TRAFFIC IMPACT STUDY

For

Surfside Crossing Proposed Residential & Retail Development

Property Located at:

Memorial Drive (CR 40 A) & 8th Avenue/9th Avenue Block 405 – Lots 5, 6 & 7 Township of Neptune, Monmouth County, NJ



1904 Main Street | 245 Main Street, Suite #110 Lake Como, NJ 07719 | Chester, NJ 07930 (732) 681-0760

Nick Verderese, PE NJ PE License #38991 Justin P. Taylor, PE, PTOE NJ PE License #45988

April 4, 2022 Revised: November 22, 2022

2241-99-002T



INTRODUCTION

It is proposed to construct a residential development with ground floor retail on a parcel of land that is currently undeveloped, located along the southbound side of Memorial Drive between 8th Avenue and 9th Avenue in the Township of Neptune, Monmouth County, New Jersey (see Figure 1 in Appendix A). The site is designated as Block 405 – Lots 5, 6 and 7 on the Township of Neptune Tax Maps. It is proposed to construct a four-story 70-unit residential building with 1,976 SF of retail space and parking on the ground floor ("The Project"). The site is located within the TRV – Transit Village Zone. It is proposed to provide access to the site via a full movement driveway along 8th Avenue and a full movement driveway along 9th Avenue.

Dynamic Traffic LLC has been retained to prepare this study to assess the traffic impact associated with the construction of The Project on the adjacent roadway network. This study documents the methodology, analyses, findings and conclusions of our study and includes:

- A detailed field inspection was conducted to obtain an inventory of existing roadway geometry, traffic control, and location and geometry of existing driveways and intersections.
- Existing traffic data was collected via manual turning movement (MTM) counts during the weekday AM and weekday PM peak periods at the intersections of:
 - o Memorial Drive (CR 40 A) & 8th Avenue
 - o Memorial Drive (CR 40 A) & 9th Avenue
- Projections of traffic to be generated by the proposed development were prepared utilizing trip generation data as published by the Institute of Transportation Engineers. Site traffic was then assigned to the adjacent street system based upon the anticipated directional distribution.
- Capacity analyses were conducted for the Existing, No Build, and Build conditions for the study intersections.
- The proposed points of ingress and egress were inspected for adequacy of geometric design, spacing and/or alignment to streets and driveways on the opposite side of the street, relationship to other driveways adjacent to the development, and conformance with accepted design standards.
- The site plan as designed was reviewed for sufficiency in accommodating large wheel base vehicles such as delivery trucks, refuse trucks, and emergency vehicles.
- The proposed site circulation and parking as shown on the site plan were reviewed for conformance with the Residential Site Improvement Standards ("RSIS") (N.J.A.C. 5:21) and Township Ordinance.



EXISTING CONDITIONS

A review of the existing roadway conditions near the proposed site was conducted to provide the basis for assessing the traffic impact of the development. This included field investigations of the surrounding roadways and intersections, collection of traffic volume data, and extensive analyses.

Existing Roadway Conditions

The following are descriptions of the roadways in the study area:

Memorial Drive (CR 40 A) is an Urban Major Collector roadway under Monmouth County jurisdiction with a general north/south orientation. In the vicinity of the site the posted speed limit is 40 MPH and the roadway provides two travel lanes in each direction. Curb is provided along both sides of the roadway, while sidewalk is only provided along the southbound side of the roadway. Memorial Drive provides a straight horizontal alignment with a slightly curved horizontal alignment to the north of the site and a relatively flat vertical alignment. The land uses along Memorial Drive in the vicinity of The Project are mixed industrial, commercial and residential. Additionally, the Bradley Beach train station is located less than 800 feet from the site along the northbound side of Memorial Drive.

8th Avenue is a local roadway under Township of Neptune jurisdiction with a general east/west orientation. In the vicinity of the site the speed limit is not posted and the roadway provides one travel lane in each direction. Curb and sidewalk are provided along both sides of the roadway. 8th Avenue provides a straight horizontal alignment and a relatively flat vertical alignment. The land uses along 8th Avenue in the vicinity of The Project are primarily residential.

9th Avenue is a local roadway under Township of Neptune jurisdiction with a general east/west orientation. In the vicinity of the site the posted speed limit is 25 MPH and the roadway provides one travel lane in each direction. Curb and sidewalk are provided along both sides of the roadway. 9th Avenue provides a straight horizontal alignment and a relatively flat vertical alignment. The land uses along 9th Avenue in the vicinity of The Project are primarily residential.

Existing Traffic Volumes

Manual turning movement (MTM) counts were conducted on Thursday, March 10, 2022 from 7:00 to 9:00 AM and from 4:30 to 6:30 PM at the following intersections:

- Memorial Drive (CR 40 A) & 8th Avenue
- Memorial Drive (CR 40 A) & 9th Avenue

Review of the collected traffic data reveals that the weekday morning peak street hour (PSH) occurs between 7:30 - 8:30 AM and the weekday evening PSH occurs between 4:45 - 5:45 PM. Figure 2, located in Appendix A, shows the existing peak hour traffic volumes at the study intersections. All traffic counts are contained in Appendix B.

COVID-19 Traffic Count Normalization

It should be noted that the COVID-19 pandemic may have affected traffic patterns at the time of the traffic counts. As a result, current traffic volumes on the surrounding roadways may be atypical at this time and not entirely representative of pre-COVID "existing" traffic conditions. Therefore, historical traffic volume data has been reviewed and compared with current traffic volumes in order to account for this effect. Specifically, this firm obtained Automatic Traffic Recorder (ATR) volumes from the NJDOT count database conducted along Memorial Drive north of 9th Avenue on Wednesday, June 19, 2019. Please note that the June volumes were utilized to also account for the seasonal increase in traffic that this area experiences.

In order to perform an appropriate comparison, the 2019 volumes were increased to better represent 2021 conditions by applying a growth rate of 2.5% per year, obtained from the NJDOT Annual Background Growth Rate Table, for a period of three (3) years. The adjusted 2019 traffic volumes were then compared to the existing 2021 traffic counts as summarized in the table below.

Table I Traffic Count Comparison

Location	Date		ial Drive Pea Counted	With Ba	fic Volume ckground wth ^[1]	COV Adjustme	ID-19 ent Factor
		AM	PM	AM	PM	AM	PM
Memorial Drive	June 2019	982	1,210	1,058	1,303	1.10	1.13
north of 9th Avenue	March 2022	964	1,152	964	1,152	1.10	1.13

[1] June 2019 data increased by 2.5% per NJDOT Annual Background Growth Rate Table compounded annually for three years.

As seen above, the current volumes were found to be lower than the historical counts grown to represent existing conditions during both peak hours. Therefore, adjustment factors of 1.10 and 1.13 were applied to the AM and PM peak hour volumes, respectively, to provide a conservative analysis. Figure 3, located in Appendix A, shows the adjusted existing peak hour traffic volumes at the study intersection.

Existing Capacity Analysis

The methodology utilized in the capacity analyses is described in the *Highway Capacity Manual*, published by the Transportation Research Board. In general, the term Level of Service (LOS) is used to provide a "qualitative" evaluation of capacity based upon certain "quantitative" calculations related to empirical values, such as traffic volume and intersection control.

An unsignalized (STOP sign controlled) driveway or side street along a through route is seldom critical from an overall capacity standpoint, however, it may be of great significance to the capacity of the minor cross-route, and it may influence the quality of traffic flow on both. When analyzing an unsignalized intersection, it is assumed that both the major street through and right turn movements are unimpeded and have the right-of-way over all side street traffic and left turns from the major street. All other turning movements in the intersection cross, merge with, or are otherwise impeded by major street movements. Traffic delays at unsignalized intersections are determined by sequentially processing these impeded movements. Table II describes the level of service ranges for unsignalized (stop controlled) intersections.



Table II Level of Service Criteria for Unsignalized Intersections

Level of Service	Average Control Delay (seconds per vehicle)
a	0.0 to 10.0
ь	10.1 to 15.0
С	15.1 to 25.0
đ	25.1 to 35.0
e	35.1 to 50.0
f	greater than 50.0

It should be noted that the analyses within the *Highway Capacity Manual* assume a random arrival for all the movements, which may not be the case if an adjacent traffic signal is present that platoons vehicles, such as the signalized intersection of Memorial Drive and 6th Avenue.

All capacity analyses were performed utilizing Synchro 11 software. It should be noted that the existing percentage of trucks and peak hour factors were used in the existing analysis. Table III summarizes the existing levels of service (LOS) and delays. All capacity analysis calculation worksheets are contained in Appendix C.

Table III Existing Levels of Service

Intersection	Direc Move		AM PSH	PM PSH
Memorial Drive & 8 th Avenue	EB	LR	b (15)	c (19)
Memorial Drive & 8° Avenue	NB	L	a (9)	a (10)
Memorial Drive & 9 th Avenue	EB	LR	c (17)	b (14)
Ivicinoriai Drive & 9° Avenue	NB	L	a (9)	a (10)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

The following are discussions pertaining to each of the existing intersections analyzed.

Memorial Drive and 8th Avenue

8th Avenue intersects Memorial Drive to form a T-intersection with the eastbound approach of 8th Avenue operating under stop control. The northbound approach of Memorial Drive provides a shared left turn/through lane and a dedicated through lane, while the southbound approach provides a dedicated through lane and a shared through/right turn lane. The eastbound approach of 8th Avenue provides a shared left/right turn lane.

A review of the existing analysis reveals that all movements operate at levels of service "C" or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.



Memorial Drive and 9th Avenue

9th Avenue intersects Memorial Drive to form a T-intersection with the eastbound approach of 9th Avenue operating under stop control. The northbound approach of Memorial Drive provides a shared left turn/through lane and a dedicated through lane, while the southbound approach provides a dedicated through lane and a shared through/right turn lane. The eastbound approach of 9th Avenue provides a shared left/right turn lane.

A review of the existing analysis reveals that all movements operate at levels of service "C" or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.



FUTURE CONDITIONS

Traffic volumes and operational analyses were developed for both the 2024 No Build and Build conditions. The No Build conditions provide a baseline for assessing the impact of the site development traffic on the roadway system. The process of developing the No Build and Build traffic volumes and the subsequent analyses is outlined below.

Regardless of whether the subject site is developed or not, traffic volumes on the surrounding roadways are expected to increase as a result of developments throughout the region. A growth rate for roadways within the study area was obtained from the NJDOT Annual Background Growth Rate Table, which indicates a growth rate of 2.5% per year.

Through consultation with the Neptune Township Planning Board staff, there are no other developments in the vicinity of the site that have been approved but not yet constructed that are identified as significant traffic generators. It was assumed that the background growth rate was adequate to account for the traffic associated with all development within the area.

