

STORMWATER MANAGEMENT, GROUNDWATER RECHARGE AND WATER QUALITY ANALYSIS

For

**Surfside Crossing
Proposed Multi-Family Mixed Use Building**

**1102 9th Ave
Block 405, Lot 5-7
Neptune Township
Monmouth County, NJ**

Prepared by:



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A handwritten signature in black ink, appearing to read 'Steven R. Cattani', is positioned above a horizontal line.

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I. SITE DESCRIPTION

The project site consists of Block 405, Lots 5-7, located at the intersection of 9th Ave, Memorial Drive, and 8th Ave. in Neptune Township, Monmouth County, New Jersey. Currently, the site is mostly open with wooded areas towards the central portion of the combined lots. In addition, the site currently has two single-family homes located on existing lot 7. The subject site is 79,034 square feet (1.81 acres). The site is bordered to the north by 9th Ave; to the east Memorial Drive; to the south 8th Ave, and the west by residential uses. The project consists of developing the parcel with a proposed multi-family mixed use building totaling 162,588 square feet over multiple floors. The building includes 70 residential units, and 1,976 SF of retail with one hundred and thirty (130) total passenger surface vehicle parking spaces, driveways, landscaping and other related site improvements.

The existing conditions of the tract have been verified by the Alta/ NSPS Land Title Survey, prepared by Dynamic Survey, LLC, dated 3/30/2022.

II. DESIGN OVERVIEW

This report has been prepared to define and analyze the stormwater drainage conditions that would occur as a result of the development of Block 405, Lots 5-7 in Neptune Township, Monmouth County, New Jersey. The project includes new stormwater management facilities to address applicable aspects of Neptune Township Stormwater Management rules and NJAC 7:8.

Based upon the fact that the proposed improvements will result in more than one (1) acre of land disturbance and increase the amount of impervious coverage by more than 0.25 acres, this project is classified as a “major development”; and therefore, has been designed to meet the stormwater runoff quantity, quality and groundwater recharge standards, set forth by Neptune Township Land Use Ordinance and NJAC 7:8. Accordingly, the following items are addressed within this report:

- Erosion control, groundwater recharge and runoff quantity standards (7:8-5.4)
- Stormwater runoff quality standards (7:8-5.5)
- Calculation of stormwater runoff and groundwater recharge (7:8-5.6)
- Standards for structural stormwater management measures (7:8-5.7)

The scope of the report includes the proposed multi-family dwelling, basins, driveways, parking areas, landscaping and other related site improvements as shown on the engineering drawings. The proposed overall site plan contains 75.1% impervious lot coverage. The storm systems on site have been designed using this coverage.

A hydrological evaluation is provided for the NJDEP Water Quality, 2, 10, 25, and 100-year storm events utilizing the Urban Hydrology for Small Watersheds TR55 method.

NJAC 7:8-5.4(a)3 states the stormwater quantity impacts can be calculating to meet one the of the following below:

i. Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction runoff hydrographs for the 2, 10 and 100-year storm events do not exceed, at any point in time, the pre-construction runoff hydrographs for the same storm events;

ii. Demonstrate through hydrologic and hydraulic analysis that there is no increase, as compared to the pre-construction condition, in the peak runoff rates of stormwater leaving the site for the two, 10 and 100-year storm events and that the increased volume or change in timing of stormwater runoff will not increase flood damage at or downstream of the site. This analysis shall include the analysis of impacts of existing land uses and projected land uses assuming full development under existing zoning and land use ordinances in the drainage area;

iii. Design stormwater management measures so that the post-construction peak runoff rates for the two, 10 and 100-year storm events are 50, 75 and 80 percent, respectively, of the pre-construction peak runoff rates. The percentages apply only to the post-construction stormwater runoff that is attributable to the portion of the site on which the proposed development or project is to be constructed.

