.013]	
10. Average velocity, V (Figure 15-4)		ft/s	2.41]	
11. $T_t = \frac{L}{3600 \text{ V}}$	Compute T _t	hr	0.028	+	+] = [0.028
Channel flow		Segment ID				٦	
12. Cross sectional flow area, a		ft ²				1	
13. Wetted perimeter, p _w		•				-	
	r = a Compute r	ft _				-	
14. Hydraulic radius, r	r = Pw Compute r	ft				-	
15. Channel slope, s		ft/ft				-	
16. Manning's roughness coeff., n						-	
$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$	Compute V	ft/s					
18. Flow length, L		ft]	
$T_t = \frac{L}{3600 \text{ V}}$	Computo T	L		+	+	1_ [0.000
19. 3600 V	Compute T _t	hr [_ =(0.000
		Segment ID				1	
12. Cross sectional flow area, a		ft ²				1	
		_				7	
13. Wetted perimeter, p _w	а	ft				-	
14. Hydraulic radius, r	$r = \frac{a}{p_w}$ Compute r	ft					
15. Channel slope, s	- Compute I	ft/ft				1	
16. Manning's roughness coeff., n		1011				1	
$V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$		ŀ				1	
17. V =n	Compute V	ft/s					

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Project	JSMC East Addition		Ву	SPT	Date				
Location	Township of Neptune, M	Ionmouth County, NJ	Checked	MI	Date				
Circle One:	Present Developed				-				
Circle One:	T_c T_t	through subarea	Existing	DA-2, Pe	rvious, Cu	rrent Stori	n	-	
NOTES: Space f		s per flow type can be used for ea	ach						
Includ	e a map, schematic, or desc	ription of flow segments.							
Sheet flow (App	licable to T _c Only)		Segment ID	1					
1 Curface does	cription (table 3-1)			Short Grass					
	oughness coeff., n (Table 15	_1\		0.15					
Flow Length	-	-1)	ft	20					
4. Two-yr 24-hr			in	3.48					
5. Land slope,			ft/ft	0.010					
•		Compute T _t	hr	0.06	-	-	-	_ [0.06
6. T _t =0.0	P ₂ ^{0.5} s ^{0.4}	, ,	·						
Shallow concer	ntrated flow		Segment ID	2					
	cription (paved or unpaved)			Paved					
8. Flow length,	L		ft	274					
9. Watercourse	e slope, s		ft/ft	0.013					
10. Average velo	ocity, V (Figure 15-4)		ft/s	2.40	_				
11. T _t =	L	Compute T _t	hr	0.03 +		4	-	=	0.03
	3600 V								
Channel flow			Segment ID						
12. Cross sectio	nal flow area, a		ft ²						
13. Wetted perir	neter, p _w		ft						
14. Hydraulic rad	dius, r	r = $\frac{a}{P_W}$ Compute r	ft						
15. Channel sloր	pe, s		ft/ft						
16. Manning's ro	oughness coeff., n 1.49 r ^{2/3} s ^{1/2}								
17.	n	Compute V	ft/s						

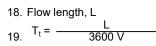
18. Flow length, L $T_t = \frac{}{360}$

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, p_w
- 14. Hydraulic radius, r
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

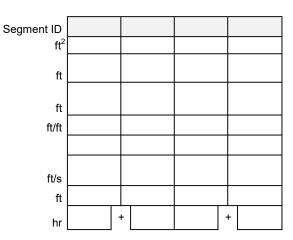
	\/ =	$1.49 r^{2/3} s^{1/2}$	
17.	v –	n	

Compute V

Compute r



Compute T_t



0.00

0.09

hr

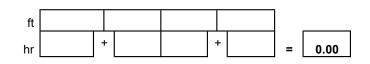
20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

Proje	ect JSMC East Addition	1		Ву	SPT	Date				
Loca	ation Township of Neptur	ne, Monmou	th County, NJ	Checked	MI	Date				
Circle	le One: Present Developed				_					
Circle	rcle One: T_c T_t through subarea			Exis	sting DA-2	, Pervious	, Future St	orm		
NOT	ES: Space for as many as two segme worksheet.	ents per flow	type can be use	ed for each					•	
	Include a map, schematic, or de	escription of f	low segments.							
Shee	et flow (Applicable to T _c Only)	·	· ·	Segment ID	1]	
	(pp			ooginoni ib	Short					
1.	Surface description (table 3-1)			-	Grass					
2.	Manning's roughness coeff., n (Table	15-1)		_	0.15					
3.	Flow Length, L			ft	20					
4.	Two-yr 24-hr rainfall, P ₂			in	4.14					
5.	Land slope, s			ft/ft	0.01					
6	$T_{r} = 0.007(nL)^{0.8}$		Compute T _t	hr	0.05	+		+	=	0.05
0.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{2}^{0.5}s^{0.4}}$			-						
Shal	llow concentrated flow			Segment ID	2]	
	Surface description (paved or unpave	ad)		oog.non.ib	Paved					
	Flow length, L	su)		ft	274					
	•			ft/ft						
	Watercourse slope, s			ļ.	0.013					
	Average velocity, V (Figure 15-4)		O	ft/s	2.40	+	1.	 +	 	
11.	$T_t = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.03	' L		' L	=	0.03
				_					-	
<u>Cha</u>	nnel flow			Segment ID						
12.	Cross sectional flow area, a			ft ²						
13.	Wetted perimeter, p _w			ft						
14.	Hydraulic radius, r	$r = \frac{a}{P_W}$	Compute r	ft						
	Channel slope, s		·	ft/ft						
16.	Manning's roughness coeff., n			•						
	$V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$			-						
17.	n		Compute V	ft/s						
18.	Flow length, L			ft		1,				
19.	$T_t = \frac{L}{3600 \text{ V}}$		Compute T _t	hr		+		+	_	0.00
10.	0000 1			''' L						0.00
				Segment ID]	
12.	Cross sectional flow area, a			ft ²						
				-						
13.	Wetted perimeter, p _w			ft		-				
1.4	Hydraulic radius, r	$r = \frac{a}{p_w}$	Compute r	ft						
	Channel slope, s	I- W	Computer	ft/ft						
	•			וטונ						
10.	Manning's roughness coeff., n $V = 1.49 \text{ r}^{2/3} \text{ s}^{1/2}$			-						
17.	$V = \frac{1.49 r^{2.3} s^{1/2}}{n}$		Compute V	ft/s						

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Drainet	ICMC Fa	a			Dv	CDT	Dete				
Project		st Addition	ammanth Canat	N.I	_By Checked	SPT	Date		-		
Location Circle One:	Present	Developed Developed	onmouth Count	y, NJ	_ Criecked	MI	Date		-		
Circle One:	T _c		through subarea		Pronosec	I DΔ-1 Imi	pervious, (Current St	orm		
		•	<u>.</u>			2 DA-1, IIII	oci vious, c	Janeni Ot			
NOTES: Space for worksh	-	as two segments	per now type car	n be used for ea	acn						
Include	e a map, scł	nematic, or desc	ription of flow seg	ments.							
Sheet flow (Appl	licable to T _c	Only)			Segment ID	1					
					_						
Surface desc	• `	•				Asphalt					
2. Manning's ro	=	eff., n (Table 15-	1)			0.011					
3. Flow Length,					ft .	100					
4. Two-yr 24-hr					in	3.48					
5. Land slope, s					ft/ft	0.014	+		+		
6. $T_t = \frac{0.0}{1}$	007(nL) ^{0.8}	_		Compute T _t	hr	0.022	r			=	0.022
ſ	r ₂ s				ı						
Shallow concen					Segment ID	2					
7. Surface desc	cription (pav	ed or unpaved)				Paved					
8. Flow length,					ft	243					
9. Watercourse	•				ft/ft	0.010					
10. Average velo		•			ft/s	1.90	. 1				
11. T _t =	2600 V	_		Compute T _t	hr	0.036	+	-	+	=	0.036
•	3000 V										
Channel flow					Segment ID	3	4	5	6		
12. Cross section	nal flow area	а, а			ft ²	1.22718	1.76715	4.90874	7.068583		
13. Wetted perin	neter, p _w				ft	3.92699	4.71239	7.85398	9.424778		
14. Hydraulic rad	dius, r		$r = \frac{a}{P_w}$	Compute r	ft	0.31	0.38	0.63	0.75		
15. Channel slop	oe, s				ft/ft	0.012	0.015	0.016	0.005		
16. Manning's ro	-					0.013	0.013	0.013	0.013		
V =		³ s ^{1/2}		Community V	£11-	F C0	7.25	40.00	6.00		
17.	n			Compute V	ft/s	5.68	7.35	10.60	6.69		

18. Flow length, L

81

0.004

376

0.014

31

0.001

12

0.000

			Segment ID	7		
12. Cross sectional flow area, a			ft ²	9.62113		
13. Wetted perimeter, p _w			ft	10.9956		
	r =a					
14. Hydraulic radius, r	p_w	Compute r	ft	0.88		
15. Channel slope, s			ft/ft	0.003		
16. Manning's roughness coeff., n				0.013		
$V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$	_					
17. v –	_	Compute V	ft/s	5.74		
18. Flow length, L			ft	612		

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Pro	ject JSMC East Additio	n		Ву	SPT	Date				
Loc	ation Township of Neptu	ne, Monmout	h County, NJ	Checked	MI	Date				
Circ	cle One: Present Develope									
Circ	cle One: T _c T _t	through subar	ea	Propo	osed DA-1,	Imperviou	s, Future	Storm		
NO	TES: Space for as many as two segm worksheet.	ents per flow ty	pe can be use	ed for each						
	Include a map, schematic, or c	lescription of flo	ow segments.							
She	eet flow (Applicable to T _c Only)			Segment ID	1					
1.	Surface description (table 3-1)				Asphalt					
2.	Manning's roughness coeff., n (Table	e 15-1)			0.011					
3.	Flow Length, L			ft	100					
4.	Two-yr 24-hr rainfall, P ₂			in	4.14					
5.	Land slope, s			ft/ft	0.014					
6.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{c}^{0.5}s^{0.4}}$		Compute T _t	hr	0.020	+	-	+	=	0.020
	allow concentrated flow			Segment ID	2					
7.	Surface description (paved or unpav	ed)		-	Paved					
8.	Flow length, L			ft	243					
9.	Watercourse slope, s			ft/ft	0.010					
10.	Average velocity, V (Figure 15-4)			ft/s	1.9					
11	$T_{t} = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.036	+	-	+	=	0.036
	3600 V									
Cha	annel flow			Segment ID	3	4	5	6		
12.	Cross sectional flow area, a			ft ²	1.227185	1.76715	4.90874	7.06858		
13.	Wetted perimeter, p _w	0		ft	3.926991	4.71239	7.85398	9.42478		
14.	Hydraulic radius, r	$r = \frac{a}{P_W}$	Compute r	ft	0.31	0.38	0.63	0.75		
15.	Channel slope, s			ft/ft	0.012	0.015	0.016	0.005		
16.	Manning's roughness coeff., n				0.013	0.013	0.013	0.013		
17.	$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$		Compute V	ft/s	5.68	7.35	10.60	6.69		
	Flow length, L		Compute v	ft	81	376	31	12		
19.	_ I		Compute T _t	hr		0.014		0.000	=	0.019
				Segment ID	7					
12.	Cross sectional flow area, a			ft ²	9.621128					
13.	Wetted perimeter, p _w			ft	10.99557					
11	Hydroulio radius r	$r = \frac{a}{p_w}$	Computer	Er	0.00					
	Hydraulic radius, r	PW	Compute r	ft ft/ft	0.88					
	Channel slope, s			ft/ft	0.003					
10.	Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$				0.013					
17.	V =n		Compute V	ft/s	5.74					

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

ft	612					
hr	0.030	+		+	=	0.030

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Project	JSMC Eas	st Addition			_By	SPT	Date	1/6/2025	_		
Location	Township	of Neptune, M	Ionmouth Co	unty, NJ	Checked	MI	Date		_		
Circle One:	Present	Developed									
Circle One:	T _c	T_{t}	through subar	rea	Propose	ed DA-1, P	ervious, C	urrent Sto	rm		
NOTES: Space for worksh	•	is two segments	s per flow type	can be used for ea	ach						
Include	e a map, sch	nematic, or desc	cription of flow	segments.							
Sheet flow (Appl	licable to T _c	Only)			Segment ID	1					
Surface description	cription (table	∍ 3-1)				Short Grass					
2. Manning's ro	ughness coe	eff., n (Table 15	-1)			0.240					
3. Flow Length,	L				ft	37					
4. Two-yr 24-hr	rainfall, P ₂				in	3.48					
5. Land slope,	S				ft/ft	0.015				ı	
6. $T_t = \frac{0.0}{I}$	$\frac{0.07(nL)^{0.8}}{2^{0.5}s^{0.4}}$	_		Compute T _t	hr	0.116	+		+	= [0.116
Shallow concen	trated flow				Segment ID	2					
7. Surface desc	cription (pave	ed or unpaved)				Paved					
8. Flow length,	L				ft	109					
9. Watercourse	slope, s				ft/ft	0.003					
10. Average velo	city, V (Figu	re 15-4)			ft/s	1.11	<u> </u>			ı	
11. T _t =	L 3600 V	_		Compute T _t	hr	0.027	+		+	= [0.027
Channel flow					Segment ID	3	4	5	6		
12. Cross section	nal flow area	ı, a			ft ²	1.22718	1.76715	4.90874	7.068583		
13. Wetted perin	neter, p _w		-		ft	3.92699	4.71239	7.85398	9.424778		
14. Hydraulic rac	dius, r		$r = \frac{a}{p_w}$	Compute r	ft	0.31	0.38	0.63	0.75		
15. Channel slop	oe, s				ft/ft	0.012	0.015	0.016	0.005		
16. Manning's ro	ughness coe	∍ff., n				0.013	0.013	0.013	0.013		

Compute V

Compute T_t

18. Flow length, L $T_t = \frac{}{36}$

5.68

81

0.004

ft/s

7.35

376

0.014

10.60

31

0.001

6.69

12

0.000

				Segment ID	7	8	
12. Cross sectional flow area, a				ft ²	9.62113		
13. Wetted perimeter, p _w				ft	10.9956		
14. Hydraulic radius, r	r =	<u>а</u> р _w	Compute r	ft	0.88		
15. Channel slope, s				ft/ft	0.003		
16. Manning's roughness coeff., n					0.013		
$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$	_		Compute V	ft/s	5.74		
18. Flow length, L				ft	612		
_{+ -} L						. T	.

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Compute T_t

Location Township of Neptune, Monmouth County, NJ Checked MI Date Circle One: Present Developed Circle One: T _c T _t through subarea Proposed DA-1, Pervious, Future Storm NOTES: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schematic, or description of flow segments. Sheet flow (Applicable to T _c Only) Segment ID Short Grass 1. Surface description (table 3-1) 2. Manning's roughness coeff., n (Table 15-1) 3. Flow Length, L 4. Two-yr 24-hr rainfall, P ₂ 5. Land slope, s 6. T _t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}} Compute T _t Shallow concentrated flow 7. Surface description (paved or unpaved) Segment ID Proposed DA-1, Pervious, Future Storm 1. Surface description (paved or unpaved)	
Circle One: T_c T_t through subarea Proposed DA-1, Pervious, Future Storm NOTES: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schematic, or description of flow segments. Sheet flow (Applicable to T_c Only) Segment ID Short Grass Manning's roughness coeff., T_c (Table 15-1) Flow Length, L Two-yr 24-hr rainfall, T_c in T_c in T_c (Table 15-1) Land slope, T_c (Compute T_c hr T_c (D.106) Segment ID Shallow concentrated flow Segment ID	
NOTES: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schematic, or description of flow segments. Sheet flow (Applicable to T_c Only) Segment ID Short Grass Manning's roughness coeff., n (Table 15-1) Flow Length, L Two-yr 24-hr rainfall, P_2 Land slope, s $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t Segment ID	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	
1. Surface description (table 3-1) 2. Manning's roughness coeff., n (Table 15-1) 3. Flow Length, L 4. Two-yr 24-hr rainfall, P_2 5. Land slope, s 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t Compute T_t Shallow concentrated flow Short Grass 9.240 4.14 9.015 1. Short Grass 9.240 9	
2. Manning's roughness coeff., n (Table 15-1) 3. Flow Length, L 4. Two-yr 24-hr rainfall, P_2 5. Land slope, s 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t Compute T_t Compute T_t Segment ID 2. Manning's roughness coeff., n (Table 15-1) 0.240 4. 14 5. Land slope, s 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Segment ID 2	
3. Flow Length, L 4. Two-yr 24-hr rainfall, P ₂ 5. Land slope, s 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t Compute T_t Compute T_t Compute T_t Segment ID 2	
4. Two-yr 24-hr rainfall, P_2 in 4.14 5. Land slope, s ft/ft 0.015 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t hr 0.106 + = = Shallow concentrated flow	
5. Land slope, s 6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t Shallow concentrated flow Segment ID 2	
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$ Compute T_t hr 0.106 +	
6. $T_t = \frac{0.007 \text{(ILL)}}{P_2^{0.5} \text{s}^{0.4}}$ Shallow concentrated flow Segment ID 2	
	0.106
8. Flow length, L	
9. Watercourse slope, s ft/ft 0.003	
10. Average velocity, V (Figure 15-4) ft/s 1.11	
	0.027
11. T _t = L Compute T _t hr 0.027 + + =	
<u>Channel flow</u> Segment ID 3 4 5 6	
12. Cross sectional flow area, a ft ² 1.227185 1.76715 4.90874 7.06858	
13. Wetted perimeter, p _w ft 3.926991 4.71239 7.85398 9.42478	
14. Hydraulic radius, r $r = \frac{a}{Pw}$ Compute r ft 0.31 0.38 0.63 0.75	
15. Channel slope, s ft/ft 0.012 0.015 0.016 0.005	
16. Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$	
$V = \frac{1.49 r^{2.5} s^{3.2}}{n}$ 17. Compute V ft/s 5.68 7.35 10.60 6.69	
18. Flow length, L ft 81 376 31 12	
19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t hr 0.004 + 0.001 + 0.000 =	0.019
Segment ID 7 8	
12. Cross sectional flow area, a	
13. Wetted perimeter, p _w ft 10.99557	
$r = \frac{a}{p_w}$ 14. Hydraulic radius, r ft 0.88	
15. Channel slope, s ft/ft 0.003	
16. Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$	

17. n

Compute V

18. Flow length, L

 $T_{t} = \frac{L}{3600 \text{ V}}$

Compute T_t

20. Watershed or subarea T_c or $T_t \, (\text{add } T_t \, \text{in steps 6, 11, 19})$

Project	JSUMC E	East Addition			Ву	SPT	Date				
Location			Monmouth Count	y, NJ	Checked	MI	Date				
Circle One:	Present	(Developed)							·	_	
Circle One:	T _c	T_t	through subarea		Propose	d DA-2, Imp	ervious, C	urrent Sto	rm		
NOTES: Space f		as two segmen	ts per flow type ca	n be used for e	ach						
Include	e a map, so	hematic, or des	cription of flow seg	ments.							
Sheet flow (App	licable to T	Only)			Segment ID	1]	
Surface description	cription (tab	le 3-1)				Asphalt					
Manning's ro		•	5-1)			0.011					
Flow Length,	•	7011., 11 (Table 1	5 1)		ft	100					
4. Two-yr 24-hr					in	3.48					
5. Land slope,					ft/ft	0.008					
6. $T_t = \frac{0.0}{100}$		<u> </u>		Compute T _t	hr	0.028	-		+	= [0.028
ŀ	P ₂ s s ···									1	
Shallow concen	trated flow	<u></u>			Segment ID	2					
7. Surface desc	cription (pav	ed or unpaved)			Paved					
8. Flow length,	L				ft	347					
9. Watercourse	e slope, s				ft/ft	0.011					
10. Average velo	ocity, V (Fig	ure 15-4)			ft/s	2.13				ļ,	
11. T _t =	L 3600 V	_		Compute T _t	hr	0.045	- [+	=	0.045
Channel flow					Segment ID]	
12. Cross section	nal flow are	a.a			ft ²						
13. Wetted perin		,			ft						
14. Hydraulic rad			$r = \frac{a}{P_W}$	Compute r	ft						
15. Channel slop					ft/ft						
16. Manning's ro		eff., n									
V =	•		_								
17.	n			Compute V	ft/s						
18. Flow length,	L				ft					Ι.	

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, pw
- 14. Hydraulic radius, r
- 14. Tryuraulic raulus,
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

$$V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$$

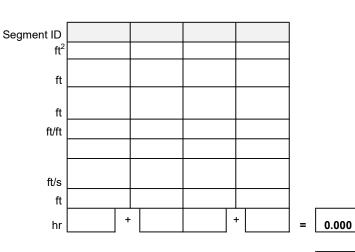
Compute V

Compute r

18. Flow length, L

19.
$$T_t = \frac{L}{3600 \text{ V}}$$

Compute T_t



hr **0.07**

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

Project JSUMC East	Addition	Ву	SPT	Date			
Location Township of	Neptune, Monmouth County, NJ	Checked	MI	Date			
Circle One: Present De	veloped						
Circle One: T _c T _t	through subarea	Prope	osed DA-2,	Imperviou	s, Future Sto	rm	
NOTES: Space for as many as two worksheet.	o segments per flow type can be use	d for each					
Include a map, schemat	tic, or description of flow segments.						
Sheet flow (Applicable to T _c Only)	\$	Segment ID	1				
1. Surface description (table 3-1))		Asphalt				
2. Manning's roughness coeff., n	ı (Table 15-1)		0.011				
3. Flow Length, L		ft	100				
4. Two-yr 24-hr rainfall, P ₂		in	4.14				
5. Land slope, s		ft/ft	0.008	<u> </u>			
6. $T_t = \frac{0.007(nL)^{0.8}}{P_0^{0.5}s^{0.4}}$	Compute T _t	hr	0.026	+	+	=	0.026
Shallow concentrated flow	,	Segment ID	2				
7. Surface description (paved or	unpaved)		Paved				
8. Flow length, L		ft	347				
9. Watercourse slope, s		ft/ft	0.011				
10. Average velocity, V (Figure 15	5-4)	ft/s	2.13				
11. $T_t = \frac{L}{3600 \text{ V}}$	Compute T _t	hr	0.045	+	+	=	0.045
Channel flow	\$	Segment ID					
12. Cross sectional flow area, a		ft ²					
13. Wetted perimeter, p _w		ft					
14. Hydraulic radius, r	r = <u>a</u> Compute r	ft					
15. Channel slope, s		ft/ft					
16. Manning's roughness coeff., n $V = \frac{1.49 \text{ r}^{2/3} \text{ s}^{1/2}}{1.49 \text{ r}^{2/3} \text{ s}^{1/2}}$	l						
17. V =n	Compute V	ft/s					
18. Flow length, L		ft		<u> </u>			
19. $T_t = \frac{L}{3600 \text{ V}}$	Compute T_t	hr		+	+	=	0.000
		1		1			
12. Cross sectional flow area, a	(Segment ID ft ²					
12. Closs sectional now alea, a		10					
13. Wetted perimeter, p _w	a	ft					
14. Hydraulic radius, r	$r = \frac{1}{p_w}$ Compute r	ft					
15. Channel slope, s		ft/ft					
16. Manning's roughness coeff., n $V = 1.49 r^{2/3} s^{1/2}$	ı						

17 n

Compute V

ft/s ft

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

hr

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

hr **0.07**

Project	JSUMC I	East Addition			_By	SPT	Date		_		
Location	Townshi	p of Neptune, I	Monmouth County	y, NJ	Checked	MI	Date		_		
Circle One:	Present	Developed								_	
Circle One: (T _c	T_{t}	through subarea		Propose	d DA-2, P	ervious, C	urrent Sto	rm	_	
	for as many	as two segment	ts per flow type car	n be used for ea	ach						
Inclu	ide a map, so	hematic, or des	cription of flow seg	ments.							
Sheet flow (Ap	•		,		Segment ID	1				ĺ	
<u> </u>	, p.1.00.010 10 1.	; • , ,			Cogmon ID	Short					
1. Surface des	scription (tab	le 3-1)				Grass					
2. Manning's i	roughness co	oeff., n (Table 15	5-1)			0.150					
3. Flow Length	h, L				ft	21					
4. Two-yr 24-ł	hr rainfall, P ₂				in	3.48					
5. Land slope	e, S				ft/ft	0.025	<u> </u>			l .	
6. T _t =0	0.007(nL) ^{0.8}	_		Compute T _t	hr	0.041	+			=	0.041
	P ₂ s				ı					1	
Shallow conce	entrated flow	<u> </u>			Segment ID	2					
7. Surface des	scription (pav	ed or unpaved)				Paved					
8. Flow length	n, L				ft	434					
9. Watercours	se slope, s				ft/ft	0.011					
10. Average ve	elocity, V (Fig	ure 15-4)			ft/s	2.13	<u> </u>	ļ.,		! ,	
11. T _t =	L 3600 V	_		Compute T _t	hr	0.057	+			=	0.057
Channel flow					Segment ID						
12. Cross secti	ional flow are	a, a			ft ²						
13. Wetted per	imeter, p _w				ft						
14. Hydraulic ra	adius, r		$r = \frac{a}{-p_w}$	Compute r	ft						
15. Channel slo	ope, s				ft/ft						
16. Manning's i	roughness co	eff., n									
	1.49 r ²		_	Compute V	ft/s						
18. Flow length				- 2pa.e v	ft						
•	L 3600 V	<u></u>			"		+		+		
19. It -	3600 V			Compute T _t	hr		·] = [0.000

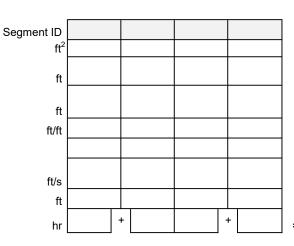
- 12. Cross sectional flow area, a
- 13. Wetted perimeter, p_w
- 14. Hydraulic radius, r
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

	V =	1.49 r ^{2/3} s ^{1/2}
17.	v –	n

18. Flow length, L
19.
$$T_t = \frac{L}{3600 \text{ V}}$$

Compute V

Compute r



0.000

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

0.10

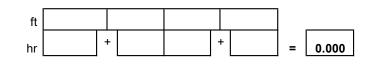
hr

Project JSUMC	East Addition		Ву	SPT	Date			
Location Townsh	ip of Neptune, Monmouth C	ounty, NJ	Checked	MI	Date			
Circle One: Present	Developed						-	
Circle One: T _c	T _t through subarea		Prop	osed DA-2	, Pervious	s, Future S	torm	
NOTES: Space for as many a	as two segments per flow type	can be use	d for each					
worksheet.								
Include a map, sch	ematic, or description of flow	segments.						
Sheet flow (Applicable to T_c	Only)		Segment ID	1				
1 Curtage description (table	2.1			Short				
 Surface description (table Manning's roughness coe 	•			Grass 0.15				
2. Manning's roughness coe3. Flow Length, L	311., 11 (Table 13-1)		ft	21				
4. Two-yr 24-hr rainfall, P ₂			in	4.14				
5. Land slope, s			ft/ft	0.025				
•	C	ompute T _t	. 1		+		+	= 0.038
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$	_	ompate It	hr [0.038				= 0.038
			ا در د					1
Shallow concentrated flow			Segment ID	2				
7. Surface description (pave	ed or unpaved)			Paved				<u> </u>
8. Flow length, L			ft	434				}
9. Watercourse slope, s	45 A)		ft/ft	0.011				<u> </u>
10. Average velocity, V (Figu		omputo T	ft/s	2.13	<u> </u> +		+	
11. $T_t = \frac{L}{3600 \text{ V}}$	_	ompute T _t	hr	0.057	·		· L	= 0.057
			г		ı		1	1
Channel flow			Segment ID					ļ
12. Cross sectional flow area	ı, a		ft ²					
13. Wetted perimeter, p _w	а		ft					
14. Hydraulic radius, r	$r = \frac{a}{P_W}$ C	compute r	ft					
15. Channel slope, s			ft/ft					
16. Manning's roughness coe								
$V = \frac{1.49 r^{2/3} s}{n}$		ompute V	ft/s					
18. Flow length, L	3.	ompato v	ft					
+ L	_				+		+	
19. T _t = 3600 V	Co	ompute T _t	hr					= 0.000
			Г					1
40. Oraca	_		Segment ID ft ²					
12. Cross sectional flow area	ı, a		π-					
13. Wetted perimeter, p _w			ft]
44 11 1 12 12 12	$r = \frac{a}{p_w}$ C		ا					
14. Hydraulic radius, r	Pw C	Compute r	ft					
15. Channel slope, s	"		ft/ft					
16. Manning's roughness coo $V = \frac{1.49 r^{2/3} s}{1.49 r^{2/3} s}$			-					1
$V = \frac{1.49 r^{2/3} s}{n}$		ompute V	ft/s					

18. Flow length, L

19.
$$T_t = \frac{L}{3600 \text{ V}}$$

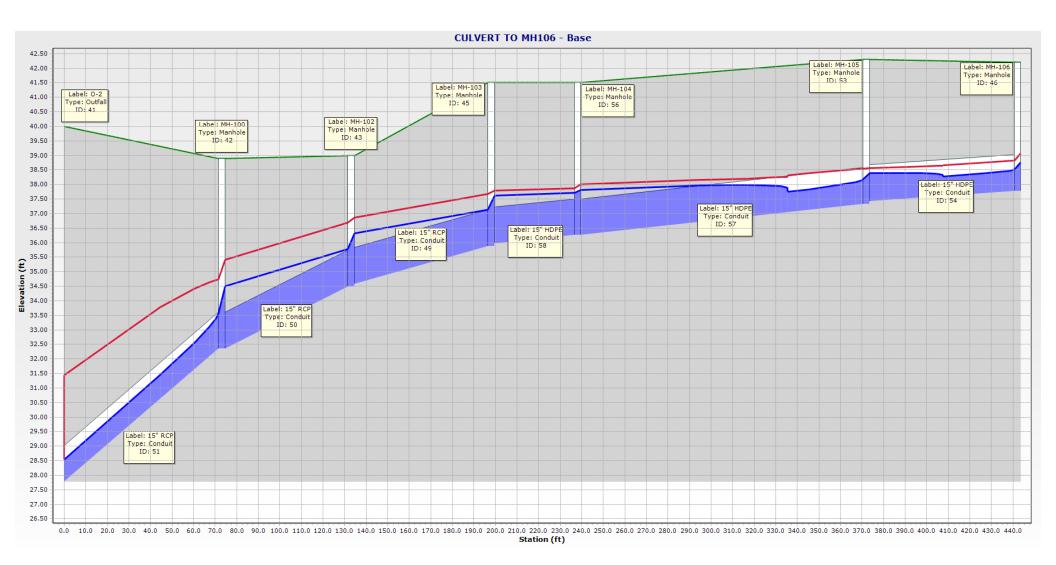
Compute T_t

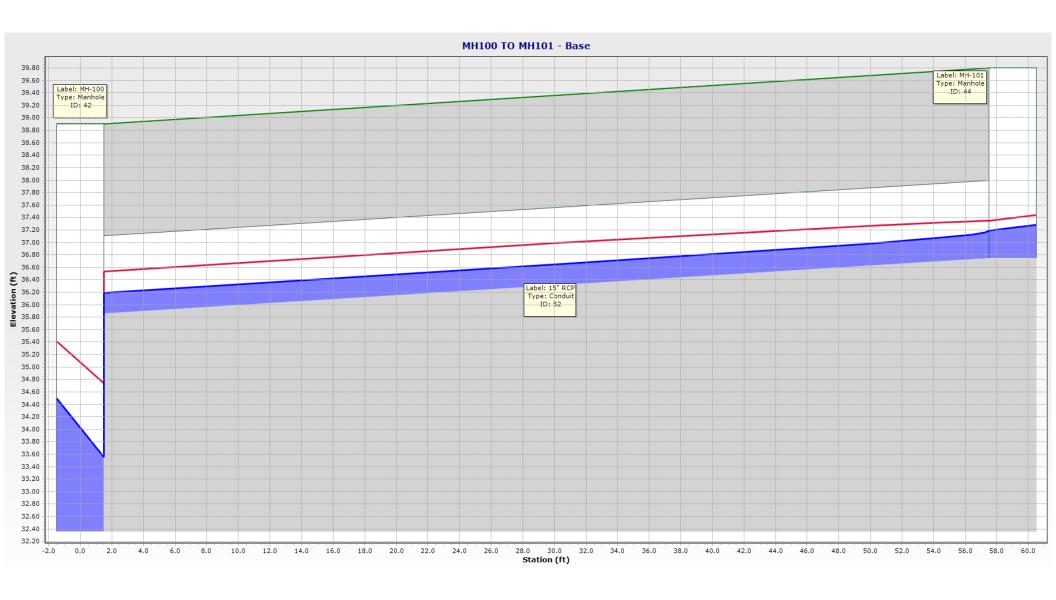


20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

FlexTable: Conduit Table

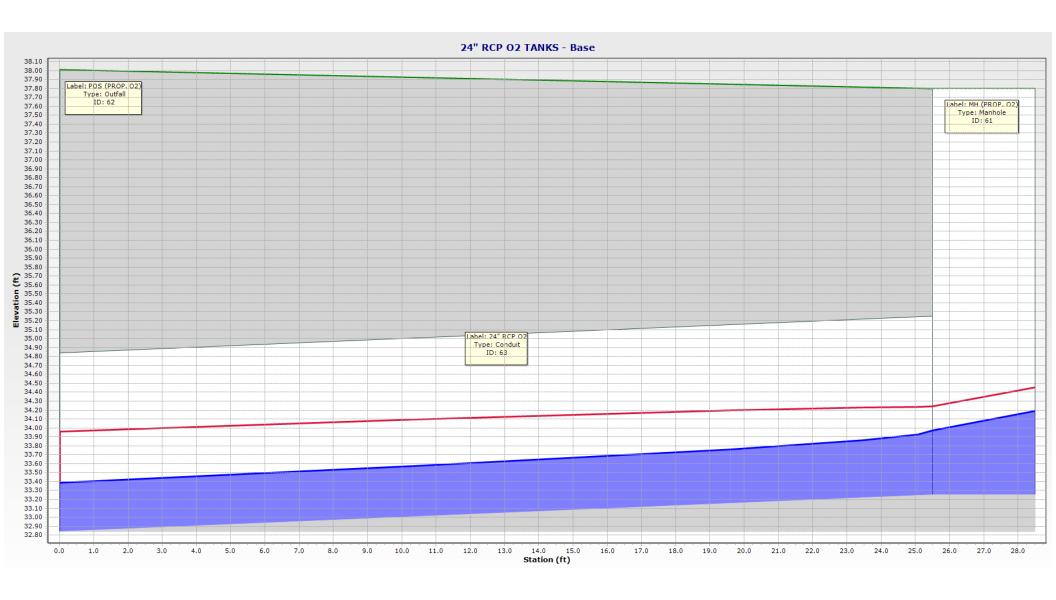
Label	Start Node	Stop Node	Invert (Start) (ft)	Invert (Stop) (ft)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Diameter (in)	Flow (cfs)	Capacity (Full Flow) (cfs)
15" RCP	MH-103	MH-102	35.88	34.58	65.0	0.020	15.0	7.21	9.14
15" RCP	MH-102	MH-100	34.48	32.36	60.0	0.035	15.0	9.41	12.14
15" RCP	MH-100	0-2	32.36	27.78	73.0	0.063	15.0	10.53	16.18
15" RCP	MH-101	MH-100	36.75	35.86	59.0	0.015	15.0	1.22	7.93
15" HDPE	MH-106	MH-105	37.78	37.43	70.0	0.005	15.0	3.31	5.94
15" HDPE	MH-105	MH-104	37.33	36.26	134.0	0.008	15.0	4.32	7.50
15" HDPE	MH-104	MH-103	36.26	35.98	40.0	0.007	15.0	4.06	7.03

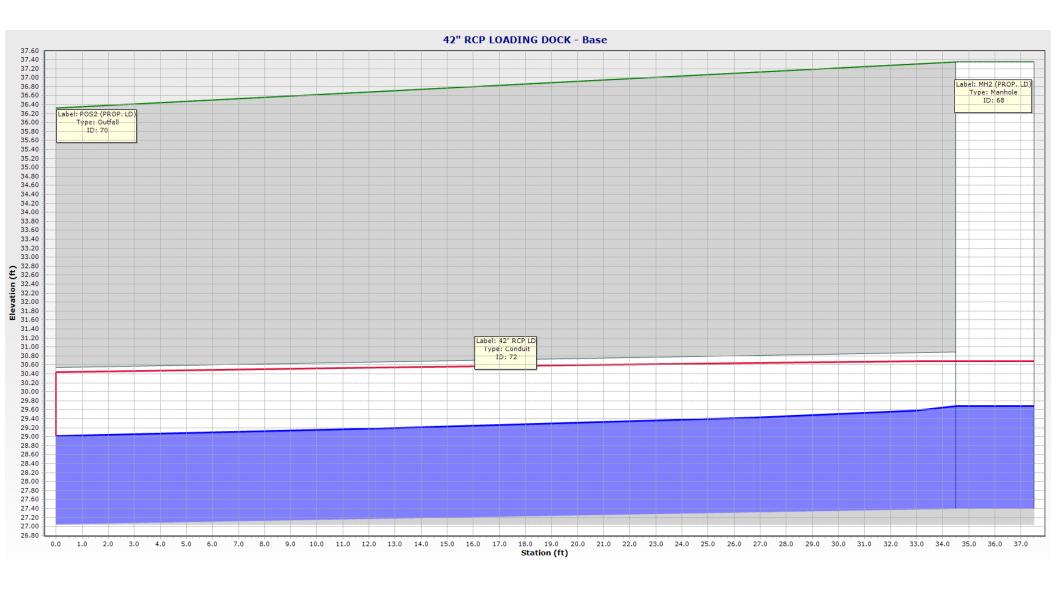




FlexTable: Conduit Table

Label	Start Node	Stop Node	Invert (Start) (ft)	Invert (Stop) (ft)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Diameter (in)	Flow (cfs)	Capacity (Full Flow) (cfs)
24" RCP O2	MH (EXIST. O2)	POS (EXIST. O2)	33.25	32.84	27.0	0.015	24.0	3.72	27.88
24" RCP O2	MH (PROP. O2)	POS (PROP. O2)	33.25	32.84	27.0	0.015	24.0	4.25	27.88
42" RCP LD	MH2 (EXIST. LD)	POS2 (EXIST. LD)	27.39	27.04	36.0	0.010	42.0	53.25	99.20
42" RCP LD	MH2 (PROP. LD)	POS2 (PROP. LD)	27.39	27.04	36.0	0.010	42.0	53.66	99.20