Future 2024 No Build traffic volumes were developed by applying the background growth rate of 2.5% for two (2) years to the study area roadways existing traffic volumes. Figure 4, in Appendix A, shows the 2024 No Build traffic volumes.

Traffic Generation

Trip generation projections for The Project were prepared utilizing trip generation research data as published under Land Use Code (LUC) 220 – Multifamily Housing (Low-Rise) Close to Rail Transit and LUC 822 – Strip Retail Plaza (<40K) in the Institute of Transportation Engineers' (ITE) publication, *Trip Generation*, 11th Edition. This publication sets forth trip generation rates based on empirical traffic count data conducted at numerous research sites. Table IV below details the traffic volumes associated with the subject project.

Table IV Trip Generation

Land Haa	1	AM PSI	ł]	PM PSH	I
Land Use	In	Out	Total	In	Out	Total
70 Residential Units	8	19	27	26	17	43
1,976 SF Retail	3	2	5	13	12	25
Total	11	21	32	39	29	68

Once the magnitude of traffic to be generated by the site is known, it is necessary to assign that traffic to the adjacent street system. The distribution of new traffic to the surrounding roadways is based on the location of primary arterial roadways, major signalized intersections and existing traffic patterns. Figures 5 and 6, located in Appendix A, illustrate the Trip Distribution and Site Generated Volumes, respectively. The Site Generated Volumes assigned to the study area network were added to the No Build traffic volumes to generate the Build traffic volumes, which are shown in Figure 7.



Future Capacity Analysis

Operational conditions at the study intersections were analyzed under the No Build and Build conditions and are summarized in Table V below.

Table V Future Levels of Service

	Direct	ction/	AM	PSH	PM	PSH
Intersection		ement	No Build	Build	No Build	Build
Memorial Drive & 8th Avenue	EB	LR	c (15)	c (15)	c (20)	c (21)
Wellional Drive & 8 Avenue	NB	L	a (9)	a (9)	a (10)	a (10)
Memorial Drive & 9th Avenue	EB	LR	c (18)	c (17)	b (15)	c (16)
Wellional Drive & 9° Avenue	NB	L	a (9)	a (9)	a (10)	a (10)
8 th Avenue & Site Driveway	EB	L	-	a (7)	-	a (7)
8 Avenue & Site Driveway	SB	LR	1	a (9)	•	a (9)
Oth Avenue & Site Driveyey	WB	L	-	a (7)	•	a (7)
9 th Avenue & Site Driveway	NB	LR	-	a (9)	-	a (9)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

Memorial Drive and 8th Avenue

With the addition of site generated traffic, each intersection movement is anticipated to operate at No Build levels of service "C" or better. See Table V for the individual movement levels of service and delays.

Memorial Drive and 9th Avenue

With the addition of site generated traffic, each intersection movement is anticipated to operate at acceptable levels of service "C" or better. See Table V for the individual movement levels of service and delays.

8th Avenue and Site Driveway

The site driveway is proposed to intersect 8th Avenue to form an unsignalized T-intersection with the southbound approach of the site driveway operating under stop control. The eastbound approach of 8th Avenue is proposed to provide a shared left turn/through lane, while the westbound approach is proposed to provide a shared through/right turn lane. The southbound approach of the site driveway is proposed to provide a shared left/right turn lane.

As designed, the driveway is anticipated to operate at levels of service "A" during the studied peak hours. See Table V for the individual movement levels of service and delays.



9th Avenue and Site Driveway

The site driveway is proposed to intersect 9th Avenue to form an unsignalized T-intersection with the northbound approach of the site driveway operating under stop control. The eastbound approach of 9th Avenue is proposed to provide a shared through/right turn lane, while the westbound approach is proposed to provide a shared left turn/through lane. The northbound approach of the site driveway is proposed to provide a shared left/right turn lane.

As designed, the driveway is anticipated to operate at levels of service "A" during the studied peak hours. See Table V for the individual movement levels of service and delays.



SITE PLAN

Site Access and Circulation

The site plan was reviewed with respect to the site access and on-site circulation design. As noted previously, access to The Project will be provided via a full movement driveway along 8th Avenue and a full movement driveway along 9th Avenue.

The parking lot will be serviced by parking aisles with widths of 24' for two-way circulation and 90-degree parking, which satisfy both the Ordinance and Residential Site Improvement Standards (RSIS) requirement of 24'. Additionally, the site will provide driveway widths between 22' and 24.5', which meet and exceed the Ordinance maximum requirement of 22' for two-way traffic. Review of the site plan design indicates that the site can sufficiently accommodate a large wheel base vehicle, such as an emergency vehicle or refuse truck, along with the automobile traffic anticipated.

Parking

The RSIS sets forth a parking requirement of 1.8 parking spaces per 1-bedroom apartment units and 2.0 parking spaces per 2-bedroom apartment unit. Additionally, the Neptune Township Ordinance sets forth a parking requirement of 1 space per 250 SF for retail sales and service uses. This equates to a parking requirement of 136 spaces for the proposed 70-unit (22 1-bedroom units & 48 2-bedroom units) multifamily residential building and 8 spaces for the proposed 1,976 SF of ground floor retail space, for a total of 144 parking spaces.

It is proposed to provide 130 parking spaces, inclusive of 22 make-ready electric vehicle charging spaces and 5 handicap spaces. As per the current Municipal Land Use Law (MLUL) (N.J.A.C. 40:55-D), electric vehicle charging stations count as two spaces for the purposes of complying with parking supply requirements, up to a maximum of 10% of the requirement. As such, the effective proposed parking supply is calculated to be 144 spaces which satisfies the RSIS and Ordinance requirement of 144 spaces.

As previously mentioned, the site is located within 800 feet of the Bradley Beach train station. Further, there are NJ Transit 2 bus lines (317 and 830) that travel along Main Street. The availability of public transportation will likely further reduce the parking demand for the site.

It is proposed to provide parking stalls with dimensions of 9'x18', which satisfy the RSIS and Ordinance minimum requirement of 9'x18'.



FINDINGS & CONCLUSIONS

Findings

Based upon the detailed analyses as documented herein, the following findings are noted:

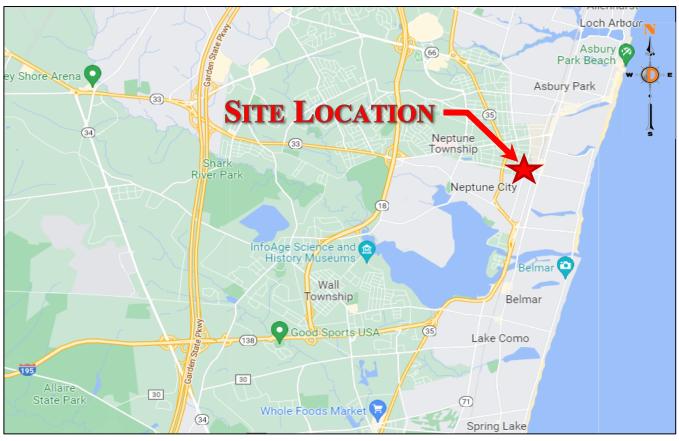
- The proposed 70-unit multifamily residential building with 1,976 SF of ground floor retail is projected to generate 11 entering trips and 21 exiting trips during the weekday morning peak hour and 39 entering trips and 29 exiting trips during the evening peak hour that are "new" to the adjacent roadway network.
- Access to the site is proposed to be provided via a full movement driveway along 8th Avenue and a full movement driveway along 9th Avenue.
- With the addition of site generated traffic, the intersection of Memorial Avenue and 8th Avenue is anticipated to operate at No Build levels of service "C" or better during the peak hours studied.
- With the addition of site generated traffic, the intersection of Memorial Avenue and 9th Avenue is anticipated to operate at levels of service "C" or better during the peak hours studied.
- As designed, the intersection of 8th Avenue and the site driveway is anticipated to operate at levels of service "A" during the peak hours studied.
- As designed, the intersection of 9th Avenue and the site driveway is anticipated to operate at levels of service "A" during the peak hours studied.
- As proposed, The Project's site driveways and internal circulation have been designed to provide for safe and efficient movement of automobiles, refuse trucks and emergency vehicles.
- The Project's site access points, internal circulation, and parking supply have been designed in accordance with the RSIS (N.J.A.C. 5:21) and Ordinance requirements.

Conclusions

Based upon our Traffic Impact Study as detailed in the body of this report, it is the professional opinion of Dynamic Traffic LLC that the adjacent street system of the Township of Neptune and Monmouth County will not experience any significant degradation in operating conditions with the construction of The Project. The site driveways are located to provide safe and efficient access to the adjacent roadway system. The site plan as proposed provides for good circulation throughout the site and provides adequate parking to accommodate The Project's needs.