Per the above requirements, runoff from the site will comply with the flow reductions indicated under iii as follows:

2-year:	50% reduction
10-year:	25% reduction
100-year:	20% reduction

This facility will comply with the Stormwater Management Best Management Practices except for several minor diminimus exceptions to the runoff quantity standards.

It is important to note that the aforementioned flow reduction requirements are only required to be applied to onsite drainage areas within the limit of disturbance to satisfy Neptune Township and NJDEP flow reduction requirements. Therefore, the proposed development satisfies the flow reduction requirements by applying the peak rate reduction requirements only to the onsite areas that are proposed to be disturbed, all remaining undisturbed areas on site will continue to discharge under their existing condition.

III. EXISTING DRAINAGE CONDITIONS

The tract has been evaluated with the following drainage sub-watershed areas as depicted on the Existing Conditions Drainage Area Map that can be found in the appendix of this report.

Existing Drainage Area 1: This study area is comprised of mostly grassed areas with portions of the existing asphalt driveways and single-family home. It is analyzed as an area to be disturbed as a result of the proposed development. The stormwater runoff currently flows from the eastern portion of the site to the western end of the property towards the existing 'B' inlet located on 8th Avenue, also known as POA 2.

Existing Drainage Area 2: This study area is comprised of mostly grassed and wooded areas with a portion of the existing asphalt driveways and single-family homes. It is analyzed as an area to be disturbed as a result of the proposed development. The stormwater runoff currently flows from the eastern portion of the site to the western end of the property towards the existing low point located offsite at the rear of the adjacent lot 4. From this low point, runoff flows toward the existing 'B' inlet located on 9th Avenue, also known as POA 1.

Existing Drainage Area 3: This study area is comprised of mostly grassed and wooded areas. It is analyzed as an area to be disturbed as a result of the proposed development. The stormwater runoff currently flows from the eastern portion of the site to the western end of the property towards the existing 'B' inlet located on 9th Avenue, also known as POA 1.

Existing Bypass to Memorial Drive: This study area is comprised of mostly grassed areas. It is analyzed as an area to be minimally disturbed as a result of the proposed development. The stormwater runoff currently flows from the southern portion of the site to the northeastern side of the property towards the existing 'B' inlet located on Memorial Drive, also known as POA 3.

Based on the Monmouth County soils survey information, the soil types native to the site include:

MONMOUTH COUNTY SOIL SURVEY INFORMATION				
SOIL TYPE (SYMBOL)	SOIL TYPE (NAME)	HYDROLOGIC SOIL GROUP	DEFAULT SOIL TYPE (NAME)	DEFAULT HYDROLOGIC SOIL GROUP
UR	Urban Lands	N/A	Fort Mott	A

Per the NJ Stormwater BMP Manual Chapter 12 Soil Testing Criteria, sites that do not have a Hydrologic Soil Group can be defined as Fort Mott, HSG A, as the site is within the Coastal Plain. The soil investigation completed by Dynamic Earth, LLC, confirms the site should be examined using HSG A by completing five (5) permeability tests that show permeability rates consistent with HSG type A soils. The soil borings and soil profile pits generally encountered deep fill material and loose natural soils near the proposed building footprint. Topsoil was encountered between approximately five inches and ten inches of topsoil at the surface.

Beneath the surficial cover, existing fill materials were encountered that generally consisted of sand, loamy sand, sandy loam, and clay with variable amounts of gravel and debris. The debris encountered included brick, metal, pvc, glass, seashells, concrete, and asphalt. The existing fill materials were encountered to depths ranging between approximately 2.5 feet and 5.5 feet below the ground surface; corresponding to elevations ranging between 12.2 feet and 7.6 feet. Beneath the existing fill materials, natural soils were encountered that generally consisted of sand, sandy loam, loam sandy clay loam, silty clay loam, clay loam, silty clay, and clay with variable amounts of gravel. The natural soils were encountered to termination and refusal depths ranging between approximately 10 feet and 12.4 feet below the ground surface; corresponding to elevations ranging between 4.5 feet and 0.7 feet. The refusal encountered was due to continuous wet cave-in of coarse-grained materials.