APPENDIX III:

West Addition Pondpack: Master Summary Report
West Addition PondPack: Rainfall Report
West Addition PondPack: Unit Hydrograph
West Addition PondPack: Routing Diagrams
West Addition Pondpack: Graphical & Numerical Hydrograph Comparisons
West Addition Time of Concentration Calculations
West Addition SewerGEMS: FlexTables
West Addition SewerGEMS: Pipe Profile

JSUMC West Addition: PondPack Report

Project Summary	
Title	JSUMC West Addition
Engineer	
Company	Dewberry Engineers Inc
Date	12/3/2024

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Current Storm		
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Subsection: User Notifications

Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	33
Label	EDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.7% is greater than 1.5% . Computed peak flow= $8.05 \text{ ft}^3/\text{s}$ Interp. peak flow= $4.77 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	32
Label	EDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 39.2 % is greater
	than 1.5 %. Computed peak flow= 0.69 ft ³ /s Interp. peak flow= 0.42 ft ³ /s. Output increment
	for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	35
Label	EDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 3.34 ft ³ /s Interp. peak flow= 2.00 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	34
Label	EDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 22.0 % is greater than 1.5 %. Computed peak flow= 0.54 ft³/s Interp. peak flow= 0.42 ft³/s. Output increment for this catchment may be too large.
Source	Warning

Subsection: User Notifications

Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	37
Label	PDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.7% is greater than 1.5% . Computed peak flow= $7.12 \text{ ft}^3/\text{s}$ Interp. peak flow= $4.22 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	36
Label	PDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 38.3 % is greater than 1.5 %. Computed peak flow= 0.85 ft³/s Interp. peak flow= 0.53 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	39
Label	PDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 3.31 ft³/s Interp. peak flow= 1.97 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 2 year
Element Type	Catchment
Element Id	38
Label	PDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 25.6 % is greater
. 1333293	than 1.5 %. Computed peak flow= 0.56 ft ³ /s Interp. peak flow= 0.41 ft ³ /s. Output increment for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	33
Label	EDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.6 % is greater than 1.5 %. Computed peak flow= 12.66 ft³/s Interp. peak flow= 7.52 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	32
Label	EDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 44.1 % is greater than 1.5 %. Computed peak flow= $1.96 \text{ft}^3/\text{s}$ Interp. peak flow= $1.10 \text{ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	35
Label	EDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 5.26 ft ³ /s Interp. peak flow= 3.14 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	34
Label	EDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 25.1 % is greater than 1.5 %. Computed peak flow= 1.37 ft ³ /s Interp. peak flow= 1.02 ft ³ /s. Output increment for this catchment may be too large.
	for this editionent may be too larger

Subsection: User Notifications

Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	37
Label	PDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.6 % is greater than 1.5 %. Computed peak flow= 11.20 ft ³ /s Interp. peak flow= 6.65 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	36
Label	PDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 43.8 % is greater than 1.5 %. Computed peak flow= 2.53 ft³/s Interp. peak flow= 1.42 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	39
Label	PDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 5.20 ft³/s Interp. peak flow= 3.11 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 10 year
Element Type	Catchment
Element Id	38
Label	PDA2 - Per - Cur
Time	(N/A)
TITLE	
Message	The difference between calculated peak flow and interpolated peak flow 29.0 % is greater than 1.5 %. Computed peak flow= 1.37 ft ³ /s Interp. peak flow= 0.97 ft ³ /s. Output increment for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	33
Label	EDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.5 % is greater than 1.5 %. Computed peak flow= 21.93 ft³/s Interp. peak flow= 13.04 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	32
Label	EDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 46.7 % is greater than 1.5 %. Computed peak flow= 5.05 ft ³ /s Interp. peak flow= 2.69 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	35
Label	EDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 9.11 ft ³ /s Interp. peak flow= 5.45 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	34
Label	EDA2 - Per - Cur
Time	(N/A)
	The difference between calculated peak flow and interpolated peak flow 26.9 % is greater
Message	than 1.5 %. Computed peak flow= 3.27 ft ³ /s Interp. peak flow= 2.39 ft ³ /s. Output increment for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	37
Label	PDA1 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.5 % is greater than 1.5 %. Computed peak flow= 19.40 ft³/s Interp. peak flow= 11.54 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	36
Label	PDA1 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 46.5 % is greater than 1.5 %. Computed peak flow= 6.65 ft³/s Interp. peak flow= 3.56 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	39
Label	PDA2 - Imp - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater than 1.5 %. Computed peak flow= 9.01 ft ³ /s Interp. peak flow= 5.40 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Current 100 year
Element Type	Catchment
Element Id	38
Label	PDA2 - Per - Cur
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 31.2 % is greater than 1.5 %. Computed peak flow= 3.25 ft ³ /s Interp. peak flow= 2.23 ft ³ /s. Output increment
	for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	43
Label	EDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.7 % is greater than 1.5 %. Computed peak flow= 9.60 ft ³ /s Interp. peak flow= 5.70 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	41
Label	EDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 41.7 % is greater than 1.5 %. Computed peak flow= 1.08 ft ³ /s Interp. peak flow= 0.63 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	47
Label	EDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 3.99 ft ³ /s Interp. peak flow= 2.38 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	45
Label	EDA2 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 26.9 % is greater than 1.5 %. Computed peak flow= 0.82 ft ³ /s Interp. peak flow= 0.60 ft ³ /s. Output increment
	for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	42
Label	PDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.7 % is greater than 1.5 %. Computed peak flow= $8.49 \text{ft}^3/\text{s}$ Interp. peak flow= $5.04 \text{ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	40
Label	PDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 44.8 % is greater
	than 1.5 %. Computed peak flow= 1.40 ft ³ /s Interp. peak flow= 0.77 ft ³ /s. Output increment
	for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	46
Label	PDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.3 % is greater than 1.5 %. Computed peak flow= 3.95 ft ³ /s Interp. peak flow= 2.36 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 2 year
Element Type	Catchment
Element Id	44
Label	PDA2 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 31.4 % is greater than 1.5 %. Computed peak flow= 0.83 ft³/s Interp. peak flow= 0.57 ft³/s. Output increment for this catchment may be too large.
Source	Warning

Subsection: User Notifications

Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	43
Label	EDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.6 % is greater than 1.5 %. Computed peak flow= 14.93 ft³/s Interp. peak flow= 8.87 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	41
Label	EDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 45.1% is greater than 1.5% . Computed peak flow= $2.68 \text{ ft}^3/\text{s}$ Interp. peak flow= $1.47 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	47
Label	EDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 6.20 ft ³ /s Interp. peak flow= 3.71 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	45
Label	EDA2 - Per - Fut
Time	(N/A)
Message	
Message	The difference between calculated peak flow and interpolated peak flow 29.6 % is greater than 1.5 %. Computed peak flow= 1.85 ft³/s Interp. peak flow= 1.31 ft³/s. Output increment for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	42
Label	PDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.6 % is greater than 1.5 %. Computed peak flow= 13.21 ft³/s Interp. peak flow= 7.85 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	40
Label	PDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 48.4% is greater than 1.5% . Computed peak flow= $3.56 \text{ ft}^3/\text{s}$ Interp. peak flow= $1.84 \text{ ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	46
Label	PDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.2 % is greater than 1.5 %. Computed peak flow= 6.14 ft ³ /s Interp. peak flow= 3.67 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 10 year
Element Type	Catchment
Element Id	44
Label	PDA2 - Per - Fut
Time	(N/A)
Message	
Picssage	The difference between calculated peak flow and interpolated peak flow 33.5 % is greater than 1.5 %. Computed peak flow= 1.84 ft³/s Interp. peak flow= 1.23 ft³/s. Output increment for this catchment may be too large.

Subsection: User Notifications

Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	43
Label	EDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.5 % is greater than 1.5 %. Computed peak flow= 27.13 ft³/s Interp. peak flow= 16.13 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	41
Label	EDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 47.3 % is greater than 1.5 %. Computed peak flow= 6.88 ft³/s Interp. peak flow= 3.63 ft³/s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	47
Label	EDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow $40.1~\%$ is greater than $1.5~\%$. Computed peak flow= $11.27~\mathrm{ft^3/s}$ Interp. peak flow= $6.75~\mathrm{ft^3/s}$. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	45
Label	EDA2 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 31.6 % is greater than 1.5 %. Computed peak flow= 4.50 ft ³ /s Interp. peak flow= 3.07 ft ³ /s. Output increment
	for this catchment may be too large.

Subsection: User Notifications

Scenario Element Type Element Id Label	Future 100 year Catchment
Element Id	
	42
Label	42
	PDA1 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.5 % is greater than 1.5 %. Computed peak flow= 24.00 ft ³ /s Interp. peak flow= 14.27 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	40
Label	PDA1 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 48.2 % is greater
	than 1.5 %. Computed peak flow= 9.32 ft ³ /s Interp. peak flow= 4.83 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	46
Label	PDA2 - Imp - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 40.1 % is greater
ricosage	than 1.5 %. Computed peak flow= 11.15 ft ³ /s Interp. peak flow= 6.68 ft ³ /s. Output increment for this catchment may be too large.
Source	Warning
Message Id	7
Scenario	Future 100 year
Element Type	Catchment
Element Id	44
Label	PDA2 - Per - Fut
Time	(N/A)
Message	The difference between calculated peak flow and interpolated peak flow 35.5 % is greater than 1.5 %. Computed peak flow= $4.41 \text{ft}^3/\text{s}$ Interp. peak flow= $2.84 \text{ft}^3/\text{s}$. Output increment for this catchment may be too large.
Source	Warning

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
EDA1 - Per - Cur	Current 2 year	2	0.056	12.25	0.42
EDA1 - Per - Cur	Current 10 year	10	0.145	12.25	1.10
EDA1 - Per - Cur	Current 100 year	100	0.367	12.25	2.69
EDA1 - Imp - Cur	Current 2 year	2	0.610	12.00	4.77
EDA1 - Imp - Cur	Current 10 year	10	0.979	12.00	7.52
EDA1 - Imp - Cur	Current 100 year	100	1.724	12.00	13.04
EDA2 - Per - Cur	Current 2 year	2	0.048	12.25	0.42
EDA2 - Per - Cur	Current 10 year	10	0.114	12.25	1.02
EDA2 - Per - Cur	Current 100 year	100	0.274	12.25	2.39
EDA2 - Imp - Cur	Current 2 year	2	0.243	12.00	2.00
EDA2 - Imp - Cur	Current 10 year	10	0.389	12.00	3.14
EDA2 - Imp - Cur	Current 100 year	100	0.686	12.00	5.45
PDA1 - Per - Cur	Current 2 year	2	0.071	12.25	0.53
PDA1 - Per - Cur	Current 10 year	10	0.187	12.25	1.42
PDA1 - Per - Cur	Current 100 year	100	0.483	12.25	3.56
PDA1 - Imp - Cur	Current 2 year	2	0.540	12.00	4.22
PDA1 - Imp - Cur	Current 10 year	10	0.866	12.00	6.65
PDA1 - Imp - Cur	Current 100 year	100	1.526	12.00	11.54
PDA2 - Per - Cur	Current 2 year	2	0.048	12.25	0.41
PDA2 - Per - Cur	Current 10 year	10	0.112	12.25	0.97
PDA2 - Per - Cur	Current 100 year	100	0.264	12.25	2.23
PDA2 - Imp - Cur	Current 2 year	2	0.240	12.00	1.97
PDA2 - Imp - Cur	Current 10 year	10	0.385	12.00	3.11
PDA2 - Imp - Cur	Current 100 year	100	0.678	12.00	5.40
PDA1 - Per - Fut	Future 2 year	2	0.106	12.25	0.77
PDA1 - Per - Fut	Future 10 year	10	0.254	12.25	1.84
PDA1 - Per - Fut	Future 100 year	100	0.666	12.00	4.83
EDA1 - Per - Fut	Future 2 year	2	0.083	12.25	0.63
EDA1 - Per - Fut	Future 10 year	10	0.195	12.25	1.47
EDA1 - Per - Fut	Future 100 year	100	0.505	12.25	3.63
PDA1 - Imp - Fut	Future 2 year	2	0.650	12.00	5.04
PDA1 - Imp - Fut	Future 10 year	10	1.027	12.00	7.85
PDA1 - Imp - Fut	Future 100 year	100	1.895	12.00	14.27
EDA1 - Imp - Fut	Future 2 year	2	0.734	12.00	5.70
EDA1 - Imp - Fut	Future 10 year	10	1.162	12.00	8.87
EDA1 - Imp - Fut	Future 100 year	100	2.143	12.00	16.13
PDA2 - Per - Fut	Future 2 year	2	0.067	12.25	0.57
PDA2 - Per - Fut	Future 10 year	10	0.146	12.25	1.23
PDA2 - Per - Fut	Future 100 year	100	0.355	12.25	2.84
EDA2 - Per - Fut	Future 2 year	2	0.068	12.25	0.60
EDA2 - Per - Fut	Future 10 year	10	0.150	12.25	1.31
EDA2 - Per - Fut	Future 100 year	100	0.369	12.25	3.07
PDA2 - Imp - Fut	Future 2 year	2	0.289	12.00	2.36

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
PDA2 - Imp - Fut	Future 10 year	10	0.457	12.00	3.67
PDA2 - Imp - Fut	Future 100 year	100	0.843	12.00	6.68
EDA2 - Imp - Fut	Future 2 year	2	0.292	12.00	2.38
EDA2 - Imp - Fut	Future 10 year	10	0.462	12.00	3.71
EDA2 - Imp - Fut	Future 100 year	100	0.852	12.00	6.75

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
Pre - POS1	Current 2 year	2	0.667	12.00	5.02
Pre - POS1	Future 2 year	2	0.818	12.00	6.14
Pre - POS1	Current 10 year	10	1.123	12.00	8.40
Pre - POS1	Future 10 year	10	1.357	12.00	10.13
Pre - POS1	Current 100 year	100	2.092	12.00	15.56
Pre - POS1	Future 100 year	100	2.647	12.00	19.66
Post - POS1	Current 2 year	2	0.611	12.00	4.51
Post - POS1	Future 2 year	2	0.756	12.00	5.61
Post - POS1	Current 10 year	10	1.053	12.00	7.77
Post - POS1	Future 10 year	10	1.281	12.00	9.53
Post - POS1	Current 100 year	100	2.009	12.00	14.84
Post - POS1	Future 100 year	100	2.562	12.00	19.10
Pre - POS2	Current 2 year	2	0.291	12.00	2.19
Pre - POS2	Future 2 year	2	0.360	12.00	2.72
Pre - POS2	Current 10 year	10	0.503	12.00	3.74
Pre - POS2	Future 10 year	10	0.612	12.00	4.56
Pre - POS2	Current 100 year	100	0.959	12.00	7.01
Pre - POS2	Future 100 year	100	1.221	12.00	8.97
Post - POS2	Current 2 year	2	0.288	12.00	2.19
Post - POS2	Future 2 year	2	0.356	12.00	2.71
Post - POS2	Current 10 year	10	0.497	12.00	3.72
Post - POS2	Future 10 year	10	0.603	12.00	4.54
Post - POS2	Current 100 year	100	0.943	12.00	6.97
Post - POS2	Future 100 year	100	1.198	12.00	8.90

Subsection: Time-Depth Curve Return Event: 10 years Label: Current Storm Storm Event: Current 10

Scenario: Current 10 year

Time-Depth Curve: Current 10	
Label	Current 10
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	10 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.01	0.02	0.02
0.50	0.03	0.04	0.04	0.05	0.06
1.00	0.06	0.07	0.08	0.08	0.09
1.50	0.10	0.10	0.11	0.12	0.12
2.00	0.13	0.14	0.14	0.15	0.16
2.50	0.17	0.17	0.18	0.19	0.20
3.00	0.20	0.21	0.22	0.23	0.24
3.50	0.24	0.25	0.26	0.27	0.28
4.00	0.29	0.29	0.30	0.31	0.32
4.50	0.33	0.34	0.35	0.35	0.36
5.00	0.37	0.38	0.39	0.40	0.41
5.50	0.42	0.43	0.44	0.45	0.45
6.00	0.46	0.47	0.48	0.49	0.50
6.50	0.52	0.53	0.54	0.55	0.56
7.00	0.57	0.59	0.60	0.61	0.62
7.50	0.64	0.65	0.66	0.68	0.69
8.00	0.71	0.72	0.74	0.75	0.77
8.50	0.78	0.80	0.81	0.83	0.85
9.00	0.86	0.88	0.90	0.92	0.94
9.50	0.96	0.98	1.00	1.03	1.05
10.00	1.08	1.10	1.13	1.16	1.19
10.50	1.22	1.25	1.29	1.33	1.37
11.00	1.41	1.47	1.52	1.58	1.65
11.50	1.72	1.82	1.93	2.07	2.27
12.00	2.61	3.17	3.37	3.51	3.62
12.50	3.72	3.79	3.86	3.92	3.97
13.00	4.03	4.07	4.11	4.15	4.19
13.50	4.22	4.25	4.28	4.31	4.34
14.00	4.36	4.39	4.41	4.44	4.46
14.50	4.48	4.50	4.52	4.54	4.56
15.00	4.58	4.59	4.61	4.63	4.64
15.50	4.66	4.67	4.69	4.70	4.72
16.00	4.73	4.75	4.76	4.78	4.79
16.50	4.80	4.82	4.83	4.84	4.85

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Subsection: Time-Depth Curve Return Event: 10 years
Label: Current Storm Storm Event: Current 10

Scenario: Current 10 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
17.00	4.87	4.88	4.89	4.90	4.91
17.50	4.92	4.94	4.95	4.96	4.97
18.00	4.98	4.99	4.99	5.00	5.01
18.50	5.02	5.03	5.04	5.05	5.06
19.00	5.07	5.08	5.09	5.09	5.10
19.50	5.11	5.12	5.13	5.14	5.15
20.00	5.15	5.16	5.17	5.18	5.19
20.50	5.20	5.20	5.21	5.22	5.23
21.00	5.24	5.24	5.25	5.26	5.27
21.50	5.27	5.28	5.29	5.30	5.30
22.00	5.31	5.32	5.32	5.33	5.34
22.50	5.34	5.35	5.36	5.36	5.37
23.00	5.38	5.38	5.39	5.40	5.40
23.50	5.41	5.42	5.42	5.43	5.43
24.00	5.44	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 100 years
Label: Current Storm Storm Event: Current 100

Scenario: Current 100 year

Time-Depth Curve: Current 100	
Label	Current 100
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	100 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.02	0.03	0.04
0.50	0.05	0.06	0.07	0.09	0.10
1.00	0.11	0.12	0.13	0.14	0.15
1.50	0.17	0.18	0.19	0.20	0.21
2.00	0.23	0.24	0.25	0.26	0.28
2.50	0.29	0.30	0.31	0.33	0.34
3.00	0.35	0.37	0.38	0.39	0.41
3.50	0.42	0.44	0.45	0.46	0.48
4.00	0.49	0.51	0.52	0.54	0.55
4.50	0.57	0.58	0.60	0.61	0.63
5.00	0.64	0.66	0.67	0.69	0.71
5.50	0.72	0.74	0.75	0.77	0.79
6.00	0.80	0.82	0.84	0.85	0.87
6.50	0.89	0.91	0.93	0.95	0.97
7.00	0.99	1.01	1.03	1.05	1.08
7.50	1.10	1.12	1.15	1.17	1.19
8.00	1.22	1.25	1.27	1.30	1.32
8.50	1.35	1.38	1.41	1.43	1.46
9.00	1.49	1.52	1.55	1.59	1.62
9.50	1.66	1.70	1.74	1.78	1.82
10.00	1.86	1.91	1.96	2.00	2.05
10.50	2.11	2.16	2.22	2.29	2.37
11.00	2.44	2.53	2.63	2.74	2.85
11.50	2.98	3.15	3.33	3.57	3.92
12.00	4.50	5.48	5.83	6.07	6.25
12.50	6.42	6.55	6.66	6.77	6.87
13.00	6.96	7.03	7.11	7.18	7.24
13.50	7.29	7.35	7.40	7.44	7.49
14.00	7.54	7.58	7.62	7.66	7.70
14.50	7.74	7.78	7.81	7.85	7.88
15.00	7.91	7.94	7.97	7.99	8.02
15.50	8.05	8.08	8.10	8.13	8.15
16.00	8.18	8.21	8.23	8.25	8.28
16.50	8.30	8.32	8.35	8.37	8.39

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Subsection: Time-Depth Curve Return Event: 100 years
Label: Current Storm Storm Event: Current 100

Scenario: Current 100 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
17.00	8.41	8.43	8.45	8.47	8.49
17.50	8.51	8.53	8.55	8.56	8.58
18.00	8.60	8.61	8.63	8.65	8.66
18.50	8.68	8.69	8.71	8.73	8.74
19.00	8.76	8.77	8.79	8.80	8.82
19.50	8.83	8.85	8.86	8.88	8.89
20.00	8.91	8.92	8.94	8.95	8.96
20.50	8.98	8.99	9.01	9.02	9.03
21.00	9.05	9.06	9.07	9.09	9.10
21.50	9.11	9.12	9.14	9.15	9.16
22.00	9.17	9.19	9.20	9.21	9.22
22.50	9.23	9.25	9.26	9.27	9.28
23.00	9.29	9.30	9.31	9.33	9.34
23.50	9.35	9.36	9.37	9.38	9.39
24.00	9.40	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 2 years Label: Current Storm Storm Event: Current 2-Year

Scenario: Current 2 year

Time-Depth Curve:	Current 2-Year
Label	Current 2-Year
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	2 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.00	0.01	0.01	0.02
0.50	0.02	0.02	0.03	0.03	0.04
1.00	0.04	0.04	0.05	0.05	0.06
1.50	0.06	0.07	0.07	0.07	0.08
2.00	0.08	0.09	0.09	0.10	0.10
2.50	0.11	0.11	0.12	0.12	0.13
3.00	0.13	0.14	0.14	0.15	0.15
3.50	0.16	0.16	0.17	0.17	0.18
4.00	0.18	0.19	0.19	0.20	0.20
4.50	0.21	0.22	0.22	0.23	0.23
5.00	0.24	0.24	0.25	0.26	0.26
5.50	0.27	0.27	0.28	0.28	0.29
6.00	0.30	0.30	0.31	0.32	0.32
6.50	0.33	0.34	0.34	0.35	0.36
7.00	0.37	0.37	0.38	0.39	0.40
7.50	0.41	0.42	0.42	0.43	0.44
8.00	0.45	0.46	0.47	0.48	0.49
8.50	0.50	0.51	0.52	0.53	0.54
9.00	0.55	0.56	0.58	0.59	0.60
9.50	0.61	0.63	0.64	0.66	0.67
10.00	0.69	0.71	0.72	0.74	0.76
10.50	0.78	0.80	0.82	0.85	0.88
11.00	0.90	0.94	0.97	1.01	1.06
11.50	1.10	1.17	1.23	1.32	1.45
12.00	1.67	2.03	2.16	2.25	2.31
12.50	2.38	2.42	2.47	2.51	2.54
13.00	2.58	2.60	2.63	2.66	2.68
13.50	2.70	2.72	2.74	2.76	2.77
14.00	2.79	2.81	2.82	2.84	2.85
14.50	2.87	2.88	2.89	2.90	2.92
15.00	2.93	2.94	2.95	2.96	2.97
15.50	2.98	2.99	3.00	3.01	3.02
16.00	3.03	3.04	3.05	3.06	3.06
16.50	3.07	3.08	3.09	3.10	3.11

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Subsection: Time-Depth Curve Return Event: 2 years
Label: Current Storm Storm Event: Current 2-Year

Scenario: Current 2 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
17.00	3.11	3.12	3.13	3.14	3.14
17.50	3.15	3.16	3.16	3.17	3.18
18.00	3.18	3.19	3.20	3.20	3.21
18.50	3.21	3.22	3.22	3.23	3.24
19.00	3.24	3.25	3.25	3.26	3.26
19.50	3.27	3.28	3.28	3.29	3.29
20.00	3.30	3.30	3.31	3.31	3.32
20.50	3.32	3.33	3.33	3.34	3.34
21.00	3.35	3.35	3.36	3.36	3.37
21.50	3.37	3.38	3.38	3.39	3.39
22.00	3.40	3.40	3.41	3.41	3.41
22.50	3.42	3.42	3.43	3.43	3.44
23.00	3.44	3.44	3.45	3.45	3.46
23.50	3.46	3.46	3.47	3.47	3.48
24.00	3.48	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 10 years Label: Future Storm Storm Event: Future 10

Scenario: Future 10 year

Time-Depth Curve: Future 10	
Label	Future 10
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	10 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.01	0.02	0.03
0.50	0.04	0.04	0.05	0.06	0.07
1.00	0.07	0.08	0.09	0.10	0.10
1.50	0.11	0.12	0.13	0.14	0.15
2.00	0.15	0.16	0.17	0.18	0.19
2.50	0.20	0.21	0.21	0.22	0.23
3.00	0.24	0.25	0.26	0.27	0.28
3.50	0.29	0.30	0.31	0.32	0.33
4.00	0.34	0.35	0.36	0.37	0.38
4.50	0.39	0.40	0.41	0.42	0.43
5.00	0.44	0.45	0.46	0.47	0.48
5.50	0.49	0.50	0.51	0.52	0.54
6.00	0.55	0.56	0.57	0.58	0.60
6.50	0.61	0.62	0.63	0.65	0.66
7.00	0.68	0.69	0.70	0.72	0.73
7.50	0.75	0.77	0.78	0.80	0.81
8.00	0.83	0.85	0.87	0.88	0.90
8.50	0.92	0.94	0.96	0.98	1.00
9.00	1.02	1.04	1.06	1.08	1.11
9.50	1.13	1.16	1.18	1.21	1.24
10.00	1.27	1.30	1.33	1.37	1.40
10.50	1.44	1.47	1.52	1.56	1.61
11.00	1.67	1.73	1.79	1.87	1.95
11.50	2.03	2.15	2.27	2.44	2.67
12.00	3.07	3.74	3.97	4.14	4.26
12.50	4.38	4.46	4.54	4.62	4.68
13.00	4.74	4.80	4.85	4.89	4.94
13.50	4.97	5.01	5.04	5.08	5.11
14.00	5.14	5.17	5.20	5.23	5.25
14.50	5.28	5.30	5.33	5.35	5.37
15.00	5.39	5.41	5.43	5.45	5.47
15.50	5.49	5.51	5.53	5.54	5.56
16.00	5.58	5.60	5.61	5.63	5.64
16.50	5.66	5.68	5.69	5.71	5.72

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Subsection: Time-Depth Curve Return Event: 10 years Label: Future Storm Storm Event: Future 10

Scenario: Future 10 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.00	5.73	5.75	5.76	5.78	5.79
17.50	5.80	5.81	5.83	5.84	5.85
18.00	5.86	5.87	5.89	5.90	5.91
18.50	5.92	5.93	5.94	5.95	5.96
19.00	5.97	5.98	5.99	6.00	6.01
19.50	6.02	6.03	6.04	6.05	6.06
20.00	6.07	6.08	6.09	6.10	6.11
20.50	6.12	6.13	6.14	6.15	6.16
21.00	6.17	6.18	6.19	6.20	6.20
21.50	6.21	6.22	6.23	6.24	6.25
22.00	6.26	6.26	6.27	6.28	6.29
22.50	6.30	6.31	6.31	6.32	6.33
23.00	6.34	6.34	6.35	6.36	6.37
23.50	6.37	6.38	6.39	6.40	6.40
24.00	6.41	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 100 years Label: Future Storm Storm Event: Future 100

Scenario: Future 100 year

Time-Depth Curve: Future 100	
Label	Future 100
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	100 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.01	0.03	0.04	0.05
0.50	0.06	0.08	0.09	0.11	0.12
1.00	0.13	0.15	0.16	0.18	0.19
1.50	0.20	0.22	0.23	0.25	0.26
2.00	0.28	0.29	0.31	0.33	0.34
2.50	0.36	0.37	0.39	0.40	0.42
3.00	0.44	0.45	0.47	0.49	0.50
3.50	0.52	0.54	0.56	0.57	0.59
4.00	0.61	0.63	0.65	0.66	0.68
4.50	0.70	0.72	0.74	0.76	0.78
5.00	0.79	0.81	0.83	0.85	0.87
5.50	0.89	0.91	0.93	0.95	0.97
6.00	0.99	1.01	1.03	1.06	1.08
6.50	1.10	1.13	1.15	1.17	1.20
7.00	1.22	1.25	1.28	1.30	1.33
7.50	1.36	1.39	1.42	1.45	1.48
8.00	1.51	1.54	1.57	1.60	1.64
8.50	1.67	1.70	1.74	1.77	1.81
9.00	1.84	1.88	1.92	1.96	2.01
9.50	2.05	2.10	2.15	2.20	2.25
10.00	2.30	2.36	2.42	2.48	2.54
10.50	2.60	2.67	2.75	2.83	2.92
11.00	3.02	3.13	3.25	3.38	3.53
11.50	3.68	3.89	4.12	4.42	4.84
12.00	5.57	6.78	7.20	7.50	7.73
12.50	7.94	8.09	8.24	8.37	8.49
13.00	8.60	8.70	8.79	8.87	8.95
13.50	9.02	9.08	9.14	9.20	9.26
14.00	9.32	9.37	9.42	9.47	9.52
14.50	9.57	9.61	9.66	9.70	9.74
15.00	9.78	9.81	9.85	9.88	9.92
15.50	9.95	9.98	10.02	10.05	10.08
16.00	10.11	10.14	10.17	10.20	10.23
16.50	10.26	10.29	10.32	10.34	10.37

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Subsection: Time-Depth Curve Return Event: 100 years Label: Future Storm Storm Event: Future 100

Scenario: Future 100 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.00	10.40	10.42	10.45	10.47	10.49
17.50	10.52	10.54	10.56	10.59	10.61
18.00	10.63	10.65	10.67	10.69	10.71
18.50	10.73	10.75	10.77	10.79	10.81
19.00	10.83	10.84	10.86	10.88	10.90
19.50	10.92	10.94	10.96	10.97	10.99
20.00	11.01	11.03	11.05	11.06	11.08
20.50	11.10	11.12	11.13	11.15	11.17
21.00	11.18	11.20	11.22	11.23	11.25
21.50	11.26	11.28	11.29	11.31	11.33
22.00	11.34	11.36	11.37	11.39	11.40
22.50	11.42	11.43	11.44	11.46	11.47
23.00	11.49	11.50	11.51	11.53	11.54
23.50	11.56	11.57	11.58	11.59	11.61
24.00	11.62	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Time-Depth Curve Return Event: 2 years Label: Future Storm Storm Event: Future 2

Scenario: Future 2 year

Time-Depth Curve: Future 2	
Label	Future 2
Start Time	0.00 hours
Increment	0.10 hours
End Time	24.00 hours
Return Event	2 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.00	0.00	0.00	0.01	0.01	0.02
0.50	0.02	0.03	0.03	0.04	0.04
1.00	0.05	0.05	0.06	0.06	0.07
1.50	0.07	0.08	0.08	0.09	0.09
2.00	0.10	0.10	0.11	0.12	0.12
2.50	0.13	0.13	0.14	0.14	0.15
3.00	0.16	0.16	0.17	0.17	0.18
3.50	0.19	0.19	0.20	0.20	0.21
4.00	0.22	0.22	0.23	0.24	0.24
4.50	0.25	0.26	0.26	0.27	0.28
5.00	0.28	0.29	0.30	0.30	0.31
5.50	0.32	0.32	0.33	0.34	0.35
6.00	0.35	0.36	0.37	0.38	0.38
6.50	0.39	0.40	0.41	0.42	0.43
7.00	0.44	0.45	0.45	0.46	0.47
7.50	0.48	0.49	0.50	0.52	0.53
8.00	0.54	0.55	0.56	0.57	0.58
8.50	0.59	0.61	0.62	0.63	0.64
9.00	0.66	0.67	0.68	0.70	0.71
9.50	0.73	0.75	0.76	0.78	0.80
10.00	0.82	0.84	0.86	0.88	0.90
10.50	0.93	0.95	0.98	1.01	1.04
11.00	1.08	1.12	1.16	1.21	1.26
11.50	1.31	1.39	1.47	1.57	1.72
12.00	1.98	2.42	2.57	2.67	2.75
12.50	2.83	2.88	2.93	2.98	3.02
13.00	3.06	3.10	3.13	3.16	3.19
13.50	3.21	3.24	3.26	3.28	3.30
14.00	3.32	3.34	3.36	3.38	3.39
14.50	3.41	3.43	3.44	3.46	3.47
15.00	3.48	3.50	3.51	3.52	3.53
15.50	3.55	3.56	3.57	3.58	3.59
16.00	3.60	3.61	3.62	3.64	3.65
16.50	3.66	3.67	3.68	3.69	3.69

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Subsection: Time-Depth Curve Return Event: 2 years Label: Future Storm Storm Event: Future 2

Scenario: Future 2 year

CUMULATIVE RAINFALL (in) Output Time Increment = 0.10 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
17.00	3.70	3.71	3.72	3.73	3.74
17.50	3.75	3.76	3.76	3.77	3.78
18.00	3.79	3.79	3.80	3.81	3.82
18.50	3.82	3.83	3.84	3.84	3.85
19.00	3.86	3.86	3.87	3.88	3.88
19.50	3.89	3.90	3.90	3.91	3.92
20.00	3.92	3.93	3.94	3.94	3.95
20.50	3.95	3.96	3.97	3.97	3.98
21.00	3.98	3.99	4.00	4.00	4.01
21.50	4.01	4.02	4.02	4.03	4.04
22.00	4.04	4.05	4.05	4.06	4.06
22.50	4.07	4.07	4.08	4.08	4.09
23.00	4.09	4.10	4.10	4.11	4.11
23.50	4.12	4.12	4.13	4.13	4.14
24.00	4.14	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Equations

Αt

Unit Hydrograph Method (Computational Notes) Definition of Terms

Total area (acres): At = Ai + Ap

	. ota: a: ca (ac: co). / ii / ii / ip
Ai	Impervious area (acres)
Ар	Pervious area (acres)
CNi	Runoff curve number for impervious area
CNp	Runoff curve number for pervious area
fLoss	f loss constant infiltration (depth/time)
gKs	Saturated Hydraulic Conductivity (depth/time)
Md	Volumetric Moisture Deficit
Psi	Capillary Suction (length)
hK	Horton Infiltration Decay Rate (time^-1)
fo	Initial Infiltration Rate (depth/time)
fc	Ultimate(capacity)Infiltration Rate (depth/time)
Ia	Initial Abstraction (length)
dt	Computational increment (duration of unit excess rainfall)
	Default dt is smallest value of 0.1333Tc, rtm, and th (Smallest dt is then adjusted to match up with Tp)
UDdt	User specified override computational main time increment
	(only used if UDdt is => .1333Tc)
D(t)	Point on distribution curve (fraction of P) for time step t
K	2 / (1 + (Tr/Tp)): default K = 0.75: (for Tr/Tp = 1.67)
Ks	Hydrograph shape factor = Unit Conversions * K: = ((1hr/3600sec) *
	(1ft/12in) * ((5280ft)**2/sq.mi)) * K Default Ks = 645.333 * 0.75 = 484
Lag	Lag time from center of excess runoff (dt) to Tp: Lag = 0.6Tc
Lag P	Total precipitation depth, inches
Pa(t)	Accumulated rainfall at time step t
Pi(t)	Incremental rainfall at time step t
• •	Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi. = (Ks * A * Q) /
qp	Tp (where Q = 1in. runoff, A=sq.mi.)
Qu(t)	Unit hydrograph ordinate (cfs) at time step t
C- (-)	and the second s

Q(t) Final hydrograph ordinate (cfs) at time step t
Rai(t) Accumulated runoff (inches) at time step t for impervious area

Rap(t) Accumulated runoff (inches) at time step t for impervious area Rip(t) Accumulated runoff (inches) at time step t for pervious area Rip(t) Incremental runoff (inches) at time step t for impervious area Rip(t) Incremental runoff (inches) at time step t for pervious area

R(t) Incremental weighted total runoff (inches)

Rtm Time increment for rainfall table

Si S for impervious area: Si = (1000/CNi) - 10 Sp S for pervious area: Sp = (1000/CNp) - 10

t Time step (row) number
Tc Time of concentration

Tb Time (hrs) of entire unit hydrograph: Tb = Tp + TrTp Time (hrs) to peak of a unit hydrograph: Tp = (dt/2) + LagTr Time (hrs) of receding limb of unit hydrograph: Tr = ratio of Tp

Subsection: Unit Hydrograph Equations

Unit Hydrograph Method Computational Notes Precipitation

Column (1) Time for time step t

Column (2) D(t) = Point on distribution curve for time step t Column (3) <math>Pi(t) = Pa(t) - Pa(t-1): Col.(4) - Preceding Col.(4)

Column (4) $Pa(t) = D(t) \times P$: $Col.(2) \times P$

Pervious Area Runoff (using SCS Runoff CN Method)

Column (5) Rap(t) = Accumulated pervious runoff for time step t

If $(Pa(t) is \le 0.2Sp)$ then use: Rap(t) = 0.0

If (Pa(t) is > 0.2Sp) then use:

Rap(t) = (Col.(4)-0.2Sp)**2 / (Col.(4)+0.8Sp)

Column (6) Rip(t) = Incremental pervious runoff for time step t

Rip(t) = Rap(t) - Rap(t-1)

Rip(t) = Col.(5) for current row - Col.(5) for preceding row.