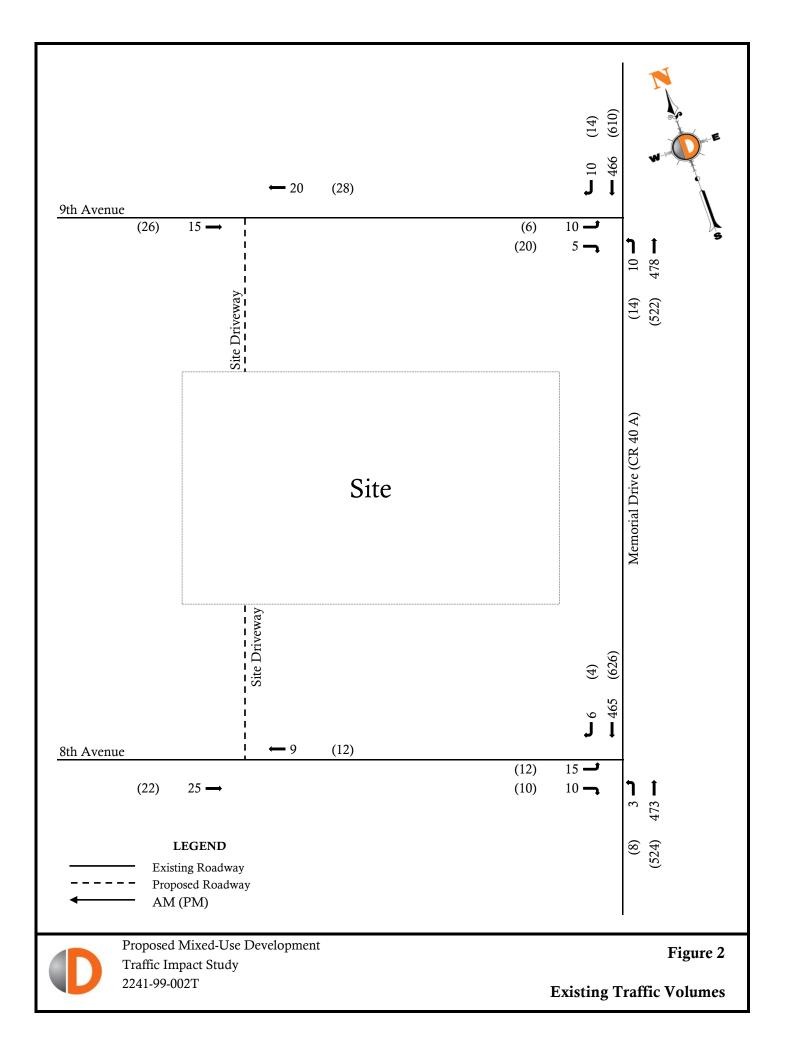
Appendix A Traffic Volume Figures

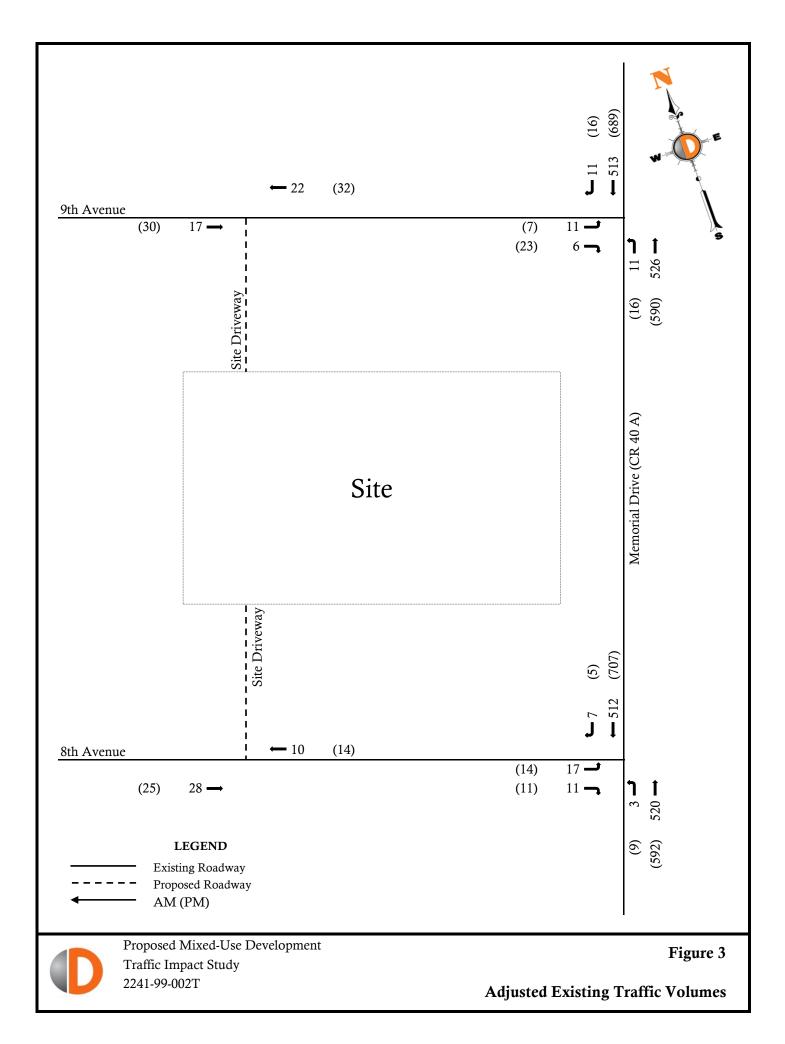


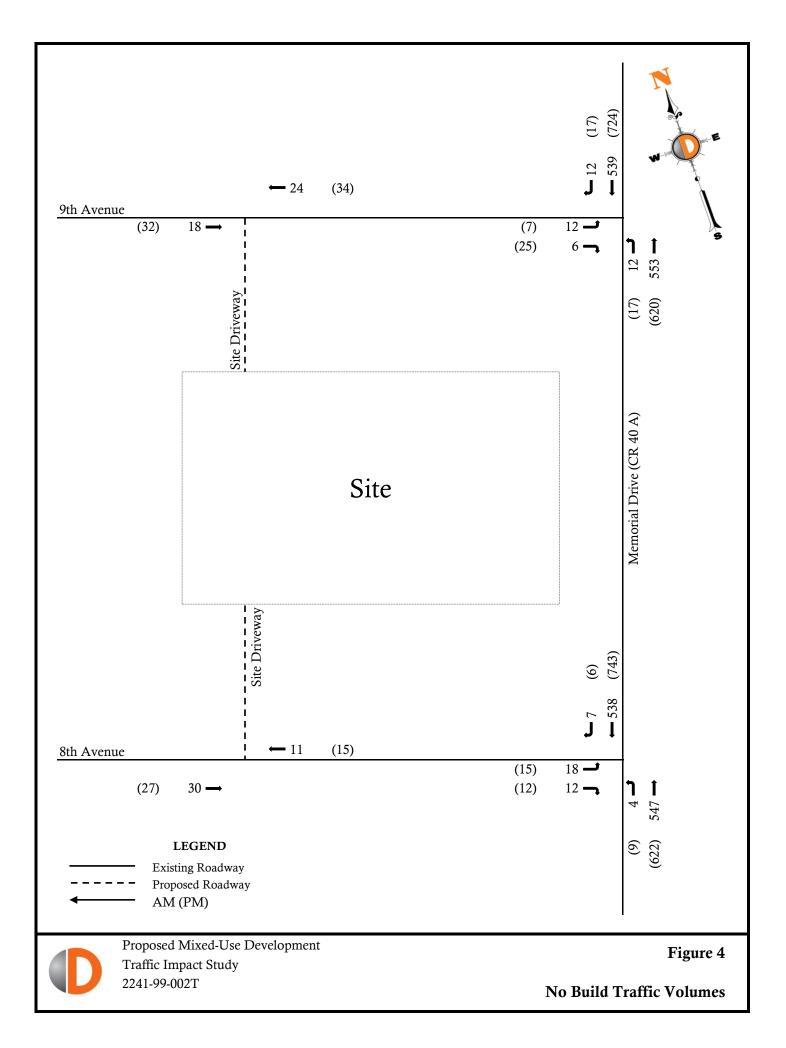


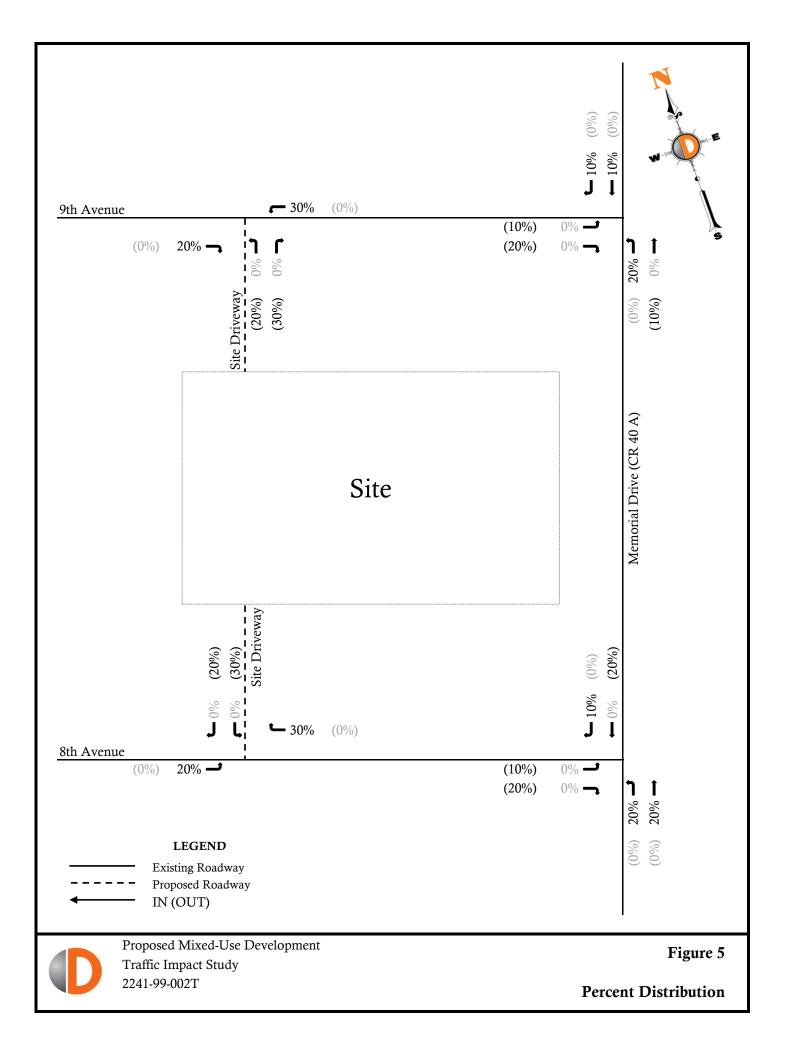


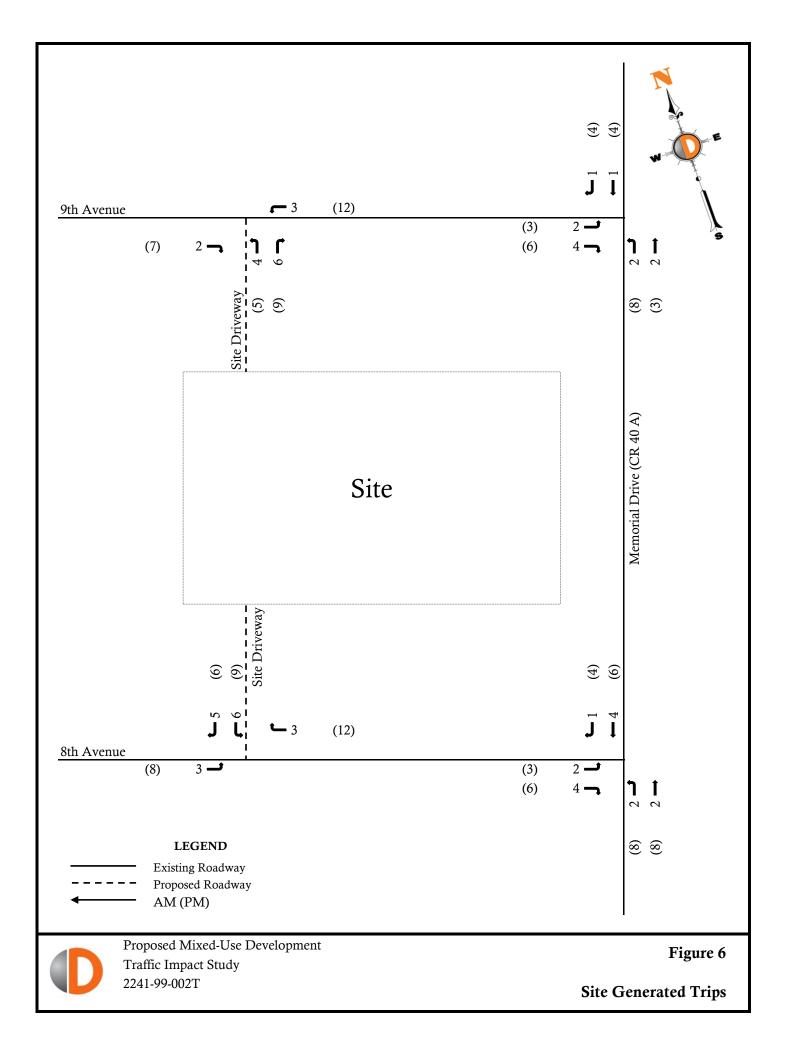
Proposed Mixed-Use Development Traffic Impact Study 2241-99-002T