Indicators of seasonal high groundwater (based on soil mottling and/or direct groundwater observations during the wet season) were encountered at depths ranging between approximately 2.5 feet and 4.6 feet below the ground surface; corresponding to elevations ranging between 12.2 feet and 8.8 feet. Groundwater was encountered at depths ranging between approximately five feet and seven feet below the ground surface; corresponding to elevations ranging between 10.5 feet and 7.5 feet. Groundwater levels are expected to fluctuate seasonally and following significant periods of precipitation. A summary of seasonal high groundwater levels encountered and permeability samples collected is presented in the chart below:

Seasonal High Groundwater and Permeability Test Summary						
Location	Surface Elevation (ft)	Estimated Seasonal High Groundwater		Sample Depth (Inches)	Permeability Results (inches/hour)	
		Depth (ft)	Elevation (ft)		Replicate A	Replicate B
SPP-1	14.7	2.5	12.2	18	Not Tested	
				40		
SPP-2	13.4	4.6	8.8	44		
				78		
SPP-3	13.1	3.3	9.8	24	8.9	7.5
				45	Not Tested	
				70		
SPP-4	13.1	3.0	10.1	24	6.5	13.8
				42	Not Tested	
				100		
SPP-5	13.9	4.0	9.9	24	12.5	6.6
SPP-6	13.2	3.8	9.4	32	>20	>20
				50	Not Tested	
SPP-7	14.5	4.2	10.3	30	14.4	18.7
				50	Not Tested	
SPP-8	13.3	3.9	9.4	36		
				60		
SPP-9	12.9	4.0	8.9	36		

Based on the laboratory testing completed and the subsurface conditions encountered at soil profile pit

excavations, the soils encountered are generally consistent with a hydraulic soil group (HSG) A.

IV. PROPOSED DRAINAGE CONDITIONS

The tract has been evaluated with the following drainage sub-watershed areas as depicted on the Contributory Drainage Area Map that can be found in the appendix of this report. Each sub-watershed area has been calculated as a separate point of analysis.

Proposed Drainage Area 1: This portion of the site mainly consists of the building roof runoff. Stormwater runoff from this area is collected by a series of roof leaders which flows to the proposed above ground small-scale infiltration basin 1. Stormwater discharged from basin 1 flows through the outlet control structure to the existing 'B' inlet on 9th avenue also known as POA 1.

Proposed Study Area 2: This portion of the site mainly consists of the building roof runoff. Stormwater runoff from this area is collected by a series of roof leaders which flows to the proposed above ground small-scale infiltration basin 2. Stormwater discharged from basin 2 flows through the outlet control structure to the subsurface small-scale infiltration basin 3.

Proposed Study Area 3: This portion of the site consists of the majority of the impervious surfaces on the site including the porous pavement, asphalt, and portions of the landscaped areas. Runoff from this drainage area sheet flows to the proposed porous pavement and then to the subsurface small-scale infiltration basin 3. Stormwater discharged from basin 3, flows through the outlet control structure to the existing 'B' inlet on 9th Avenue, also known as POA 1.

Proposed Bypass 8th Ave: This portion of the site consists of a minimal portion of the asphalt driveway and adjacent grassed areas. Runoff from this area flows undetained to the existing 'B' inlet on 8th Avenue, also known as POA 2.

Proposed Bypass 9th Ave: This portion of the site consists of a minimal portion of the asphalt driveway and adjacent grassed areas. Runoff from this area flows undetained to the existing 'B' inlet on 9th Avenue, also known as POA 1.

Proposed Bypass Memorial Drive: This portion of the site consists of a minimal portion of the asphalt driveway and adjacent grassed areas. Runoff from this area flows undetained to the existing 'B' inlet on Memorial Drive also known as POA 3.