Impervious Area Runoff

Column (7 & 8)... Did not specify to use impervious areas.

Incremental Weighted Runoff

Column (9)
$$R(t) = (Ap/At) \times Rip(t) + (Ai/At) \times Rii(t)$$
$$R(t) = (Ap/At) \times Col.(6) + (Ai/At) \times Col.(8)$$

SCS Unit Hydrograph Method

Column (10) Q(t) is computed with the SCS unit hydrograph method

using R(t) and Qu(t).

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA1 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration	0.08 hours
(Composite)	0.06 Hours
Area (User Defined)	2.340 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	8.05 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak	12.00 hours
Interpolated Output)	12.00 Hours
Flow (Peak Interpolated	4.77 ft ³ /s
Output)	
Drainaga Araa	
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.340 acres
Maximum Retention	0.20 in
(Pervious) Maximum Retention	
(Pervious, 20 percent)	0.04 in
(
Cumulative Runoff	
Cumulative Runoff Depth	2.25 :
(Pervious)	3.25 in
Runoff Volume (Pervious)	0.633 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.610 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
J. J ,	

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Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA1 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters			
Unit peak, qp	33.14 ft ³ /s		
Unit peak time, Tp	0.05 hours		
Unit receding limb, Tr	0.21 hours		
Total unit time, Tb	0.27 hours		

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA1 - Imp - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.01	0.02	0.02	0.03
2.25	0.04	0.05	0.05	0.06	0.07
3.50	0.07	0.08	0.08	0.09	0.09
4.75	0.09	0.10	0.10	0.11	0.11
6.00	0.11	0.12	0.13	0.14	0.15
7.25	0.16	0.17	0.18	0.19	0.20
8.50	0.21	0.22	0.23	0.26	0.29
9.75	0.33	0.36	0.39	0.43	0.54
11.00	0.65	0.86	1.06	1.76	4.77
12.25	2.78	1.49	0.98	0.78	0.62
13.50	0.50	0.43	0.40	0.36	0.33
14.75	0.30	0.27	0.25	0.24	0.23
16.00	0.22	0.21	0.20	0.19	0.18
17.25	0.17	0.17	0.16	0.15	0.14
18.50	0.14	0.14	0.14	0.13	0.13
19.75	0.13	0.13	0.12	0.12	0.12
21.00	0.12	0.11	0.11	0.11	0.11
22.25	0.11	0.10	0.10	0.10	0.10
23.50	0.09	0.09	0.09	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	12.66 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	7.52 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.340 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.20 in
Runoff Volume (Pervious)	1.014 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.979 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	33.14 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: EDA1 - Imp - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.02	0.04	0.06
1.75	0.07	0.08	0.10	0.11	0.12
3.00	0.12	0.13	0.14	0.15	0.15
4.25	0.16	0.17	0.17	0.18	0.18
5.50	0.19	0.20	0.20	0.21	0.23
6.75	0.24	0.26	0.27	0.29	0.30
8.00	0.32	0.33	0.35	0.37	0.38
9.25	0.43	0.48	0.53	0.58	0.63
10.50	0.68	0.86	1.04	1.36	1.69
11.75	2.78	7.52	4.37	2.34	1.54
13.00	1.22	0.97	0.78	0.67	0.62
14.25	0.57	0.52	0.47	0.42	0.39
15.50	0.37	0.36	0.35	0.33	0.32
16.75	0.30	0.29	0.27	0.26	0.25
18.00	0.23	0.22	0.22	0.22	0.21
19.25	0.21	0.21	0.20	0.20	0.19
20.50	0.19	0.19	0.18	0.18	0.18
21.75	0.17	0.17	0.17	0.16	0.16
23.00	0.15	0.15	0.15	0.14	0.14

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	21.93 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	13.04 ft³/s
Drainage Area	
	00.000
SCS CN (Composite)	98.000
Area (User Defined) Maximum Retention	2.340 acres
(Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
(5 111, 5 1111)	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	1.786 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	1.724 ac-ft
SCS Unit Hydrograph Parameters	,
	•
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	33.14 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 100 years
Label: EDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.07	0.11	0.14
1.50	0.17	0.19	0.21	0.22	0.24
2.75	0.25	0.26	0.27	0.29	0.30
4.00	0.31	0.31	0.32	0.33	0.34
5.25	0.35	0.36	0.36	0.37	0.40
6.50	0.42	0.44	0.47	0.50	0.52
7.75	0.55	0.57	0.60	0.62	0.65
9.00	0.67	0.76	0.84	0.93	1.02
10.25	1.11	1.20	1.51	1.82	2.38
11.50	2.94	4.83	13.04	7.56	4.05
12.75	2.67	2.11	1.68	1.36	1.16
14.00	1.08	0.99	0.90	0.81	0.72
15.25	0.67	0.65	0.62	0.60	0.57
16.50	0.55	0.52	0.50	0.47	0.45
17.75	0.42	0.40	0.38	0.38	0.37
19.00	0.37	0.36	0.35	0.35	0.34
20.25	0.34	0.33	0.32	0.32	0.31
21.50	0.30	0.30	0.29	0.29	0.28
22.75	0.27	0.27	0.26	0.26	0.25
24.00	0.24	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA1 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

•					
Storm Event	Future 2				
Return Event	2 years				
Duration	24.00 hours				
Depth	4.14 in				
Time of Concentration (Composite)	0.08 hours				
Area (User Defined)	2.340 acres				
Computational Time Increment	0.01 hours				
Time to Peak (Computed)	12.11 hours				
Flow (Peak, Computed)	9.60 ft ³ /s				
Output Increment	0.25 hours				
Time to Flow (Peak Interpolated Output)	12.00 hours				
Flow (Peak Interpolated Output)	5.70 ft ³ /s				
Drainage Area					
SCS CN (Composite)	98.000				
Area (User Defined)	2.340 acres				
Maximum Retention (Pervious)	0.20 in				
Maximum Retention (Pervious, 20 percent)	0.04 in				
Cumulative Runoff					
Cumulative Runoff Depth (Pervious)	3.90 in				
Runoff Volume (Pervious)	0.761 ac-ft				
Liveline arrows b Malyress - Assessment					
Hydrograph Volume (Area under h	,				
Volume	0.734 ac-ft				
SCS Unit Hydrograph Parameters					
Time of Concentration (Composite)	0.08 hours				
Computational Time Increment	0.01 hours				
Unit Hydrograph Shape Factor	483.432				
K Factor	0.749				
Receding/Rising, Tr/Tp	1.670				

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA1 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	33.14 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA1 - Imp - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.00	0.02	0.03	0.04
2.00	0.05	0.06	0.07	0.07	0.08
3.25	0.09	0.09	0.10	0.11	0.11
4.50	0.12	0.12	0.13	0.13	0.14
5.75	0.14	0.14	0.15	0.17	0.18
7.00	0.19	0.20	0.21	0.22	0.24
8.25	0.25	0.26	0.27	0.28	0.32
9.50	0.36	0.40	0.43	0.47	0.51
10.75	0.65	0.79	1.03	1.27	2.10
12.00	5.70	3.32	1.78	1.17	0.93
13.25	0.74	0.60	0.51	0.47	0.43
14.50	0.40	0.36	0.32	0.29	0.28
15.75	0.27	0.26	0.25	0.24	0.23
17.00	0.22	0.21	0.20	0.19	0.18
18.25	0.17	0.17	0.16	0.16	0.16
19.50	0.16	0.15	0.15	0.15	0.14
20.75	0.14	0.14	0.14	0.13	0.13
22.00	0.13	0.13	0.12	0.12	0.12
23.25	0.11	0.11	0.11	0.11	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

•	
Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	14.93 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	8.87 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.340 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.17 in
Runoff Volume (Pervious)	1.203 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	1.162 ac-ft
SCS Unit Hydrograph Parameters	S
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	33.14 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 10 years
Label: EDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.01	0.04	0.06	0.08
1.75	0.10	0.11	0.13	0.14	0.15
3.00	0.16	0.17	0.18	0.18	0.19
4.25	0.20	0.21	0.21	0.22	0.23
5.50	0.23	0.24	0.24	0.26	0.28
6.75	0.29	0.31	0.33	0.35	0.36
8.00	0.38	0.40	0.42	0.43	0.45
9.25	0.51	0.57	0.63	0.69	0.75
10.50	0.81	1.02	1.23	1.61	1.99
11.75	3.28	8.87	5.15	2.76	1.82
13.00	1.44	1.14	0.92	0.79	0.73
14.25	0.67	0.61	0.55	0.49	0.46
15.50	0.44	0.42	0.41	0.39	0.37
16.75	0.36	0.34	0.32	0.31	0.29
18.00	0.27	0.26	0.26	0.25	0.25
19.25	0.25	0.24	0.24	0.23	0.23
20.50	0.22	0.22	0.22	0.21	0.21
21.75	0.20	0.20	0.20	0.19	0.19
23.00	0.18	0.18	0.17	0.17	0.17

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	27.13 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	16.13 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.340 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	2.219 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	2.143 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	33.14 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: EDA1 - Imp - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.340 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

rime on left represents time for mot value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.04	0.11	0.16	0.20
1.50	0.23	0.26	0.28	0.30	0.31
2.75	0.33	0.34	0.35	0.37	0.38
4.00	0.39	0.40	0.41	0.42	0.43
5.25	0.44	0.45	0.46	0.46	0.49
6.50	0.52	0.56	0.59	0.62	0.65
7.75	0.68	0.71	0.74	0.77	0.81
9.00	0.84	0.94	1.05	1.16	1.27
10.25	1.37	1.48	1.87	2.26	2.94
11.50	3.64	5.98	16.13	9.35	5.00
12.75	3.30	2.61	2.07	1.68	1.44
14.00	1.33	1.22	1.11	1.00	0.90
15.25	0.83	0.80	0.77	0.74	0.71
16.50	0.68	0.65	0.62	0.59	0.56
17.75	0.52	0.49	0.48	0.47	0.46
19.00	0.45	0.45	0.44	0.43	0.42
20.25	0.41	0.41	0.40	0.39	0.38
21.50	0.38	0.37	0.36	0.35	0.35
22.75	0.34	0.33	0.32	0.32	0.31
24.00	0.30	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA1 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	0.69 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.42 ft³/s
Drainage Area	
SCS CN (Composite)	66.200
Area (User Defined)	0.880 acres
Maximum Retention (Pervious)	5.11 in
Maximum Retention (Pervious, 20 percent)	1.02 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.80 in
Runoff Volume (Pervious)	0.059 ac-ft
Lhadas anna h M. L. (A.	
Hydrograph Volume (Area und	ier Hydrograph curve)
Volume	0.056 ac-ft
SCS Unit Hydrograph Parame	ters
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: EDA1 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA1 - Per - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.25	0.00	0.01	0.04	0.25	0.42
12.50	0.21	0.15	0.12	0.10	0.08
13.75	0.07	0.07	0.06	0.06	0.05
15.00	0.05	0.04	0.04	0.04	0.04
16.25	0.04	0.04	0.04	0.03	0.03
17.50	0.03	0.03	0.03	0.03	0.03
18.75	0.03	0.03	0.03	0.03	0.03
20.00	0.02	0.02	0.02	0.02	0.02
21.25	0.02	0.02	0.02	0.02	0.02
22.50	0.02	0.02	0.02	0.02	0.02
23.75	0.02	0.02	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years
Label: EDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10			
Return Event	10 years			
Duration	24.00 hours			
Depth	5.44 in			
Time of Concentration (Composite)	0.13 hours			
Area (User Defined)	0.880 acres			
Computational Time Increment	0.02 hours			
Time to Peak (Computed)	12.13 hours			
Flow (Peak, Computed)	1.96 ft ³ /s			
Output Increment	0.25 hours			
Time to Flow (Peak Interpolated Output)	12.25 hours			
Flow (Peak Interpolated Output)	1.10 ft³/s			
Drainage Area				
SCS CN (Composite)	66.200			
Area (User Defined)	0.880 acres			
Maximum Retention (Pervious)	5.11 in			
Maximum Retention (Pervious, 20 percent)	1.02 in			
Cumulative Runoff				
Cumulative Runoff Depth (Pervious)	2.05 in			
Runoff Volume (Pervious)	0.150 ac-ft			
Hydrograph Volume (Area under F				
Volume	0.145 ac-ft			
SCS Unit Hydrograph Parameters				
Time of Concentration (Composite)	0.13 hours			
Computational Time Increment	0.02 hours			
Unit Hydrograph Shape Factor	483.432			
K Factor	0.749			
Receding/Rising, Tr/Tp	1.670			

Subsection: Unit Hydrograph Summary Return Event: 10 years
Label: EDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: EDA1 - Per - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.75	0.00	0.00	0.01	0.02	0.03
11.00	0.05	0.08	0.13	0.26	0.88
12.25	1.10	0.52	0.35	0.29	0.23
13.50	0.19	0.16	0.15	0.14	0.13
14.75	0.12	0.10	0.10	0.09	0.09
16.00	0.09	0.08	0.08	0.08	0.07
17.25	0.07	0.07	0.06	0.06	0.06
18.50	0.06	0.06	0.06	0.05	0.05
19.75	0.05	0.05	0.05	0.05	0.05
21.00	0.05	0.05	0.05	0.05	0.05
22.25	0.04	0.04	0.04	0.04	0.04
23.50	0.04	0.04	0.04	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years
Label: EDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	5.05 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	2.69 ft³/s
Drainage Area	
SCS CN (Composite)	66.200
Area (User Defined)	0.880 acres
Maximum Retention (Pervious)	5.11 in
Maximum Retention (Pervious, 20 percent)	1.02 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.21 in
Runoff Volume (Pervious)	0.382 ac-ft
Hydrograph Volume (Area under l	Hydrograph curve)
Volume	0.367 ac-ft
SCS Unit Hydrograph Parameters	3
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years
Label: EDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: EDA1 - Per - Cur

Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
7.25	0.00	0.00	0.01	0.01	0.02
8.50	0.03	0.03	0.04	0.05	0.06
9.75	0.08	0.10	0.12	0.14	0.19
11.00	0.25	0.36	0.49	0.85	2.52
12.25	2.69	1.20	0.81	0.65	0.52
13.50	0.42	0.36	0.33	0.30	0.28
14.75	0.25	0.23	0.21	0.20	0.19
16.00	0.19	0.18	0.17	0.16	0.16
17.25	0.15	0.14	0.13	0.13	0.12
18.50	0.12	0.12	0.12	0.11	0.11
19.75	0.11	0.11	0.11	0.11	0.10
21.00	0.10	0.10	0.10	0.10	0.09
22.25	0.09	0.09	0.09	0.09	0.08
23.50	0.08	0.08	0.08	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: EDA1 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	1.08 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.63 ft³/s
Drainage Area	
-	
SCS CN (Composite)	66.200
Area (User Defined)	0.880 acres
Maximum Retention (Pervious)	5.11 in
Maximum Retention (Pervious, 20 percent)	1.02 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.18 in
Runoff Volume (Pervious)	0.087 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.083 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
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Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: EDA1 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA1 - Per - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.75	0.00	0.00	0.02	0.04	0.10
12.00	0.44	0.63	0.31	0.22	0.18
13.25	0.14	0.12	0.10	0.09	0.09
14.50	0.08	0.07	0.07	0.06	0.06
15.75	0.06	0.06	0.05	0.05	0.05
17.00	0.05	0.05	0.04	0.04	0.04
18.25	0.04	0.04	0.04	0.04	0.04
19.50	0.03	0.03	0.03	0.03	0.03
20.75	0.03	0.03	0.03	0.03	0.03
22.00	0.03	0.03	0.03	0.03	0.03
23.25	0.03	0.03	0.03	0.02	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres
Computational Time	0.02 havea
Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	2.68 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.47 ft³/s
Drainage Area	
SCS CN (Composite)	66,200
Area (User Defined)	0.880 acres
Maximum Retention (Pervious)	5.11 in
Maximum Retention (Pervious, 20 percent)	1.02 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.77 in
Runoff Volume (Pervious)	0.203 ac-ft
Hydrograph Volume (Area under	Hvdrograph curve)
Volume	0.195 ac-ft
SCS Unit Hydrograph Parameters	S
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA1 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft³/s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 10 years
Label: EDA1 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.00	0.00	0.00	0.01	0.01	0.02
10.25	0.03	0.04	0.06	0.09	0.14
11.50	0.21	0.39	1.25	1.47	0.68
12.75	0.46	0.37	0.30	0.24	0.21
14.00	0.19	0.18	0.16	0.15	0.13
15.25	0.12	0.12	0.12	0.11	0.11
16.50	0.10	0.10	0.09	0.09	0.09
17.75	0.08	0.08	0.07	0.07	0.07
19.00	0.07	0.07	0.07	0.07	0.07
20.25	0.06	0.06	0.06	0.06	0.06
21.50	0.06	0.06	0.06	0.06	0.05
22.75	0.05	0.05	0.05	0.05	0.05
24.00	0.05	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	6.88 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	3.63 ft³/s
Drainage Area	
SCS CN (Composite)	66.200
Area (User Defined)	0.880 acres
Maximum Retention (Pervious)	5.11 in
Maximum Retention (Pervious, 20 percent)	1.02 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	7.15 in
Runoff Volume (Pervious)	0.525 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.505 ac-ft
SCS Unit Hydrograph Parameter	rs
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA1 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	7.67 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: EDA1 - Per - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	0.880 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
6.25	0.00	0.00	0.01	0.01	0.02
7.50	0.03	0.03	0.04	0.05	0.06
8.75	0.07	0.08	0.10	0.12	0.14
10.00	0.17	0.19	0.22	0.30	0.39
11.25	0.54	0.73	1.23	3.53	3.63
12.50	1.60	1.07	0.85	0.68	0.55
13.75	0.47	0.43	0.40	0.36	0.33
15.00	0.30	0.27	0.26	0.25	0.24
16.25	0.23	0.22	0.21	0.20	0.19
17.50	0.18	0.17	0.17	0.16	0.16
18.75	0.15	0.15	0.15	0.15	0.14
20.00	0.14	0.14	0.14	0.13	0.13
21.25	0.13	0.13	0.12	0.12	0.12
22.50	0.12	0.11	0.11	0.11	0.11
23.75	0.10	0.10	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA2 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year			
Return Event	2 years			
Duration	24.00 hours			
Depth	3.48 in			
Time of Concentration (Composite)	0.05 hours			
Area (User Defined)	0.930 acres			
Computational Time Increment	0.01 hours			
Time to Peak (Computed)	12.10 hours			
Flow (Peak, Computed)	3.34 ft ³ /s			
Output Increment	0.25 hours			
Time to Flow (Peak Interpolated Output)	12.00 hours			
Flow (Peak Interpolated Output)	2.00 ft ³ /s			
Drainage Area				
SCS CN (Composite)	98.000			
Area (User Defined)	0.930 acres			
Maximum Retention (Pervious)	0.20 in			
Maximum Retention (Pervious, 20 percent)	0.04 in			
Cumulative Runoff				
Cumulative Runoff Depth (Pervious)	3.25 in			
Runoff Volume (Pervious)	0.252 ac-ft			
Hydrograph Volume (Area unde	er Hydrograph curve)			
Volume	0.243 ac-ft			
CCC Unit Hydromanh Davis				
SCS Unit Hydrograph Paramete	#IS			
Time of Concentration (Composite)	0.05 hours			
Computational Time Increment	0.01 hours			
Unit Hydrograph Shape Factor	483.432			
K Factor	0.749			
Receding/Rising, Tr/Tp	1.670			

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA2 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	21.07 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA2 - Imp - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.00	0.01	0.01	0.01
2.25	0.02	0.02	0.02	0.02	0.03
3.50	0.03	0.03	0.03	0.03	0.04
4.75	0.04	0.04	0.04	0.04	0.04
6.00	0.05	0.05	0.05	0.06	0.06
7.25	0.07	0.07	0.07	0.08	0.08
8.50	0.08	0.09	0.09	0.11	0.12
9.75	0.13	0.14	0.16	0.17	0.22
11.00	0.26	0.35	0.43	0.78	2.00
12.25	0.93	0.59	0.38	0.31	0.24
13.50	0.20	0.17	0.16	0.14	0.13
14.75	0.12	0.11	0.10	0.09	0.09
16.00	0.09	0.08	0.08	0.08	0.07
17.25	0.07	0.07	0.06	0.06	0.06
18.50	0.06	0.05	0.05	0.05	0.05
19.75	0.05	0.05	0.05	0.05	0.05
21.00	0.05	0.05	0.04	0.04	0.04
22.25	0.04	0.04	0.04	0.04	0.04
23.50	0.04	0.04	0.04	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

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Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	5.26 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	3.14 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.930 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.20 in
Runoff Volume (Pervious)	0.403 ac-ft
Hydrograph Volume (Area under	r Hydrograph curve)
Volume	0.389 ac-ft
SCS Unit Hydrograph Paramete	rs
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	21.07 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 10 years
Label: EDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.01	0.02	0.02
1.75	0.03	0.03	0.04	0.04	0.05
3.00	0.05	0.05	0.06	0.06	0.06
4.25	0.06	0.07	0.07	0.07	0.07
5.50	0.08	0.08	0.08	0.09	0.09
6.75	0.10	0.10	0.11	0.12	0.12
8.00	0.13	0.13	0.14	0.15	0.15
9.25	0.17	0.19	0.21	0.23	0.25
10.50	0.27	0.35	0.42	0.56	0.68
11.75	1.23	3.14	1.46	0.92	0.59
13.00	0.48	0.37	0.31	0.26	0.25
14.25	0.22	0.21	0.18	0.17	0.15
15.50	0.15	0.14	0.14	0.13	0.13
16.75	0.12	0.11	0.11	0.10	0.10
18.00	0.09	0.09	0.09	0.09	0.08
19.25	0.08	0.08	0.08	0.08	0.08
20.50	0.08	0.07	0.07	0.07	0.07
21.75	0.07	0.07	0.07	0.06	0.06
23.00	0.06	0.06	0.06	0.06	0.06

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	9.11 ft³/s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	5.45 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.930 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	0.710 ac-ft
Hydrograph Volume (Area und	ler Hydrograph curve)
Volume	0.686 ac-ft
SCS Unit Hydrograph Parame	ters
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters		
Unit peak, qp	21.07 ft ³ /s	
Unit peak time, Tp	0.03 hours	
Unit receding limb, Tr	0.13 hours	
Total unit time, Tb	0.17 hours	

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 100 years
Label: EDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.03	0.04	0.06
1.50	0.07	0.08	0.08	0.09	0.09
2.75	0.10	0.10	0.11	0.11	0.12
4.00	0.12	0.13	0.13	0.13	0.14
5.25	0.14	0.14	0.14	0.15	0.16
6.50	0.17	0.18	0.19	0.20	0.21
7.75	0.22	0.23	0.24	0.25	0.26
9.00	0.27	0.31	0.34	0.38	0.41
10.25	0.45	0.48	0.62	0.73	0.98
11.50	1.18	2.14	5.45	2.53	1.60
12.75	1.03	0.83	0.65	0.53	0.46
14.00	0.43	0.39	0.36	0.32	0.29
15.25	0.27	0.26	0.25	0.24	0.23
16.50	0.22	0.21	0.20	0.19	0.18
17.75	0.17	0.16	0.15	0.15	0.15
19.00	0.15	0.14	0.14	0.14	0.14
20.25	0.13	0.13	0.13	0.13	0.12
21.50	0.12	0.12	0.12	0.11	0.11
22.75	0.11	0.11	0.10	0.10	0.10
24.00	0.10	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA2 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres
,	
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	3.99 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	2.38 ft³/s
Drainage Area	
·	00.000
SCS CN (Composite)	98.000
Area (User Defined) Maximum Retention	0.930 acres
(Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.90 in
Runoff Volume (Pervious)	0.303 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.292 ac-ft
SCS Unit Hydrograph Parameter	 S
Time of Concentration	
(Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
	2.0.0

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA2 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	21.07 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA2 - Imp - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time on lore represents time for most value in cach form					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.00	0.01	0.01	0.02
2.00	0.02	0.02	0.03	0.03	0.03
3.25	0.03	0.04	0.04	0.04	0.04
4.50	0.05	0.05	0.05	0.05	0.05
5.75	0.06	0.06	0.06	0.07	0.07
7.00	0.08	0.08	0.08	0.09	0.09
8.25	0.10	0.10	0.11	0.11	0.13
9.50	0.14	0.16	0.17	0.19	0.20
10.75	0.27	0.31	0.42	0.51	0.93
12.00	2.38	1.11	0.70	0.45	0.36
13.25	0.28	0.23	0.20	0.19	0.17
14.50	0.16	0.14	0.13	0.12	0.11
15.75	0.11	0.10	0.10	0.10	0.09
17.00	0.09	0.08	0.08	0.07	0.07
18.25	0.07	0.07	0.07	0.06	0.06
19.50	0.06	0.06	0.06	0.06	0.06
20.75	0.06	0.06	0.05	0.05	0.05
22.00	0.05	0.05	0.05	0.05	0.05
23.25	0.05	0.04	0.04	0.04	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

,	
Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	6.20 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	3.71 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.930 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.17 in
Runoff Volume (Pervious)	0.478 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.462 ac-ft
SCS Unit Hydrograph Parameter	s
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	21.07 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: EDA2 - Imp - Fut

Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.01	0.02	0.03	0.03
1.75	0.04	0.05	0.05	0.05	0.06
3.00	0.06	0.07	0.07	0.07	0.08
4.25	0.08	0.08	0.08	0.09	0.09
5.50	0.09	0.09	0.10	0.10	0.11
6.75	0.12	0.12	0.13	0.14	0.15
8.00	0.15	0.16	0.17	0.17	0.18
9.25	0.21	0.23	0.25	0.27	0.30
10.50	0.32	0.42	0.49	0.66	0.80
11.75	1.45	3.71	1.72	1.09	0.70
13.00	0.56	0.44	0.36	0.31	0.29
14.25	0.26	0.24	0.22	0.20	0.18
15.50	0.17	0.17	0.16	0.15	0.15
16.75	0.14	0.13	0.13	0.12	0.11
18.00	0.11	0.10	0.10	0.10	0.10
19.25	0.10	0.10	0.09	0.09	0.09
20.50	0.09	0.09	0.09	0.08	0.08
21.75	0.08	0.08	0.08	0.08	0.07
23.00	0.07	0.07	0.07	0.07	0.07

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	11.27 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	6.75 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.930 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	0.882 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.852 ac-ft
	0.002 40 10
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters		
Unit peak, qp	21.07 ft ³ /s	
Unit peak time, Tp	0.03 hours	
Unit receding limb, Tr	0.13 hours	
Total unit time, Tb	0.17 hours	

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 100 years Label: EDA2 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.930 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

rime on leterapieseries and for mise value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.02	0.05	0.07	0.08
1.50	0.09	0.10	0.11	0.12	0.12
2.75	0.13	0.14	0.14	0.15	0.15
4.00	0.15	0.16	0.16	0.17	0.17
5.25	0.17	0.18	0.18	0.18	0.20
6.50	0.21	0.22	0.23	0.25	0.26
7.75	0.27	0.28	0.30	0.31	0.32
9.00	0.33	0.38	0.42	0.47	0.50
10.25	0.55	0.59	0.76	0.90	1.21
11.50	1.46	2.65	6.75	3.13	1.97
12.75	1.27	1.02	0.80	0.66	0.57
14.00	0.53	0.48	0.44	0.39	0.35
15.25	0.33	0.32	0.30	0.29	0.28
16.50	0.27	0.26	0.24	0.23	0.22
17.75	0.21	0.20	0.19	0.19	0.18
19.00	0.18	0.18	0.17	0.17	0.17
20.25	0.16	0.16	0.16	0.16	0.15
21.50	0.15	0.15	0.14	0.14	0.14
22.75	0.13	0.13	0.13	0.13	0.12
24.00	0.12	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: EDA2 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	0.54 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.42 ft³/s
Drainage Area	
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention (Pervious, 20 percent)	0.86 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.00 in
Runoff Volume (Pervious)	0.049 ac-ft
Hydrograph Volume (Area und	der Hydrograph curve)
Volume	0.048 ac-ft
	0.0.10 40 10
SCS Unit Hydrograph Parame	eters
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: EDA2 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.71 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours
Total unit time, Tb	0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA2 - Per - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
(Hours)	(11-75)	(11-75)	(IL-/5)	(IL-/5)	(11-75)
11.00	0.00	0.01	0.02	0.05	0.20
12.25	0.42	0.19	0.13	0.10	0.08
13.50	0.07	0.06	0.05	0.05	0.05
14.75	0.04	0.04	0.03	0.03	0.03
16.00	0.03	0.03	0.03	0.03	0.03
17.25	0.03	0.02	0.02	0.02	0.02
18.50	0.02	0.02	0.02	0.02	0.02
19.75	0.02	0.02	0.02	0.02	0.02
21.00	0.02	0.02	0.02	0.02	0.02
22.25	0.02	0.02	0.02	0.02	0.02
23.50	0.01	0.01	0.01	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	1.37 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.02 ft³/s
Drainage Area	
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention (Pervious, 20 percent)	0.86 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.37 in
Runoff Volume (Pervious)	0.116 ac-ft
Hydrograph Volume (Area under F	Hydrograph curve)
Volume	0.114 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.71 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours
Total unit time, Tb	0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: EDA2 - Per - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.00	0.00	0.00	0.00	0.01	0.01
10.25	0.02	0.02	0.03	0.05	0.08
11.50	0.11	0.21	0.59	1.02	0.43
12.75	0.28	0.22	0.18	0.15	0.12
14.00	0.11	0.10	0.10	0.09	0.08
15.25	0.07	0.07	0.07	0.06	0.06
16.50	0.06	0.06	0.05	0.05	0.05
17.75	0.05	0.04	0.04	0.04	0.04
19.00	0.04	0.04	0.04	0.04	0.04
20.25	0.04	0.04	0.04	0.04	0.03
21.50	0.03	0.03	0.03	0.03	0.03
22.75	0.03	0.03	0.03	0.03	0.03
24.00	0.03	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	3.27 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	2.39 ft³/s
Drainage Area	
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention (Pervious, 20 percent)	0.86 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.69 in
Runoff Volume (Pervious)	0.280 ac-ft
Hydrograph Volume (Area under l	Hydrograph curve)
Volume	0.274 ac-ft
SCS Unit Hydrograph Parameters	3
Time of Concentration (Composite)	0.18 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
•	

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.71 ft ³ /s
Unit peak time, Tp	0.12 hours
Unit receding limb, Tr	0.48 hours
Total unit time, Tb	0.60 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: EDA2 - Per - Cur

Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.18 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
6.50	0.00	0.00	0.01	0.01	0.01
7.75	0.02	0.02	0.02	0.03	0.03
9.00	0.04	0.05	0.06	0.07	0.08
10.25	0.10	0.11	0.15	0.20	0.27
11.50	0.37	0.61	1.56	2.39	0.95
12.75	0.60	0.48	0.38	0.31	0.26
14.00	0.24	0.22	0.20	0.18	0.16
15.25	0.15	0.14	0.14	0.13	0.13
16.50	0.12	0.12	0.11	0.11	0.10
17.75	0.09	0.09	0.09	0.08	0.08
19.00	0.08	0.08	0.08	0.08	0.08
20.25	0.07	0.07	0.07	0.07	0.07
21.50	0.07	0.07	0.07	0.06	0.06
22.75	0.06	0.06	0.06	0.06	0.06
24.00	0.06	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: EDA2 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	0.82 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.60 ft ³ /s
Drainage Area	
	60.000
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention	0.86 in
(Pervious, 20 percent)	0.00 111
Cumulative Runoff	
Cumulative Runoff Depth	
(Pervious)	1.42 in
Runoff Volume (Pervious)	0.070 ac-ft
Hydrograph Volume (Area under I	Hydrograph curve)
Volume	0.068 ac-ft
SCS Unit Hydrograph Parameters	<u> </u>
	•
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: EDA2 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.93 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: EDA2 - Per - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
10.25	0.00	0.00	0.01	0.01	0.03
11.50	0.05	0.10	0.33	0.60	0.26
12.75	0.17	0.14	0.11	0.09	0.08
14.00	0.07	0.07	0.06	0.06	0.05
15.25	0.05	0.05	0.04	0.04	0.04
16.50	0.04	0.04	0.04	0.03	0.03
17.75	0.03	0.03	0.03	0.03	0.03
19.00	0.03	0.03	0.03	0.03	0.03
20.25	0.02	0.02	0.02	0.02	0.02
21.50	0.02	0.02	0.02	0.02	0.02
22.75	0.02	0.02	0.02	0.02	0.02
24.00	0.02	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	1.85 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.31 ft³/s
Drainage Area	
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention (Pervious, 20 percent)	0.86 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.13 in
Runoff Volume (Pervious)	0.154 ac-ft
Hydrograph Volume (Area under H	ydrograph curve)
Volume	0.150 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: EDA2 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.93 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: EDA2 - Per - Fut

Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time on lest opicocito time for mot value in cach rom					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
8.25	0.00	0.00	0.00	0.01	0.01
9.50	0.01	0.02	0.03	0.03	0.04
10.75	0.06	0.08	0.12	0.17	0.30
12.00	0.85	1.31	0.54	0.35	0.28
13.25	0.22	0.18	0.15	0.14	0.13
14.50	0.12	0.11	0.10	0.09	0.09
15.75	0.08	0.08	0.08	0.07	0.07
17.00	0.07	0.06	0.06	0.06	0.06
18.25	0.05	0.05	0.05	0.05	0.05
19.50	0.05	0.05	0.05	0.05	0.05
20.75	0.04	0.04	0.04	0.04	0.04
22.00	0.04	0.04	0.04	0.04	0.04
23.25	0.04	0.04	0.03	0.03	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	4.50 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	3.07 ft³/s
Drainage Area	
SCS CN (Composite)	69.990
Area (User Defined)	0.590 acres
Maximum Retention (Pervious)	4.29 in
Maximum Retention (Pervious, 20 percent)	0.86 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	7.70 in
Runoff Volume (Pervious)	0.378 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	0.369 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: EDA2 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.93 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: EDA2 - Per - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.590 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
5.50	0.00	0.00	0.01	0.01	0.01
6.75	0.02	0.02	0.03	0.03	0.04
8.00	0.04	0.05	0.05	0.06	0.07
9.25	0.08	0.10	0.11	0.13	0.15
10.50	0.18	0.23	0.29	0.40	0.54
11.75	0.87	2.22	3.07	1.22	0.78
13.00	0.62	0.49	0.40	0.33	0.30
14.25	0.28	0.26	0.23	0.21	0.19
15.50	0.18	0.18	0.17	0.16	0.16
16.75	0.15	0.14	0.14	0.13	0.12
18.00	0.12	0.11	0.11	0.11	0.10
19.25	0.10	0.10	0.10	0.10	0.10
20.50	0.09	0.09	0.09	0.09	0.09
21.75	0.09	0.08	0.08	0.08	0.08
23.00	0.08	0.08	0.07	0.07	0.07

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	7.12 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	4.22 ft³/s
Drainage Area	
	98.000
SCS CN (Composite) Area (User Defined)	96.000 2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.25 in
Runoff Volume (Pervious)	0.560 ac-ft
Hydrograph Volume (Area und	der Hydrograph curve)
Volume	0.540 ac-ft
SCS Unit Hydrograph Parame	eters
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	29.32 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA1 - Imp - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.01	0.01	0.02	0.03
2.25	0.04	0.04	0.05	0.05	0.06
3.50	0.06	0.07	0.07	0.08	0.08
4.75	0.08	0.09	0.09	0.10	0.10
6.00	0.10	0.11	0.12	0.13	0.14
7.25	0.14	0.15	0.16	0.17	0.18
8.50	0.19	0.20	0.21	0.23	0.26
9.75	0.29	0.32	0.35	0.38	0.48
11.00	0.58	0.76	0.94	1.55	4.22
12.25	2.46	1.32	0.87	0.69	0.55
13.50	0.44	0.38	0.35	0.32	0.29
14.75	0.26	0.24	0.22	0.21	0.20
16.00	0.19	0.19	0.18	0.17	0.16
17.25	0.15	0.15	0.14	0.13	0.13
18.50	0.12	0.12	0.12	0.12	0.12
19.75	0.11	0.11	0.11	0.11	0.11
21.00	0.10	0.10	0.10	0.10	0.10
22.25	0.09	0.09	0.09	0.09	0.09
23.50	0.08	0.08	0.08	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	11.20 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	6.65 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.20 in
Runoff Volume (Pervious)	0.897 ac-ft
Hydrograph Volume (Area under F	Hydrograph curve)
Volume	0.866 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	29.32 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA1 - Imp - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.02	0.04	0.05
1.75	0.06	0.07	0.08	0.09	0.10
3.00	0.11	0.12	0.12	0.13	0.14
4.25	0.14	0.15	0.15	0.16	0.16
5.50	0.17	0.17	0.18	0.19	0.20
6.75	0.22	0.23	0.24	0.26	0.27
8.00	0.28	0.30	0.31	0.32	0.34
9.25	0.38	0.42	0.47	0.51	0.56
10.50	0.60	0.76	0.92	1.21	1.49
11.75	2.46	6.65	3.86	2.07	1.36
13.00	1.08	0.86	0.69	0.59	0.55
14.25	0.50	0.46	0.42	0.37	0.34
15.50	0.33	0.32	0.31	0.29	0.28
16.75	0.27	0.25	0.24	0.23	0.22
18.00	0.20	0.20	0.19	0.19	0.19
19.25	0.18	0.18	0.18	0.17	0.17
20.50	0.17	0.17	0.16	0.16	0.16
21.75	0.15	0.15	0.15	0.14	0.14
23.00	0.14	0.13	0.13	0.13	0.12