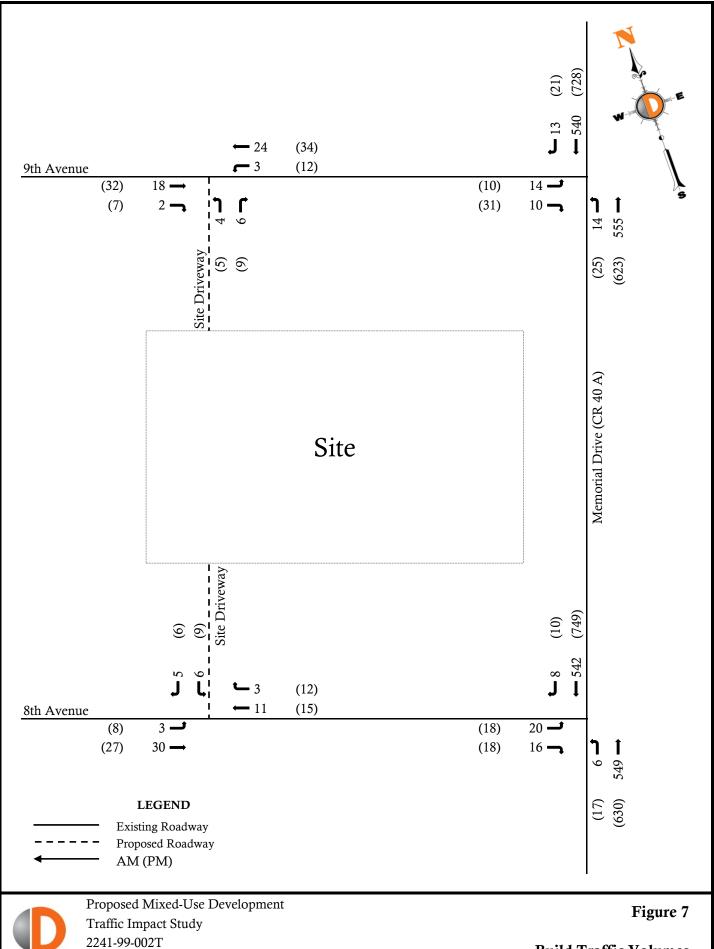












Appendix B Project Information

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 8th Avenue File Name: Memorial Drive and 8th Ave - AM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

					Groups	1 1111100			_ , _ ,	- IIIUCKS	(' ' ' '					1
		8	th Aver	nue			Me	morial l	Drive			Me	morial I	Drive		
		E	astbou	nd			N	orthbou	und			S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
07:00 AM	3	0	2	0	5	5	88	0	0	93	0	83	1	0	84	182
07:15 AM	3	0	2	0	5	0	100	0	0	100	0	101	1	0	102	207
07:30 AM	3	0	5	0	8	1	130	0	0	131	0	114	0	0	114	253
07:45 AM	2	0	1	0	3	2	112	0	0	114	0	133	1_	0	134	251
Total	11	0	10	0	21	8	430	0	0	438	0	431	3	0	434	893
08:00 AM	4	0	1	0	5	0	126	0	0	126	0	111	4	0	115	246
08:15 AM	6	0	3	0	9	0	105	0	0	105	0	97	1	0	98	212
08:30 AM	2	0	4	0	6	2	116	0	0	118	0	102	3	0	105	229
08:45 AM	4	0	2	0	6	0	116	0	0	116	0	90	2	0	92	214
Total	16	0	10	0	26	2	463	0	0	465	0	400	10	0	410	901
Grand Total	27	0	20	0	47	10	893	0	0	903	0	831	13	0	844	1794
Apprch %	57.4	0	42.6	0		1.1	98.9	0	0		0	98.5	1.5	0		
Total %	1.5	0	1.1	0	2.6	0.6	49.8	0	0	50.3	0	46.3	0.7	0	47	
Cars	26	0	16	0	42	9	798	0	0	807	0	801	13	0	814	1663
% Cars	96.3	0	80	0	89.4	90	89.4	0	0	89.4	0	96.4	100	0	96.4	92.7
Trucks (SU)	1	0	4	0	5	1	72	0	0	73	0	22	0	0	22	100
% Trucks (SU)	3.7	0	20	0	10.6	10	8.1	0	0	8.1	0	2.6	0	0	2.6	5.6
Trucks (TT)	0	0	0	0	0	0	23	0	0	23	0	8	0	0	8	31
% Trucks (TT)	0	0	0	0	0	0	26	0	0	2.5	0	1	0	0	0.9	17

Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 8th Avenue File Name: Memorial Drive and 8th Ave - AM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 2

		81	h Aven	ue			Me	morial I	Orive			Mei	morial I	Drive		
		Ę	astbou	nd			N	<u>orthbo</u> u	ınd			So	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	lysis Fro	m 07:00	O AM to	08:45 A	AM - Peak	1 of 1		_					_			
Peak Hour for E	Entire Int	ersection	n Begir	ns at 07	:30 AM											
07:30 AM	3	0	5	0	8	1	130	0	0	131	0	114	0	0	114	253
07:45 AM	2	0	1	0	3	2	112	0	0	114	0	133	1	0	134	251
08:00 AM	4	0	1	0	5	0	126	0	0	126	0	111	4	0	115	246
08:15 AM	6	0	3	0	9	0	105	0	0	105	0	97	1_	0	98	212
Total Volume	15	0	10	0	25	3	473	0	0	476	0	455	6	0	461	962
% App. Total	60	0	40	0		0.6	99.4	0	0		0	98.7	1.3	0		
PHF	.625	.000	.500	.000	.694	.375	.910	.000	.000	.908	.000	.855	.375	.000	.860	.951
Cars	15	0	8	0	23	3	416	0	0	419	0	448	6	0	454	896
% Cars	100	0	80.0	0	92.0	100	87.9	0	0	88.0	0	98.5	100	0	98.5	93.1
Trucks (SU)	0	0	2	0	2	0	44	0	0	44	0	4	0	0	4	50
% Trucks (SU)	0	0	20.0	0	8.0	0	9.3	0	0	9.2	0	0.9	0	0	0.9	5.2
Trucks (TT)	0	0	0	0	0	0	13	0	0	13	0	3	0	0	3	16
% Trucks (TT)	0	0	0	0	0	0	2.7	0	0	2.7	0	0.7	0	0	0.7	1.7

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 8th Avenue File Name: Memorial Drive and 8th Ave - PM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

Groups Printed- Cars - Trucks (50) - Trucks (11)														_		
		8	th Aver	nue			Me	morial l	Drive			Me	morial I	Drive		
		E	astbou	nd			N	orthbou	ınd			S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
04:30 PM	3	0	0	3	6	4	110	0	0	114	0	167	0	0	167	287
04:45 PM	4	0	2	0	6	1	129	0	0	130	0	146	1	0	147	283
Total	7	0	2	3	12	5	239	0	0	244	0	313	1	0	314	570
,											'					
05:00 PM	2	0	4	1	7	3	140	0	0	143	0	159	2	0	161	311
05:15 PM	4	0	3	0	7	0	113	0	0	113	0	168	0	0	168	288
05:30 PM	2	0	1	0	3	4	140	0	0	144	0	153	1	0	154	301
05:45 PM	3	0	1	0	4	0	130	0	0	130	0	130	2	0	132	266
Total	11	0	9	1	21	7	523	0	0	530	0	610	5	0	615	1166
06:00 PM	1	0	3	1	5	0	118	0	0	118	0	109	1	0	110	233
06:15 PM	3	0	4	0	7	0	116	0	0	116	0	103	1	0	104	227
Grand Total	22	0	18	5	45	12	996	0	0	1008	0	1135	8	0	1143	2196
Apprch %	48.9	0	40	11.1		1.2	98.8	0	0		0	99.3	0.7	0		
Total %	1	0	0.8	0.2	2	0.5	45.4	0	0	45.9	0	51.7	0.4	0	52	
Cars	21	0	15	5	41	10	933	0	0	943	0	1123	7	0	1130	2114
% Cars	95.5	0	83.3	100	91.1	83.3	93.7	0	0	93.6	0	98.9	87.5	0	98.9	96.3
Trucks (SU)	1	0	3	0	4	2	60	0	0	62	0	10	1	0	11	77
% Trucks (SU)	4.5	0	16.7	0	8.9	16.7	6	0	0	6.2	0	0.9	12.5	0	1	3.5
Trucks (TT)	0	0	0	0	0	0	3	0	0	3	0	2	0	0	2	5
% Trucks (TT)	0	0	0	0	0	0	0.3	0	0	0.3	0	0.2	0	0	0.2	0.2

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 8th Avenue File Name: Memorial Drive and 8th Ave - PM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 2

			h Aven					morial <mark>I</mark> orthboເ					morial [outhbou			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	alysis Fro	m 04:30	PM to	06:15 F	PM - Peak	1 of 1		_					_			
Peak Hour for	Entire Int	ersection	n Begir	ns at 04	:45 PM											
04:45 PM	1	0	2	0	6	1	129	0	0	130	0	146	1	0	147	283
05:00 PM	2	0	4	1	7	3	140	0	0	143	0	159	2	0	161	311
05:15 PM	4	0	3	0	7	0	113	0	0	113	0	168	0	0	168	288
05:30 PM	2	0	1_	0	3	4	140	0_	0	144	0	153	1_	0	154	301
Total Volume	12	0	10	1	23	8	522	0	0	530	0	626	4	0	630	1183
_ % App. Total	52.2	0	43.5	4.3		1.5	98.5	0	0		0	99.4	0.6	0		
PHF	.750	.000	.625	.250	.821	.500	.932	.000	.000	.920	.000	.932	.500	.000	.938	.951
Cars	11	0	8	1	20	7	487	0	0	494	0	623	3	0	626	1140
% Cars	91.7	0	80.0	100	87.0	87.5	93.3	0	0	93.2	0	99.5	75.0	0	99.4	96.4
Trucks (SU)	1	0	2	0	3	1	34	0	0	35	0	3	1	0	4	42
% Trucks (SU)	8.3	0	20.0	0	13.0	12.5	6.5	0	0	6.6	0	0.5	25.0	0	0.6	3.6
Trucks (TT)	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
% Trucks (TT)	0	0	0	0	0	0	0.2	0	0	0.2	0	0	0	0	0	0.1