V. NON-STRUCTURAL STORMWATER MANAGEMENT STRATEGIES

The proposed project has been designed to the maximum extent practicable by incorporating the nonstructural stormwater management strategies set forth in NJAC 7:8-5.3 as follows:

1. **Protect areas that provide water quality benefits or areas particularly susceptible to erosion and sediment lost:** The proposed impervious surface is minimized wherever possible under the proposed condition; therefore, increasing the water quality benefits on the site. By implementation of the porous pavement and infiltration basins, the proposed development meets the water quality requirements set forth by NJAC 7:8.
2. **Minimize impervious surfaces and break up or disconnect the flow of runoff over impervious surfaces:** The impervious surfaces have been minimized wherever possible. Impervious surfaces have been diverted to multiple structural BMPs capable of providing water quality treatment.
3. **Maximize the protection of natural drainage features and vegetation:** In the proposed condition, there is a 68.4% increase in impervious coverage. A Landscaping Plan has been prepared to compensate for the loss of existing vegetation due to the development.
4. **Minimize the decrease in the “time of concentration” from pre-construction to post-construction. “Time of concentration” is defined as the time it takes for runoff to travel from the hydraulically most distant point of the drainage area to the point of interest within a watershed:** The decrease in the time of concentration has been minimized by maintaining existing overland flow slopes to the maximum extent practical.
5. **Minimize land disturbance including clearing and grading:** Land disturbance has been minimized where feasible. The site disturbance is limited to the development area.
6. **Minimize soil compaction:** Soil compaction will be minimized in the basins and proposed lawn and landscape areas.
7. **Provide low-maintenance landscaping that encourages retention and planting of native vegetation and minimizes the use of lawns, fertilizers and pesticides:** The project proposes low-maintenance trees, shrubs, and ground cover on the site. Refer to the Landscape Plan for plant information.
8. **Provide vegetated open-channel conveyance systems discharging into and through stable vegetated areas:** Due to the site constraints, it is not feasible to design a vegetated open-channel conveyance system on this project.
9. **Provide other source controls to prevent or minimize the use or exposure of pollutants at the site in order to prevent or minimizes the release of those pollutants into stormwater runoff:** The proposed small-scale infiltration basins provide 80% TSS removal. In addition, the porous pavement will provide 80% TSS removal prior to discharging to basin 3. Basin 3 will provide additional water quality measures due to infiltration.

VI. DESIGN METHODOLOGY

The intention of the proposed stormwater management facilities for this project is to comply with applicable required measures from Neptune Township Land Use Ordinance and NJAC 7:8. In order to prepare the stormwater calculations for the subject project, an investigation of the property and topography was

performed. An on-site review of the tract was performed by Dynamic Engineering Consultants, PC, verifying the existing site conditions and land cover characteristics. Dynamic Survey was contracted to prepare the ALTA/NSPS Land Title Survey for the existing site.

Based on our review of the existing site conditions and the Topographic Survey, the Drainage Area Maps for the existing and proposed site conditions as defined within this report were established. A grading plan was developed for the proposed site improvements with consideration to the existing drainage patterns. The plan was then designed to ensure runoff from the proposed development could be directed to stormwater management facilities to the maximum extent practicable in order to address the applicable sections of Neptune Township Stormwater Management rules and NJAC 7:8.

The two (2) above ground small-scale infiltration basins will temporarily store stormwater runoff from the site. An outlet control structure for each basin has been implemented to release stormwater runoff at a controlled rate to satisfy the stormwater quantity requirements. Overflow from the above ground basins is routed via the emergency spillways to the existing downstream stormwater management facilities. The above ground infiltration basins have been designed to meet the requirements set forth by NJAC 7:8.

The porous pavement drains to a subsurface small-scale infiltration basin that temporarily stores and attenuates stormwater runoff from the site. An outlet control structure has been implemented to release stormwater runoff at a controlled rate to satisfy the stormwater quantity requirements. The subsurface small-scale infiltration basin has been designed to meet the recharge requirements set forth by NJAC 7:8.