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	19.40 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	11.54 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	1.580 ac-ft
Lludus susus Naturas (A	an I leading arrangle accounts \
Hydrograph Volume (Area und	er nydrograph curve)
Volume	1.526 ac-ft
SCS Unit Hydrograph Paramet	ters
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	29.32 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 100 years
Label: PDA1 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.06	0.09	0.12
1.50	0.15	0.17	0.18	0.20	0.21
2.75	0.22	0.23	0.24	0.25	0.26
4.00	0.27	0.28	0.29	0.29	0.30
5.25	0.31	0.31	0.32	0.33	0.35
6.50	0.37	0.39	0.42	0.44	0.46
7.75	0.48	0.51	0.53	0.55	0.57
9.00	0.60	0.67	0.75	0.82	0.90
10.25	0.98	1.06	1.33	1.61	2.10
11.50	2.60	4.28	11.54	6.69	3.58
12.75	2.36	1.87	1.48	1.20	1.03
14.00	0.95	0.87	0.80	0.72	0.64
15.25	0.59	0.57	0.55	0.53	0.51
16.50	0.48	0.46	0.44	0.42	0.40
17.75	0.38	0.35	0.34	0.33	0.33
19.00	0.32	0.32	0.31	0.31	0.30
20.25	0.30	0.29	0.29	0.28	0.27
21.50	0.27	0.26	0.26	0.25	0.25
22.75	0.24	0.24	0.23	0.23	0.22
24.00	0.22	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

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Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	8.49 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	5.04 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.90 in
Runoff Volume (Pervious)	0.674 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.650 ac-ft
SCS Unit Hydrograph Parameter	rs .
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	29.32 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA1 - Imp - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.00	0.01	0.02	0.03
2.00	0.04	0.05	0.06	0.06	0.07
3.25	0.08	0.08	0.09	0.09	0.10
4.50	0.10	0.11	0.11	0.12	0.12
5.75	0.12	0.13	0.14	0.15	0.16
7.00	0.17	0.18	0.19	0.20	0.21
8.25	0.22	0.23	0.24	0.25	0.28
9.50	0.32	0.35	0.38	0.42	0.45
10.75	0.57	0.69	0.91	1.13	1.86
12.00	5.04	2.93	1.57	1.04	0.82
13.25	0.65	0.53	0.45	0.42	0.38
14.50	0.35	0.32	0.28	0.26	0.25
15.75	0.24	0.23	0.22	0.21	0.20
17.00	0.19	0.18	0.17	0.17	0.16
18.25	0.15	0.15	0.14	0.14	0.14
19.50	0.14	0.14	0.13	0.13	0.13
20.75	0.13	0.12	0.12	0.12	0.12
22.00	0.11	0.11	0.11	0.11	0.10
23.25	0.10	0.10	0.10	0.09	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

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Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	13.21 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	7.85 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.17 in
Runoff Volume (Pervious)	1.065 ac-ft
Hydrograph Volume (Area under	, , , , , , , , , , , , , , , , , , ,
Volume	1.027 ac-ft
SCS Unit Hydrograph Parameters	s
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	3
Unit peak, qp	29.32 ft ³ /s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA1 - Imp - Fut

Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.01	0.04	0.06	0.07
1.75	0.09	0.10	0.11	0.12	0.13
3.00	0.14	0.15	0.16	0.16	0.17
4.25	0.18	0.18	0.19	0.19	0.20
5.50	0.20	0.21	0.21	0.23	0.24
6.75	0.26	0.28	0.29	0.31	0.32
8.00	0.34	0.35	0.37	0.38	0.40
9.25	0.45	0.50	0.56	0.61	0.66
10.50	0.72	0.90	1.09	1.43	1.76
11.75	2.91	7.85	4.56	2.44	1.61
13.00	1.27	1.01	0.82	0.70	0.65
14.25	0.60	0.54	0.49	0.44	0.40
15.50	0.39	0.38	0.36	0.34	0.33
16.75	0.32	0.30	0.29	0.27	0.26
18.00	0.24	0.23	0.23	0.22	0.22
19.25	0.22	0.21	0.21	0.21	0.20
20.50	0.20	0.19	0.19	0.19	0.18
21.75	0.18	0.18	0.17	0.17	0.17
23.00	0.16	0.16	0.15	0.15	0.15

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.11 hours
Flow (Peak, Computed)	24.00 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	14.27 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	2.070 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	1.963 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	1.895 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.08 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	29.32 ft³/s
Unit peak time, Tp	0.05 hours
Unit receding limb, Tr	0.21 hours
Total unit time, Tb	0.27 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA1 - Imp - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.08 hours
Area (User Defined)	2.070 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.04	0.10	0.14	0.18
1.50	0.20	0.23	0.25	0.26	0.28
2.75	0.29	0.30	0.31	0.33	0.33
4.00	0.34	0.35	0.36	0.37	0.38
5.25	0.39	0.40	0.40	0.41	0.44
6.50	0.46	0.49	0.52	0.55	0.57
7.75	0.60	0.63	0.66	0.68	0.71
9.00	0.74	0.83	0.93	1.02	1.12
10.25	1.22	1.31	1.65	2.00	2.60
11.50	3.22	5.29	14.27	8.27	4.43
12.75	2.92	2.31	1.83	1.48	1.27
14.00	1.18	1.08	0.98	0.89	0.79
15.25	0.73	0.71	0.68	0.65	0.63
16.50	0.60	0.57	0.54	0.52	0.49
17.75	0.46	0.44	0.42	0.41	0.41
19.00	0.40	0.39	0.39	0.38	0.37
20.25	0.37	0.36	0.35	0.35	0.34
21.50	0.33	0.33	0.32	0.31	0.31
22.75	0.30	0.29	0.29	0.28	0.27
24.00	0.27	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA1 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	0.85 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.53 ft³/s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
O	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	0.75 in
Runoff Volume (Pervious)	0.074 ac-ft
Hydrograph Volume (Area und	ler Hydrograph curve)
Volume	0.071 ac-ft
SCS Unit Hydrograph Parame	ters
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	10.37 ft³/s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA1 - Per - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.50	0.00	0.04	0.29	0.53	0.27
12.75	0.19	0.16	0.13	0.11	0.09
14.00	0.09	0.08	0.08	0.07	0.06
15.25	0.06	0.06	0.05	0.05	0.05
16.50	0.05	0.05	0.04	0.04	0.04
17.75	0.04	0.04	0.04	0.03	0.03
19.00	0.03	0.03	0.03	0.03	0.03
20.25	0.03	0.03	0.03	0.03	0.03
21.50	0.03	0.03	0.03	0.03	0.03
22.75	0.03	0.03	0.03	0.03	0.02
24.00	0.02	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	2.53 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.42 ft³/s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.96 in
Runoff Volume (Pervious)	0.195 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	0.187 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	10.37 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA1 - Per - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres

rime on left represents time for mist value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.00	0.00	0.01	0.01	0.03	0.05
11.25	0.10	0.16	0.32	1.12	1.42
12.50	0.67	0.46	0.37	0.30	0.25
13.75	0.21	0.20	0.18	0.17	0.15
15.00	0.14	0.13	0.12	0.12	0.11
16.25	0.11	0.11	0.10	0.10	0.09
17.50	0.09	0.08	0.08	0.08	0.07
18.75	0.07	0.07	0.07	0.07	0.07
20.00	0.07	0.07	0.07	0.07	0.06
21.25	0.06	0.06	0.06	0.06	0.06
22.50	0.06	0.06	0.05	0.05	0.05
23.75	0.05	0.05	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years
Label: PDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	6.65 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	3.56 ft ³ /s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.07 in
Runoff Volume (Pervious)	0.503 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.483 ac-ft
SCS Unit Hydrograph Parameter	s
Time of Concentration (Composite)	0.13 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	10.37 ft ³ /s
Unit peak time, Tp	0.09 hours
Unit receding limb, Tr	0.35 hours
Total unit time, Tb	0.43 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA1 - Per - Cur

Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.13 hours
Area (User Defined)	1.190 acres

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
7.50	0.00	0.01	0.01	0.02	0.03
8.75	0.04	0.05	0.06	0.08	0.10
10.00	0.12	0.14	0.17	0.24	0.32
11.25	0.46	0.64	1.10	3.30	3.56
12.50	1.59	1.07	0.86	0.69	0.56
13.75	0.47	0.44	0.41	0.37	0.34
15.00	0.30	0.28	0.27	0.26	0.25
16.25	0.24	0.23	0.22	0.21	0.20
17.50	0.19	0.18	0.17	0.16	0.16
18.75	0.16	0.16	0.15	0.15	0.15
20.00	0.15	0.14	0.14	0.14	0.14
21.25	0.13	0.13	0.13	0.13	0.12
22.50	0.12	0.12	0.12	0.11	0.11
23.75	0.11	0.11	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: PDA1 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	1.40 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.77 ft³/s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
O	
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.12 in
Runoff Volume (Pervious)	0.111 ac-ft
Lludro group Volume / Area	Lludrograph
Hydrograph Volume (Area under	
Volume	0.106 ac-ft
SCS Unit Hydrograph Parameter	 S
Time of Concentration	
(Composite)	0.12 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA1 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	11.24 ft ³ /s
Unit peak time, Tp	0.08 hours
Unit receding limb, Tr	0.32 hours
Total unit time, Tb	0.40 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA1 - Per - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.00	0.00	0.01	0.04	0.12	0.57
12.25	0.77	0.39	0.28	0.23	0.18
13.50	0.15	0.13	0.12	0.11	0.10
14.75	0.10	0.09	0.08	0.08	0.07
16.00	0.07	0.07	0.07	0.06	0.06
17.25	0.06	0.06	0.05	0.05	0.05
18.50	0.05	0.05	0.05	0.05	0.05
19.75	0.04	0.04	0.04	0.04	0.04
21.00	0.04	0.04	0.04	0.04	0.04
22.25	0.04	0.04	0.04	0.04	0.03
23.50	0.03	0.03	0.03	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

- /	
Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	3.56 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.84 ft³/s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.67 in
Runoff Volume (Pervious)	0.264 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.254 ac-ft
SCS Unit Hydrograph Parameters	5
Time of Concentration (Composite)	0.12 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA1 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	11.24 ft ³ /s
Unit peak time, Tp	0.08 hours
Unit receding limb, Tr	0.32 hours
Total unit time, Tb	0.40 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA1 - Per - Fut

Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.25	0.00	0.00	0.01	0.02	0.03
10.50	0.05	0.07	0.11	0.17	0.26
11.75	0.50	1.68	1.84	0.88	0.60
13.00	0.49	0.39	0.32	0.27	0.25
14.25	0.24	0.22	0.20	0.18	0.16
15.50	0.16	0.15	0.15	0.14	0.14
16.75	0.13	0.12	0.12	0.11	0.11
18.00	0.10	0.10	0.10	0.09	0.09
19.25	0.09	0.09	0.09	0.09	0.09
20.50	0.08	0.08	0.08	0.08	0.08
21.75	0.08	0.08	0.07	0.07	0.07
23.00	0.07	0.07	0.07	0.06	0.06

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.13 hours
Flow (Peak, Computed)	9.32 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	4.83 ft ³ /s
Drainage Area	
SCS CN (Composite)	65.120
Area (User Defined)	1.190 acres
Maximum Retention (Pervious)	5.36 in
Maximum Retention (Pervious, 20 percent)	1.07 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	7.00 in
Runoff Volume (Pervious)	0.694 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.666 ac-ft
SCS Unit Hydrograph Parameter	ers
Time of Concentration (Composite)	0.12 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA1 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	11.24 ft ³ /s
Unit peak time, Tp	0.08 hours
Unit receding limb, Tr	0.32 hours
Total unit time, Tb	0.40 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA1 - Per - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.12 hours
Area (User Defined)	1.190 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
6.25	0.00	0.00	0.01	0.01	0.02
7.50	0.03	0.04	0.05	0.06	0.07
8.75	0.08	0.10	0.12	0.15	0.18
10.00	0.21	0.25	0.29	0.38	0.51
11.25	0.71	0.96	1.64	4.83	4.59
12.50	2.10	1.42	1.13	0.90	0.73
13.75	0.62	0.58	0.53	0.49	0.44
15.00	0.39	0.36	0.35	0.34	0.33
16.25	0.31	0.30	0.29	0.27	0.26
17.50	0.25	0.23	0.22	0.21	0.21
18.75	0.20	0.20	0.20	0.20	0.19
20.00	0.19	0.19	0.18	0.18	0.18
21.25	0.17	0.17	0.17	0.16	0.16
22.50	0.16	0.15	0.15	0.15	0.14
23.75	0.14	0.14	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years

Label: PDA2 - Imp - Cur Storm Event: Current 2-Year Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	3.31 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	1.97 ft³/s

Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.920 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in

Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.25 in
Runoff Volume (Pervious)	0.249 ac-ft

Hydrograph Volume (Area under Hydrograph curve)		
Volume	0.240 ac-ft	

	0.2.0 00.0	
SCS Unit Hydrograph Parameters		
Time of Concentration (Composite)	0.05 hours	
Computational Time Increment	0.01 hours	
Unit Hydrograph Shape Factor	483.432	
K Factor	0.749	
Receding/Rising, Tr/Tp	1.670	

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Imp - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters				
Unit peak, qp	20.85 ft ³ /s			
Unit peak time, Tp	0.03 hours			
Unit receding limb, Tr	0.13 hours			
Total unit time, Tb	0.17 hours			

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA2 - Imp - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
1.00	0.00	0.00	0.01	0.01	0.01
2.25	0.02	0.02	0.02	0.02	0.03
3.50	0.03	0.03	0.03	0.03	0.04
4.75	0.04	0.04	0.04	0.04	0.04
6.00	0.05	0.05	0.05	0.06	0.06
7.25	0.06	0.07	0.07	0.08	0.08
8.50	0.08	0.09	0.09	0.11	0.12
9.75	0.13	0.14	0.16	0.17	0.22
11.00	0.26	0.35	0.42	0.77	1.97
12.25	0.92	0.58	0.37	0.30	0.24
13.50	0.19	0.17	0.16	0.14	0.13
14.75	0.12	0.10	0.10	0.09	0.09
16.00	0.09	0.08	0.08	0.08	0.07
17.25	0.07	0.06	0.06	0.06	0.06
18.50	0.05	0.05	0.05	0.05	0.05
19.75	0.05	0.05	0.05	0.05	0.05
21.00	0.05	0.04	0.04	0.04	0.04
22.25	0.04	0.04	0.04	0.04	0.04
23.50	0.04	0.04	0.04	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10				
Return Event	10 years				
Duration	24.00 hours				
Depth	5.44 in				
Time of Concentration (Composite)	0.05 hours				
Area (User Defined)	0.920 acres				
Computational Time Increment	0.01 hours				
Time to Peak (Computed)	12.10 hours				
Flow (Peak, Computed)	5.20 ft ³ /s				
Output Increment	0.25 hours				
Time to Flow (Peak Interpolated Output)	12.00 hours				
Flow (Peak Interpolated Output)	3.11 ft³/s				
Drainage Area					
SCS CN (Composite)	98.000				
Area (User Defined)	0.920 acres				
Maximum Retention (Pervious)	0.20 in				
Maximum Retention (Pervious, 20 percent)	0.04 in				
Cumulative Runoff					
Cumulative Runoff Depth (Pervious)	5.20 in				
Runoff Volume (Pervious)	0.399 ac-ft				
Lludrograph Values / Assauration					
Hydrograph Volume (Area under					
Volume	0.385 ac-ft				
SCS Unit Hydrograph Parameter	s				
Time of Concentration (Composite)	0.05 hours				
Computational Time Increment	0.01 hours				
Unit Hydrograph Shape Factor	483.432				
K Factor	0.749				
Receding/Rising, Tr/Tp	1.670				

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Imp - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	20.85 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA2 - Imp - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.00	0.01	0.02	0.02
1.75	0.03	0.03	0.04	0.04	0.05
3.00	0.05	0.05	0.06	0.06	0.06
4.25	0.06	0.07	0.07	0.07	0.07
5.50	0.08	0.08	0.08	0.09	0.09
6.75	0.10	0.10	0.11	0.11	0.12
8.00	0.13	0.13	0.14	0.14	0.15
9.25	0.17	0.19	0.21	0.23	0.25
10.50	0.27	0.35	0.41	0.55	0.67
11.75	1.22	3.11	1.44	0.91	0.59
13.00	0.47	0.37	0.30	0.26	0.24
14.25	0.22	0.20	0.18	0.16	0.15
15.50	0.15	0.14	0.14	0.13	0.12
16.75	0.12	0.11	0.11	0.10	0.10
18.00	0.09	0.09	0.09	0.08	0.08
19.25	0.08	0.08	0.08	0.08	0.08
20.50	0.07	0.07	0.07	0.07	0.07
21.75	0.07	0.07	0.06	0.06	0.06
23.00	0.06	0.06	0.06	0.06	0.06

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

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Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	9.01 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	5.40 ft ³ /s
Drainage Area	
· · · · · · · · · · · · · · · · · · ·	98.000
SCS CN (Composite) Area (User Defined)	96.000 0.920 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	9.16 in
Runoff Volume (Pervious)	0.702 ac-ft
Hydrograph Volume (Area unde	ar Hydrograph curve)
Volume	0.678 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Imp - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	20.85 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA2 - Imp - Cur

Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

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Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.01	0.03	0.04	0.06
1.50	0.07	0.07	0.08	0.09	0.09
2.75	0.10	0.10	0.11	0.11	0.12
4.00	0.12	0.12	0.13	0.13	0.13
5.25	0.14	0.14	0.14	0.15	0.16
6.50	0.17	0.18	0.19	0.20	0.21
7.75	0.22	0.23	0.24	0.24	0.26
9.00	0.27	0.30	0.33	0.37	0.40
10.25	0.44	0.47	0.61	0.72	0.97
11.50	1.17	2.12	5.40	2.50	1.58
12.75	1.02	0.82	0.64	0.53	0.45
14.00	0.42	0.38	0.35	0.31	0.28
15.25	0.26	0.25	0.24	0.23	0.22
16.50	0.21	0.20	0.20	0.18	0.18
17.75	0.17	0.16	0.15	0.15	0.15
19.00	0.14	0.14	0.14	0.14	0.13
20.25	0.13	0.13	0.13	0.12	0.12
21.50	0.12	0.12	0.12	0.11	0.11
22.75	0.11	0.10	0.10	0.10	0.10
24.00	0.10	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	3.95 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	2.36 ft³/s
Drainage Area	
	00.000
SCS CN (Composite)	98.000
Area (User Defined)	0.920 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.90 in
Runoff Volume (Pervious)	0.299 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.289 ac-ft
SCS Unit Hydrograph Parameter	re
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Imp - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	20.85 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA2 - Imp - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.75	0.00	0.00	0.01	0.01	0.02
2.00	0.02	0.02	0.03	0.03	0.03
3.25	0.03	0.04	0.04	0.04	0.04
4.50	0.05	0.05	0.05	0.05	0.05
5.75	0.06	0.06	0.06	0.07	0.07
7.00	0.07	0.08	0.08	0.09	0.09
8.25	0.10	0.10	0.11	0.11	0.13
9.50	0.14	0.16	0.17	0.19	0.20
10.75	0.26	0.31	0.42	0.51	0.92
12.00	2.36	1.10	0.69	0.45	0.36
13.25	0.28	0.23	0.20	0.18	0.17
14.50	0.15	0.14	0.12	0.12	0.11
15.75	0.11	0.10	0.10	0.09	0.09
17.00	0.09	0.08	0.08	0.07	0.07
18.25	0.07	0.07	0.06	0.06	0.06
19.50	0.06	0.06	0.06	0.06	0.06
20.75	0.06	0.05	0.05	0.05	0.05
22.00	0.05	0.05	0.05	0.05	0.05
23.25	0.04	0.04	0.04	0.04	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	6.14 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	3.67 ft ³ /s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.920 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	6.17 in
Runoff Volume (Pervious)	0.473 ac-ft
Hydrograph Volume (Area under H	ydrograph curve)
Volume	0.457 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	20.85 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 10 years
Label: PDA2 - Imp - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time on left represents time for mist value in each row.					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.50	0.00	0.01	0.02	0.03	0.03
1.75	0.04	0.04	0.05	0.05	0.06
3.00	0.06	0.07	0.07	0.07	0.08
4.25	0.08	0.08	0.08	0.09	0.09
5.50	0.09	0.09	0.10	0.10	0.11
6.75	0.12	0.12	0.13	0.14	0.14
8.00	0.15	0.16	0.16	0.17	0.18
9.25	0.20	0.22	0.25	0.27	0.30
10.50	0.32	0.41	0.49	0.66	0.79
11.75	1.44	3.67	1.70	1.08	0.69
13.00	0.56	0.44	0.36	0.31	0.29
14.25	0.26	0.24	0.21	0.19	0.18
15.50	0.17	0.17	0.16	0.15	0.15
16.75	0.14	0.13	0.13	0.12	0.11
18.00	0.11	0.10	0.10	0.10	0.10
19.25	0.10	0.10	0.09	0.09	0.09
20.50	0.09	0.09	0.08	0.08	0.08
21.75	0.08	0.08	0.08	0.07	0.07
23.00	0.07	0.07	0.07	0.07	0.07

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres
Computational Time Increment	0.01 hours
Time to Peak (Computed)	12.10 hours
Flow (Peak, Computed)	11.15 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.00 hours
Flow (Peak Interpolated Output)	6.68 ft³/s
Drainage Area	
SCS CN (Composite)	98.000
Area (User Defined)	0.920 acres
Maximum Retention (Pervious)	0.20 in
Maximum Retention (Pervious, 20 percent)	0.04 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	11.38 in
Runoff Volume (Pervious)	0.872 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	0.843 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.05 hours
Computational Time Increment	0.01 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Imp - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	20.85 ft ³ /s
Unit peak time, Tp	0.03 hours
Unit receding limb, Tr	0.13 hours
Total unit time, Tb	0.17 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA2 - Imp - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.05 hours
Area (User Defined)	0.920 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

rime on leterapieseries and for more value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
0.25	0.00	0.02	0.05	0.07	0.08
1.50	0.09	0.10	0.11	0.12	0.12
2.75	0.13	0.13	0.14	0.15	0.15
4.00	0.15	0.16	0.16	0.17	0.17
5.25	0.17	0.18	0.18	0.18	0.20
6.50	0.21	0.22	0.23	0.25	0.26
7.75	0.27	0.28	0.29	0.30	0.32
9.00	0.33	0.38	0.41	0.46	0.50
10.25	0.55	0.58	0.76	0.90	1.20
11.50	1.44	2.62	6.68	3.09	1.95
12.75	1.26	1.01	0.79	0.65	0.56
14.00	0.52	0.47	0.44	0.39	0.35
15.25	0.32	0.31	0.30	0.29	0.28
16.50	0.27	0.25	0.24	0.23	0.22
17.75	0.20	0.19	0.19	0.18	0.18
19.00	0.18	0.17	0.17	0.17	0.17
20.25	0.16	0.16	0.16	0.15	0.15
21.50	0.15	0.14	0.14	0.14	0.14
22.75	0.13	0.13	0.13	0.12	0.12
24.00	0.12	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Per - Cur Storm Event: Current 2-Year

Label: PDA2 - Per - Cur Scenario: Current 2 year

Storm Event	Current 2 Vear
Storm Event	Current 2-Year
Return Event	2 years
Duration Depth	24.00 hours 3.48 in
Time of Concentration	J. 10 III
(Composite)	0.17 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.17 hours
Flow (Peak, Computed)	0.56 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.41 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.05 in
Runoff Volume (Pervious)	0.049 ac-ft
,	
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.048 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Per - Cur Storm Event: Current 2-Year

Scenario: Current 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.73 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA2 - Per - Cur

Storm Event: Current 2-Year

Scenario: Current 2 year

Storm Event	Current 2-Year
Return Event	2 years
Duration	24.00 hours
Depth	3.48 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.75	0.00	0.00	0.01	0.03	0.06
12.00	0.22	0.41	0.18	0.12	0.10
13.25	0.08	0.07	0.06	0.05	0.05
14.50	0.05	0.04	0.04	0.03	0.03
15.75	0.03	0.03	0.03	0.03	0.03
17.00	0.03	0.03	0.02	0.02	0.02
18.25	0.02	0.02	0.02	0.02	0.02
19.50	0.02	0.02	0.02	0.02	0.02
20.75	0.02	0.02	0.02	0.02	0.02
22.00	0.02	0.02	0.02	0.02	0.01
23.25	0.01	0.01	0.01	0.01	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	1.37 ft³/s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.97 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	2.45 in
Runoff Volume (Pervious)	0.114 ac-ft
Hydrograph Volume (Area unde	er Hydrograph curve)
Volume	0.112 ac-ft
SCS Unit Hydrograph Paramete	ers
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670
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Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Per - Cur Storm Event: Current 10

Scenario: Current 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.73 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 10 years

Label: PDA2 - Per - Cur

Storm Event: Current 10

Scenario: Current 10 year

Storm Event	Current 10
Return Event	10 years
Duration	24.00 hours
Depth	5.44 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
9.00	0.00	0.00	0.01	0.01	0.01
10.25	0.02	0.03	0.04	0.05	0.08
11.50	0.12	0.21	0.61	0.97	0.41
12.75	0.27	0.21	0.17	0.14	0.12
14.00	0.11	0.10	0.09	0.08	0.08
15.25	0.07	0.07	0.06	0.06	0.06
16.50	0.06	0.05	0.05	0.05	0.05
17.75	0.04	0.04	0.04	0.04	0.04
19.00	0.04	0.04	0.04	0.04	0.04
20.25	0.04	0.04	0.03	0.03	0.03
21.50	0.03	0.03	0.03	0.03	0.03
22.75	0.03	0.03	0.03	0.03	0.03
24.00	0.03	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.15 hours
Flow (Peak, Computed)	3.25 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	2.23 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	5.81 in
Runoff Volume (Pervious)	0.271 ac-ft
11.1. 1.1. 1.1.	
Hydrograph Volume (Area und	er Hydrograph curve)
Volume	0.264 ac-ft
SCS Unit Hydrograph Paramet	ers
Time of Concentration (Composite)	0.17 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Per - Cur Storm Event: Current 100

Scenario: Current 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.73 ft³/s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.45 hours
Total unit time, Tb	0.57 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA2 - Per - Cur

Storm Event: Current 100

Scenario: Current 100 year

Storm Event	Current 100
Return Event	100 years
Duration	24.00 hours
Depth	9.40 in
Time of Concentration (Composite)	0.17 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
6.25	0.00	0.00	0.00	0.01	0.01
7.50	0.01	0.02	0.02	0.03	0.03
8.75	0.04	0.04	0.05	0.06	0.07
10.00	0.08	0.10	0.11	0.15	0.20
11.25	0.27	0.37	0.60	1.58	2.23
12.50	0.89	0.57	0.45	0.36	0.29
13.75	0.24	0.23	0.21	0.19	0.17
15.00	0.16	0.14	0.14	0.13	0.13
16.25	0.12	0.12	0.11	0.11	0.10
17.50	0.10	0.09	0.09	0.08	0.08
18.75	0.08	0.08	0.08	0.08	0.07
20.00	0.07	0.07	0.07	0.07	0.07
21.25	0.07	0.07	0.06	0.06	0.06
22.50	0.06	0.06	0.06	0.06	0.06
23.75	0.05	0.05	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 2 years
Label: PDA2 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.16 hours
Flow (Peak, Computed)	0.83 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	0.57 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	1.49 in
Runoff Volume (Pervious)	0.069 ac-ft
Hydrograph Volume (Area under F	lydrograph curve)
Volume	0.067 ac-ft
SCS Unit Hydrograph Parameters	
Time of Concentration (Composite)	0.16 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 2 years Label: PDA2 - Per - Fut Storm Event: Future 2

Scenario: Future 2 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.97 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.43 hours
Total unit time, Tb	0.53 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 2 years

Label: PDA2 - Per - Fut

Storm Event: Future 2

Scenario: Future 2 year

Storm Event	Future 2
Return Event	2 years
Duration	24.00 hours
Depth	4.14 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

rime on fert represents time for mot value in each rown					
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.00	0.00	0.00	0.00	0.01	0.02
11.25	0.03	0.05	0.11	0.35	0.57
12.50	0.25	0.17	0.14	0.11	0.09
13.75	0.08	0.07	0.07	0.06	0.06
15.00	0.05	0.05	0.04	0.04	0.04
16.25	0.04	0.04	0.04	0.03	0.03
17.50	0.03	0.03	0.03	0.03	0.03
18.75	0.03	0.03	0.03	0.03	0.02
20.00	0.02	0.02	0.02	0.02	0.02
21.25	0.02	0.02	0.02	0.02	0.02
22.50	0.02	0.02	0.02	0.02	0.02
23.75	0.02	0.02	(N/A)	(N/A)	(N/A)

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

- /	
Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.14 hours
Flow (Peak, Computed)	1.84 ft³/s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	1.23 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	3.23 in
Runoff Volume (Pervious)	0.151 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.146 ac-ft
SCS Unit Hydrograph Parameters	5
Time of Concentration (Composite)	0.16 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 10 years Label: PDA2 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.97 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.43 hours
Total unit time, Tb	0.53 hours

Subsection: Unit Hydrograph (Hydrograph Table) Return Event: 10 years
Label: PDA2 - Per - Fut Storm Event: Future 10

Scenario: Future 10 year

Storm Event	Future 10
Return Event	10 years
Duration	24.00 hours
Depth	6.41 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
8.00	0.00	0.00	0.00	0.01	0.01
9.25	0.01	0.02	0.02	0.03	0.04
10.50	0.04	0.06	0.08	0.12	0.17
11.75	0.30	0.87	1.23	0.51	0.34
13.00	0.27	0.21	0.18	0.15	0.14
14.25	0.13	0.12	0.11	0.09	0.09
15.50	0.08	0.08	0.08	0.07	0.07
16.75	0.07	0.07	0.06	0.06	0.06
18.00	0.05	0.05	0.05	0.05	0.05
19.25	0.05	0.05	0.05	0.05	0.04
20.50	0.04	0.04	0.04	0.04	0.04
21.75	0.04	0.04	0.04	0.04	0.04
23.00	0.04	0.04	0.03	0.03	0.03

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres
Computational Time Increment	0.02 hours
Time to Peak (Computed)	12.14 hours
Flow (Peak, Computed)	4.41 ft ³ /s
Output Increment	0.25 hours
Time to Flow (Peak Interpolated Output)	12.25 hours
Flow (Peak Interpolated Output)	2.84 ft³/s
Drainage Area	
SCS CN (Composite)	70.960
Area (User Defined)	0.560 acres
Maximum Retention (Pervious)	4.09 in
Maximum Retention (Pervious, 20 percent)	0.82 in
Cumulative Runoff	
Cumulative Runoff Depth (Pervious)	7.83 in
Runoff Volume (Pervious)	0.366 ac-ft
Hydrograph Volume (Area under	Hydrograph curve)
Volume	0.355 ac-ft
SCS Unit Hydrograph Parameter	s
Time of Concentration (Composite)	0.16 hours
Computational Time Increment	0.02 hours
Unit Hydrograph Shape Factor	483.432
K Factor	0.749
Receding/Rising, Tr/Tp	1.670

Subsection: Unit Hydrograph Summary Return Event: 100 years Label: PDA2 - Per - Fut Storm Event: Future 100

Scenario: Future 100 year

SCS Unit Hydrograph Parameters	
Unit peak, qp	3.97 ft ³ /s
Unit peak time, Tp	0.11 hours
Unit receding limb, Tr	0.43 hours
Total unit time, Tb	0.53 hours

Subsection: Unit Hydrograph (Hydrograph Table)

Return Event: 100 years

Label: PDA2 - Per - Fut

Storm Event: Future 100

Scenario: Future 100 year

Storm Event	Future 100
Return Event	100 years
Duration	24.00 hours
Depth	11.62 in
Time of Concentration (Composite)	0.16 hours
Area (User Defined)	0.560 acres

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.25 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
5.25	0.00	0.00	0.01	0.01	0.01
6.50	0.01	0.02	0.02	0.03	0.03
7.75	0.04	0.04	0.05	0.06	0.06
9.00	0.07	0.08	0.10	0.11	0.13
10.25	0.15	0.18	0.22	0.29	0.40
11.50	0.53	0.86	2.22	2.84	1.14
12.75	0.74	0.59	0.46	0.38	0.31
14.00	0.29	0.27	0.24	0.22	0.20
15.25	0.18	0.17	0.17	0.16	0.16
16.50	0.15	0.14	0.14	0.13	0.12
17.75	0.12	0.11	0.10	0.10	0.10
19.00	0.10	0.10	0.10	0.10	0.09
20.25	0.09	0.09	0.09	0.09	0.09
21.50	0.08	0.08	0.08	0.08	0.08
22.75	0.08	0.07	0.07	0.07	0.07
24.00	0.07	(N/A)	(N/A)	(N/A)	(N/A)

Subsection: Addition Summary Return Event: 2 years
Label: Post - POS1 Storm Event: Current 2-Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Cur

Node Inflows

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	0.540	12.00	4.22
Flow (From)	PDA1 - Per - Cur	0.071	12.25	0.53
Flow (In)	Post - POS1	0.611	12.00	4.51

Subsection: Addition Summary Return Event: 2 years
Label: Post - POS1 Storm Event: Future 2

Scenario: Future 2 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Fut

Node Inflows

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	0.650	12.00	5.04
Flow (From)	PDA1 - Per - Fut	0.106	12.25	0.77
Flow (In)	Post - POS1	0.756	12.00	5.61

Subsection: Addition Summary Return Event: 10 years
Label: Post - POS1 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Cur

Node Inflows

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	0.866	12.00	6.65
Flow (From)	PDA1 - Per - Cur	0.187	12.25	1.42
Flow (In)	Post - POS1	1.053	12.00	7.77

Subsection: Addition Summary Return Event: 10 years
Label: Post - POS1 Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	1.027	12.00	7.85
Flow (From)	PDA1 - Per - Fut	0.254	12.25	1.84
Flow (In)	Post - POS1	1.281	12.00	9.53

Subsection: Addition Summary Return Event: 100 years
Label: Post - POS1 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Cur	1.525	12.00	11.54
Flow (From)	PDA1 - Per - Cur	0.483	12.25	3.56
Flow (In)	Post - POS1	2.009	12.00	14.84

Subsection: Addition Summary Return Event: 100 years
Label: Post - POS1 Storm Event: Future 100

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Post - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA1 - Imp - Fut	1.895	12.00	14.27
Flow (From)	PDA1 - Per - Fut	0.666	12.00	4.83
Flow (In)	Post - POS1	2.562	12.00	19.10

Subsection: Addition Summary Return Event: 2 years
Label: Post - POS2 Storm Event: Current 2-Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.240	12.00	1.97
Flow (From)	PDA2 - Per - Cur	0.048	12.25	0.41
Flow (In)	Post - POS2	0.288	12.00	2.19

Subsection: Addition Summary Return Event: 2 years
Label: Post - POS2 Storm Event: Future 2

Scenario: Future 2 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.289	12.00	2.36
Flow (From)	PDA2 - Per - Fut	0.067	12.25	0.57
Flow (In)	Post - POS2	0.356	12.00	2.71

Subsection: Addition Summary Return Event: 10 years Label: Post - POS2 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.385	12.00	3.11
Flow (From)	PDA2 - Per - Cur	0.112	12.25	0.97
Flow (In)	Post - POS2	0.497	12.00	3.72

Subsection: Addition Summary Return Event: 10 years
Label: Post - POS2 Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.457	12.00	3.67
Flow (From)	PDA2 - Per - Fut	0.146	12.25	1.23
Flow (In)	Post - POS2	0.603	12.00	4.54

Subsection: Addition Summary Return Event: 100 years
Label: Post - POS2 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Cur	0.678	12.00	5.40
Flow (From)	PDA2 - Per - Cur	0.264	12.25	2.23
Flow (In)	Post - POS2	0.943	12.00	6.97

Subsection: Addition Summary Return Event: 100 years Label: Post - POS2 Storm Event: Future 100

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Post - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	PDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	PDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	PDA2 - Imp - Fut	0.843	12.00	6.68
Flow (From)	PDA2 - Per - Fut	0.355	12.25	2.84
Flow (In)	Post - POS2	1.198	12.00	8.90

Subsection: Addition Summary Return Event: 2 years
Label: Pre - POS1 Storm Event: Current 2-Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	0.610	12.00	4.77
Flow (From)	EDA1 - Per - Cur	0.056	12.25	0.42
Flow (In)	Pre - POS1	0.667	12.00	5.02

Subsection: Addition Summary Return Event: 2 years
Label: Pre - POS1 Storm Event: Future 2

Scenario: Future 2 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	0.734	12.00	5.70
Flow (From)	EDA1 - Per - Fut	0.083	12.25	0.63
Flow (In)	Pre - POS1	0.818	12.00	6.14

Subsection: Addition Summary Return Event: 10 years
Label: Pre - POS1 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	0.979	12.00	7.52
Flow (From)	EDA1 - Per - Cur	0.145	12.25	1.10
Flow (In)	Pre - POS1	1.123	12.00	8.40

Subsection: Addition Summary Return Event: 10 years
Label: Pre - POS1 Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	1.161	12.00	8.87
Flow (From)	EDA1 - Per - Fut	0.195	12.25	1.47
Flow (In)	Pre - POS1	1.357	12.00	10.13