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 9th Avenue File Name: Memorial Drive and 9th Ave - AM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

					Groups	1 milet	ı- cais	- IIUCK	<u>3 (30)</u>	- IIIUCKS	(' ' ')					,
		9	th Aver	nue			Me	morial I	Drive			Me	morial I	Drive		
		E	astbou	nd			N	orthbou	ınd			S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
07:00 AM	4	0	1	0	5	2	88	0	0	90	0	78	1	0	79	174
07:15 AM	1	0	3	0	4	2	100	0	0	102	0	106	1	0	107	213
07:30 AM	4	0	1	0	5	3	130	0	0	133	0	111	3	0	114	252
07:45 AM	3	0	2	0	5	2	112	1_	0	115	0	125	3	0	128	248
Total	12	0	7	0	19	9	430	1	0	440	0	420	8	0	428	887
,											1					
08:00 AM	2	0	2	0	4	3	126	0	0	129	0	111	3	0	114	247
08:15 AM	1	0	0	0	1	2	105	0	0	107	0	105	1	0	106	214
08:30 AM	1	0	2	0	3	5	116	0	0	121	0	102	4	0	106	230
08:45 AM	2	0	1_	0	3	4	116	0	0	120	0	84	1_	0	85	208
Total	6	0	5	0	11	14	463	0	0	477	0	402	9	0	411	899
- 1											II.					ı
Grand Total	18	0	12	0	30	23	893	1	0	917	0	822	17	0	839	1786
Apprch %	60	0	40	0		2.5	97.4	0.1	0		0	98	2	0		
Total %	1_	0	0.7	0	1.7	1.3	50	0.1	0	51.3	0	46	1_	0	47	
Cars	16	0	8	0	24	20	798	1	0	819	0	731	14	0	745	1588
% Cars	88.9	0	66.7	0	80	87	89.4	100	0	89.3	0	88.9	82.4	0	88.8	88.9
Trucks (SU)	2	0	3	0	5	1	72	0	0	73	0	73	3	0	76	154
% Trucks (SU)	11.1	0	25	0	16.7	4.3	8.1	0	0	. 8	0	8.9	17.6	0	9.1	8.6
Trucks (TT)	0	0	1	0	1	2	23	0	0	25	0	18	0	0	18	44
% Trucks (TT)	0	0	8.3	0	3.3	8.7	26	0	0	27	0	22	0	0	21	2.5

Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 9th Avenue File Name: Memorial Drive and 9th Ave - AM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 2

		91	h Aven	ue	Memorial Drive Memorial Drive											
		E	astbou	nd			N	orthbou	ınd			S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	lysis Fro	m 07:00	O AM to	08:45 A	AM - Peak	1 of 1										
Peak Hour for E	Intire Int	ersection	n Begir	ns at 07	:30 AM											
07:30 AM	4	0	1	0	5	3	130	0	0	133	0	111	3	0	114	252
07:45 AM	3	0	2	0	5	2	112	1	0	115	0	125	3	0	128	248
08:00 AM	2	0	2	0	4	3	126	0	0	129	0	111	3	0	114	247
08:15 AM	1_	0	0	0	1	2	105	0	0	107	0	105	1	0	106	214
Total Volume	10	0	5	0	15	10	473	1	0	484	0	452	10	0	462	961
% App. Total	66.7	0	33.3	0		2.1	97.7	0.2	0		0	97.8	2.2	0		
PHF	.625	.000	.625	.000	.750	.833	.910	.250	.000	.910	.000	.904	.833	.000	.902	.953
Cars	8	0	3	0	11	8	416	1	0	425	0	409	7	0	416	852
% Cars	80.0	0	60.0	0	73.3	80.0	87.9	100	0	87.8	0	90.5	70.0	0	90.0	88.7
Trucks (SU)	2	0	2	0	4	1	44	0	0	45	0	37	3	0	40	89
% Trucks (SU)	20.0	0	40.0	0	26.7	10.0	9.3	0	0	9.3	0	8.2	30.0	0	8.7	9.3
Trucks (TT)	0	0	0	0	0	1	13	0	0	14	0	6	0	0	6	20
% Trucks (TT)	0	0	0	0	0	10.0	2.7	0	0	2.9	0	1.3	0	0	1.3	2.1

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 9th Avenue File Name: Memorial Drive and 9th Ave - PM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

Groups Printed- Cars - Trucks (50) - Trucks (11)														•		
		9	th Aver	nue			Me	morial l	Drive			Me	morial l	Drive		
		E	astbou	nd			N	orthbo	und			S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
04:30 PM	6	0	3	0	9	4	110	0	0	114	0	163	3	0	166	289
04:45 PM	1	0	3	0	4	3	129	0	0	132	0	146	2	0	148	284
Total	7	0	6	0	13	7	239	0	0	246	0	309	5	0	314	573
05:00 PM	1	0	4	0	5	3	140	0	0	143	0	155	3	0	158	306
05:15 PM	1	0	7	0	8	5	113	0	0	118	0	164	5	0	169	295
05:30 PM	3	0	6	0	9	3	140	0	0	143	0	143	4	0	147	299
05:45 PM	2	0	4	0	6	0	130	0	0	130	0	124	3	0	127	263
Total	7	0	21	0	28	11	523	0	0	534	0	586	15	0	601	1163
06:00 PM	3	0	3	0	6	1	118	0	0	119	0	98	6	0	104	229
06:15 PM	1	0	1	0	2	2	116	0	0	118	0	100	1	0	101	221
Grand Total	18	0	31	0	49	21	996	0	0	1017	0	1093	27	0	1120	2186
Apprch %	36.7	0	63.3	0		2.1	97.9	0	0		0	97.6	2.4	0		
Total %	0.8	0	1.4	0	2.2	1_	45.6	0	0	46.5	0	50	1.2	0	51.2	
Cars	18	0	27	0	45	16	933	0	0	949	0	1028	24	0	1052	2046
% Cars	100	0	87.1	0	91.8	76.2	93.7	0	0	93.3	0	94.1	88.9	0	93.9	93.6
Trucks (SU)	0	0	3	0	3	5	60	0	0	65	0	53	3	0	56	124
% Trucks (SU)	0	0	9.7	0	6.1	23.8	6	0	0	6.4	0	4.8	11.1	0	5	5.7
Trucks (TT)	0	0	1	0	1	0	3	0	0	3	0	12	0	0	12	16
% Trucks (TT)	0	0	3.2	0	2	0	0.3	0	0	0.3	0	1.1	0	0	1.1	0.7

Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

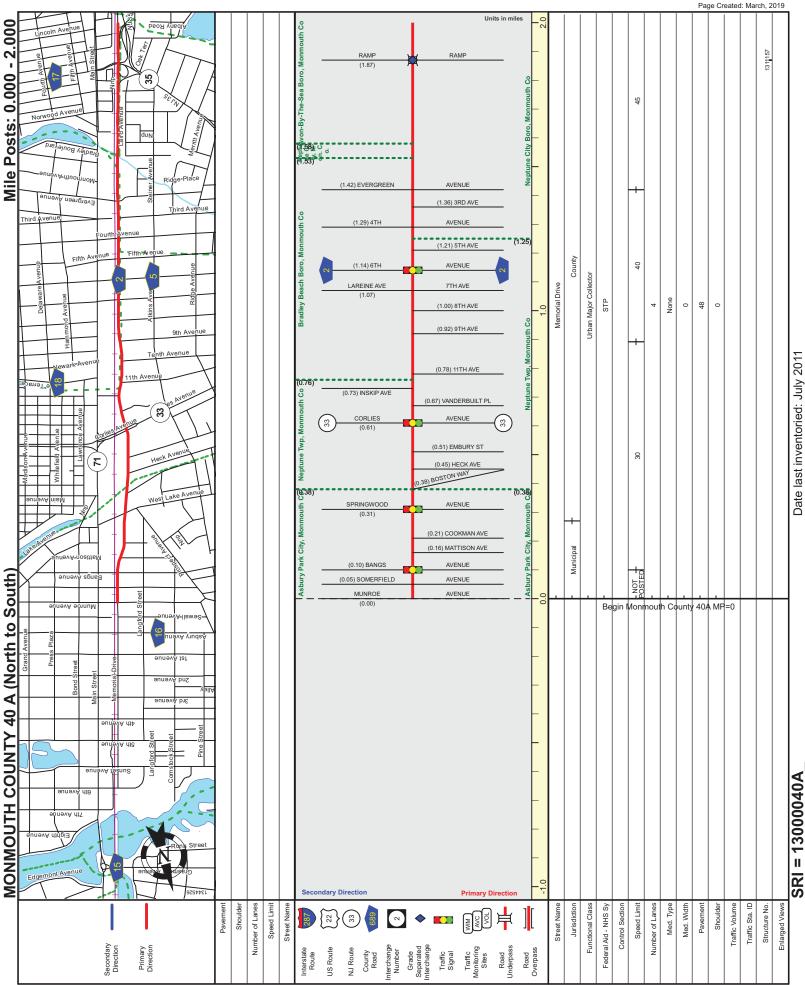
1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: 9th Avenue File Name: Memorial Drive and 9th Ave - PM

N/S: Memorial Drive Site Code : 00000000 Town/County: Neptune/Monmouth Start Date : 3/10/2022

Job #: 2241-99-002T Page No : 2

		91	h Aven	ue			Mei	morial I	Orive			Mei	morial I	Drive		
		Ę	astbou	nd			N ₂	orthbou	ınd			S	outhbo			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Anal	lysis Fro	m 04:30	0 PM to	06:15 F	PM - Peak	1 of 1										
Peak Hour for E	ntire Int	ersectio	n Begir	s at 04	:45 PM											
04:45 PM	1	0	3	0	4	3	129	0	0	132	0	146	2	0	148	284
05:00 PM	1	0	4	0	5	3	140	0	0	143	0	155	3	0	158	306
05:15 PM	1	0	7	0	8	5	113	0	0	118	0	164	5	0	169	295
05:30 PM	3	0	6	0	9	3	140	0	0	143	0	143	4	0	147	299
Total Volume	6	0	20	0	26	14	522	0	0	536	0	608	14	0	622	1184
% App. Total	23.1	0	76.9	0		2.6	97.4	0	0		0	97.7	2.3	0		
PHF	.500	.000	.714	.000	.722	.700	.932	.000	.000	.937	.000	.927	.700	.000	.920	.967
Cars	6	0	17	0	23	12	487	0	0	499	0	573	11	0	584	1106
% Cars	100	0	85.0	0	88.5	85.7	93.3	0	0	93.1	0	94.2	78.6	0	93.9	93.4
Trucks (SU)	0	0	2	0	2	2	34	0	0	36	0	33	3	0	36	74
% Trucks (SU)	0	0	10.0	0	7.7	14.3	6.5	0	0	6.7	0	5.4	21.4	0	5.8	6.3
Trucks (TT)	0	0	1	0	1	0	1	0	0	1	0	2	0	0	2	4
% Trucks (TT)	0	0	5.0	0	3.8	0	0.2	0	0	0.2	0	0.3	0	0	0.3	0.3