According to the NJAC 7:8-5.5(a), a TSS removal rate of 80% is required for stormwater runoff generated from vehicular pavement areas as a result of a major development. By the use of small-scale infiltration basins, and porous pavement, the proposed site meets the 80% TSS removal.

VII. STORMWATER MANAGEMENT BASIN DESIGN AND RUNOFF QUANTITY STANDARDS

In order to meet the stormwater runoff quantity and water quality requirements set forth in NJAC 7:8, the site design incorporates porous pavement, a subsurface small-scale infiltration basin, and two (2) above ground small-scale infiltration basins. Runoff from a portion of the roof area flows through a series of roof leaders to each of the above ground small-scale infiltration basins. Discharge from basin 2 flows through the stormwater conveyance systems to the subsurface small-scale infiltration basin 3. Additional runoff to basin 3 flows over land by sheet flow to the pervious paving areas, filters through the pervious paving and is then collected by the subsurface small-scale infiltration basin 3. Outflow from basins 1 and 3 is combined and routed to the existing 'B' inlet on 9th Avenue, also known as POA 1.

Minimal areas adjacent to each road frontage will have runoff that is undetained to each POA. It is important to note that the impervious surfaces that bypass to each POA are less in the proposed condition as compared to existing. An outline of the Drainage Areas is located in the Runoff Curve Number (CN) Calculations Existing & Proposed in the appendix of the report.

A summary of the pre and post development flows are shown in the charts below:

Pre-development and Post Development Peak Runoff Results

PRE VS. POST SUMMARY CHART (FLOW TO POA 1)						
DESIGN STORM	EXISTING DISTURBED AREA (CFS)	HYDRO-GRAPH #	RUNOFF RATE REDUCTION OF DISTURBED AREA	MAXIMUM TOTAL ALLOWABLE RUNOFF RATE (CFS)	PROPOSED RUNOFF RATE (CFS)	HYDRO-GRAPH #
2 Year	0.22	24	50%	0.11	0.10	28
10 Year	0.34	24	25%	0.26	0.15	28
100 Year	2.19	24	20%	1.75	1.56	28

PRE VS. POST SUMMARY CHART (FLOW TO POA 2)						
DESIGN STORM	EXISTING DISTURBED AREA (CFS)	HYDRO-GRAPH #	RUNOFF RATE REDUCTION OF DISTURBED AREA	MAXIMUM TOTAL ALLOWABLE RUNOFF RATE (CFS)	PROPOSED RUNOFF RATE (CFS)	HYDRO-GRAPH #
2 Year	0.12	10	50%	0.06	0.129*	33
10 Year	0.19	10	25%	0.14	0.202*	33
100 Year	0.38	10	20%	0.30	0.378*	33

*De minimis exception requested.

PRE VS. POST SUMMARY CHART (FLOW TO POA 3)						
DESIGN STORM	EXISTING DISTURBED AREA (CFS)	HYDRO-GRAPH #	RUNOFF RATE REDUCTION OF DISTURBED AREA	MAXIMUM TOTAL ALLOWABLE RUNOFF RATE (CFS)	PROPOSED RUNOFF RATE (CFS)	HYDRO-GRAPH #
2 Year	0.00	29	50%	0.00	0.00	30
10 Year	0.00	29	25%	0.00	0.00	30
100 Year	0.041	29	20%	0.03	0.095*	30

*Diminimis exception requested.

VIII. GROUNDWATER RECHARGE & WATER QUALITY

As required by NJAC 7:8-5.5, a TSS removal rate of 80% is required for stormwater generated by the water quality design storm as a result of a major development. The design for the subject development meets the obligation for TSS removal by utilizing porous pavement, one (1) subsurface infiltration basin, and two (2) above ground infiltration basins.