Subsection: Addition Summary Return Event: 100 years
Label: Pre - POS1 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Cur	1.724	12.00	13.04
Flow (From)	EDA1 - Per - Cur	0.367	12.25	2.69
Flow (In)	Pre - POS1	2.092	12.00	15.56

Subsection: Addition Summary Return Event: 100 years
Label: Pre - POS1 Storm Event: Future 100

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Pre - POS1'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA1 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA1 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA1 - Imp - Fut	2.143	12.00	16.13
Flow (From)	EDA1 - Per - Fut	0.505	12.25	3.63
Flow (In)	Pre - POS1	2.647	12.00	19.66

Subsection: Addition Summary Return Event: 2 years
Label: Pre - POS2 Storm Event: Current 2-Year

Scenario: Current 2 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.243	12.00	2.00
Flow (From)	EDA2 - Per - Cur	0.048	12.25	0.42
Flow (In)	Pre - POS2	0.291	12.00	2.19

Subsection: Addition Summary Return Event: 2 years
Label: Pre - POS2 Storm Event: Future 2

Scenario: Future 2 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.292	12.00	2.38
Flow (From)	EDA2 - Per - Fut	0.068	12.25	0.60
Flow (In)	Pre - POS2	0.360	12.00	2.72

Subsection: Addition Summary Return Event: 10 years
Label: Pre - POS2 Storm Event: Current 10

Scenario: Current 10 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.389	12.00	3.14
Flow (From)	EDA2 - Per - Cur	0.114	12.25	1.02
Flow (In)	Pre - POS2	0.503	12.00	3.74

Subsection: Addition Summary Return Event: 10 years
Label: Pre - POS2 Storm Event: Future 10

Scenario: Future 10 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.462	12.00	3.71
Flow (From)	EDA2 - Per - Fut	0.150	12.25	1.31
Flow (In)	Pre - POS2	0.612	12.00	4.56

Subsection: Addition Summary Return Event: 100 years
Label: Pre - POS2 Storm Event: Current 100

Scenario: Current 100 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Cur
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Cur

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Cur	0.686	12.00	5.45
Flow (From)	EDA2 - Per - Cur	0.274	12.25	2.39
Flow (In)	Pre - POS2	0.959	12.00	7.01

Subsection: Addition Summary Return Event: 100 years
Label: Pre - POS2 Storm Event: Future 100

Scenario: Future 100 year

Summary for Hydrograph Addition at 'Pre - POS2'

Upstream Link	Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA2 - Imp - Fut
<catchment node="" outflow="" to=""></catchment>	EDA2 - Per - Fut

Inflow Type	Element	Volume (ac-ft)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA2 - Imp - Fut	0.852	12.00	6.75
Flow (From)	EDA2 - Per - Fut	0.369	12.25	3.07
Flow (In)	Pre - POS2	1.221	12.00	8.97

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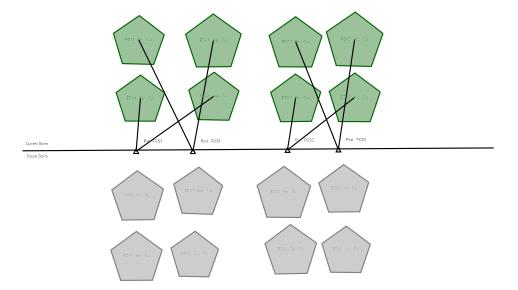
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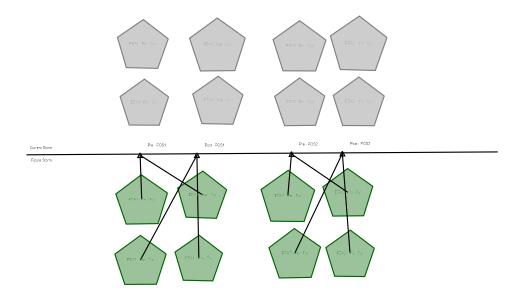
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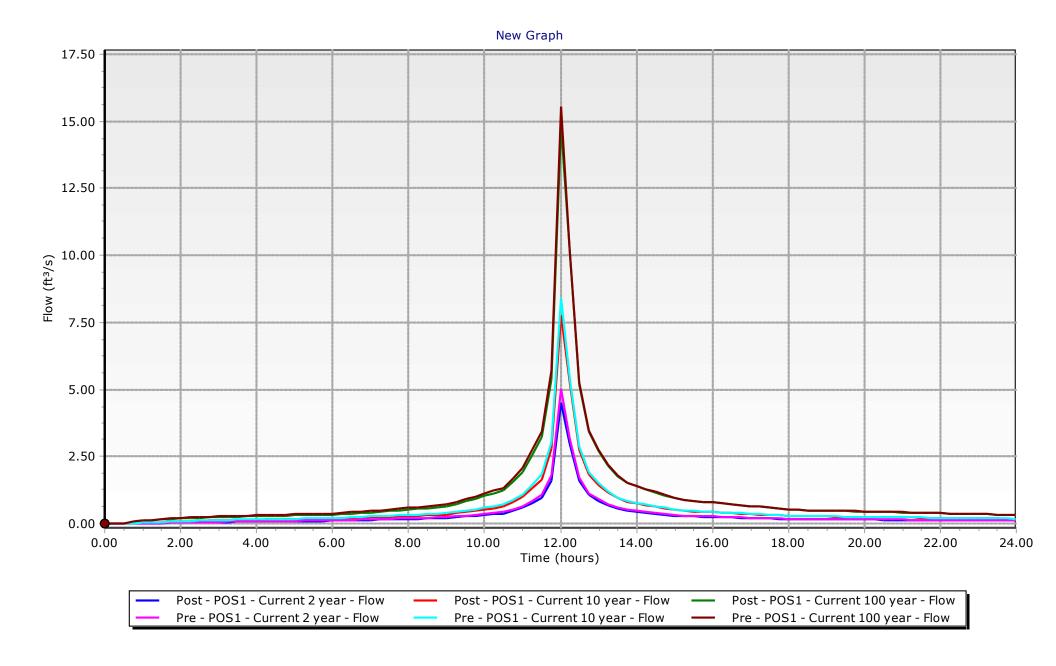
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Scenario: Current 2 year



Scenario: Future 2 year

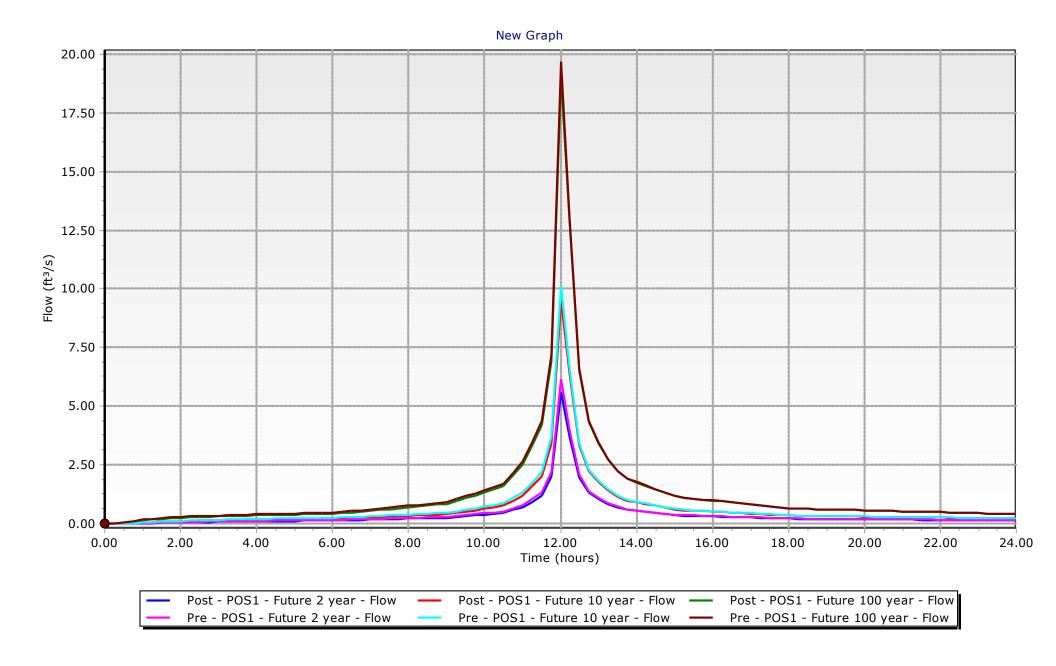




West Addition Current Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1 2 Year Storm Event 10-Year Storm Event 100-Year Storm Event										
Time	Pre-Dev	Year Storm Ever	nt Reduction	10-Year Storm Event Pre-Dev Post-Dev Reduction			Pre-Dev Post-Dev Reduction			
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.00	
0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.06	0.01	
1.00	0.00	0.00	0.00	0.02	0.02	0.00	0.11	0.09	0.02	
1.25	0.01	0.01	0.00	0.04	0.04	0.00	0.14	0.12	0.02	
1.50	0.02	0.01	0.01	0.06	0.05	0.01	0.17	0.15	0.02	
1.75	0.02	0.02	0.00	0.07	0.06	0.01	0.19	0.17	0.02	
2.00	0.03	0.03	0.00	0.08	0.07	0.01	0.21	0.18	0.03	
2.25	0.04	0.04	0.00	0.10	0.08	0.02	0.22	0.20	0.02	
2.50	0.05	0.04	0.01	0.11	0.09	0.02	0.24	0.21	0.03	
2.75	0.05	0.05	0.00	0.12	0.10	0.02	0.25	0.22	0.03	
3.00	0.06	0.05	0.01	0.12	0.11	0.01	0.26	0.23	0.03	
3.25	0.07	0.06	0.01	0.13	0.12	0.01	0.27	0.24	0.03	
3.50	0.07	0.06	0.01	0.14	0.12	0.02	0.29	0.25	0.04	
3.75	0.08	0.07	0.01	0.15	0.13	0.02	0.30	0.26	0.04	
4.00	0.08	0.07	0.01	0.15	0.14	0.01	0.31	0.27	0.04	
4.25	0.09	0.08	0.01	0.16	0.14	0.02	0.31	0.28	0.03	
4.50	0.09	0.08	0.01	0.17	0.15	0.02	0.32	0.29	0.03	
4.75	0.09	0.08	0.01	0.17	0.15	0.02	0.33	0.29	0.04	
5.00	0.10	0.09	0.01	0.18	0.16	0.02	0.34	0.30	0.04	
5.25	0.10	0.09	0.01	0.18	0.16	0.02	0.35	0.31	0.04	
5.50	0.11	0.10	0.01	0.19	0.17	0.02	0.36	0.31	0.05	
5.75	0.11	0.10	0.01	0.20	0.17	0.03	0.36	0.32	0.04	
6.00	0.11	0.10	0.01	0.20	0.18	0.02	0.37	0.33	0.04	
6.25	0.12	0.11	0.01	0.21	0.19	0.02	0.40	0.35	0.05	
6.50	0.13	0.12	0.01	0.23	0.20	0.03	0.42	0.37	0.05	
6.75	0.14	0.13	0.01	0.24	0.22	0.02	0.44	0.39	0.05	
7.00	0.15	0.14	0.01	0.26	0.23	0.03	0.47	0.42	0.05	
7.25	0.16	0.14	0.02	0.27	0.24	0.03	0.50	0.44	0.06	
7.50	0.17	0.15	0.02	0.29	0.26	0.03	0.53	0.46	0.07	
7.75	0.18	0.16	0.02	0.30	0.27	0.03	0.56	0.49	0.07	
8.00	0.19	0.17	0.02	0.32	0.28	0.04	0.59	0.52	0.07	
8.25	0.20	0.18	0.02	0.33	0.30	0.03	0.62	0.55	0.07	
8.50	0.21	0.19	0.02	0.35	0.31	0.04	0.65	0.58	0.07	
8.75	0.22	0.20	0.02	0.37	0.32	0.05	0.68	0.61	0.07	
9.00	0.23	0.21	0.02	0.38	0.34	0.04	0.71	0.64	0.07	
9.25	0.26	0.23	0.03	0.43	0.38	0.05	0.81	0.73	0.08	
9.50	0.29	0.26	0.03	0.48	0.42	0.06	0.91	0.82	0.09	
9.75	0.33	0.29	0.04	0.53	0.47	0.06	1.01	0.92	0.09	
10.00	0.36	0.32	0.04	0.58	0.51	0.07	1.12	1.02	0.10	
10.25	0.39	0.35	0.04	0.64	0.56	0.08	1.22	1.13	0.09	
10.50	0.43	0.38	0.05	0.70	0.62	0.08	1.33	1.23	0.10	
10.75	0.54	0.48	0.06	0.89	0.79	0.10	1.69	1.57	0.12	
11.00	0.65	0.58	0.07	1.09	0.98	0.11	2.07	1.93	0.14	
11.25	0.86	0.76	0.10	1.44	1.30	0.14	2.73	2.56	0.17	
11.50	1.07	0.94	0.13	1.82	1.65	0.17	3.43	3.24	0.19	
11.75	1.80	1.59	0.21	3.04	2.78	0.26	5.68	5.38	0.30	
12.00	5.02	4.51	0.51	8.40	7.77	0.63	15.56	14.84	0.72	
12.25	3.20	2.99	0.21	5.46	5.29	0.17	10.26	10.25	0.01	
12.50	1.70	1.59	0.11	2.85	2.74	0.11	5.25	5.17	0.08	
12.75	1.13	1.06	0.07	1.90	1.83	0.07	3.48	3.44	0.04	
13.00	0.90	0.85	0.05	1.51	1.45	0.06	2.76	2.73	0.03	
13.25	0.72	0.68	0.04	1.20	1.16	0.04	2.19	2.17	0.02	

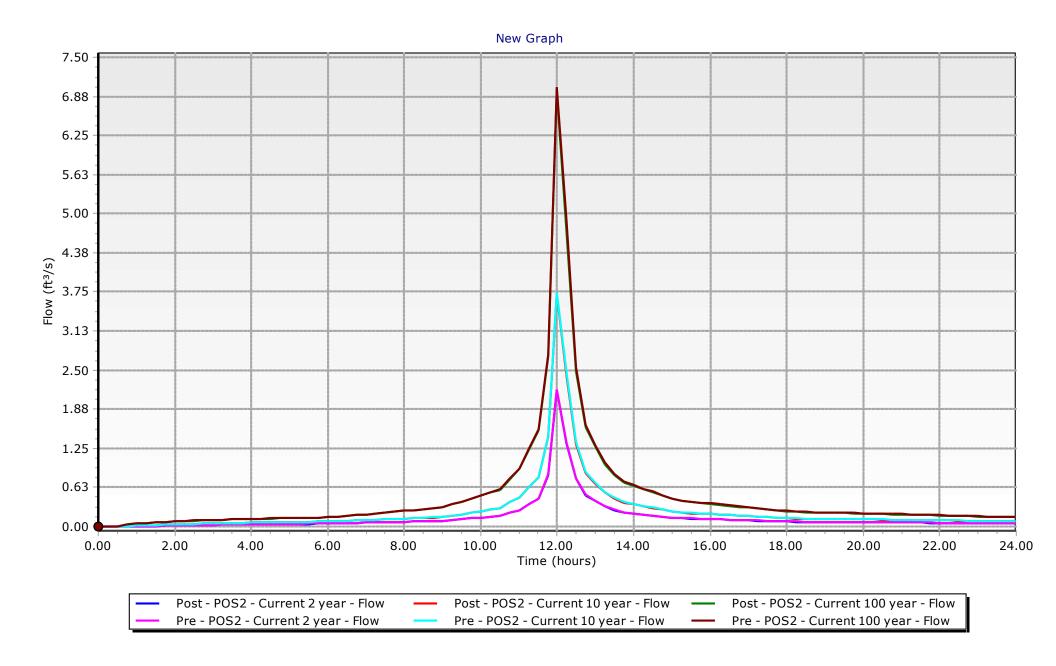
13.50 0.58 0.55 0.03 0.97 0.94 0.03 1.78 1.76 0.02 13.75 0.50 0.47 0.03 0.83 0.81 0.02 1.52 1.50 0.02 14.00 0.46 0.44 0.02 0.77 0.75 0.02 1.40 1.39 0.01 14.15 0.43 0.40 0.03 0.71 0.69 0.02 1.29 1.28 0.01 14.50 0.39 0.37 0.02 0.65 0.63 0.02 1.18 1.17 0.01 14.75 0.35 0.33 0.02 0.59 0.57 0.02 1.07 1.06 0.01 15.00 0.32 0.30 0.02 0.59 0.57 0.02 1.07 1.06 0.01 15.25 0.29 0.28 0.01 0.48 0.47 0.01 0.88 0.87 0.01 15.55 0.28 0.27 0.01 0.47 0.45 0.02 0.85 0.84 0.01 15.75 0.27 0.26 0.01 0.45 0.44 0.01 0.82 0.81 0.01 16.00 0.26 0.25 0.01 0.43 0.42 0.01 0.75 0.74 0.01 16.50 0.24 0.23 0.01 0.43 0.42 0.01 0.75 0.74 0.01 16.50 0.24 0.23 0.01 0.40 0.39 0.01 0.75 0.74 0.01 16.75 0.22 0.21 0.01 0.36 0.35 0.01 0.75 0.74 0.01 16.75 0.23 0.22 0.01 0.36 0.35 0.01 0.75 0.74 0.01 17.26 0.22 0.21 0.01 0.36 0.35 0.01 0.66 0.65 0.01 17.75 0.23 0.02 0.01 0.34 0.33 0.01 0.66 0.65 0.01 17.75 0.21 0.20 0.01 0.34 0.33 0.01 0.66 0.65 0.01 17.75 0.29 0.19 0.01 0.34 0.33 0.01 0.62 0.62 0.00 17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.59 0.59 0.00 17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.56 0.56 0.00 17.75 0.19 0.18 0.01 0.27 0.28 0.01 0.59 0.59 0.00 18.75 0.16 0.16 0.01 0.27 0.26 0.01 0.48 0.49 0.01 18.75 0.16 0.16 0.16 0.01 0.27 0.26 0.01 0.49 0.49 0.00 19.70 0.16 0.16 0.10 0.28 0.27 0.01 0.49 0.49 0.00 19.75 0.16 0.16 0.16 0.01 0.27 0.27 0.26 0.01 0.48 0.44 0.00 19.75 0.16 0.16 0.15 0.01 0.26 0.25 0.01 0.48 0.44 0.00 19.75 0.16 0.16 0.16 0.01 0.24 0.										
14.00	13.50	0.58	0.55	0.03	0.97	0.94	0.03	1.78	1.76	0.02
14.25	13.75	0.50	0.47	0.03	0.83	0.81	0.02	1.52	1.50	0.02
14.50	14.00	0.46	0.44	0.02	0.77	0.75	0.02	1.40	1.39	0.01
14.75	14.25	0.43	0.40	0.03	0.71	0.69	0.02	1.29	1.28	0.01
15.00	14.50	0.39	0.37	0.02	0.65	0.63	0.02	1.18	1.17	0.01
15.25	14.75	0.35	0.33	0.02	0.59	0.57	0.02	1.07	1.06	0.01
15.50	15.00	0.32	0.30	0.02	0.52	0.51	0.01	0.95	0.94	0.01
15.75	15.25	0.29	0.28	0.01	0.48	0.47	0.01	0.88	0.87	0.01
16.00		0.28	0.27	0.01	0.47	0.45	0.02	0.85	0.84	0.01
16.25	15.75	0.27	0.26	0.01	0.45	0.44	0.01	0.82	0.81	0.01
16.50	16.00	0.26	0.25	0.01	0.43	0.42	0.01	0.78	0.78	0.00
16.75	16.25	0.25	0.24	0.01	0.41	0.40	0.01	0.75	0.74	0.01
17.00 0.22 0.21 0.01 0.36 0.35 0.01 0.66 0.65 0.01 17.25 0.21 0.20 0.01 0.34 0.33 0.01 0.62 0.62 0.00 17.50 0.20 0.19 0.01 0.33 0.32 0.01 0.59 0.59 0.00 17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.56 0.56 0.00 18.00 0.18 0.17 0.01 0.29 0.28 0.01 0.53 0.52 0.01 18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.75 0.16 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.26 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.01 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.43 0.43 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.01 0.43 0.43 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.43 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 21.25 0.14 0.13 0.01 0.23 0.23 0.01 0.43 0.44 0.00 21.25 0.15 0.14 0.13 0.01 0.23 0.23 0.01 0.43 0.44 0.00 21.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.25 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.12 0.01 0.21 0.21 0.00 0.35 0.38 0.00 22.25 0.13 0.12 0.01 0.21 0.20 0.19 0.01 0.35 0.35 0.00 22.25 0.13 0.12 0.01 0.21 0.20 0.19 0.01 0.35 0.35 0.00 22.25 0.13 0.12 0.01 0.21 0.20 0.19 0.01 0.35 0.35 0.00 22.25 0.13 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 22.25 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00 22.25 0.11 0.11 0.01 0.	16.50	0.24	0.23	0.01	0.40	0.39	0.01	0.72	0.71	0.01
17.25 0.21 0.20 0.01 0.34 0.33 0.01 0.62 0.62 0.00 17.50 0.20 0.19 0.01 0.33 0.32 0.01 0.59 0.59 0.00 17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.56 0.56 0.00 18.00 0.18 0.17 0.01 0.29 0.28 0.01 0.53 0.52 0.01 18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.50 0.17 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.01 0.47 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.75 0.14 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.01 0.43 0.42 0.01 21.00 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.55 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.55 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.55 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.10 0.21 0.21 0.20 0.01 0.33 0.38 0.01 22.25 0.13 0.12 0.11 0.01 0.21 0.20 0.01 0.35 0.35 0.36 0.00 22.25 0.15 0.11 0.11 0.00 0.19 0.	16.75	0.23	0.22	0.01	0.38	0.37	0.01	0.69	0.68	0.01
17.50 0.20 0.19 0.01 0.33 0.32 0.01 0.59 0.59 0.00 17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.56 0.56 0.00 18.00 0.18 0.17 0.01 0.29 0.28 0.01 0.53 0.52 0.01 18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.50 0.17 0.16 0.01 0.27 0.27 0.20 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.48 0.48 0.00 19.00 0.16 0.15 0.01 0.26 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.01 0.47 0.46 0.01 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.43 0.43 0.00 20.75 0.14 0.14 0.01 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.01 0.43 0.42 0.01 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.00 22.25 0.13 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.75 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.25 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.55 0.15 0.14 0.11 0.01 0.24 0.23 0.01 0.35 0.38 0.01 22.55 0.13 0.13 0.10 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.55 0.13 0.12 0.01 0.21 0.21 0.00 0.35 0.35 0.00 22.75 0.12 0.12 0.01 0.21 0.20 0.00 0.35 0.35 0.00 22.75 0.11 0.11 0.01 0.20 0.19 0.18 0.00 0.33 0.33 0.00 23.75 0.11 0.11 0.11 0.00 0.18 0.	17.00	0.22	0.21	0.01	0.36	0.35	0.01	0.66	0.65	0.01
17.75 0.19 0.18 0.01 0.31 0.30 0.01 0.56 0.56 0.00 18.00 0.18 0.17 0.01 0.29 0.28 0.01 0.53 0.52 0.01 18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.50 0.17 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.25 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.75 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 22.50 0.15 0.14 0.11 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.39 0.00 22.25 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.21 0.00 0.35 0.35 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.35 0.35 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.35 0.36 0.00 22.75 0.12 0.12 0.01 0.21 0.20 0.01 0.35 0.35 0.00 22.75 0.12 0.11 0.01 0.19 0.19 0.19 0.00 0.35 0.34 0.01 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.	17.25	0.21	0.20	0.01	0.34	0.33	0.01	0.62	0.62	0.00
18.00 0.18 0.17 0.01 0.29 0.28 0.01 0.53 0.52 0.01 18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.50 0.17 0.16 0.01 0.27 0.26 0.01 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.49 0.49 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.02 0.01 0.47 0.46 0.01 20.00 0.15 0.14 0.01 0.24	17.50	0.20	0.19	0.01	0.33	0.32	0.01	0.59	0.59	0.00
18.25 0.17 0.16 0.01 0.28 0.27 0.01 0.51 0.50 0.01 18.50 0.17 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.44 0.44 0.01 20.00 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.25 0.15 0.14 0.01 0.24	17.75	0.19	0.18	0.01	0.31	0.30	0.01	0.56	0.56	0.00
18.50 0.17 0.16 0.01 0.27 0.27 0.00 0.50 0.49 0.01 18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.24 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.25 0.15 0.14 0.01	18.00	0.18	0.17	0.01	0.29	0.28	0.01	0.53	0.52	0.01
18.75 0.16 0.16 0.00 0.27 0.26 0.01 0.49 0.49 0.00 19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.25 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.42 0.00 20.25 0.15	18.25	0.17	0.16	0.01	0.28	0.27	0.01	0.51	0.50	0.01
19.00 0.16 0.15 0.01 0.27 0.26 0.01 0.48 0.48 0.00 19.25 0.16 0.15 0.01 0.26 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.05 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.25 0.00 0.46 0.46 0.06 0.00 20.00 0.15 0.14 0.01 0.25 0.25 0.25 0.00 0.46 0.46 0.46 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42	18.50	0.17	0.16	0.01	0.27	0.27	0.00	0.50	0.49	0.01
19.25 0.16 0.15 0.01 0.26 0.26 0.00 0.48 0.47 0.01 19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.05 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.01 21.25 0.14 0.13 0.01	18.75	0.16	0.16	0.00	0.27	0.26	0.01	0.49	0.49	0.00
19.50 0.16 0.15 0.01 0.26 0.25 0.01 0.47 0.46 0.01 19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.01 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.25 0.13 0.13 0.00 0.22	19.00	0.16	0.15	0.01	0.27	0.26	0.01	0.48	0.48	0.00
19.75 0.15 0.15 0.00 0.25 0.25 0.00 0.46 0.46 0.00 20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.01 0.00 21.75 0.13 0.13	19.25	0.16	0.15	0.01	0.26	0.26	0.00	0.48	0.47	0.01
20.00 0.15 0.14 0.01 0.25 0.24 0.01 0.45 0.45 0.00 20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.04 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.02 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40	19.50	0.16	0.15	0.01	0.26	0.25	0.01	0.47	0.46	0.01
20.25 0.15 0.14 0.01 0.24 0.24 0.00 0.44 0.44 0.00 20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.01 0.41 0.41 0.00 21.75 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.40 0.40 22.00 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.39 0.39 0.00 22.00 0.13	19.75	0.15	0.15	0.00	0.25	0.25	0.00	0.46	0.46	0.00
20.50 0.15 0.14 0.01 0.24 0.23 0.01 0.43 0.43 0.00 20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.02 0.00 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.40 0.00 21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50	20.00	0.15	0.14	0.01	0.25	0.24	0.01	0.45	0.45	0.00
20.75 0.14 0.14 0.00 0.24 0.23 0.01 0.43 0.42 0.01 21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.41 0.00 21.75 0.13 0.13 0.00 0.22 0.22 0.01 0.39 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12	20.25	0.15	0.14	0.01	0.24	0.24	0.00	0.44	0.44	0.00
21.00 0.14 0.13 0.01 0.23 0.23 0.00 0.42 0.42 0.00 21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.40 0.00 21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.35 0.36 0.36 0.00 23.00 0.12	20.50	0.15	0.14	0.01	0.24	0.23	0.01	0.43	0.43	0.00
21.25 0.14 0.13 0.01 0.23 0.22 0.01 0.41 0.41 0.00 21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.35 0.00 23.25 0.12 0.11 0.01	20.75	0.14	0.14	0.00	0.24	0.23	0.01	0.43	0.42	0.01
21.50 0.13 0.13 0.00 0.22 0.22 0.00 0.40 0.40 0.00 21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.34 0.34 0.01 23.50 0.11 0.11 0.00 0.18		0.14	0.13	0.01	0.23	0.23	0.00	0.42	0.42	0.00
21.75 0.13 0.13 0.00 0.22 0.21 0.01 0.39 0.39 0.00 22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.18 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11	21.25	0.14	0.13	0.01	0.23	0.22	0.01	0.41	0.41	0.00
22.00 0.13 0.12 0.01 0.21 0.21 0.00 0.39 0.38 0.01 22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.06 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.33 0.00	21.50	0.13	0.13	0.00	0.22	0.22	0.00	0.40	0.40	0.00
22.25 0.13 0.12 0.01 0.21 0.20 0.01 0.38 0.38 0.00 22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.18 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00	21.75	0.13	0.13	0.00	0.22	0.21	0.01	0.39	0.39	0.00
22.50 0.12 0.12 0.00 0.20 0.20 0.00 0.37 0.37 0.00 22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.18 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.33 0.00	22.00	0.13	0.12	0.01	0.21	0.21	0.00	0.39	0.38	0.01
22.75 0.12 0.12 0.00 0.20 0.20 0.00 0.36 0.36 0.00 23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.18 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00	22.25	0.13	0.12	0.01	0.21	0.20	0.01	0.38	0.38	0.00
23.00 0.12 0.11 0.01 0.20 0.19 0.01 0.35 0.35 0.00 23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.19 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00		0.12	0.12	0.00	0.20	0.20	0.00	0.37	0.37	0.00
23.25 0.12 0.11 0.01 0.19 0.19 0.00 0.35 0.34 0.01 23.50 0.11 0.11 0.00 0.19 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00	22.75	0.12	0.12	0.00	0.20	0.20	0.00	0.36	0.36	0.00
23.50 0.11 0.11 0.00 0.19 0.18 0.01 0.34 0.34 0.00 23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00	23.00	0.12	0.11	0.01	0.20	0.19	0.01	0.35	0.35	0.00
23.75 0.11 0.11 0.00 0.18 0.18 0.00 0.33 0.33 0.00	23.25	0.12	0.11	0.01	0.19	0.19	0.00	0.35	0.34	0.01
	23.50	0.11	0.11	0.00	0.19	0.18	0.01	0.34	0.34	0.00
24.00 0.11 0.10 0.01 0.18 0.17 0.01 0.32 0.32 0.00	23.75	0.11	0.11	0.00	0.18	0.18	0.00	0.33	0.33	0.00
	24.00	0.11	0.10	0.01	0.18	0.17	0.01	0.32	0.32	0.00



West Addition Future Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

	2 Year Storm Event			1	0-Year Storm Eve	ent	100-Year Storm Event			
Time	Pre-Dev	Post-Dev		Pre-Dev	Post-Dev		Pre-Dev	Post-Dev	Reduction	
(Hours)	Flow (CFS)	Flow (CFS)	Reduction (CFS)	Flow (CFS)	Flow (CFS)	Reduction (CFS)	Flow (CFS)	Flow (CFS)	(CFS)	
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.04	0.04	0.00	
0.75	0.00	0.00	0.00	0.01	0.01	0.00	0.11	0.10	0.01	
1.00	0.00	0.00	0.00	0.04	0.04	0.00	0.16	0.14	0.02	
1.25	0.02	0.01	0.01	0.06	0.06	0.00	0.20	0.18	0.02	
1.50	0.03	0.02	0.01	0.08	0.07	0.01	0.23	0.20	0.03	
1.75	0.04	0.03	0.01	0.10	0.09	0.01	0.26	0.23	0.03	
2.00	0.05	0.04	0.01	0.11	0.10	0.01	0.28	0.25	0.03	
2.25	0.06	0.05	0.01	0.13	0.11	0.02	0.30	0.26	0.04	
2.50	0.07	0.06	0.01	0.14	0.12	0.02	0.31	0.28	0.03	
2.75	0.07	0.06	0.01	0.15	0.13	0.02	0.33	0.29	0.04	
3.00	0.08	0.07	0.01	0.16	0.14	0.02	0.34	0.30	0.04	
3.25	0.09	0.08	0.01	0.17	0.15	0.02	0.35	0.31	0.04	
3.50	0.09	0.08	0.01	0.18	0.16	0.02	0.37	0.33	0.04	
3.75	0.10	0.09	0.01 0.02	0.18	0.16 0.17	0.02	0.38	0.33 0.34	0.05	
4.00 4.25	0.11	0.09	0.02	0.19	0.17	0.02	0.39	0.34	0.05 0.05	
4.25	0.11	0.10	0.01	0.20	0.18	0.02	0.40	0.36	0.05	
4.75	0.12	0.10	0.02	0.21	0.18	0.03	0.41	0.37	0.05	
5.00	0.13	0.11	0.01	0.22	0.19	0.02	0.43	0.38	0.05	
5.25	0.13	0.12	0.01	0.23	0.20	0.03	0.44	0.39	0.05	
5.50	0.14	0.12	0.02	0.23	0.20	0.03	0.45	0.40	0.05	
5.75	0.14	0.12	0.02	0.24	0.21	0.03	0.46	0.40	0.06	
6.00	0.14	0.13	0.01	0.24	0.21	0.03	0.46	0.41	0.05	
6.25	0.15	0.14	0.01	0.26	0.23	0.03	0.50	0.44	0.06	
6.50	0.17	0.15	0.02	0.28	0.24	0.04	0.53	0.46	0.07	
6.75	0.18	0.16	0.02	0.29	0.26	0.03	0.57	0.50	0.07	
7.00	0.19	0.17	0.02	0.31	0.28	0.03	0.60	0.53	0.07	
7.25	0.20	0.18	0.02	0.33	0.29	0.04	0.64	0.57	0.07	
7.50	0.21	0.19	0.02	0.35	0.31	0.04	0.68	0.61	0.07	
7.75	0.22	0.20	0.02	0.36	0.32	0.04	0.72	0.64	0.08	
8.00	0.24	0.21	0.03	0.38	0.34	0.04	0.76	0.68	0.08	
8.25	0.25	0.22	0.03	0.40	0.35	0.05	0.79	0.72	0.07	
8.50	0.26	0.23	0.03	0.42	0.37	0.05	0.83	0.76	0.07	
8.75	0.27	0.24	0.03	0.43	0.38	0.05	0.88	0.80	0.08	
9.00	0.28	0.25	0.03	0.45	0.40	0.05	0.92	0.84	0.08	
9.25	0.32	0.28	0.04	0.51	0.45	0.06	1.04	0.95	0.09	
9.50	0.36	0.32	0.04	0.58	0.51	0.07	1.16	1.07	0.09	
9.75	0.40	0.35	0.05	0.64	0.57	0.07	1.30	1.20	0.10	
10.00	0.43	0.38	0.05	0.71	0.63	0.08	1.43	1.33	0.10	
10.25	0.47	0.42	0.05	0.78	0.70	0.08	1.57	1.46	0.11	
10.50	0.51	0.45	0.06	0.85	0.76	0.09	1.71	1.60	0.11	
10.75	0.65	0.57	0.08	1.08	0.97	0.11	2.16	2.03	0.13	
11.00	0.79	0.69	0.10	1.33	1.20	0.13	2.65	2.51	0.14	
11.25	1.04	0.92 1.17	0.12	1.75	1.60	0.15 0.17	3.48 4.37	3.31 4.18	0.17	
11.50 11.75	1.31 2.21	1.17	0.14 0.23	2.20 3.67	2.03 3.41	0.17	7.21	6.94	0.19 0.27	
12.00	6.14	5.61	0.23	10.13	9.53	0.60	19.66	19.10	0.27	
12.25	3.95	3.71	0.24	6.62	6.39	0.80	12.98	12.87	0.56	
12.25	2.08	1.96	0.12	3.43	3.32	0.23	6.60	6.53	0.11	
12.75	1.39	1.31	0.08	2.28	2.21	0.07	4.37	4.34	0.07	
13.00	1.10	1.05	0.05	1.81	1.76	0.07	3.46	3.44	0.03	
13.25	0.88	0.83	0.05	1.44	1.40	0.03	2.75	2.73	0.02	
13.50	0.88	0.68	0.03	1.17	1.40	0.03	2.73	2.73	0.02	
13.75	0.71	0.58	0.03	1.00	0.97	0.03	1.90	1.89	0.01	
14.00	0.57	0.54	0.03	0.93	0.90	0.03	1.76	1.75	0.01	
14.00	0.57	1 0.54	0.03	0.55	0.50	0.03	1.70	1./3	0.01	