Appendix C Capacity Analysis

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			414	ΦÞ	
Traffic Vol, veh/h	17	11	3	520	512	7
Future Vol, veh/h	17	11	3	520	512	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_	None	-	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	20	0	12	2	0
Mvmt Flow	18	12	3	547	539	7
		-		- · · ·	- 500	
N. A	N. C.					
	Minor2		Major1		/lajor2	
Conflicting Flow All	823	273	546	0	-	0
Stage 1	543	-	-	-	-	-
Stage 2	280	-	-	-	-	-
Critical Hdwy	6.8	7.3	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.5	2.2	-	-	-
Pot Cap-1 Maneuver	316	673	1033	-	-	-
Stage 1	552	-	-	-	-	-
Stage 2	748	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	315	673	1033	-	-	-
Mov Cap-2 Maneuver	315	-	_	-	-	-
Stage 1	550	-	-	-	-	-
Stage 2	748	-	_	-	_	-
y -						
	==		L ID		0.5	
Approach	EB		NB		SB	
HCM Control Delay, s	14.8		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)	I.	1033	-	398	-	-
HCM Lane V/C Ratio		0.003		0.074		
					-	-
HCM Control Delay (s) HCM Lane LOS		8.5	0	14.8	-	-
	\	A	Α	В	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

0.4					
EBL	EBR	NBL	NBT	SBT	SBR
14	11	9	592	707	5
14	11	9	592	707	5
0	0	0	0	0	0
		Free	Free	Free	Free
-	None	_	None	-	None
0	-	-	-	-	-
	-	-	0	0	-
0	-	_			-
95	95	95			95
				1	25
		9			5
	-		V_V		
	375	749	0	-	0
	-	-	-	-	-
	-	-	-	-	-
6.96	7.3	4.36	-	-	-
5.96	-	-	-	-	-
5.96	-	-	-	-	-
3.58	3.5	2.33	-	-	-
204	574	787	-	-	-
414	-	-	-	-	-
683	-	-	-	-	-
			-	-	-
201	574	787	-	-	-
201	-	-	-	-	-
	-	-	-	-	-
	_	_	_	_	_
19.1		0.2		0	
С					
, †	NDI	NDT	ERI n1	CDT	SBR
l .					
					-
					-
				-	-
	Λ	Λ	С	_	-
)	A 0	A -	0.3	_	
	EBL 14 14 0 Stop 0 4,# 0 95 8 15 Minor2 1077 747 330 6.96 5.96 5.96 3.58 204 414 683 201 407 683 EB	EBL EBR 14 11 14 11 0 0 Stop Stop - None 0 95 95 8 20 15 12 Minor2 N 1077 375 747 330 6.96 7.3 5.96 5.96 3.58 3.5 204 574 414 683 201 574 201 407 683 EB 19.1 C at NBL 787 0.012	EBL EBR NBL 14 11 9 14 11 9 0 0 0 0 Stop Stop Free - None - 0 95 95 95 8 20 13 15 12 9 Minor2 Major1 1077 375 749 747 330 6.96 7.3 4.36 5.96 5.96 5.96 3.58 3.5 2.33 204 574 787 414 683 201 574 787 201 407 683 EB NB 19.1 0.2 C it NBL NBT 787 - 0.012 - 9.6 0.1	EBL EBR NBL NBT 14 11 9 592 14 11 9 592 0 0 0 0 0 Stop Stop Free Free - None 0 0 95 95 95 95 8 20 13 7 15 12 9 623 Minor2 Major1 N 1077 375 749 0 747 330 5.96	EBL EBR NBL NBT SBT Y A † † † † † † † † † † † † † † † † † † †

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥.	LDI/	NDL	44	↑ ↑	ODIC
Traffic Vol, veh/h	'T' 18	12	4	↔ T 547	T № 538	7
Future Vol, veh/h	18	12	4	547	538	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	riee -	None	riee -	None
Storage Length	0	None -	_	None -	-	NOHE -
Veh in Median Storage	-	_	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	95	20	95	12	2	95
Mvmt Flow	19	13	4	576	566	7
IVIVIIIL FIOW	19	13	4	3/0	200	1
Major/Minor N	Minor2	N	Major1	N	//ajor2	
Conflicting Flow All	866	287	573	0	-	0
Stage 1	570	-	-	-	-	-
Stage 2	296	-	-	-	-	-
Critical Hdwy	6.8	7.3	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	_
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.5	2.2	-	_	-
Pot Cap-1 Maneuver	296	659	1010	-	-	-
Stage 1	535	-	-	_	-	-
Stage 2	735	-	-	-	-	-
Platoon blocked, %	, 00			_	-	_
Mov Cap-1 Maneuver	294	659	1010	_		_
Mov Cap-1 Maneuver	294	-		_	-	_
Stage 1	532	-		-	-	-
Stage 1	735	-	-	-	-	_
Sidye Z	133	-	_	-	_	-
Approach	EB		NB		SB	
прргодоп			0.1		0	
HCM Control Delay, s	15.4		0.1			
	15.4 C		0.1			
HCM Control Delay, s			0.1			
HCM Control Delay, s HCM LOS	С	NIDI		EDI 4	CDT	CDD
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	С	NBL	NBT I	EBLn1	SBT	SBR
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	С	1010	NBT I	378	-	-
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	C ut	1010 0.004	NBT I	378 0.084	-	-
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	C ut	1010 0.004 8.6	NBT I	378 0.084 15.4	- - -	- - -
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	C et	1010 0.004	NBT I	378 0.084	-	-

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			414	† }	
Traffic Vol, veh/h	15	12	9	622	743	6
Future Vol, veh/h	15	12	9	622	743	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	8	20	13	7	1	25
Mvmt Flow	16	13	9	655	782	6
NA : (NA)	14' 6					
	Minor2		Major1		//ajor2	
Conflicting Flow All	1131	394	788	0	-	0
Stage 1	785	-	-	-	-	-
Stage 2	346	-	-	-	-	-
Critical Hdwy	6.96	7.3	4.36	-	-	-
Critical Hdwy Stg 1	5.96	-	-	-	-	-
Critical Hdwy Stg 2	5.96	-	-	-	-	-
Follow-up Hdwy	3.58	3.5	2.33	-	-	-
Pot Cap-1 Maneuver	188	557	759	-	-	-
Stage 1	395	-	-	-	-	-
Stage 2	670	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	184	557	759	-	-	-
Mov Cap-2 Maneuver	184	-	-	-	-	-
Stage 1	387	-	-	-	-	-
Stage 2	670	-	-	-	-	-
y -						
			L ID		0.5	
Approach	EB		NB		SB	
HCM Control Delay, s	20.4		0.2		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)	IV.	759	-	262	-	-
HCM Lane V/C Ratio		0.012		0.108	-	-
		9.8	0.1	20.4		
HCM Control Delay (s) HCM Lane LOS				20.4 C	-	-
HCM Lane LOS HCM 95th %tile Q(veh	١	A 0	Α	0.4	-	-
HOW SOUL WILLE CALACT)	U	-	0.4	-	-

0.5					
EBL	EBR	NBL	NBT	SBT	SBR
20	16	6	549	542	8
20	16	6	549	542	8
0	0	0	0	0	0
		Free	Free	Free	Free
-	None	_	None	-	None
0	-	-	-	_	-
	-	-	0	0	-
0	-	_			-
	95	95			95
					0
					8
_			0.0		
	290	579	0	-	0
	-	-	-	-	-
	-	-	-	-	-
6.8	7.3	4.1	-	-	-
5.8	-	-	-	-	-
5.8	-	-	-	-	-
3.5	3.5	2.2	-	-	-
292	656	1005	-	-	-
532	-	-	-	-	-
731	-	-	-	-	-
			-	-	-
289	656	1005	-	-	-
289	-	-	-	-	-
	-	-	-	_	-
	_	_	_	_	_
15.4		0.1		0	
С					
+	NDI	NDT	ERI n1	CDT	SBR
· ·					
					-
					-
				-	-
)				-	-
	0	_	0.3	_	_
	EBL 20 20 0 Stop - 0,# 0 0 95 0 21 Minor2 876 575 301 6.8 5.8 3.5 292 532 731 289 289 527 731 EBB 15.4 C	EBL EBR 20 16 20 16 0 0 Stop Stop - None 0 9, # 0 95 95 0 20 21 17 Minor2 N 876 290 575 301 6.8 7.3 5.8 5.8 3.5 3.5 292 656 532 731 289 656 289 527 731 EB 15.4 C tt NBL 1005 0.006 8.6 A	EBL EBR NBL 20 16 6 20 16 6 0 0 0 0 Stop Stop Free - None - 0 95 95 95 0 20 0 21 17 6 Minor2 Major1 876 290 579 575 301 6.8 7.3 4.1 5.8 5.8 3.5 3.5 2.2 292 656 1005 532 731 289 656 1005 289 731 EB NB 15.4 0.1 C ot NBL NBT 1005 - 0.006 - 8.6 0 A A	EBL EBR NBL NBT 20 16 6 549 20 16 6 549 0 0 0 0 0 Stop Stop Free Free - None 0 0 95 95 95 95 0 20 0 12 21 17 6 578 Minor2 Major1 N 876 290 579 0 575 301 6.8 7.3 4.1 - 5.8 5.8 5.8 5.8 292 656 1005 - 532 731 289 656 1005 - 289 731 EB NB 15.4 0.1 C It NBL NBT EBLn1 1005 - 385 0.006 - 0.098 8.6 0 15.4 A A C	EBL EBR NBL NBT SBT 20 16 6 549 542 20 16 6 549 542 0 0 0 0 0 0 0 Stop Stop Free Free Free - None - None 0 0 0 95 95 95 95 95 0 20 0 12 2 21 17 6 578 571 Minor2 Major1 Major2 876 290 579 0 - 575 301 6.8 7.3 4.1 5.8