Nine (9) test pits have been conducted for the four (4) BMP's on site; however, only eight (8) pits will be used in design of the BMP's. SPP-4 and -5 were used in designing Basin 1, which has a basin bottom area of 180

SF. SPP-6 and -7 were used in designing Basin 2, which has a basin bottom area of 440 SF. SPP-2, -3, and -9 were used in designing Basin 3 which has an area of 8,430 SF. Lastly, the pervious pavement was designed using SPP-8 and -3, which has an area of 6,450 SF. See soil pit testing results in Section III of this report for further information.

Recharge: The Post-Development Annual Recharge Deficit has been calculated using the New Jersey Groundwater Recharge Spreadsheet. Per the NJGRS Spreadsheet and soils investigation, the site development does not result in a recharge deficit. Refer to the NJ Groundwater Recharge Spreadsheet in the Appendix of this report.

Water Quality: The stormwater management design for the project satisfies the requirements set forth in NJAC 7:8-5.5(a) by utilizing pervious paving and small-scale infiltration basins. The pervious pavement will provide the minimum TSS removal rate of 80%. Per NJDEP BMP Manual Chapter 9.6, pervious pavement provides an 80% TSS removal. The project meets the contributory drainage area requirement of 3:1 (drainage area to area of pervious pavement), with a maximum slope of less than 5%. For either of the above ground small-scale infiltration basins (#1 & #2), all impervious runoff comes from the roof, and is considered clean; therefore, these basins do not need additional water quality measures. Additionally, the stormwater will be evacuated from the infiltration basins within 72 hours. As a result, the water quality requirements of the Neptune Township Land Development Ordinance and NJAC 7:8 are met.

IX. STABILITY ANALYSIS

Per the NJ Soil Erosion Standards, Section 21, “Standard for Off-Site Stability,” compliance has been met for the site. The conditions of the NJ SESC Standards Section 21-1 have been satisfied using the point of discharge method with a well-defined channel.

- a. **Retain pre-developed runoff characteristics. Do not increase the rate of runoff from development.**
Discharge rates from the proposed stormwater improvements are above the flow rates in the existing conditions; therefore method ‘b’ below will be used.
- b. **Analyze the waterway or channel for stability under the planned rate of discharge using the Standard for Grassed Waterways or Standard for Channel Stabilization, as appropriate.** Peak flows from the 2- and 10-year storms shall be analyzed. The 2- and 10-year storms have existing peak flowrates of 0.22 cfs and 0.34 cfs, respectively. The proposed conditions, during the stability analysis, meets the requirements set forth in the Standard for Channel Stabilization by not exceeding the maximum flow rates for the existing 12” pipe at POA 1. A summary of the flow rates are shown in the chart below:

PRE VS. POST SUMMARY CHART (STUDY POINT – STABILITY)			
<u>Design Storm</u>	<u>Existing Runoff Rate (CFS)</u>	<u>Proposed Runoff Rate (CFS)</u>	<u>Proposed Pipe Velocity (fps)</u>
2 Year	0.22	0.68	2.95
10 Year	0.34	2.31	3.5

- c. **Modify the waterway or channel to a stable design condition.** The combined flows to the existing stormwater facilities show that the proposed flow rates are in a stable condition as the discharge rate is lower than the maximum allowable flow rate in the existing 12” pipe.

X. CONCLUSION

The proposed overall development has been designed with provisions for the safe and efficient control of stormwater runoff in a manner that will not adversely impact the existing drainage patterns, adjacent roadways, or adjacent parcels. The 80% TSS removal obligations set forth by NJAC 7:8 have been satisfied by utilizing three (3) infiltration basins and pervious pavement. Recharge requirements are met utilizing the above ground infiltration basins and underground infiltration basin. The diminimus runoff quantity exceptions requested do not adversely affect the surrounding stormwater collection systems. The exceptions involve runoff rates of less than 0.5cfs in the 100 year storm condition.

With this stated, it is evident that the proposed development will not have a negative impact on the existing drainage pattern, water quality, or groundwater recharge on site or within the vicinity of the subject parcel.