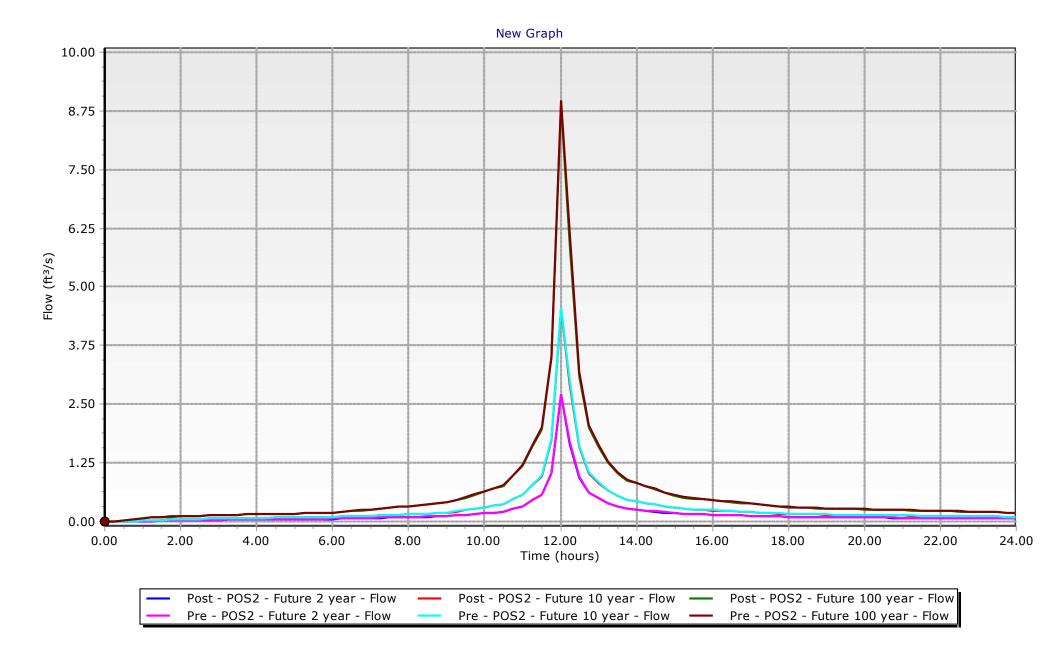
14.25	0.52	0.50	0.02	0.85	0.83	0.02	1.62	1.61	0.01
14.50	0.48	0.45	0.03	0.78	0.76	0.02	1.48	1.47	0.01
14.75	0.43	0.41	0.02	0.70	0.69	0.01	1.34	1.33	0.01
15.00	0.38	0.37	0.01	0.63	0.61	0.02	1.19	1.19	0.00
15.25	0.36	0.34	0.02	0.58	0.57	0.01	1.10	1.10	0.00
15.50	0.34	0.33	0.01	0.56	0.55	0.01	1.06	1.06	0.00
15.75	0.33	0.32	0.01	0.54	0.53	0.01	1.02	1.02	0.00
16.00	0.32	0.30	0.02	0.52	0.51	0.01	0.98	0.98	0.00
16.25	0.30	0.29	0.01	0.50	0.49	0.01	0.94	0.94	0.00
16.50	0.29	0.28	0.01	0.48	0.46	0.02	0.90	0.90	0.00
16.75	0.28	0.27	0.01	0.46	0.45	0.01	0.86	0.86	0.00
17.00	0.27	0.26	0.01	0.43	0.42	0.01	0.82	0.82	0.00
17.25	0.25	0.24	0.01	0.41	0.40	0.01	0.78	0.78	0.00
17.50	0.24	0.23	0.01	0.39	0.38	0.01	0.74	0.74	0.00
17.75	0.23	0.22	0.01	0.37	0.36	0.01	0.70	0.70	0.00
18.00	0.21	0.21	0.00	0.35	0.34	0.01	0.66	0.66	0.00
18.25	0.21	0.20	0.01	0.34	0.33	0.01	0.63	0.63	0.00
18.50	0.20	0.19	0.01	0.33	0.32	0.01	0.62	0.62	0.00
18.75	0.20	0.19	0.01	0.32	0.32	0.00	0.61	0.61	0.00
19.00	0.20	0.19	0.01	0.32	0.31	0.01	0.60	0.60	0.00
19.25	0.19	0.19	0.00	0.31	0.31	0.00	0.59	0.59	0.00
19.50	0.19	0.18	0.01	0.31	0.30	0.01	0.58	0.58	0.00
19.75	0.19	0.18	0.01	0.30	0.30	0.00	0.57	0.57	0.00
20.00	0.18	0.18	0.00	0.30	0.29	0.01	0.56	0.56	0.00
20.25	0.18	0.17	0.01	0.29	0.29	0.00	0.55	0.55	0.00
20.50	0.18	0.17	0.01	0.29	0.28	0.01	0.54	0.54	0.00
20.75	0.17	0.17	0.00	0.28	0.28	0.00	0.53	0.53	0.00
21.00	0.17	0.16	0.01	0.28	0.27	0.01	0.52	0.52	0.00
21.25	0.17	0.16	0.01	0.27	0.27	0.00	0.51	0.51	0.00
21.50	0.16	0.16	0.00	0.27	0.26	0.01	0.50	0.50	0.00
21.75	0.16	0.16	0.00	0.26	0.26	0.00	0.49	0.49	0.00
22.00	0.16	0.15	0.01	0.26	0.25	0.01	0.48	0.48	0.00
22.25	0.15	0.15	0.00	0.25	0.25	0.00	0.47	0.47	0.00
22.50	0.15	0.15	0.00	0.25	0.24	0.01	0.46	0.46	0.00
22.75	0.15	0.14	0.01	0.24	0.24	0.00	0.45	0.45	0.00
23.00	0.14	0.14	0.00	0.23	0.23	0.00	0.44	0.44	0.00
23.25	0.14	0.14	0.00	0.23	0.23	0.00	0.43	0.43	0.00
23.50	0.14	0.13	0.01	0.22	0.22	0.00	0.42	0.42	0.00
23.75	0.13	0.13	0.00	0.22	0.21	0.01	0.41	0.41	0.00
24.00	0.13	0.13	0.00	0.21	0.21	0.00	0.40	0.40	0.00



West Addition Current Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1 2 Year Storm Event 10-Year Storm Event 100-Year Storm Event										
Time	Pre-Dev	Year Storm Ever	nt Reduction	10-Year Storm Event Pre-Dev Post-Dev Reduction			Pre-Dev Post-Dev Reduction			
(Hours)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	Flow (CFS)	Flow (CFS)	(CFS)	
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.00	
0.75	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.06	0.01	
1.00	0.00	0.00	0.00	0.02	0.02	0.00	0.11	0.09	0.02	
1.25	0.01	0.01	0.00	0.04	0.04	0.00	0.14	0.12	0.02	
1.50	0.02	0.01	0.01	0.06	0.05	0.01	0.17	0.15	0.02	
1.75	0.02	0.02	0.00	0.07	0.06	0.01	0.19	0.17	0.02	
2.00	0.03	0.03	0.00	0.08	0.07	0.01	0.21	0.18	0.03	
2.25	0.04	0.04	0.00	0.10	0.08	0.02	0.22	0.20	0.02	
2.50	0.05	0.04	0.01	0.11	0.09	0.02	0.24	0.21	0.03	
2.75	0.05	0.05	0.00	0.12	0.10	0.02	0.25	0.22	0.03	
3.00	0.06	0.05	0.01	0.12	0.11	0.01	0.26	0.23	0.03	
3.25	0.07	0.06	0.01	0.13	0.12	0.01	0.27	0.24	0.03	
3.50	0.07	0.06	0.01	0.14	0.12	0.02	0.29	0.25	0.04	
3.75	0.08	0.07	0.01	0.15	0.13	0.02	0.30	0.26	0.04	
4.00	0.08	0.07	0.01	0.15	0.14	0.01	0.31	0.27	0.04	
4.25	0.09	0.08	0.01	0.16	0.14	0.02	0.31	0.28	0.03	
4.50	0.09	0.08	0.01	0.17	0.15	0.02	0.32	0.29	0.03	
4.75	0.09	0.08	0.01	0.17	0.15	0.02	0.33	0.29	0.04	
5.00	0.10	0.09	0.01	0.18	0.16	0.02	0.34	0.30	0.04	
5.25	0.10	0.09	0.01	0.18	0.16	0.02	0.35	0.31	0.04	
5.50	0.11	0.10	0.01	0.19	0.17	0.02	0.36	0.31	0.05	
5.75	0.11	0.10	0.01	0.20	0.17	0.03	0.36	0.32	0.04	
6.00	0.11	0.10	0.01	0.20	0.18	0.02	0.37	0.33	0.04	
6.25	0.12	0.11	0.01	0.21	0.19	0.02	0.40	0.35	0.05	
6.50	0.13	0.12	0.01	0.23	0.20	0.03	0.42	0.37	0.05	
6.75	0.14	0.13	0.01	0.24	0.22	0.02	0.44	0.39	0.05	
7.00	0.15	0.14	0.01	0.26	0.23	0.03	0.47	0.42	0.05	
7.25	0.16	0.14	0.02	0.27	0.24	0.03	0.50	0.44	0.06	
7.50	0.17	0.15	0.02	0.29	0.26	0.03	0.53	0.46	0.07	
7.75	0.18	0.16	0.02	0.30	0.27	0.03	0.56	0.49	0.07	
8.00	0.19	0.17	0.02	0.32	0.28	0.04	0.59	0.52	0.07	
8.25	0.20	0.18	0.02	0.33	0.30	0.03	0.62	0.55	0.07	
8.50	0.21	0.19	0.02	0.35	0.31	0.04	0.65	0.58	0.07	
8.75	0.22	0.20	0.02	0.37	0.32	0.05	0.68	0.61	0.07	
9.00	0.23	0.21	0.02	0.38	0.34	0.04	0.71	0.64	0.07	
9.25	0.26	0.23	0.03	0.43	0.38	0.05	0.81	0.73	0.08	
9.50	0.29	0.26	0.03	0.48	0.42	0.06	0.91	0.82	0.09	
9.75	0.33	0.29	0.04	0.53	0.47	0.06	1.01	0.92	0.09	
10.00	0.36	0.32	0.04	0.58	0.51	0.07	1.12	1.02	0.10	
10.25	0.39	0.35	0.04	0.64	0.56	0.08	1.22	1.13	0.09	
10.50	0.43	0.38	0.05	0.70	0.62	0.08	1.33	1.23	0.10	
10.75	0.54	0.48	0.06	0.89	0.79	0.10	1.69	1.57	0.12	
11.00	0.65	0.58	0.07	1.09	0.98	0.11	2.07	1.93	0.14	
11.25	0.86	0.76	0.10	1.44	1.30	0.14	2.73	2.56	0.17	
11.50	1.07	0.94	0.13	1.82	1.65	0.17	3.43	3.24	0.19	
11.75	1.80	1.59	0.21	3.04	2.78	0.26	5.68	5.38	0.30	
12.00	5.02	4.51	0.51	8.40	7.77	0.63	15.56	14.84	0.72	
12.25	3.20	2.99	0.21	5.46	5.29	0.17	10.26	10.25	0.01	
12.50	1.70	1.59	0.11	2.85	2.74	0.11	5.25	5.17	0.08	
12.75	1.13	1.06	0.07	1.90	1.83	0.07	3.48	3.44	0.04	
13.00	0.90	0.85	0.05	1.51	1.45	0.06	2.76	2.73	0.03	
13.25	0.72	0.68	0.04	1.20	1.16	0.04	2.19	2.17	0.02	

13.50	0.58	0.55	0.03	0.97	0.94	0.03	1.78	1.76	0.02
13.75	0.50	0.47	0.03	0.83	0.81	0.02	1.52	1.50	0.02
14.00	0.46	0.44	0.02	0.77	0.75	0.02	1.40	1.39	0.01
14.25	0.43	0.40	0.03	0.71	0.69	0.02	1.29	1.28	0.01
14.50	0.39	0.37	0.02	0.65	0.63	0.02	1.18	1.17	0.01
14.75	0.35	0.33	0.02	0.59	0.57	0.02	1.07	1.06	0.01
15.00	0.32	0.30	0.02	0.52	0.51	0.01	0.95	0.94	0.01
15.25	0.29	0.28	0.01	0.48	0.47	0.01	0.88	0.87	0.01
15.50	0.28	0.27	0.01	0.47	0.45	0.02	0.85	0.84	0.01
15.75	0.27	0.26	0.01	0.45	0.44	0.01	0.82	0.81	0.01
16.00	0.26	0.25	0.01	0.43	0.42	0.01	0.78	0.78	0.00
16.25	0.25	0.24	0.01	0.41	0.40	0.01	0.75	0.74	0.01
16.50	0.24	0.23	0.01	0.40	0.39	0.01	0.72	0.71	0.01
16.75	0.23	0.22	0.01	0.38	0.37	0.01	0.69	0.68	0.01
17.00	0.22	0.21	0.01	0.36	0.35	0.01	0.66	0.65	0.01
17.25	0.21	0.20	0.01	0.34	0.33	0.01	0.62	0.62	0.00
17.50	0.20	0.19	0.01	0.33	0.32	0.01	0.59	0.59	0.00
17.75	0.19	0.18	0.01	0.31	0.30	0.01	0.56	0.56	0.00
18.00	0.18	0.17	0.01	0.29	0.28	0.01	0.53	0.52	0.01
18.25	0.17	0.16	0.01	0.28	0.27	0.01	0.51	0.50	0.01
18.50	0.17	0.16	0.01	0.27	0.27	0.00	0.50	0.49	0.01
18.75	0.16	0.16	0.00	0.27	0.26	0.01	0.49	0.49	0.00
19.00	0.16	0.15	0.01	0.27	0.26	0.01	0.48	0.48	0.00
19.25	0.16	0.15	0.01	0.26	0.26	0.00	0.48	0.47	0.01
19.50	0.16	0.15	0.01	0.26	0.25	0.01	0.47	0.46	0.01
19.75	0.15	0.15	0.00	0.25	0.25	0.00	0.46	0.46	0.00
20.00	0.15	0.14	0.01	0.25	0.24	0.01	0.45	0.45	0.00
20.25	0.15	0.14	0.01	0.24	0.24	0.00	0.44	0.44	0.00
20.50	0.15	0.14	0.01	0.24	0.23	0.01	0.43	0.43	0.00
20.75	0.14	0.14	0.00	0.24	0.23	0.01	0.43	0.42	0.01
21.00	0.14	0.13	0.01	0.23	0.23	0.00	0.42	0.42	0.00
21.25	0.14	0.13	0.01	0.23	0.22	0.01	0.41	0.41	0.00
21.50	0.13	0.13	0.00	0.22	0.22	0.00	0.40	0.40	0.00
21.75	0.13	0.13	0.00	0.22	0.21	0.01	0.39	0.39	0.00
22.00	0.13	0.12	0.01	0.21	0.21	0.00	0.39	0.38	0.01
22.25	0.13	0.12	0.01	0.21	0.20	0.01	0.38	0.38	0.00
22.50	0.12	0.12	0.00	0.20	0.20	0.00	0.37	0.37	0.00
22.75	0.12	0.12	0.00	0.20	0.20	0.00	0.36	0.36	0.00
23.00	0.12	0.11	0.01	0.20	0.19	0.01	0.35	0.35	0.00
23.25	0.12	0.11	0.01	0.19	0.19	0.00	0.35	0.34	0.01
23.50	0.11	0.11	0.00	0.19	0.18	0.01	0.34	0.34	0.00
23.75	0.11	0.11	0.00	0.18	0.18	0.00	0.33	0.33	0.00
24.00	0.11	0.10	0.01	0.18	0.17	0.01	0.32	0.32	0.00



West Addition Future Precipitation PRE- VS POST- HYDROGRAPH FLOW RATE COMPARISON FOR POS-1

	2	2 Year Storm Eve	nt	1	0-Year Storm Eve	ent	10	0-Year Storm Eve	ent
Time	Pre-Dev	Post-Dev		Pre-Dev	Post-Dev		Pre-Dev	Post-Dev	Reduction
(Hours)	Flow (CFS)	Flow (CFS)	Reduction (CFS)	Flow (CFS)	Flow (CFS)	Reduction (CFS)	Flow (CFS)	Flow (CFS)	(CFS)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.04	0.04	0.00
0.75	0.00	0.00	0.00	0.01	0.01	0.00	0.11	0.10	0.01
1.00	0.00	0.00	0.00	0.04	0.04	0.00	0.16	0.14	0.02
1.25	0.02	0.01	0.01	0.06	0.06	0.00	0.20	0.18	0.02
1.50	0.03	0.02	0.01	0.08	0.07	0.01	0.23	0.20	0.03
1.75	0.04	0.03	0.01	0.10	0.09	0.01	0.26	0.23	0.03
2.00	0.05	0.04	0.01	0.11	0.10	0.01	0.28	0.25	0.03
2.25	0.06	0.05	0.01	0.13	0.11	0.02	0.30	0.26	0.04
2.50	0.07	0.06	0.01	0.14	0.12	0.02	0.31	0.28	0.03
2.75	0.07	0.06	0.01	0.15	0.13	0.02	0.33	0.29	0.04
3.00	0.08	0.07	0.01	0.16	0.14	0.02	0.34	0.30	0.04
3.25	0.09	0.08	0.01	0.17	0.15	0.02	0.35	0.31	0.04
3.50	0.09	0.08	0.01	0.18	0.16	0.02	0.37	0.33	0.04
3.75	0.10	0.09	0.01 0.02	0.18	0.16 0.17	0.02	0.38	0.33 0.34	0.05
4.00	0.11		 		 	0.02			0.05
4.25 4.50	0.11 0.12	0.10 0.10	0.01	0.20	0.18 0.18	0.02	0.40	0.35 0.36	0.05 0.05
4.75	0.12	0.10	0.02	0.21	0.18	0.03	0.41	0.37	0.05
5.00	0.12	0.11	0.01	0.22	0.19	0.02	0.43	0.38	0.05
5.25	0.13	0.12	0.01	0.23	0.20	0.03	0.44	0.39	0.05
5.50	0.14	0.12	0.02	0.23	0.20	0.03	0.45	0.40	0.05
5.75	0.14	0.12	0.02	0.24	0.21	0.03	0.46	0.40	0.06
6.00	0.14	0.13	0.01	0.24	0.21	0.03	0.46	0.41	0.05
6.25	0.15	0.14	0.01	0.26	0.23	0.03	0.50	0.44	0.06
6.50	0.17	0.15	0.02	0.28	0.24	0.04	0.53	0.46	0.07
6.75	0.18	0.16	0.02	0.29	0.26	0.03	0.57	0.50	0.07
7.00	0.19	0.17	0.02	0.31	0.28	0.03	0.60	0.53	0.07
7.25	0.20	0.18	0.02	0.33	0.29	0.04	0.64	0.57	0.07
7.50	0.21	0.19	0.02	0.35	0.31	0.04	0.68	0.61	0.07
7.75	0.22	0.20	0.02	0.36	0.32	0.04	0.72	0.64	0.08
8.00	0.24	0.21	0.03	0.38	0.34	0.04	0.76	0.68	0.08
8.25	0.25	0.22	0.03	0.40	0.35	0.05	0.79	0.72	0.07
8.50	0.26	0.23	0.03	0.42	0.37	0.05	0.83	0.76	0.07
8.75	0.27	0.24	0.03	0.43	0.38	0.05	0.88	0.80	0.08
9.00	0.28	0.25	0.03	0.45	0.40	0.05	0.92	0.84	0.08
9.25	0.32	0.28	0.04	0.51	0.45	0.06	1.04	0.95	0.09
9.50	0.36	0.32	0.04	0.58	0.51	0.07	1.16	1.07	0.09
9.75	0.40	0.35	0.05	0.64	0.57	0.07	1.30	1.20	0.10
10.00	0.43	0.38	0.05	0.71	0.63	0.08	1.43	1.33	0.10
10.25	0.47	0.42	0.05	0.78	0.70	0.08	1.57	1.46	0.11
10.50	0.51	0.45	0.06	0.85	0.76	0.09	1.71	1.60	0.11
10.75	0.65	0.57	0.08	1.08	0.97	0.11	2.16	2.03	0.13
11.00	0.79 1.04	0.69 0.92	0.10	1.33 1.75	1.20	0.13	2.65 3.48	2.51 3.31	0.14
11.25 11.50	1.04	1.17	0.12 0.14	2.20	1.60 2.03	0.15 0.17	4.37	4.18	0.17 0.19
11.75	2.21	1.17	0.14	3.67	3.41	0.17	7.21	6.94	0.19
12.00	6.14	5.61	0.53	10.13	9.53	0.60	19.66	19.10	0.27
12.25	3.95	3.71	0.24	6.62	6.39	0.23	12.98	12.87	0.30
12.50	2.08	1.96	0.12	3.43	3.32	0.23	6.60	6.53	0.11
12.75	1.39	1.31	0.08	2.28	2.21	0.07	4.37	4.34	0.03
13.00	1.10	1.05	0.05	1.81	1.76	0.05	3.46	3.44	0.03
13.25	0.88	0.83	0.05	1.44	1.40	0.04	2.75	2.73	0.02
13.50	0.71	0.68	0.03	1.17	1.14	0.03	2.23	2.22	0.01
13.75	0.61	0.58	0.03	1.00	0.97	0.03	1.90	1.89	0.01
14.00	0.57	0.54	0.03	0.93	0.90	0.03	1.76	1.75	0.01
14.00	0.57	0.54	0.03	0.55	0.50	0.03	1.70	1./3	1 0.01

14.25	0.52	0.50	0.02	0.85	0.83	0.02	1.62	1.61	0.01
14.50	0.48	0.45	0.03	0.78	0.76	0.02	1.48	1.47	0.01
14.75	0.43	0.41	0.02	0.70	0.69	0.01	1.34	1.33	0.01
15.00	0.38	0.37	0.01	0.63	0.61	0.02	1.19	1.19	0.00
15.25	0.36	0.34	0.02	0.58	0.57	0.01	1.10	1.10	0.00
15.50	0.34	0.33	0.01	0.56	0.55	0.01	1.06	1.06	0.00
15.75	0.33	0.32	0.01	0.54	0.53	0.01	1.02	1.02	0.00
16.00	0.32	0.30	0.02	0.52	0.51	0.01	0.98	0.98	0.00
16.25	0.30	0.29	0.01	0.50	0.49	0.01	0.94	0.94	0.00
16.50	0.29	0.28	0.01	0.48	0.46	0.02	0.90	0.90	0.00
16.75	0.28	0.27	0.01	0.46	0.45	0.01	0.86	0.86	0.00
17.00	0.27	0.26	0.01	0.43	0.42	0.01	0.82	0.82	0.00
17.25	0.25	0.24	0.01	0.41	0.40	0.01	0.78	0.78	0.00
17.50	0.24	0.23	0.01	0.39	0.38	0.01	0.74	0.74	0.00
17.75	0.23	0.22	0.01	0.37	0.36	0.01	0.70	0.70	0.00
18.00	0.21	0.21	0.00	0.35	0.34	0.01	0.66	0.66	0.00
18.25	0.21	0.20	0.01	0.34	0.33	0.01	0.63	0.63	0.00
18.50	0.20	0.19	0.01	0.33	0.32	0.01	0.62	0.62	0.00
18.75	0.20	0.19	0.01	0.32	0.32	0.00	0.61	0.61	0.00
19.00	0.20	0.19	0.01	0.32	0.31	0.01	0.60	0.60	0.00
19.25	0.19	0.19	0.00	0.31	0.31	0.00	0.59	0.59	0.00
19.50	0.19	0.18	0.01	0.31	0.30	0.01	0.58	0.58	0.00
19.75	0.19	0.18	0.01	0.30	0.30	0.00	0.57	0.57	0.00
20.00	0.18	0.18	0.00	0.30	0.29	0.01	0.56	0.56	0.00
20.25	0.18	0.17	0.01	0.29	0.29	0.00	0.55	0.55	0.00
20.50	0.18	0.17	0.01	0.29	0.28	0.01	0.54	0.54	0.00
20.75	0.17	0.17	0.00	0.28	0.28	0.00	0.53	0.53	0.00
21.00	0.17	0.16	0.01	0.28	0.27	0.01	0.52	0.52	0.00
21.25	0.17	0.16	0.01	0.27	0.27	0.00	0.51	0.51	0.00
21.50	0.16	0.16	0.00	0.27	0.26	0.01	0.50	0.50	0.00
21.75	0.16	0.16	0.00	0.26	0.26	0.00	0.49	0.49	0.00
22.00	0.16	0.15	0.01	0.26	0.25	0.01	0.48	0.48	0.00
22.25	0.15	0.15	0.00	0.25	0.25	0.00	0.47	0.47	0.00
22.50	0.15	0.15	0.00	0.25	0.24	0.01	0.46	0.46	0.00
22.75	0.15	0.14	0.01	0.24	0.24	0.00	0.45	0.45	0.00
23.00	0.14	0.14	0.00	0.23	0.23	0.00	0.44	0.44	0.00
23.25	0.14	0.14	0.00	0.23	0.23	0.00	0.43	0.43	0.00
23.50	0.14	0.13	0.01	0.22	0.22	0.00	0.42	0.42	0.00
23.75	0.13	0.13	0.00	0.22	0.21	0.01	0.41	0.41	0.00
24.00	0.13	0.13	0.00	0.21	0.21	0.00	0.40	0.40	0.00

Project	JSMC West Addition		Ву	SPT	Date		_		
Location	Township of Neptune, M	onmouth County, NJ	_ _Checked	MI	Date		_		
Circle One:	Present Developed							_	
Circle One:	T_c T_t	through subarea	Existing	DA-1, Imp	ervious, C	urrent St	orm		
NOTES: Space f		s per flow type can be used for ea	ch						
Includ	e a map, schematic, or desc	ription of flow segments.							
Sheet flow (App	licable to T _c Only)		Segment ID	1					
Surface des	cription (table 3-1)			Asphalt					
2. Manning's ro	oughness coeff., n (Table 15-	-1)		0.011					
3. Flow Length	, L		ft	100					
4. Two-yr 24-hi	rainfall, P ₂		in	3.48					
5. Land slope,	S		ft/ft	0.016					
6. T _t =0.	007(nL) ^{0.8}	Compute T _t	hr	0.021	+		+] =	0.021
	$P_2^{0.5} s^{0.4}$								
Shallow concer	ntrated flow		Segment ID	2					
7. Surface des	cription (paved or unpaved)			Paved					
8. Flow length,	L		ft	260					
9. Watercourse	e slope, s		ft/ft	0.010					
_	ocity, V (Figure 15-4)		ft/s	2.00				ļ ,	
11. T _t =	L 2600 V	Compute T _t	hr	0.036	+		+	_ =	0.036
	3000 V								
Channel flow			Segment ID	3	4	5	6		
12. Cross section	nal flow area, a		ft ²	3.14159	4.90874				
13. Wetted perir	neter, p _w		ft	6.28319	7.85398				
14. Hydraulic ra	dius, r	$r = \frac{a}{Pw}$ Compute r	ft	0.50	0.63				
15. Channel slo	pe, s		ft/ft	0.005	0.004				
	oughness coeff., n			0.013	0.013				
V =	1.49 r ^{2/3} s ^{1/2}	Committee	£L/_	E 44	F 26				
17.	n	Compute V	ft/s	5.11	5.36			1	

18. Flow length, L

264

0.014

124



- 16. Manning's roughness coeff., n $V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$ Compute V
- 18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$



ft/s

ft

Project	JSMC West Addition	on		Ву	SPT	Date				
Location	Township of Neptu	une, Monmo	uth County, I	NJ Checked	MI	Date				
Circle One:	Present Develope	ed								
Circle One:	T_c T_t	through sub	area	Exis	ting DA-1, I	mpervious	s, Future S	torm		
NOTES: Space for workshe	as many as two segmet.	nents per flow	type can be	used for each						
Include a	a map, schematic, or o	description of	flow segmen	ts.						
Sheet flow (Applic	able to T _c Only)		-	Segment ID	1					
Surface descr	iption (table 3-1)				Asphalt					
2. Manning's rou	ghness coeff., n (Tabl	e 15-1)			0.011					
3. Flow Length, L				ft	100					
4. Two-yr 24-hr r	ainfall, P ₂			in	4.14					
5. Land slope, s				ft/ft	0.016		1		ı	
6. $T_t = \frac{0.00}{P_2}$	1 <mark>7(nL)^{0.8} 20.5s^{0.4}</mark>		Compute T _t	hr	0.019	+		+ []	=	0.019
Shallow concentr	ated flow			Segment ID	2					
•	iption (paved or unpav	(pd)		ocginicii: ib	Paved					
8. Flow length, L	phon (paved of unpav	,cu)		ft	260					
9. Watercourse	elone e			ft/ft	0.01					
	sity, V (Figure 15-4)			ft/s	2					
•	, , ,		Compute T _t			<u> </u> +	1.	+	_ [0.036
11. T _t =36	300 V		Compute 1 _t	hr	0.036				- [0.036
Channel flow				Segment ID	3					
12. Cross sections	al flow area, a			ft ²	3.141593	4.90874				
13. Wetted perime	eter, p _w			ft	6.283185	7.85398				
14. Hydraulic radio	us, r	$r = \frac{a}{-p_w}$	Compute r	ft	0.50	0.63				
15. Channel slope	e, S		•	ft/ft	0.005	0.004				
16. Manning's rou	ghness coeff., n				0.013	0.013				
V =	1.49 r ^{2/3} s ^{1/2}	_								
17.	n		Compute V	ft/s	5.11	5.36				
18. Flow length, L	1			ft	264	124	1	1	ļ	
19. T _t =	300 V		Compute T _t	hr	0.014	0.006		+	=	0.021
				Segment ID						
12. Cross sections	al flow area a			ft ²						
13. Wetted perime	eter, p _w	а		ft						
14. Hydraulic radio	us, r	$r = \frac{a}{p_w}$	Compute r	ft						
15. Channel slope			•	ft/ft						
16. Manning's rou	ghness coeff., n									
V -	$1.49 r^{2/3} s^{1/2}$									

17. n

Compute V

ft/s

ft

hr

+ + = 0.000

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

20. Watershed or subarea T_c or $T_t \, (\text{add } T_t \, \text{in steps 6, 11, 19})$

Project	JSMC West Addition			Ву	SPT	Date				
Location	Township of Neptune, M	onmouth County,	NJ	Checked	MI	Date				
Circle One:	Present Developed									
Circle One:	T_c T_t	through subarea		Existing	g DA-1, Pe	rvious, Cu	rrent Stori	n		
NOTES: Space for worksh	or as many as two segments neet.	per flow type can b	e used for ea	ch						
Include	e a map, schematic, or desci	ription of flow segm	ents.							
Sheet flow (Appl	icable to T _c Only)			Segment ID	1					
Surface descr	cription (table 3-1)				Short Grass					
	ughness coeff., n (Table 15-	1)			0.150					
3. Flow Length,	L			ft	47					
4. Two-yr 24-hr	rainfall, P ₂			in	3.48					
5. Land slope, s	3			ft/ft	0.020					
6. $T_t = \frac{0.0}{F}$	007(nL) ^{0.8}	(Compute T _t	hr	0.086	+			=	0.086
Shallow concen	2 -			Segment ID	2					
	cription (paved or unpaved)			Segmentib	Paved					
8. Flow length,	,			ft	182					
9. Watercourse				ft/ft	0.010					
	ocity, V (Figure 15-4)			ft/s	2.00					
		(Compute T _t	hr		+			=	0.025
11. T _t =	3600 V			·						
Channel flow				Segment ID	3	4	5	6		
12. Cross section	nal flow area, a			ft ²	1.22718	3.14159	4.90874			
13. Wetted perim	neter, p _w			ft	3.92699	6.28319	7.85398			
14. Hydraulic rad	lius, r	$r = \frac{a}{P_w}$	Compute r	ft	0.31	0.50	0.63			
15. Channel slop	oe, s			ft/ft	0.009	0.005	0.004			
16. Manning's ro					0.013	0.013	0.013			
V =	1.49 r ^{2/3} s ^{1/2}		Compute V	ft/s	5.01	5.11	5.36			

23

0.001

264

0.014

124

0.006

18. Flow length, L

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, pw
- 14. Hydraulic radius, r

Compute r

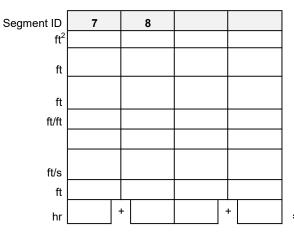
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

 $V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$

Compute V

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



0.000

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

0.13 hr

Project JSMC West A	ddition		By	SPT	Date				
Location Township of N	Neptune, Monmou	ıth County, NJ	Checked	MI	Date				
Circle One: Present Deve	eloped								
Circle One: T _c T _t	through suba	area	Exi	sting DA-1,	Pervious,	Future St	orm		
NOTES: Space for as many as two worksheet.	segments per flow	type can be us	ed for each						
Include a map, schematic	c, or description of	flow segments.							
$\underline{\textbf{Sheet flow}} \text{ (Applicable to T}_{c} \text{ Only)}$		S	Segment ID	1				ı	
Surface description (table 3-1)				Short Grass				ı	
Manning's roughness coeff., n	(Table 15-1)			0.15				ı	
3. Flow Length, L	,		ft	47				ı	
4. Two-yr 24-hr rainfall, P ₂			in	4.14				ı	
5. Land slope, s			ft/ft	0.02				ı	
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$		Compute T _t	hr	0.078	+	-	+	=	0.078
-		c	Segment ID	2				ı	
Shallow concentrated flow 7. Surface description (paved or u	innaved)		segment iD	Paved				ı	
8. Flow length, L	mpavoa)		ft	182				ı	
9. Watercourse slope, s			ft/ft	0.01				ı	
10. Average velocity, V (Figure 15-	4)		ft/s	2				ı	
11. $T_t = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.025	+		+	=	0.025
Channel flow		c	Segment ID	2		_	c	ı	
Channel flow12. Cross sectional flow area, a			ft ²	3 1.227185	3.14159	5 4.90874	6	ı	
13. Wetted perimeter, p _w			ft	3.926991	6.28319	7.85398		ı	
14. Hydraulic radius, r	r = - a - Pw-	Compute r	ft	0.31	0.50	0.63		ı	
15. Channel slope, s		Compator	ft/ft	0.009	0.005	0.004		ı	
16. Manning's roughness coeff., n				0.013	0.013	0.013		ı	
$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$								ı	
		Compute V	ft/s	5.01	5.11	5.36		ı	
18. Flow length, L $T_{t} = \frac{L}{3600 \text{ V}}$			ft	23	+ 264	124	+	ı	
19. T _t - 3600 V		Compute T _t	hr	0.001	0.014	0.006		=	0.022
		9	Segment ID	7	8			ı	
12. Cross sectional flow area, a		·	ft ²	•				ı	
13. Wetted perimeter, p _w	_		ft					ı	
14. Hydraulic radius, r	$r = \frac{a}{p_w}$	Compute r	ft					ı	
15. Channel slope, s			ft/ft					ı	
16. Manning's roughness coeff., n $V = \frac{1.49 \text{ r}^{2/3} \text{ s}^{1/2}}{\text{ s}^{1/2}}$								ı	

17. n

Compute V

ft/s

ft

hr

+ = 0.000

18. Flow length, L

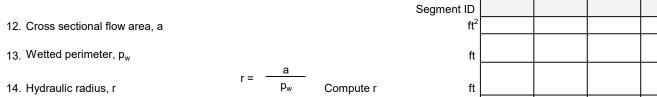
9. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Project	JSMC W	est Addition			_Ву	SPT	Date		_		
Location	Townshi	p of Neptune, M	onmouth County	, NJ	Checked	MI	Date				
Circle One:	Present	Developed									
Circle One:	T _c	T _t	through subarea		Existing	DA-2, Imp	ervious, C	urrent Sto	rm		
	e for as many ksheet.	as two segments	per flow type car	be used for ea	ach						
Incl	ude a map, so	hematic, or desci	ription of flow seg	ments.							
Sheet flow (A	pplicable to T	Only)			Segment ID	1					
1. Surface de	escription (tab	le 3-1)				Asphalt					
2. Manning's	roughness co	oeff., n (Table 15-	-1)			0.011					
3. Flow Leng	jth, L				ft	100					
4. Two-yr 24	-hr rainfall, P ₂				in	3.48					
5. Land slop	e, s				ft/ft	0.024	<u> </u>				
6. T _t =	0.007(nL) ^{0.8}	_		Compute T _t	hr	0.018	+		+	=	0.018
	. 2 -				I					İ	
Shallow cond		=			Segment ID	2					
		ved or unpaved)				Paved					
8. Flow length	th, L				ft	68					
9. Watercou	• •				ft/ft	0.031					
10. Average v	elocity, V (Fig	ure 15-4)			ft/s	3.58		1	<u> </u>		
11. T _t =	L 3600 V			Compute T _t	hr	0.005	+		+	=	0.005
Channel flow					Segment ID						
Channel flow					Segment ib	0.7854	1.76715				
12. Cross sec		а, а									
13. Wetted pe			$r = \frac{a}{P_w}$	Compute r	ft ft	3.14159 0.25	4.71239				
14. Hydraulic			· Pw	Compute r	ft ft/ft	0.005	0.38				
15. Channel s	-	off n			II/II		0.010				
16. Manning's						0.013	0.013				
17. V =	1.49 r ² n			Compute V	ft/s	3.22	5.96				
18. Flow length	th, L				ft	108	306				
	1					1	1		1		

0.009



- 14. Hydraulic radius, r15. Channel slope, sPw Compute
- 16. Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V
- 18. Flow length, L

 19. $T_t = \frac{L}{3600 \text{ V}}$ Compute T_t ft

 hr

 +

 =

 0.000

 hr

 0.05

ft/ft

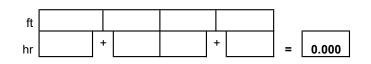
ft/s

Proj	ect JSMC West Additi	on		Ву	SPT	Date				
Loca	ation Township of Nept	une, Monmou	th County, NJ	Checked	MI	Date				
Circ	ele One: Present Develope	ed						-		
Circ	ele One: T _c T _t	through suba	irea	Exis	ting DA-2, I	mpervious	s, Future S	Storm		
ГОИ	TES: Space for as many as two segr worksheet.	nents per flow	type can be us	ed for each					-	
	Include a map, schematic, or	description of	flow segments.							
<u>She</u>	eet flow (Applicable to T _c Only)			Segment ID	1					
1.	Surface description (table 3-1)				Asphalt					
2.	Manning's roughness coeff., n (Tab	le 15-1)			0.011					
	o ,			ft	100					
4.	Two-yr 24-hr rainfall, P ₂			in	4.14					
5.	Land slope, s			ft/ft	0.024	<u> </u>				
6.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{2}^{0.5}s^{0.4}}$		Compute T _t	hr	0.017	+		+	=	0.017
Sha	Illow concentrated flow			Segment ID	2]	
7.	Surface description (paved or unpar	ved)			Paved					
8.	Flow length, L			ft	68					
9.	Watercourse slope, s			ft/ft	0.031					
10.	Average velocity, V (Figure 15-4)			ft/s	3.58					
11.	$T_t = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.005	+		+	=	0.005
<u>Cha</u>	nnel flow			Segment ID						
12.	Cross sectional flow area, a			ft ²	0.785398	1.76715				
13.	Wetted perimeter, p _w			ft	3.141593	4.71239				
14.	Hydraulic radius, r	$r = \frac{a}{-p_w}$	Compute r	ft	0.25	0.38				
15.	Channel slope, s			ft/ft	0.005	0.010				
16.	Manning's roughness coeff., n				0.013	0.013				
	$V = $ 1.49 $r^{2/3} s^{1/2}$	_								
17.	n		Compute V	ft/s	3.22	5.96				
18.	Flow length, L			ft	108	306				
19.	$T_t = \frac{L}{3600 \text{ V}}$		Compute T _t	hr	0.009	0.014		+	=	0.024
				Segment ID						
12.	Cross sectional flow area, a			ft ²						
13.	Wetted perimeter, p _w			ft						
14	Hydraulic radius, r	$r = \frac{a}{p_w}$	Compute r	ft						
	Channel slope, s	1 44	Compute 1	ft/ft					-	
	Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$			1011					 	
17.	V = 1.491 S	-	Compute V	ft/s						