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EDK	INDL			אמט
Lane Configurations	10	40	17	4 ↑	↑ ↑	40
Traffic Vol, veh/h	18	18	17	630	749	10
Future Vol, veh/h	18	18	17	630	749	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	•	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	8	20	13	7	1	25
Mvmt Flow	19	19	18	663	788	11
Major/Minor I	Minor2	N	Jaior1	N	Major2	
			Major1			^
Conflicting Flow All	1162	400	799	0	-	0
Stage 1	794	-	-	-	-	-
Stage 2	368	-	-	-	-	-
Critical Hdwy	6.96	7.3	4.36	-	-	-
Critical Hdwy Stg 1	5.96	-	-	-	-	-
Critical Hdwy Stg 2	5.96	-	-	-	-	-
Follow-up Hdwy	3.58	3.5	2.33	-	-	-
Pot Cap-1 Maneuver	179	552	752	-	-	-
Stage 1	391	-	-	-	-	-
Stage 2	653	-	_	-	-	-
Platoon blocked, %	300			_	_	_
Mov Cap-1 Maneuver	172	552	752	_	_	_
Mov Cap-1 Maneuver	172	- 552	102	_		
Stage 1	376		-		<u>-</u>	<u>-</u>
		-	-	-	-	-
Stage 2	653	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	21.1		0.5		0	
HCM LOS	C		3.0			
TIOWI EOU	J					
Minor Lane/Major Mvm	ıt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		752	-	262	_	-
		0.024	-	0.145	-	-
HCM Lane V/C Ratio						
HCM Lane V/C Ratio HCM Control Delay (s)		9.9	0.2	21.1	-	-
		9.9 A	0.2 A	21.1 C	-	-
HCM Control Delay (s)						

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			41	↑ ↑	02.1
Traffic Vol, veh/h	11	6	11	526	513	11
Future Vol, veh/h	11	6	11	526	513	11
Conflicting Peds, #/hr	0	0	0	020	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Olop	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storag			_	0	0	
Grade, %	0			0	0	
		- 0 <i>E</i>	-			95
Peak Hour Factor	95	95	95	95	95	
Heavy Vehicles, %	20	40	20	12	10	30
Mvmt Flow	12	6	12	554	540	12
Major/Minor	Minor2	N	Major1	N	Major2	
Conflicting Flow All	847	276	552	0		0
Stage 1	546	-	-	-	_	-
Stage 2	301	_	_	_	_	_
Critical Hdwy	7.2	7.7	4.5	_	_	_
Critical Hdwy Stg 1	6.2	- 1.1	7.5	_	_	_
Critical Hdwy Stg 2	6.2	_	_		_	_
Follow-up Hdwy	3.7	3.7	2.4	-	_	_
		619	899	-		-
Pot Cap-1 Maneuver	268		699	-	-	-
Stage 1	496	-	-	-	-	-
Stage 2	674	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver		619	899	-	-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	487	-	-	-	-	-
Stage 2	674	-	-	-	-	-
,						
A mana a ab	ED		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	16.5		0.3		0	
HCM LOS	С					
Minor Lane/Major Mvr	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		899	-			<u> </u>
HCM Lane V/C Ratio		0.013		0.054	-	-
	\					-
HCM Control Delay (s HCM Lane LOS	7	9.1	0.1	16.5	-	-
	.\	A	Α	C	-	-
HCM 95th %tile Q(veh	1)	0	-	0.2	-	-

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	רטוג	NDL	41	↑ \$	אופט
Traffic Vol, veh/h	'T' 7	23	16	4 T 590	T № 689	16
Future Vol, veh/h	7	23	16	590	689	16
Conflicting Peds, #/hr	0	0	0	0	009	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Siop -	None	riee -	None	riee -	
		None -	-		-	
Storage Length Veh in Median Storage	0,# 0		-	0	0	-
		-				
Grade, %	0	- 07	- 07	0	0	- 07
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	15	14	7	6	21
Mvmt Flow	7	24	16	608	710	16
Major/Minor I	Minor2	N	/lajor1	N	Major2	
Conflicting Flow All	1054	363	726	0		0
Stage 1	718	-		-	_	-
Stage 2	336	_	<u>-</u>	<u>-</u>	_	_
Critical Hdwy	6.8	7.2	4.38	_	_	
Critical Hdwy Stg 1	5.8	1.2	7.50	_		
Critical Hdwy Stg 2	5.8	_		_	-	
	3.5	3.45	2.34	•	-	-
Follow-up Hdwy Pot Cap-1 Maneuver	225	598	798	-	-	-
•	449	530	130	-	-	-
Stage 1		-	-	-	-	-
Stage 2	702	-	-	-	-	-
Platoon blocked, %	040	F00	700	-	-	-
Mov Cap-1 Maneuver	218	598	798	-	-	-
Mov Cap-2 Maneuver	218	-	-	-	-	-
Stage 1	436	-	-	-	-	-
Stage 2	702	-	-	-	-	-
Approach	EB		NB		SB	
	14.1		0.4		0	
HCM LOS			0.4		U	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		798	-		-	-
HCM Lane V/C Ratio		0.021		0.073	_	_
HCM Control Delay (s)		9.6	0.1	14.1	_	_
HCM Lane LOS		3.0 A	Α	В	<u> </u>	<u>-</u>
HCM 95th %tile Q(veh)		0.1	-	0.2		
				U.L		

0.4 EBL 12 12 12 r 0 Stop 0 ge, # 0 95 20 13	6 6 0 Stop None 95 40 6	NBL 12 12 0 Free 95 20 13	NBT 553 553 0 Free None 0 0 95 12 582	\$BT \$539 539 0 Free - 0 0 95 10 567	SBR 12 12 0 Free None 95 30
12 12 12 r 0 Stop - 0 ge, # 0 95 20 13	6 6 0 Stop None - - - 95 40 6	12 12 0 Free - - - - 95 20	553 553 0 Free None 0 0 95	539 539 0 Free - 0 0 95	12 12 0 Free None - - - 95
12 12 12 r 0 Stop - 0 ge, # 0 95 20 13	6 6 0 Stop None - - - 95 40 6	12 12 0 Free - - - - 95 20	553 553 0 Free None 0 0 95	539 539 0 Free - 0 0 95	12 12 0 Free None - - - 95
12 12 r 0 Stop - 0 ge, # 0 95 20 13	6 6 0 Stop None - - - 95 40 6	12 0 Free - - - - 95 20	553 553 0 Free None - 0 0 95 12	539 539 0 Free - 0 0 95 10	12 0 Free None - - - 95
12 r 0 Stop - 0 ge, # 0 95 20 13	6 0 Stop None - - 95 40 6	12 0 Free - - - - 95 20	553 0 Free None - 0 0 95 12	539 0 Free - 0 0 95 10	12 0 Free None - - - 95
r 0 Stop - 0 ge, # 0 95 20 13	0 Stop None - - - 95 40 6	0 Free - - - - 95 20	0 Free None - 0 0 95 12	0 Free - 0 0 95 10	0 Free None - - - 95
Stop - 0 ge, # 0 95 20 13	Stop None - - - 95 40 6	Free 95 20	Free None - 0 0 95 12	Free - 0 0 0 95 10	Free None - - - 95
0 ge, # 0 95 20 13	None - - - 95 40 6	- - - 95 20	None 0 0 95 12	0 0 0 95 10	None - - - 95
0 ge, # 0 95 20 13	95 40 6	- - 95 20	0 0 95 12	0 0 0 95 10	- - - 95
ge, # 0 0 95 20 13 Minor2	95 40 6	95 20	0 0 95 12	0 0 95 10	- - 95
0 95 20 13 Minor2	95 40 6	95 20	0 95 12	95 10	95
95 20 13 Minor2	95 40 6	95 20	95 12	95 10	95
20 13 Minor2	40 6	20	12	10	
13 Minor2	6				.3()
Minor2		13	582	56/	
				001	13
	N				
20.1		Major1	١	/lajor2	
891			0	-	0
		_	-	_	-
		_	_	_	-
		4.5	-	-	-
		-	_	_	_
		_	_	_	_
		24	_	_	_
			_	_	_
		-	_	<u>-</u>	_
		_	_	_	_
000	_				_
r 2/5	605	876	-		-
		070			_
		-	-		-
		-	-		-
000	_	<u>-</u>	-	<u>-</u>	-
EB		NB		SB	
s 17.5		0.3		0	
С					
	NDI	NDT	EDL 4	CDT	CDD
/rnt				SBI	SBR
				-	-
				-	-
(s)				-	-
				-	-
eh)	0	-	0.2	_	_
/r	891 574 317 7.2 6.2 3.7 250 479 660 - 245 468 660 EB 5 17.5 C	891 290 574 - 317 - 7.2 7.7 6.2 - 6.2 - 3.7 3.7 250 605 479 - 660 245 605 - 245 - 468 - 660 - EB 5 17.5 C mt NBL 876 0.014 8) 9.2 A	891 290 580 574 317 7.2 7.7 4.5 6.2 6.2 3.7 3.7 2.4 250 605 876 479 660 245 605 876 - 245 468 660 EB NB 5 17.5 0.3 C mt NBL NBT 876 - 0.014 - 5) 9.2 0.1 A A	891 290 580 0 574 317 7.2 7.7 4.5 - 6.2 6.2 3.7 3.7 2.4 - 250 605 876 - 479 660 245 605 876 - 468 660 EB NB 5 17.5 0.3 C mt NBL NBT EBLn1 876 - 306 0.014 - 0.062 5) 9.2 0.1 17.5 A A C	891 290 580 0 - 574 317 7.2 7.7 4.5 6.2 6.2 3.7 3.7 2.4 250 605 876 479 660 245 605 876 468 660 5245 660 660 70 245 70 245 70 250 605 876 70 250 605 876 70 250 605 876 70 250 605 876 70 250 876 876 70 250 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 70 250 876 876 876 70 250 876 876 876 70 250 876 876 876 876 876 876 876 876 876 876

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			41	†	
Traffic Vol, veh/h	7	25	17	620	724	17
Future Vol, veh/h	7	25	17	620	724	17
Conflicting Peds, #/hr	0	0	0	020	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	
Storage Length	0	-	_	-		-
Veh in Median Storage			_	0	0	
Grade, %	0	-	_	0	0	_
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	15	14	7	6	21
Mvmt Flow	7	26	18	639	746	18
Major/Minor	Minor2	N	Major1	N	Major2	
Conflicting Flow All	1111	382	764	0	_	0
Stage 1	755		_	-	_	-
Stage 2	356	_	_	_	_	_
Critical Hdwy	6.8	7.2	4.38	_	_	_
Critical Hdwy Stg 1	5.8	- 1.2	1.00	_	_	_
Critical Hdwy Stg 2	5.8		_	_	_	
Follow-up Hdwy	3.5	3.45	2.34	-	-	-
		580	771	-	-	-
Pot Cap-1 Maneuver	206	500	771	-	-	-
Stage 1	430	-	-	-	-	-
Stage 2	686	-	-	-	-	-
Platoon blocked, %	,			-	-	-
Mov Cap-1 Maneuver	199	580	771	-	-	-
Mov Cap-2 Maneuver	199	-	-	-	-	-
Stage 1	415	-	-	-	-	-
Stage 2	686	-	-	-	-	-
A mara a ab	ED		ND		O.D.	
Approach	EB		NB		SB	
HCM Control Delay, s	14.6		0.5		0	
HCM LOS	В					
Minor Lane/Major Mvm	ıt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)		771	-		OD I	אופט
HCM Lane V/C Ratio				0.081	-	-
		0.023			-	-
HCM Control Delay (s)		9.8	0.2		-	-
HCM Lane LOS		A	Α	В	-	-
HCM 95th %tile Q(veh)		0.1	-	0.3	-	-

10: Memorial Drive (CR 40 A) & 9th Avenue

Intersection Int Delay, s/veh 0.5 **EBL EBR** Movement **NBL NBT** SBT **SBR** Lane Configurations W 41 **†** Traffic Vol, veh/h 14 10 555 540 13 Future Vol, veh/h 14 10 14 555 540 13 Conflicting Peds, #/hr 0 0 0 0 0 Free Sign Control Stop Stop Free Free Free RT Channelized None None None Storage Length 0 Veh in Median Storage, # 0 0 0 Grade, % 0 0 0 Peak Hour Factor 95 95 95 95 95 95 Heavy Vehicles, % 20 40 20 12 10 30 Mvmt Flow 15 11 15 584 568 14 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 897 291 582 0 0 Stage 1 575 Stage 2 322 Critical Hdwy 7.2 7.7 4.5 Critical Hdwy Stg 1 6.2 Critical Hdwy Stg 2 6.2 Follow-up Hdwy 3.7 3.7 2.4 Pot Cap-1 Maneuver 248 604 874 Stage 1 479 _ _ Stage 2 656 Platoon blocked, % 242 604 Mov Cap-1 Maneuver 874 Mov Cap-2 Maneuver 242 Stage 1 467 Stage 2 656 Approach EB NB SB HCM Control Delay, s 17.1 0.3 0 **HCM LOS** С Minor Lane/Major Mvmt NBL NBT EBLn1 SBT **SBR** Capacity (veh/h) 874 323 HCM Lane V/C Ratio 0.017 - 0.078

HCM Control Delay (s)

HCM 95th %tile Q(veh)

HCM Lane LOS

9.2

Α

0.1

0.1

Α

17.1

С

0.3

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			41	↑ ⊅	
Traffic Vol, veh/h	10	31	25	623	728	21
Future Vol, veh/h	10	31	25	623	728	21
Conflicting Peds, #/hr	0	0	0	023	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	-	0	0	
Grade, %	0	- 07	- 07	0	0	- 07
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	15	14	7	6	21
Mvmt Flow	10	32	26	642	751	22
Major/Minor	Minor2	N	Major1	N	//ajor2	
Conflicting Flow All	1135	387	773	0	-,	0
Stage 1	762	-		-	_	-
Stage 2	373	_	_	_		_
Critical Hdwy	6.8	7.2	4.38			
	5.8	1.2	4.30		-	-
Critical Hdwy Stg 1		-	-	-	-	-
Critical Hdwy Stg 2	5.8	2.45	-	-	-	-
Follow-up Hdwy	3.5	3.45	2.34	-	-	-
Pot Cap-1 Maneuver	199	576	764	-	-	-
Stage 1	427	-	-	-	-	-
Stage 2	672	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	188	576	764	-	-	-
Mov Cap-2 Maneuver	188	-	-	-	_	-
Stage 1	404	-	-	-	-	-
Stage 2	672	_	_	_	_	-
J	J					
Approach	EB		NB		SB	
HCM Control Delay, s	15.6		0.6		0	
HCM LOS	С					
Minor Lane/Major Mvr	nt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)		764	-	383	ODT	אופט
HCM Lane V/C Ratio					-	-
		0.034	-	0.11	-	-
HCM Control Delay (s		9.9	0.2	15.6	-	-
HCM Lane LOS	\	A	Α	C	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	-	-

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	1>	1151	₩	UDIK
Traffic Vol, veh/h	3	30	11	3	6	5
Future Vol, veh/h	3	30	11	3	6	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -		riee -	None	Stop -	None
Storage Length	_	-	-	NOHE -	0	INUITE
	- +	0	0		0	_
Veh in Median Storage						-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	2	8	0	2	2	2
Mvmt Flow	4	35	13	4	7	6
Major/Minor	Major1	N	Major2	N	Minor2	
Conflicting Flow All	17	0	-	0	58	15
Stage 1	-	-	_	-	15	-
Stage 2	_	_	_	_	43	_
Critical Hdwy	4.12		-	_	6.42	6.22
•			-		5.42	
Critical Hdwy Stg 1	-	-	-	-		-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1600	-	-	-	949	1065
Stage 1	-	-	-	-	1008	-
Stage 2	-	-	-	-	979	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1600	-	-	-	946	1065
Mov Cap-2 Maneuver	_	-	-	-	946	-
Stage 1	_	_	-	_	1005	_
Stage 2	_	_	_	_	979	_
Olago 2					010	
Approach	EB		WB		SB	
HCM Control Delay, s	0.7		0		8.7	
HCM LOS					Α	
NA:	-4	EDI	EDT	WDT	WDD	ODL 4
Minor Lane/Major Mvn	nt	EBL	EBT	WBT		SBLn1
Capacity (veh/h)		1600	-	-	-	997
HCM Lane V/C Ratio		0.002	-	-		0.013
HCM Control Delay (s)		7.3	0	-	-	8.7
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh)	0	-	-	-	0

Build - PM 30: 8th Avenue & Site Driveway

Intersection						
Int Delay, s/veh	2.5					
	EBL	EDT	WDT	WDD	CDI	CDD
Movement	ERL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	€	∱	40	Y	^
Traffic Vol, veh/h	8	27	15	12	9	6
Future Vol, veh/h	8	27	15	12	9	6
Conflicting Peds, #/hr	0 Eroo	0 Eroo	0 Eroo	0 Eroo	0 Stop	0 Stop
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	- 0	-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	73	73	73	73	73	- 73
Peak Hour Factor		13				73
Heavy Vehicles, %	2		17	16	2	2
Mvmt Flow	11	37	21	16	12	8
Major/Minor I	Major1	N	Major2		Minor2	
Conflicting Flow All	37	0	-	0	88	29
Stage 1	-	-	-	-	29	-
Stage 2	-	-	-	-	59	-
Critical Hdwy	4.12	-	_	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	_	_	5.42	-
Follow-up Hdwy	2.218	-	_	_	3.518	
Pot Cap-1 Maneuver	1574	_	_	_	913	1046
Stage 1	-	-	-	-	994	-
Stage 2				_	964	_
Platoon blocked, %		-	-	_	JU-T	
Mov Cap-1 Maneuver	1574	-	-	-	907	1046
Mov Cap-2 Maneuver	1074	_	_	-	907	1040
Stage 1	-	-	_		987	-
	-	-	-	-	987	
Stage 2	-	-	-	-	904	-
Approach	EB		WB		SB	
HCM Control Delay, s	1.7		0		8.8	
HCM LOS					Α	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR :	SBI n1
	· C		וטו	וטייי		
Capacity (veh/h)		1574	-	-	-	958
HCM Control Dolay (s)		0.007	0	-		0.021
HCM Control Delay (s) HCM Lane LOS		7.3		-	-	8.8
	\	A	Α	-	-	A
HCM 95th %tile Q(veh)		0	-	-	-	0.1

Build - AM 40: Site Driveway & 9th Avenue

Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			4	¥	
Traffic Vol, veh/h	18	2	3	24	4	6
Future Vol, veh/h	18	2	3	24	4	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		_	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	27	2	2	25	2	2
Mvmt Flow	23	3	4	30	5	8
Major/Minor	Major1		Majara		Minor1	
	Major1		Major2			O.F.
Conflicting Flow All	0	0	26	0	63	25
Stage 1	-	-	-	-	25	-
Stage 2	-	-	- 4.40	-	38	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1588	-	943	1051
Stage 1	-	-	-	-	998	-
Stage 2	-	-	-	-	984	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1588	-	940	1051
Mov Cap-2 Maneuver	-	-	-	-	940	-
Stage 1	-	-	-	-	998	-
Stage 2	-	-	-	-	981	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.6	
HCM LOS	U		0.0		0.0 A	
TION LOS					А	
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1004	-		1588	-
HCM Lane V/C Ratio		0.012	-	-	0.002	-
HCM Control Delay (s))	8.6	-	-	7.3	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh	1)	0	-	-	0	-

Build - PM 40: Site Driveway & 9th Avenue

Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	LDIK	1100	₩	¥	HOIL
Traffic Vol, veh/h	32	7	12	34	T 5	9
Future Vol, veh/h	32	7	12	34	5	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -	None	riee -	None	Stop -	
	-	None -	-		0	None -
Storage Length			-	0	0	
Veh in Median Storage		-				-
Grade, %	0	- 70	- 70	0	0	- 75
Peak Hour Factor	75	75	75	75	75	75
Heavy Vehicles, %	12	2	2	18	2	2
Mvmt Flow	43	9	16	45	7	12
Major/Minor	Major1		Major2	N	Minor1	
Conflicting Flow All	0	0	52	0	125	48
Stage 1	-	-	-	-	48	-
Stage 2	_	_	_	_	77	_
Critical Hdwy	_	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	<u>-</u>	_	1.12	_	5.42	V.LL
Critical Hdwy Stg 2	-		_		5.42	_
Follow-up Hdwy	<u>-</u>	_	2.218	_	3.518	
Pot Cap-1 Maneuver	-	-	1554		870	1021
•	-	-	1004	-	974	1021
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	946	-
Platoon blocked, %	-	-	4554	-	000	1001
Mov Cap-1 Maneuver	-	-	1554	-	860	1021
Mov Cap-2 Maneuver	-	-	-	-	860	-
Stage 1	-	-	-	-	974	-
Stage 2	-	-	-	-	936	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.9		8.8	
•	U		1.9			
HCM LOS					Α	
Minor Lane/Major Mvn	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		957	-		1554	-
HCM Lane V/C Ratio		0.02	-	-	0.01	-
HCM Control Delay (s)		8.8	_	_	7.3	0
HCM Lane LOS		A	_	_	A	A
HCM 95th %tile Q(veh)	0.1	_	_	0	-
1.5m cour /ouio a(von	1	J. 1			- 0	