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

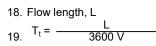
Project	JSMC West Addition		Ву	SPT	Date				
Location	Township of Neptune,	Monmouth County, NJ	Checked	MI	Date				
Circle One:	Present Developed								
Circle One: (T_c T_t	through subarea	Existing	g DA-1, Pe	rvious, Cu	rrent Storn	n		
	for as many as two segmen sheet.	ts per flow type can be used for ea	ach						
Includ	de a map, schematic, or des	scription of flow segments.							
Sheet flow (App	plicable to T _c Only)		Segment ID	1 Short					
1. Surface des	scription (table 3-1)			Grass					
2. Manning's r	oughness coeff., n (Table 1	5-1)		0.150					
3. Flow Length	h, L		ft	75					
4. Two-yr 24-h	nr rainfall, P ₂		in	3.48					
5. Land slope,	, S		ft/ft	0.012				l .	
6. T _t =0	.007(nL) ^{0.8} P ₂ ^{0.5} s ^{0.4}	Compute T _t	hr	0.153	+			= [0.153
Shallow conce	ntrated flow		Segment ID	2					
7. Surface des	scription (paved or unpaved))		Paved					
8. Flow length	, L		ft	57					
9. Watercours	e slope, s		ft/ft	0.023					
10. Average ve	locity, V (Figure 15-4)		ft/s	3.00				ļ ,	
11. T _t =	L 3600 V	Compute T _t	hr	0.005	+			=	0.005
Channel flow			Segment ID						
12. Cross section	onal flow area, a		ft ²	1.76715	1.76715				
13. Wetted peri	imeter, p _w	2	ft	4.71239	4.71239				
14. Hydraulic ra	adius, r	$r = \frac{a}{P_W}$ Compute r	ft	0.38	0.38				
15. Channel slo	ppe, s		ft/ft	0.007	0.010				
16. Manning's r	roughness coeff., n 1.49 r ^{2/3} s ^{1/2} n			0.013	0.013				
17. V =	n	Compute V	ft/s	4.99	5.96				
18. Flow length			ft	120	306				
19. T _t =	3600 V	Compute T _t	hr	0.007	0.014	+		=	0.021

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, p_w
- 14. Hydraulic radius, r
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

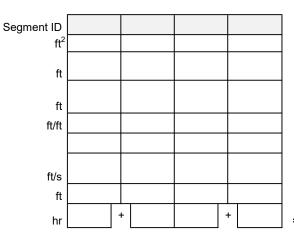
	V =	$1.49 r^{2/3} s^{1/2}$	
17.	v –	n	

Compute V

Compute r



Compute T_t



0.000

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

0.18 hr

Project	JSMC West Addition	on		Ву	SPT	Date		
Location	Township of Neptu	ıne, Monmou	th County, NJ	Checked	MI	Date		
Circle One:	Present Develope	d	-					•
Circle One:	T_c T_t	through suba	ırea	Exi	sting DA-1	, Pervious	, Future St	orm
NOTES: Space for worksh	or as many as two segm eet.	ents per flow	type can be use	ed for each				
Include	a map, schematic, or d	lescription of f	low segments.					
Sheet flow (Appli	cable to T _c Only)			Segment ID	1			
					Short			
	ription (table 3-1)				Grass			
_	ughness coeff., n (Table	e 15-1)			0.15			
3. Flow Length,				ft	75			
4. Two-yr 24-hr	_			in	4.14			
5. Land slope, s				ft/ft	0.012	<u> </u>	 	<u> </u>
6. $T_t = \frac{0.0}{F}$	$\frac{07(nL)^{0.8}}{2^{0.5}s^{0.4}}$		Compute T _t	hr	0.140	+		+
Shallow concent	rated flow			Segment ID	2			
•	ription (paved or unpav	od)		Segment ib	Paved			
8. Flow length, I		eu)		ft	57			
9. Watercourse				ft/ft	0.023			
	city, V (Figure 15-4)			ft/s	3			
_	, , ,		Compute T _t			+		+
11. T _t =3	8600 V		Compute 1 _t	hr	0.005			
Channel flow				Segment ID				
12. Cross section	nal flow area, a			ft ²	1.767146	1.76715		
13. Wetted perim				ft	4.712389	4.71239		
14. Hydraulic rad	ius, r	$r = \frac{a}{P_W}$	Compute r	ft	0.38	0.38		
15. Channel slop			'	ft/ft	0.007	0.010		
16. Manning's ro					0.013	0.013		
V =	1.49 r ^{2/3} s ^{1/2}	_						
			Compute V	ft/s	4.99	5.96		
18. Flow length, I				ft	120	306		<u> </u>
19. T _t =	8600 V		Compute T _t	hr	0.007	0.014		+
10 0	al flavores -			Segment ID ft ²				
12. Cross section	nai tiow area, a			π				
13. Wetted perim	eter, p _w			ft				
		$r = \frac{a}{p_w}$						
14. Hydraulic rad		p_w	Compute r	ft				
15. Channel slop				ft/ft				
16. Manning's ro	-							
V =	1.49 r ^{2/3} s ^{1/2}	_				1		

Compute V

ft/s ft 0.000 hr

0.17

18. Flow length, L

3600 V

Compute T_t

20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

EDA2-Per - 2/2

Project	JSMC W	est Addition			_By	SPT	Date		_		
Location	Townshi	p of Neptune, M	onmouth Coun	ty, NJ	_Checked	MI	Date		_		
Circle One:	Present	Developed								_	
Circle One:	T _c	T_t	through subarea	ı	Proposed	l DA-1, lm	pervious, (Current S	torm	-	
NOTES: Space works	•	as two segments	per flow type ca	n be used for ea	ich						
Includ	le a map, sc	hematic, or desci	ription of flow se	gments.							
Sheet flow (App	olicable to T _o	Only)			Segment ID	1					
1. Surface des	cription (tab	le 3-1)				Asphalt					
2. Manning's re	oughness co	eff., n (Table 15-	1)			0.011					
3. Flow Length	ı, L				ft	100					
4. Two-yr 24-h	r rainfall, P ₂				in	3.48					
5. Land slope,	S				ft/ft	0.016					
6. T _t =0.	007(nL) ^{0.8}	_		Compute T _t	hr	0.021	+		+] =	0.021
	P ₂ s				1		1		1	,	
Shallow concer	ntrated flow	<u></u>			Segment ID	2					
7. Surface des	cription (pav	ved or unpaved)				Paved					
8. Flow length,	L				ft	260					
9. Watercourse	e slope, s				ft/ft	0.010					
10. Average vel	ocity, V (Fig	ure 15-4)			ft/s	2.00					
11. T _t =	Z600 V	<u> </u>		Compute T _t	hr	0.036	+		+] =	0.036
	3000 V										
Channel flow					Segment ID	3	4	5	6		
12. Cross section	nal flow are	a, a			ft ²	3.14159	4.90874				
13. Wetted period	meter, p _w				ft	6.28319	7.85398				
14. Hydraulic ra	dius, r		$r = \frac{a}{P_W}$	Compute r	ft	0.50	0.63				
15. Channel slo	pe, s				ft/ft	0.005	0.004				
16. Manning's ro	oughness co 1.49 r ²	oeff., n ' ^{/3} s ^{1/2}				0.013	0.013				
17 V =	n	-		Compute V	ft/s	5.11	5.36				

264

0.014

124

0.006

18. Flow length, L

L 3600 V

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, pw

14. Hydraulic radius, r

15. Channel slope, s

16. Manning's roughness coeff., n

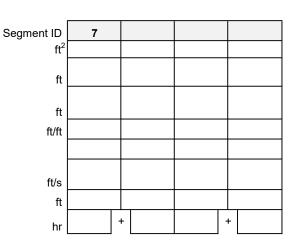
 $V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$

Compute V

Compute r

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



0.000

0.08

hr

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

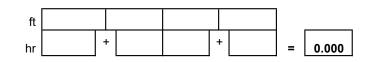
PDA1-Imper - 2/2

Project	JSMC West Addit	tion		Ву	SPT	Date		_		
Location	Township of Nep	tune, Monmou	th County, NJ	Checked	MI	Date		_		
Circle One:	Present Develop	ed							_	
Circle One:	T_c T_t	through suba	area	Prope	osed DA-1,	Imperviou	s, Future	Storm	_	
	as many as two seg	ments per flow	type can be use	ed for each						
workshe										
	a map, schematic, or	description of	flow segments.							
Sheet flow (Applic	able to T _c Only)			Segment ID	1					
1. Surface descri	iption (table 3-1)				Asphalt					
2. Manning's roug	ghness coeff., n (Tab	ole 15-1)			0.011					
3. Flow Length, L				ft	100					
4. Two-yr 24-hr ra	ainfall, P ₂			in	4.14					
5. Land slope, s				ft/ft	0.016					
6. $T_t = \frac{0.00}{P_2}$	07(nL) ^{0.8}		Compute T _t	hr	0.019	+		+	=	0.019
P ₂	$^{0.5}$ s ^{0.4}									
Shallow concentra	ated flow			Segment ID	2					
	 iption (paved or unpa	aved)		J	Paved					
8. Flow length, L		,		ft	260					
9. Watercourse s				ft/ft	0.01					
	ity, V (Figure 15-4)			ft/s	2					
-			Compute T _t	hr		+		+	=	0.036
11. T _t =36	600 V		, ,						1	
									I	
Channel flow				Segment ID	3					
12. Cross sectiona				ft ²		4.90874				
13. Wetted perime		r = <u>a</u>		ft	6.283185	7.85398				
14. Hydraulic radiu		r = -pw	Compute r	ft	0.50	0.63				
15. Channel slope				ft/ft	0.005	0.004				
16. Manning's rou	-				0.013	0.013				
V =	1.49 r * S * n	_	Compute V	ft/s	5.11	5.36				
18. Flow length, L			- '	ft	264	124				
т –					Ι.			+		
19. 1t - 36	600 V		Compute T _t	hr	0.014	0.006			=	0.021
									1	
10. 0	- I <i>6</i> I			Segment ID ft ²	4					
12. Cross sectiona	al flow area, a			IL						
13. Wetted perime	eter, p _w			ft						
		$r = \frac{a}{p_w}$								
14. Hydraulic radiu		$\rho_{\rm w}$	Compute r	ft						
15. Channel slope				ft/ft						
16. Manning's rou	ghness coeff., n 1.49 r ^{2/3} s ^{1/2}								1	
V =	1.49 r s n		Compute V	ft/s					1	

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

Project	JSMC W	est Addition			_By	SPT	Date				
Location	Townshi	ip of Neptune, M	onmouth Count	y, NJ	Checked	MI	Date				
Circle One:	Present	Developed								_	
Circle One:	T_c	T_t	through subarea		Propose	ed DA-1, P	ervious, C	urrent Sto	rm		
NOTES: Space i		as two segments	per flow type ca	n be used for ea	ach						
Includ	de a map, so	chematic, or desci	ription of flow sec	gments.							
Sheet flow (App	olicable to T	only)			Segment ID	1					
Surface des	cription (tab	le 3-1)				Short Grass					
2. Manning's ro	oughness co	peff., n (Table 15-	1)			0.150					
3. Flow Length	ı, L				ft	74					
4. Two-yr 24-h	r rainfall, P ₂				in	3.48					
5. Land slope,	s				ft/ft	0.033					
6. T _t =0.	$\frac{.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$	_		Compute T _t	hr	0.101	+	-	+	=	0.101
Shallow concer	ntrated flow	<u>/</u>			Segment ID						
7. Surface des	cription (pa	ved or unpaved)									
8. Flow length,	, L				ft						
9. Watercourse	e slope, s				ft/ft						
10. Average vel	ocity, V (Fig	ure 15-4)			ft/s						
11. T _t =	L 3600 V			Compute T _t	hr		+			=	0.000
Channel flow					Segment ID	2	3	4			
12. Cross section	onal flow are	a. a			ft ²	1.76715	3.14159	4.90874			
13. Wetted peri		,			ft	4.71239	6.28319	7.85398			
14. Hydraulic ra			$r = \frac{a}{P_w}$	Compute r	ft	0.38	0.50	0.63			
15. Channel slo	pe, s			•	ft/ft	0.005	0.005	0.004			
16. Manning's ro	-					0.013	0.013	0.013			
17. V =	1.49 r ² r			Compute V	ft/s	4.21	5.11	5.36			

18. Flow length, L

198

0.011

124

0.006

138

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, pw
- 14. Hydraulic radius, r

15. Channel slope, s

16. Manning's roughness coeff., n

 $V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$

Compute V

Compute r

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

Segment ID	4	5	6	
ft ²				
ft				
ft				
ft/ft				
ft/s				
ft				
		+		+
hr		r		T

0.000

0.13

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

PDA1-Per - 2/2

Project	JSMC West Addition	on		Ву	SPT	Date					
Location	Township of Neptu	ıne, Monmou	ıth County, NJ		MI	Date					
Circle One:	Present Develope		•	_							
Circle One:	T_c T_t	through suba	area	Prop	oosed DA-1	I, Pervious	, Future S	torm			
NOTES: Space for	as many as two segm	ents per flow	type can be us	ed for each							
NOTES: Space for as many as two segments per flow type can be used for each worksheet.											
Include	a map, schematic, or c	description of	flow segments.								
Sheet flow (Applic	able to T _c Only)			Segment ID	1						
					Short						
Surface descri	•				Grass						
-	ghness coeff., n (Table	e 15-1)		_	0.150						
3. Flow Length, L				ft	74						
4. Two-yr 24-hr r	aintali, P ₂			in	4.14						
5. Land slope, s	0.0			ft/ft	0.033	<u> </u> .					
6. $T_t = \frac{0.00}{P_t}$	07(nL) ^{0.8}		Compute T _t	hr	0.092	+		+	=	0.092	
P ₂	2 S S					I	ı		ı		
Shallow concentr	ated flow		9	Segment ID							
7. Surface descr	iption (paved or unpav	ed)									
8. Flow length, L				ft							
9. Watercourse	slope, s			ft/ft							
10. Average veloc	ity, V (Figure 15-4)			ft/s		<u> </u>					
11. T _t =36	L		Compute T_t	hr		+		+	=	0.000	
` 36	600 V										
Channel flow			5	Segment ID	3	4	5	6			
12. Cross sectiona	al flow area, a			ft ²	1.767146	3.14159	4.90874				
13. Wetted perime	eter, p _w	_		ft	4.712389	6.28319	7.85398				
14. Hydraulic radio	us, r	r = - a - Pw -	Compute r	ft	0.38	0.50	0.63				
15. Channel slope	e, s			ft/ft	0.005	0.005	0.004				
16. Manning's rou					0.013	0.013	0.013				
17. V =	1.49 r ^{2/3} s ^{1/2}	-	Community V	£1/0	4.04	F 44	F 20				
	П		Compute V	ft/s	4.21	5.11	5.36				
18. Flow length, L	L			ft	138	198	124				
19. T _t =	600 V		Compute T_t	hr	0.009	0.011	0.006	+	=	0.026	
			9	Segment ID	7	8					
12. Cross sections	al flow area, a			ft ²							
13. Wetted perime	eter, p _w			ft							
, o	/ 1 W	<u>, _ a _</u>		10							
14. Hydraulic radio	us, r	$r = \frac{a}{p_w}$	Compute r	ft							
15. Channel slope	e, s			ft/ft							
16. Manning's rou											
V =	1.49 r ^{2/3} s ^{1/2}	_									

17. n

Compute V

18. Flow length, L

 $T_t = \frac{L}{3600 \text{ V}}$

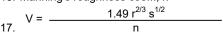
Compute T_t

20. Watershed or subarea T_c or $T_t \, (\text{add } T_t \, \text{in steps 6, 11, 19})$

hr

Project	JSMC We	est Addition			_By	SPT	Date				
Location	Township	of Neptune, N	Ionmouth County	, NJ	Checked	MI	Date		_		
Circle One:	Present	Developed							-		
Circle One:	T _c	T_t	through subarea		Proposed	I DA-2, Im	pervious, C	Current St	orm		
NOTES: Space for worksl	,	as two segment	s per flow type can	be used for ea	ach						
Include	e a map, sch	nematic, or desc	ription of flow segr	ments.							
Sheet flow (App	licable to T _c	Only)			Segment ID	1					
Surface description	cription (table	e 3-1)				Asphalt					
2. Manning's ro	oughness co	eff., n (Table 15	-1)			0.011					
3. Flow Length,	, L				ft	100					
4. Two-yr 24-hr	rainfall, P ₂				in	3.48					
5. Land slope,	S				ft/ft	0.024					
6. $T_t = \frac{0.0}{I}$	007(nL) ^{0.8} P ₂ ^{0.5} s ^{0.4}	_		Compute T _t	hr	0.018	+	-	+	=	0.018
Shallow concen	trated flow				Segment ID	2					
7. Surface desc	cription (pave	ed or unpaved)				Paved					
8. Flow length,	L				ft	54					
9. Watercourse	slope, s				ft/ft	0.190					
10. Average velo	ocity, V (Figu	ıre 15-4)			ft/s	2.80					
11. T _t =	L 3600 V	_		Compute T _t	hr	0.005	+	-	+	= [0.005
Channel flow					Segment ID						
12. Cross section	nal flow area	a, a			ft ²	0.7854	1.76715				
13. Wetted perin	neter, p _w		2		ft	3.14159	4.71239				
14. Hydraulic rad	dius, r		$r = \frac{a}{P_w}$	Compute r	ft	0.25	0.38				
15. Channel slop	oe, s				ft/ft	0.005	0.010				
16. Manning's ro	oughness co	eff., n				0.013	0.013				
V =	1.49 r ^{2/3}	~ S" ²	-	Compute V	ft/s	3.22	5.96				
18. Flow length,				Compate V	ft	116	306				
T _t =		_		Compute T _t	hr		+ 0.014		+	= [0.024

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, pw
- 14. Hydraulic radius, r
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

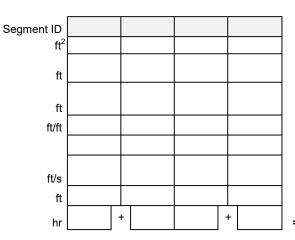


Compute V

Compute r

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



0.000

0.05

hr

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

Proj	ect JSMC West Addition	on	Ву	SPT	Date		
Loca		ine, Monmouth County, NJ	Checked	MI	Date		
Circl	le One: Present Develope	d			-		
Circl	le One: T_c T_t	through subarea	Exis	ting DA-2,	Impervious	, Future Storm	-
NOT	ES: Space for as many as two segm worksheet.	ents per flow type can be us	ed for each				
	Include a map, schematic, or o	lescription of flow segments.					
<u>She</u>	et flow (Applicable to T _c Only)		Segment ID	1			
1.	Surface description (table 3-1)			Asphalt			
2.	Manning's roughness coeff., n (Table	e 15-1)		0.011			
3.	Flow Length, L		ft	100			
4.	Two-yr 24-hr rainfall, P ₂		in	4.14			
5.	Land slope, s		ft/ft	0.024			
6.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{2}^{0.5}s^{0.4}}$	Compute T _t	hr	0.017	+	+] =
	2		Sogment ID	0]
	Surface description (payed or upper	ad)	Segment ID	2 Payed			-
	Surface description (paved or unpav	ea)	tı.	Paved			
	Flow length, L		ft	54			
	Watercourse slope, s		ft/ft	0.19			1
	Average velocity, V (Figure 15-4)	0	ft/s	2.8	+		
11.	$T_t = \frac{L}{3600 \text{ V}}$	Compute T _t	hr	0.005	⁺ []] =
<u>Cha</u>	nnel flow		Segment ID				
12.	Cross sectional flow area, a		ft ²	0.785398	1.76715		
13.	Wetted perimeter, p _w		ft	3.141593	4.71239		
14.	Hydraulic radius, r	r = <u>a</u> Compute r	ft	0.25	0.38		
15.	Channel slope, s	·	ft/ft	0.005	0.010		
	Manning's roughness coeff., n			0.013	0.013		1
	$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$	_					1
17.		Compute V	ft/s	3.22	5.96		-
18.	Flow length, L		ft	116	306		-
19.	$T_t = \frac{L}{3600 \text{ V}}$	Compute T_t	hr	0.010	0.014	+	=
			1		1 1		1
4.5	0 " 1"		Segment ID				
12.	Cross sectional flow area, a		ft ²				1
13.	Wetted perimeter, p _w		ft				
		$r = \frac{a}{p_w}$ Compute r					
14.	Hydraulic radius, r	p _w Compute r	ft				
15.	Channel slope, s		ft/ft				
16.	Manning's roughness coeff., n $V = \frac{1.49 r^{2/3} s^{1/2}}{}$						

17 n

Compute V

18. Flow length, L

9. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t

20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

Project	JSMC We	st Addition			Ву	SPT	Date				
Location	Township	of Neptune, N	Monmouth Count	y, NJ	Checked	MI	Date				
Circle One:	Present	Qeveloped									
Circle One:	T _c	T_{t}	through subarea		Propose	ed DA-1, Po	ervious, Cı	urrent Sto	m		
NOTES: Space f		as two segment	s per flow type car	n be used for ea	ich						
		nematic or desc	cription of flow seg	ımente							
	•		cription of now seg	jillellis.	0	4					
Sheet flow (App	nicable to 1 _c	Offiy)			Segment ID	1 Short					
Surface des	cription (table	e 3-1)				Grass					
2. Manning's ro	oughness coe	eff., n (Table 15	5-1)			0.240					
3. Flow Length	, L				ft	41					
4. Two-yr 24-hi	r rainfall, P ₂				in	3.48					
5. Land slope,	s				ft/ft	0.015					
6. T _t =0.	007(nL) ^{0.8}	_		Compute T_t	hr	0.125	+			=	0.125
	P ₂ ^{0.5} s ^{0.4}										
Shallow concer	ntrated flow				Segment ID	2					
7. Surface des	cription (pave	ed or unpaved)				Paved					
8. Flow length,	L				ft	67					
9. Watercourse	e slope, s				ft/ft	0.007					
10. Average vel	ocity, V (Figu	ire 15-4)			ft/s	1.70					
11. T _t =	L	_		Compute T_t	hr	0.011	+			=	0.011
	3600 V										
Channel flow					Segment ID						
12. Cross section	nal flow area	ı, а			ft ²	1.76715	1.76715				
13. Wetted perir	meter, p _w				ft	4.71239	4.71239				
14. Hydraulic ra	dius, r		$r = \frac{a}{P_W}$	Compute r	ft	0.38	0.38				
15. Channel slo	pe, s				ft/ft	0.005	0.010				
16. Manning's ro	oughness coe	eff., n				0.013	0.013				
V =	1.49 r ^{2/3}	³ s ^{1/2}	_	Compute V	ft/s	4.21	5.96				
18. Flow length,				Joinputo v	ft	228	306				
•	L 3600 V	_			"		<u> </u>		+		
19.	3600 V			Compute T _t	hr	0.015	0.014			=	0.029

- 12. Cross sectional flow area, a
- 13. Wetted perimeter, p_w
- 14. Hydraulic radius, r
- 15. Channel slope, s
- 16. Manning's roughness coeff., n

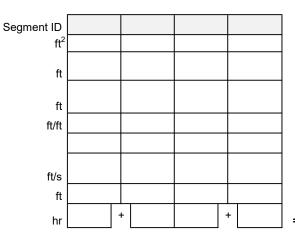
$$V = \frac{1.49 \, r^{2/3} \, s^{1/2}}{n}$$

Compute V

Compute r

18. Flow length, L 19. $T_t = \frac{L}{3600 \text{ V}}$

Compute T_t



0.000

0.17

hr

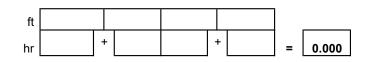
20. Watershed or subarea T_{c} or T_{t} (add T_{t} in steps 6, 11, 19)

Pro	ject JSMC West Addition	JSMC West Addition				Date				
Loc	ation Township of Neptu	ine, Monmouth Co	ounty, NJ	Checked	MI	Date				
Circ	cle One: Present Develope	d								
Circ	cle One: T_c T_t	through subarea		Prop	osed DA-1	, Pervious	, Future S	torm		
NO	TES: Space for as many as two segm worksheet.	ents per flow type	can be use	ed for each						
	Include a map, schematic, or o	description of flow s	segments.							
She	eet flow (Applicable to T _c Only)			Segment ID	1					
1.	Surface description (table 3-1)				Short Grass					
	Manning's roughness coeff., n (Table	e 15-1)			0.24					
3.		,		ft	41					
4.	Two-yr 24-hr rainfall, P ₂			in	4.14					
5.	Land slope, s			ft/ft	0.015					
6	$\tau = 0.007(nL)^{0.8}$	Со	mpute T _t	hr	0.115	+		+	=	0.115
0.	$T_{t} = \frac{0.007(nL)^{0.8}}{P_{2}^{0.5}s^{0.4}}$			•					'	
Sha	allow concentrated flow			Segment ID	2					
	Surface description (paved or unpav	ed)		3	Paved					
	Flow length, L	,		ft	67					
	Watercourse slope, s			ft/ft	0.007					
	Average velocity, V (Figure 15-4)			ft/s	1.70					
	,	Со	mpute T _t	hr		+	-	+	=	0.011
11.	$T_t = \frac{L}{3600 \text{ V}}$			•					,	
Cha	annel flow			Segment ID						
	Cross sectional flow area, a			ft ²	1.767146	1.76715				
	Wetted perimeter, p _w			ft	4.712389	4.71239				
	Hydraulic radius, r	$r = \frac{a}{Pw}$	ompute r	ft	0.38	0.38				
	Channel slope, s	0.	omputo i	ft/ft	0.005	0.010				
	Manning's roughness coeff., n				0.013	0.013				
	$V = \frac{1.49 r^{2/3} s^{1/2}}{1.49 r^{2/3} s^{1/2}}$	_								
17.	n	Co	ompute V	ft/s	4.21	5.96				
18.	Flow length, L			ft	228	306				
19.	$T_t = \frac{L}{3600 \text{ V}}$	Со	mpute T _t	hr	0.015	0.014		+	=	0.029
				Segment ID						
12.	Cross sectional flow area, a			ft ²						
13.	Wetted perimeter, p _w			ft						
. • •	. , , , , , , , , , , , , , , , , , , ,	r = <u>a</u>								
14.	Hydraulic radius, r	$r = \frac{a}{p_w}$ Co	ompute r	ft						
15.	Channel slope, s			ft/ft						
16.	Manning's roughness coeff., n									
17.	$V = \frac{1.49 r^{2/3} s^{1/2}}{n}$	Cc	ompute V	ft/s						
	••	00		.,,,						

18. Flow length, L

19. $T_t = \frac{L}{3600 \text{ V}}$

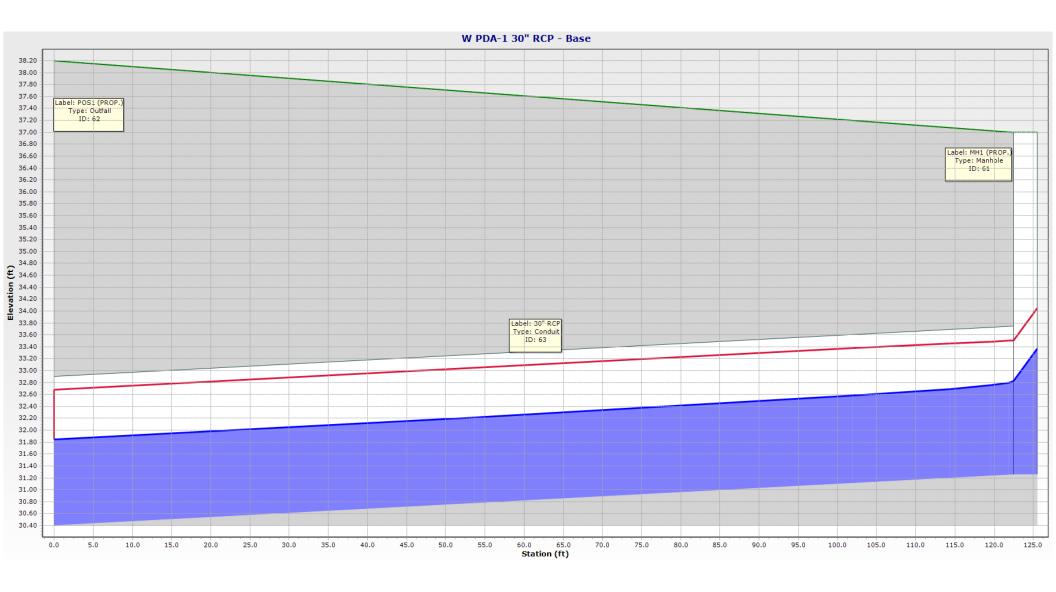
Compute T_t

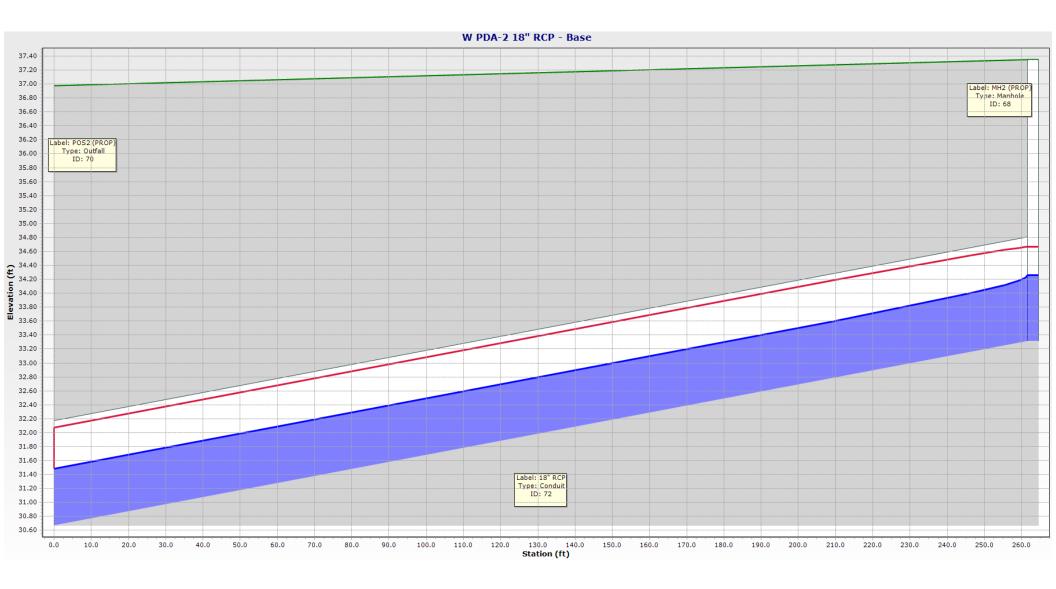


20. Watershed or subarea T_c or T_t (add T_t in steps 6, 11, 19)

FlexTable: Conduit Table

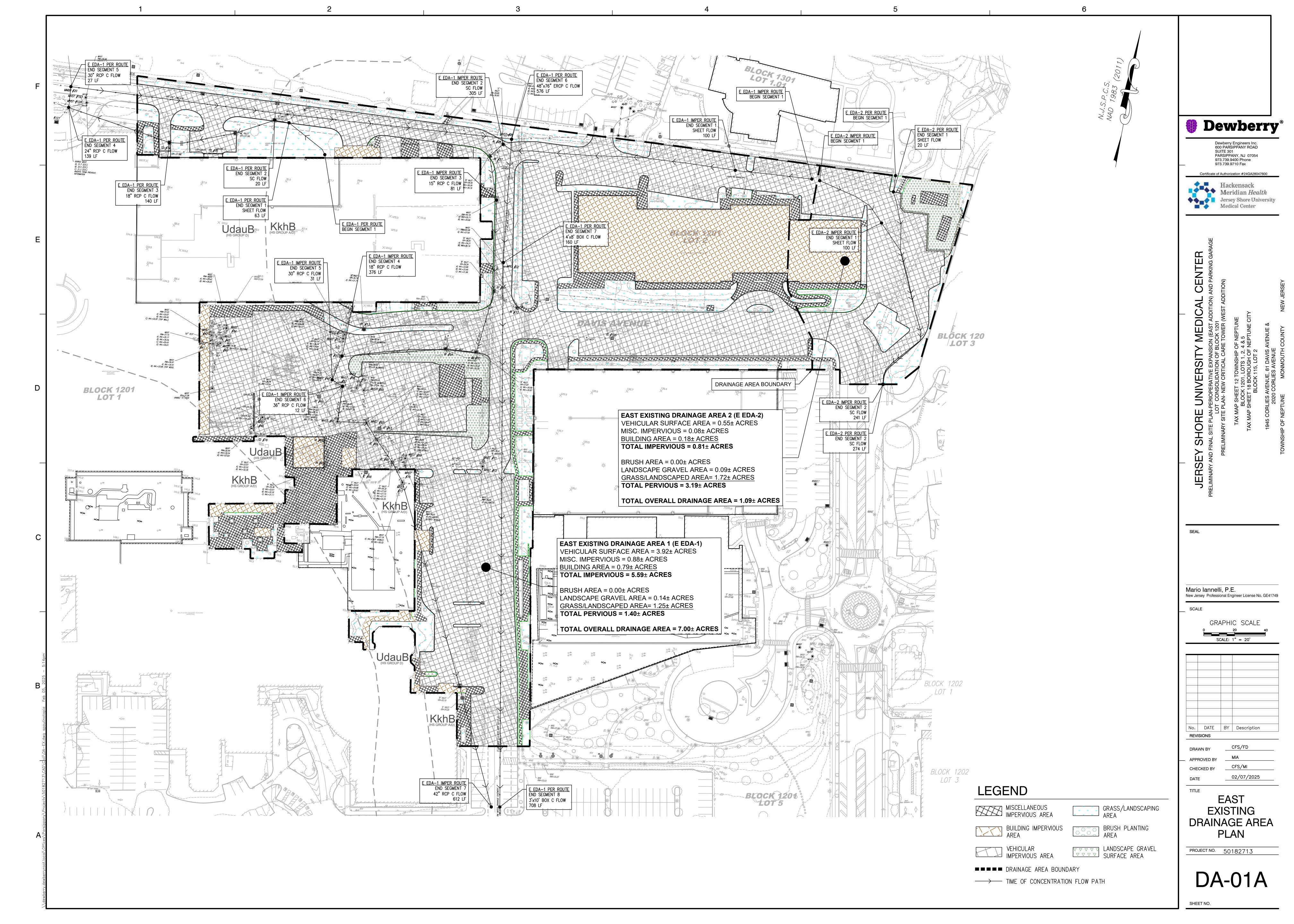
Label	Start Node	Stop Node	Invert (Start) (ft)	Invert (Stop) (ft)	Length (Unified) (ft)	Slope (Calculated)	Diameter (in)	Flow (cfs)	Capacity (Full Flow)
			()	()	()	`(ft/ft)	. ,	,	(cfs)
30" RCP	MH1 (EXIST.)	POS1 (EXIST.)	31.25	30.40	124.0	0.007	30.0	21.95	33.96
30" RCP	MH1 (PROP.)	POS1 (PROP.)	31.25	30.40	124.0	0.007	30.0	21.55	33.96
18" RCP	MH2 (EXIST)	POS2 (EXIST)	33.31	30.67	263.0	0.010	18.0	5.50	10.52
18" RCP	MH2 (PROP)	POS2 (PROP)	33.31	30.67	263.0	0.010	18.0	6.05	10.52

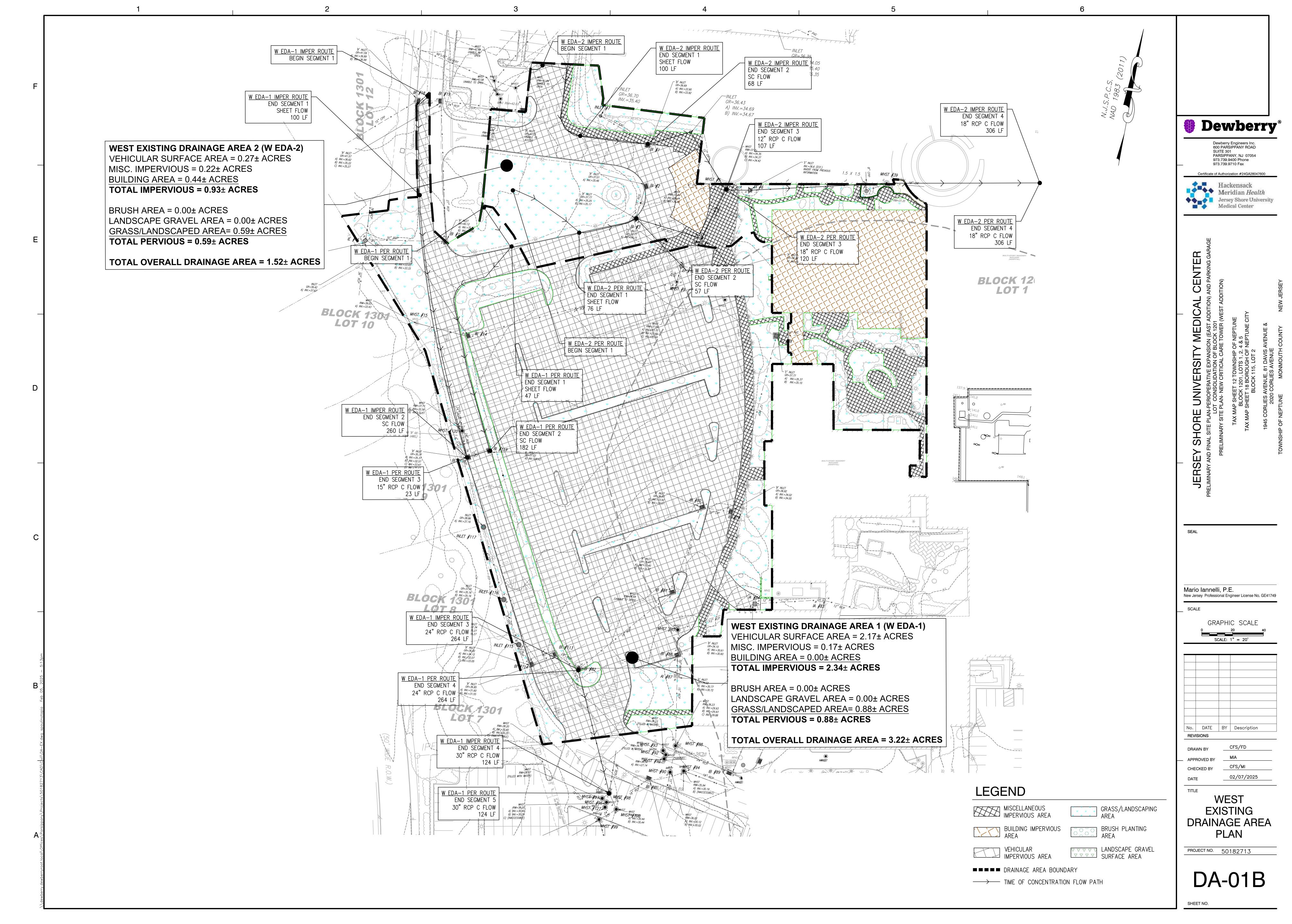


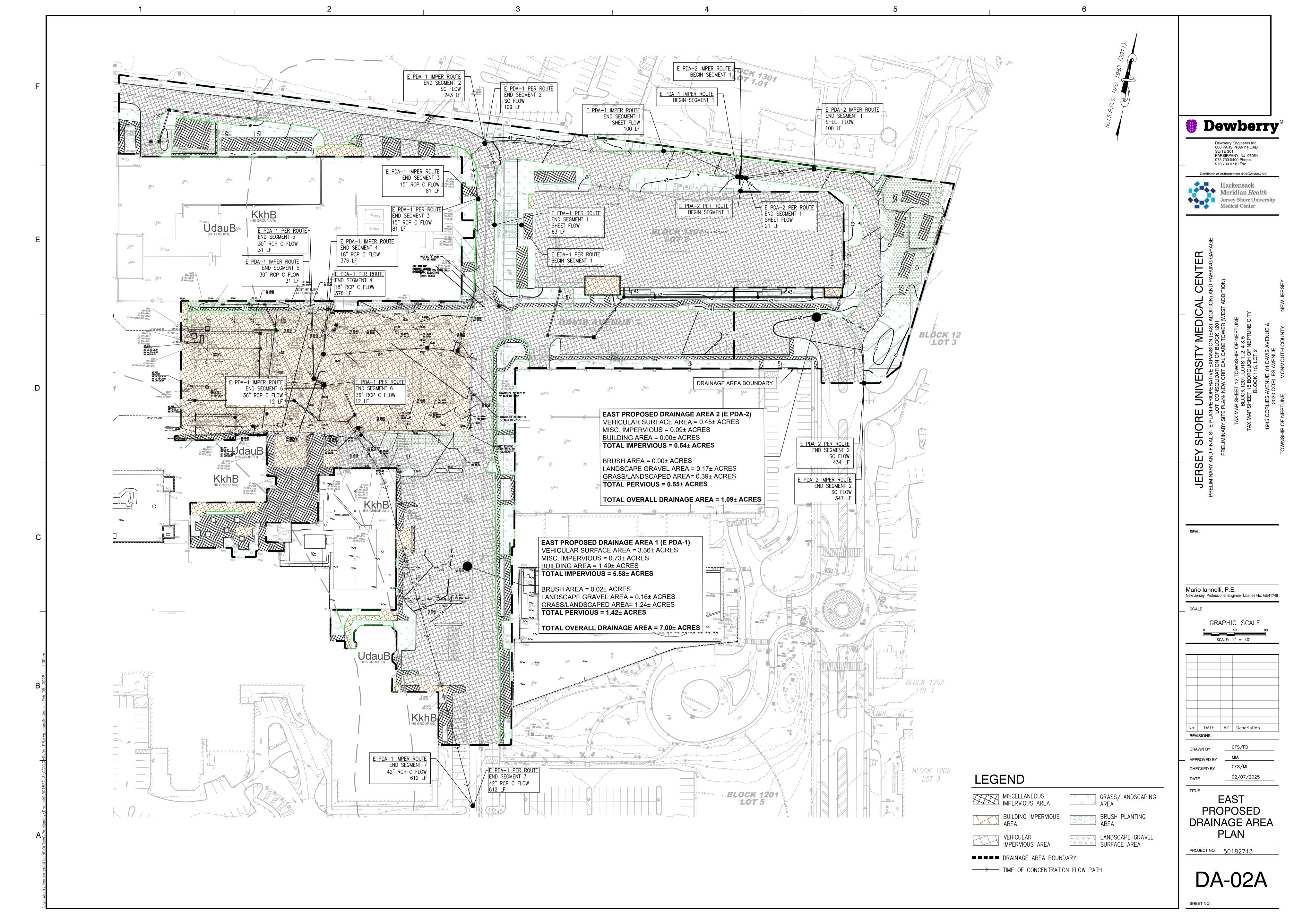


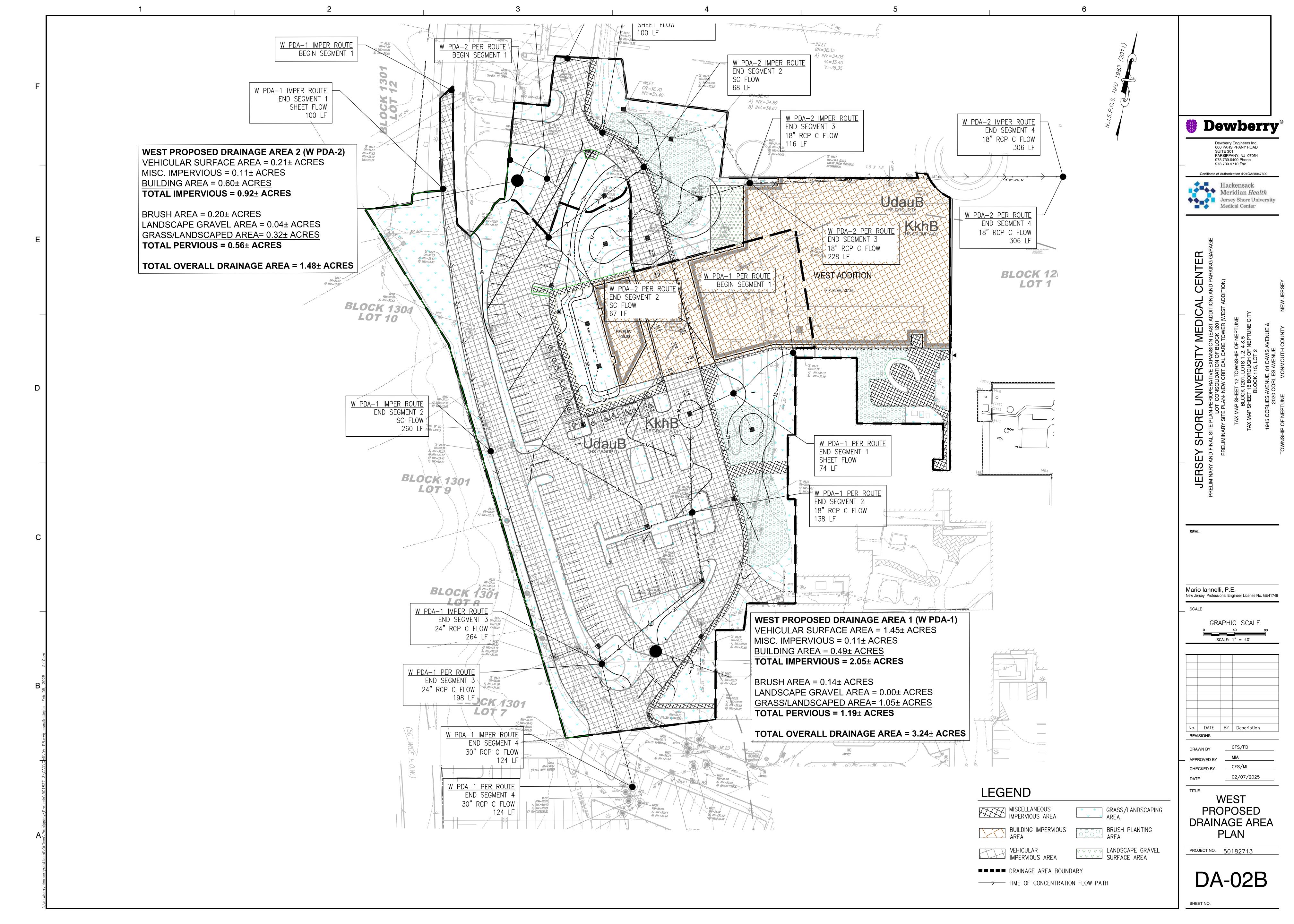
APPENDIX IV:

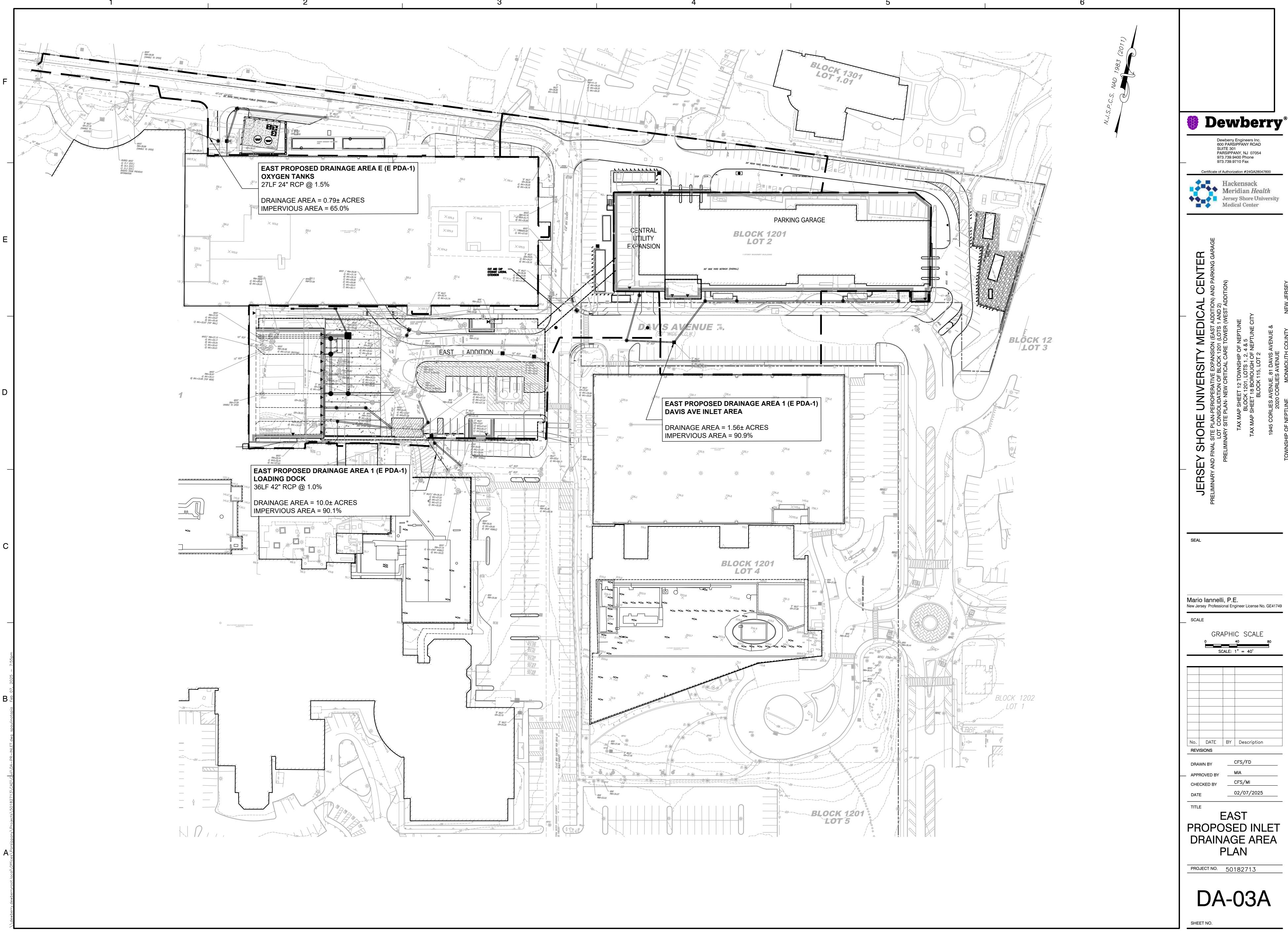
DA-01A East Existing Site Drainage Area Plan DA-01B West Existing Site Drainage Area Plan DA-02A East Proposed Site Drainage Area Plan DA-02B West Proposed Site Drainage Area Plan DA-03A East Proposed Inlet Area Area Plan DA-03B West Proposed Inlet Area Area Plan

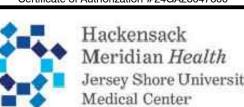


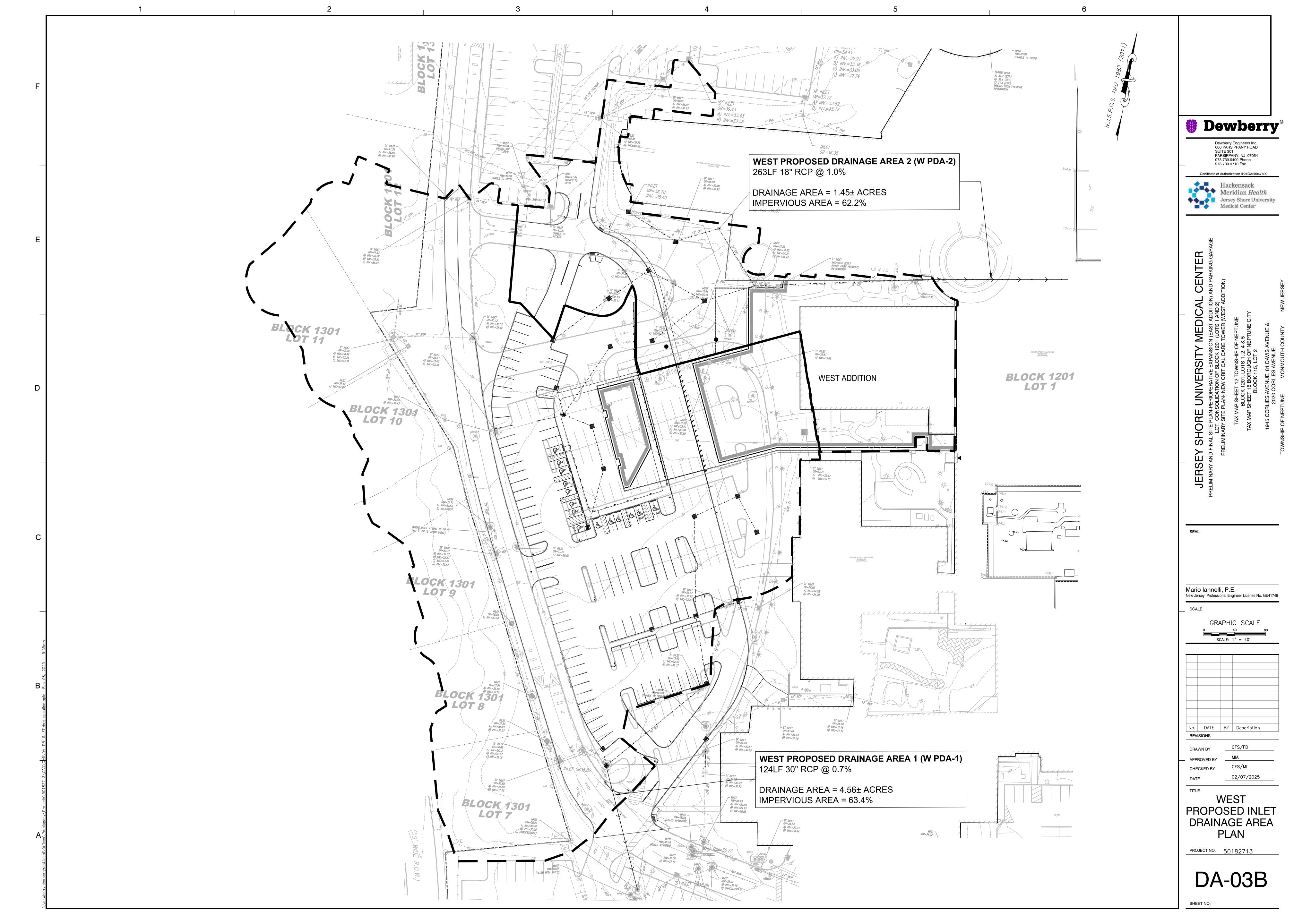












APPENDIX V:

Operations & Maintenance Manual



Operations & Maintenance Manual for Stormwater Management Facilities

Jersey Shore University Medical Center Parking Garage, Perioperative Expansion – East, & Critical Care Tower – West

Block 1201 Lots 1, 2, 4, & 5 1945 Corlies Avenue & 81 Davis Avenue Township of Neptune Monmouth County, NJ 07753

February 07, 2025

Prepared for: **Jersey Shore University Medical Center** 1945 State Highway Route 33 Neptune, NJ 07753

Prepared by: **Dewberry Engineers Inc.**600 Parsippany Road Suite 301
Parsippany, NJ 07054
Certificate of Authorization
No. 24GA28047600
(973) 739-9400

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PART I: PROJECT DETAILS

A. INTRODUCTION

This Operations and Maintenance ("O&M") Manual has been prepared for Jersey Shore University Medical Center, applicant of the properties known as Lots 1, 2, 4 & 5 of Block 1202 in the Township of Neptune, Monmouth County, NJ. The drainage features and landcover are proposed to be maintained by the site owners. Failure to repair and maintain the drainage system may be cause for the Township of Neptune to undertake all reasonably necessary repairs or maintenance and to charge such to the owners as a municipal lien on those properties.

This Operations and Maintenance ("O&M") Manual has been prepared in accordance with the standards of the New Jersey Stormwater Best Management Practices (BMP) Manual, last revised March 2021. Chapter 8 of the BMP Manual outlines the necessity for regular inspections and maintenance of stormwater facilities:

"Regular and thorough maintenance is necessary for stormwater management measures to perform effectively and reliably. They have also demonstrated that failure to perform such maintenance can lead to diminished performance, deterioration, and failure, in addition to a range of health and safety problems including mosquito breeding, vermin, and the potential for drowning. The potential for such problems to develop is accentuated by many of the very features and characteristics that allow stormwater management measures to do their job, including standing or slowing moving water, dense vegetation, forebays, trash racks, dams, and the need to continually function in all types of weather. As implied by their name, stormwater management measures are also expected to become the repositories for sediment, nutrients, trash, debris, and other pollutants targeted by the NJDEP Stormwater Management Rules. For this reason, stormwater management measures share maintenance requirements with more mundane items as vacuum cleaner bags, car motor filters, and floor mats, all of which require regular inspection and cleaning, sediment and debris removal, and periodic replacement.

In recognition of these needs and potential problems, the NJDEP Stormwater Management Rules require that a maintenance plan be developed for all stormwater management measures incorporated into the design of a major development. This maintenance plan must contain specific preventative and corrective maintenance tasks, schedules, cost estimates, and the name, address, and telephone number of the person or persons responsible for the measures' maintenance."

B. DESCRIPTION OF FACILITIES

The applicant proposes a new parking garage, Central Utility Plant (CUP) expansion, and 4-story perioperative expansion. In addition, the perioperative expansion will be constructed above the existing loading dock on Lot 1 and maintain select utilities including existing drainage network. Furthermore, the applicant is preliminarily proposing the construction of a new 11-story critical care tower in Lot 1. Improvements will decrease the total onsite impervious surface areas and motor vehicle surface areas as compared from existing to proposed conditions. The proposed stormwater pipe network will connect to the existing onsite storm drainage system. No new stormwater outfalls are proposed. Runoff quantity standards are met through land cover management techniques and runoff quality and groundwater recharge standards are not applicable. No green infrastructure BMPs are proposed.

It is necessary that the inlets and pipes be regularly inspected for erosion and damage and cleared of any trash, sediment and/or debris that may collect. Failure to do so may result in blockage or clogging of outlet pipes and could cause excessive surface runoff and flooding. If outlet pipes do not operate properly the proposed management structures will not perform their intended function.

Rainfall events are random and inspection and maintenance of these facilities prior to them is impractical. Therefore the collection, detention and control structures must be consistently ready to function as designed. The maintenance required to accomplish this must, therefore, be performed thoroughly and on a regular basis, regardless of how often the facilities are called upon for stormwater management. The key to performing this level of maintenance lies in establishing and sustaining a comprehensive, regularly scheduled maintenance program. This manual is intended to provide guidance and instruction to project personnel for the proper operation and maintenance of the stormwater collection structures.

C. PROJECT CONTACTS:

Township Engineer: William Schwarz, Jr., P.E

Department of Engineering

2201 Heck Avenue Neptune, NJ 07756 Phone: 732-897-4162

Township Construction

Official:

Joseph Ciccone

Construction Department

25 Neptune Blvd Neptune, NJ 07753

Phone: 732-988-5200 ext. 266

NJDEP: Bureau of Nonpoint Pollution Control

501 East State Street

P.O. Box 419

Trenton, NJ 08625-0419 Phone: 609-633-7021 Fax: 609-984-2147

Emergency Hotline - 1-877-WARNDEP

Design Engineer: Mario Iannelli, P.E.

Dewberry Engineers Inc. 600 Parsippany Rd., Suite 301

Parsippany, NJ 07054 Phone: 973-576-9675

<u>Project Applicant</u> Jersey Shore University Medical Center

1945 State Highway Route 33

Neptune, NJ 07753

PART II: ROUTINE INSPECTION AND MAINTENANCE

In discussing proper inspection and maintenance procedures, it is important to remember that there are two distinct reasons for proper inspection and maintenance of the facilities: to protect against loss of life and major property damage; and to ensure the proper functioning of the system to alleviate flooding. Proper inspection and maintenance procedures will include routine inspection and maintenance requirements for the outfall, inlets, and pipes.

A. PRE-CONSTRUCTION

During the construction activities, the vegetated areas must be protected from compaction by construction equipment, and no material shall be stockpiled in these areas.

B. STORMWATER MANAGEMENT FACILITIES

The stormwater management facilities have been designed to control stormwater flows and volumes. Without proper routine inspection and maintenance, they may lose their capability to function properly.

A consulting Professional Engineer should perform regularly scheduled maintenance inspection of the stormwater facilities for clogging and excessive debris and sediment accumulation at least four times annually as well as after every storm exceeding 1 inch of rainfall. Sediment removal should take place only when the system is thoroughly dry. Disposal of debris, trash, sediment, and other waste material should be done at suitable disposal/recycling sites and in compliance with all applicable local, state, and federal waste regulations. The primary purpose of these inspections is to ascertain the operational condition and safety of the facilities. Inspections will also provide information on the effectiveness of regularly scheduled Preventative and Corrective Maintenance procedures and identify where changes in the extent and scheduling of the procedures are warranted.

C. PREVENTIVE MAINTENANCE PROCEDURES:

The purpose of Preventive Maintenance is to maximize the effectiveness of the stormwater facilities. These procedures are as follows:

1. <u>Maintenance of Vegetated, Grass, and Landscaped Areas:</u>

Vegetated areas must be inspected at least annually for erosion and scour. Biweekly inspections of vegetation health should be performed during the first growing season or until the vegetation is established. Once established, inspections of vegetation health, density, and diversity should be performed at least twice a year during both the growing and non-growing season. All vegetation deficiencies should be addressed without the use of fertilizers and pesticides whenever possible. All vegetated areas should be inspected at least once a year for

unwanted growth, which should be removed with minimum disruption to the remaining vegetation.

Grass, tree and shrub areas require periodic fertilizing, de-thatching, and soil conditioning in order to maintain healthy growth and to provide soil stabilization. Grading and landscaping around facility inlets should be mowed, trimmed and debris should be removed. The application of fertilizers should follow manufacturer's instructions to reduce run-off of these compounds into the stormwater collection system. Additionally, provisions should be made to re-seed and re-establish grass cover in areas damaged by sediment accumulation, soil erosion or other causes. These tasks should be performed or at least evaluated on a quarterly basis.

Lawn areas should be mowed at least once a month during the growing season and shall be mowed on a regular basis as necessary to maintain the lawn at a height of 2 to 3 inches. Any dead or bare lawn areas shall be re-seeded in accordance with the original procedures as outlined in the Soil Erosion and Sediment Control Plans using the same mix and seeding rates. No lawn areas should be mowed twice per growing season.

The trees and shrubs shall be maintained regularly to ensure good health and exhibit an attractive appearance. Their maintenance should include fertilization twice annually with one application in the spring and another in early fall. Trees and shrubs shall be pruned in the late winter or early spring. However, dead branches should be removed as soon as they are noticed. The structure must be inspected for unwanted tree growth annually.

D. CORRECTIVE MAINTENANCE PROCEDURES:

1. Structural Repairs:

All structural components must be inspected for cracking, subsidence, spalling, erosion, and deterioration at least annually. Structural damage to outlets and inlet structures, access points and roadways as a result of flood events, settlement or other causes must be repaired promptly. The urgency of the repairs will depend upon the nature of the damage and its effects on the safety and operation of the facility. The analysis of the structural damage and the design and performance of structural repairs should only be undertaken by a consulting Professional Engineer.

2. Erosion Repair:

Vegetative cover or other protective measures are necessary to prevent the loss of soil due to the forces of wind and water. Where a re-seeding program has not been effective in maintaining a non-erosive vegetative cover, other methods such as rip-rap, geotextile fabrics, sod or regrading shall be utilized.

3. Vegetative Cover Repair:

Vegetative cover should be maintained at 85 percent. If vegetative cover has greater than 50 percent damage, the area should be re-established in accordance with original plan specifications.

4. Snow and Ice Removal:

Accumulations of snow and ice can threaten the proper drainage of water to stormwater inlets. Provision of the equipment, material and personnel to monitor and remove snow and ice from critical areas will assure the proper drainage of stormwater during the winter months.

E. SUMMARY OF MAINTENANCE PROCEDURES:

Preventive Maintenance

1. Maintenance of Grass and Landscaped Areas

Corrective Maintenance

- 1. Structural Repairs
- 2. Erosion Repair
- 3. Vegetative Cover Repair
- 4. Snow and Ice Removal

PART III: MAINTENANCE EQUIPMENT & MATERIALS

1. Grass Maintenance Equipment

- A. Riding Mowers
- B. Hand Mower
- C. Gas Powered Trimmer
- D. Seed Spreaders
- E. Fertilizer Spreaders
- F. De-Thatching Equipment
- G. Pesticide and Herbicide Application Equipment
- H. Grass Clipping and Leaf Collection Equipment

2. Vegetative Equipment

- A. Saws
- B. Pruning Shears
- C. Hedge Trimmers
- D. Wood Chippers

3. Transportation Equipment

- A. Trucks for Transportation of Material and Equipment
- B. Vehicles for Transportation of Personnel

4. Debris, Trash, Snow, Sediment And Water Removal Equipment

- A. Loader/Backhoe
- B. Portable Pump for Dewatering

5. <u>Miscellaneous Equipment</u>

- A. Shovels
- B. Rakes
- C. Picks
- D. Wheel Barrows
- E. Brooms

6. Maintenance Materials

- A. Topsoil
- B. Fill
- C. Seed
- D. Lawn Treatment (Fertilizer, Lime, Pesticides, Herbicides, etc.)
- E. Mulch

PART IV: REPORTING

The reporting of all maintenance work and inspections provides valuable data on the facility condition. Review of this information will also help to establish more efficient and beneficial maintenance procedures and practices. All completed forms shall be directed to the Municipal Engineer for review and subsequent follow-up recommendation. From field personnel to the maintenance director, everyone should be encouraged to report any problems or suggest any changes to the site.

PART V: PERMITTING

The owner shall be responsible to obtain all permits (Federal, State, County and Municipal agencies) required to maintain the stormwater management facilities associated with this project and outlined in this operation and maintenance report. In addition, all maintenance and repair activities shall be done in accordance with all Federal, State and local requirements including, but not limited to, OSHA.

Inspection Checklist / Maintenance Actions

Pipe Network

Check	list (circle one): Quarterly / Annual / Monthly / Special Event Inspection
Checklist No	Inspection Date:
	Date of most recent rain event:
	Rain Condition (circle one): Drizzle / Shower / Downpour / Other
	Ground Condition (circle one): Dry / Moist / Ponding / Submerged / Snow accumulation

Per the Township of Parsippany-Troy Hills, all inspection reports and maintenance records shall be submitted to the Township Engineer on an annual basis no later than March 1st of the following calendar year. Failure to do so will subject the owner to violation and penalties as set forth in the Township Ordinance.

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The inspection items and preventative/corrective maintenance actions listed below represent general requirements. The design engineer and/or responsible party shall adjust the items and actions to better meet the conditions of the site, the specific design targets, and the requirements of regulatory authorities.

	For Inspector			For Maintenance Crew		
Component No. Component Name	Inspection Item and Inspection Item No.			Preventative / Corrective Maintenance Actions		
	1	Missing or damaged rim or grate.	Y N	Repair or replace Work Order #		
A Inlets and Manholes	2	Damaged structures (subsidence, spalling, erosion, or deterioration) and/or reinforcing exposed.	Y N	Repair structure Work Order #		
	3	Standing water or debris.	Y N	Determine reason for downstream clog and remove the debris.		
В	1	Outlet components skewed, misaligned, or missing.	Y N	Repair or replace Work Order #		
Outfall	2	Cracked or damaged pipe.	Y N	Repair Work Order #		
C	1	Erosion and/or sediment accumulation.	Y N	Determine source of erosion and/or reason for sediment accumulation and address source.		
Vegetation	2	Dying vegetation and/or barren land where vegetation should be.	Y N	Replant vegetation in-kind based on associated Landscaping Plan. Determine reason for barren land.		
Note:						

Follow Up Items (Component No	.): -			
Associated Work Orders: #	,#,#	,#	, #	
Inspector Name	Signature	Date		

Report issues to the local authority and mosquito commission as required by local ordinances and regulatory authorities.

File this checklist in the Maintenance Log after performing maintenance.

Preventative Maintenance Record

	Corresponding Checklist No			
Comp	onent No, Inspection Item No			
Work Logs				
Activities	Components	Date Completed		
Sediment/debris removal Sediment removal should	A – Inlets and Manholes			
be taken place when the system is thoroughly dry	B – Outfall			
Dying/dead vegetation replacement	C – Vegetation			
remaining vegetation. All use of fertilizers, pesticide health must not compromise t	es, mechanical treatments, and other means type), and (quantity per usage	s to ensure optimum vegetation agement measure. The fertilizer		
Debris, sediment, and trash are handled (onsite / by (contractor name) to disposal site). (See Part I: Maintenance Plan – Disposal Plan Section)				
Crew member: / Date: Date:				
Supervisor:	/I	Date:		

File this Preventative Maintenance Record in the Maintenance Log after performing maintenance.

Corrective Maintenance Record

1.	. Work Order # Date Issued					
2.	Issue to be resolved:					
3.	The issue was from Corresponding Inspection Item No.	Corresponding Checklist				
4.	Required Actions					
	Actions	Planned Date	Date Completed			
5.	Responsible person(s):					
6.	Special requirements o Time of the season or weather condition: Tools/equipment: Subcontractor (name or specific type):					
Ap	Approved by/Date (name/signature)					
Ve	rification of completion by		3			

File this Corrective Maintenance Record in the Maintenance Log after performing maintenance.

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BRUSH PLANTING MAINTENANCE GUIDELINES:

GENERAL NOTES:

- 1. After the first growing season, all planting areas shall be inspected for dying or dead plant material and such plant material shall be replaced as per the specified landscape guarantee time period of two years. All replacement plants should be of the same species as shown on the approved site plan.
- 2. Immediately during the first growing season and periodically throughout future years, the entire planting area shall be inspected for problem weeds and invasive species and should be hand pulled or spot spayed with an approved herbicide. To control competing vegetation, herbicides as a direct spray can be used around the hardwoods and shrubs in the spring and fall.
- 3. Erosion control measures should be strictly maintained during the first growing season so that All planted areas are properly stabilized,
- 4. After first and subsequent growing seasons, the following care should be administered to each specific plant group listed. It is important to note that each plant group listed below, has specific needs and their maintenance should be tailored to those needs as such. The primary goal of the brush planting area is to maintain a well kept natural planting area that is not overgrown. The design concept was to mimic nature by planting in layers- from planting tall canopy trees to understory shrubs, followed by herbaceous plants and ornamental grasses. This should create a dynamic textured landscape with multi season interest.
- 5. During the initial planting precautions should be undertaken to prevent and prohibit animals from grazing until plant material is well established. Such precautions could be fencing, animal trapping, eco friendly plant sprays, etc.
- 6. A monitoring plan should be developed to obtain data relevant to the design, survivability, maintenance and objectives of the planting plan. This plan shall measure the survival, growth and visual value of each plant to determine possible future plant replacement options.
- 7. The planting area shall be routinely inspected for different drainage patterns and ponding of water the might affect the survivability of the plants.
- 8. Periodic evaluations of the planting design shall be made to determine if changes to the design concept should be made in relation to species selection and planting design.
- 9. All plant material shall be planted in a naturalized pattern. Naturalized patterns shall result in a relatively even distribution of each species across the planting area at the specific density. Planting in uneven distribution of species shall not be accepted.
- 10. All planting areas shall be mulched. The mulch layer shall be standard landscape style, double or triple shredded hardwood mulch. The mulch layer should be well aged (stockpiled or stored for at least twelve (12) months, uniform in color and free of other materials such as weed seeds, soil, roots, etc. The mulch shall be applied to a maximum depth of three (3) inches. Grass clippings should not be used as a mulch material.
- 11. While the proposed plantings are mostly all native plants and will be highly drought tolerant once established, irrigation is recommended for the first 1-2 years during the months of July to September for newly planted native trees, shrubs, perennials and ornamental grasses.
- 12. The following are the individual plant types planted onsite and their specific maintenance guidelines.



SHADE /ORNAMENTAL TREES AND EVERGREENS.

Throughout the growing season, All plant material shall be inspected for dead branches, plant diseases, proper installation techniques, animal diseases, etc. and should be properly addressed as soon as possible, All sucker growth shall be removed from all plant material and light pruning is recommended on a yearly basis. Fertilizer shall be applied on a regular schedule as specified to all plant material. Weed and mulch all surface areas surrounding plant material as required. Remove all diseased fallen leaf material in planting area. All vegetation conflicts with adjacent plant material shall be removed as to not inhibit growth. All dead trees and evergreens are to be replaced with native species shown on landscape plan approved by the NJDEP

DECIDUOUS AND EVERGREEN SHRUBS:

Throughout the growing season, All shrubs shall also be inspected for dead branches, plant diseases, proper installation techniques, animal damage, etc. and should be properly addressed. Fertilizer shall be applied on a regular basis as specified. All plant material shall be pruned to maintain their specific growth characteristics of that plant. All spent flowers shall be removed (deadheading) to encourage more flowers to bloom and keep plants looking tidy. It will also prevent unwarranted self seeding. Each plant characteristic shall determine the level and amount of pruning for that specific plant The goal is to maintain a relatively ordered look to the landscape. All surrounding areas shall be weeded and mulched as required. All dead shrubs are to be replaced with native species shown on landscape plan approved by the NJDEP

PERENNIALS:

In early spring, all perennial beds shall be inspected for dead plant material and shall be replaced with new plant material. All beds should also be inspected for animal damage and plant diseases and be addressed as required. It is also at this time that all healthy plant material be cleared of all normal decaying leaves and spent flowers from the previous growing season, so that new growth can be encouraged to maintain a healthy plant. During the summer growing season, all plants shall be pruned to maintain their specific growth characteristic and not interfere with the growth of adjacent plant material. As time progresses in the life of the plant, each plant type should be evaluated to determine if overgrown plants should be divided or replaced with new plant material. All plant material shall be fertilized as specified and all beds shall be weeded and mulched as required. All dead plant material shall be replaced by similar species originally planted.

ORNAMENTAL GRASSES:

In early spring, all plant material shall be inspected for plant diseases, plant damage, etc. and should be properly addressed. All dead plant material shall be removed and replaced. To help keep ornamental grasses healthy and vibrant, cutting them back every year is essential, cutting back grasses permits the plant to put their energy into producing fresh vibrant foliage. The time of year for cutting can occur anytime from late fall to early spring. The time is based on the visual preference of the owner. As with perennials, overgrown grasses should be evaluated to determine if they should be divided or removed and replaced with new plant material. After the plant has been established a minimal amount of fertilizer should be needed as the plant matures. All dead plant material shall be replaced by similar species originally planted.