APPENDIX

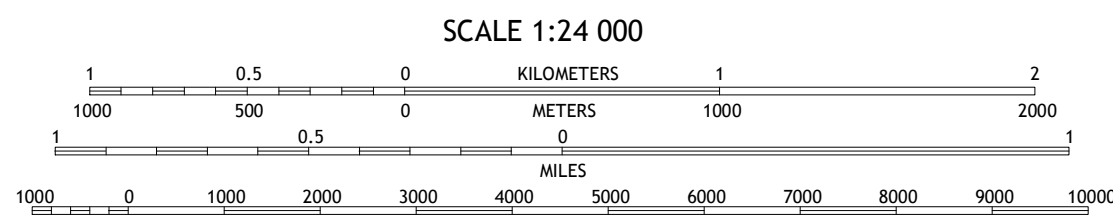
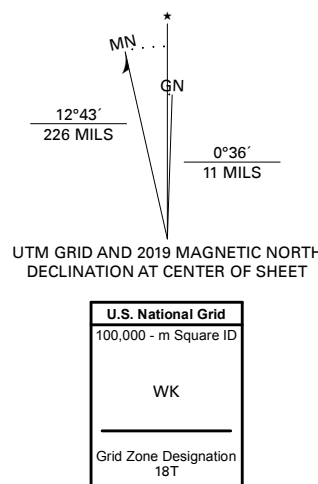
USGS MAP



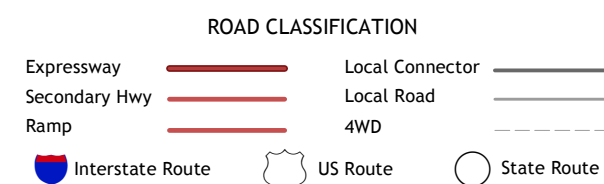
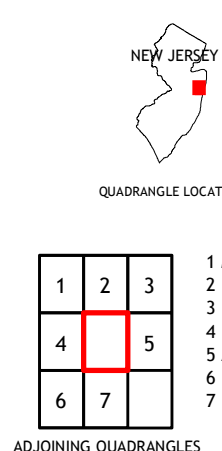
Produced by the United States Geological Survey

North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84). Projection and
1 000-meter grid/Universal Transverse Mercator, Zone 18T
This map is not a legal document. Boundaries may be
generalized for this map scale. Private lands within government
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Imagery.....NAIP, July 2015 - September 2015
Roads.....U.S. Census Bureau, 2016
Names.....GNIS, 1979 - 2019
Hydrography.....National Hydrography Dataset, 2002 - 2009
Contours.....National Elevation Dataset, 2012
Boundaries.....Multiple sources; see metadata file 2017 - 2018
Wetlands.....FWS National Wetlands Inventory 2007 - 2008



CONTOUR INTERVAL 20 FEET
NORTH AMERICAN VERTICAL DATUM OF 1988
This map was produced to conform with the
National Geospatial Program US Topo Product Standard, 2011.
A metadata file associated with this product is draft version 0.6.18



ASBURY PARK, NJ
2019



NRCS SOILS MAPS

Custom Soil Resource Report for **Monmouth County, New Jersey**



April 4, 2022

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies. State agencies including the Agricultural Experiment Stations, and local agencies. The National Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Monmouth County, New Jersey
Survey Area Data: Version 15, Aug 31, 2021
Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 25, 2020—Oct 15, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

MAP LEGEND

- Area of Interest (AOI)
- Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points
- Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spill Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features
- Streams and Canals
- Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads
- Aerial Photography

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
UR	Urban land	2.5	100.0%
Totals for Area of Interest		2.5	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Monmouth County, New Jersey

UR—Urban land

Map Unit Setting

National map unit symbol: 4j92
Elevation: 0 to 170 feet
Mean annual precipitation: 30 to 64 inches
Mean annual air temperature: 46 to 79 degrees F
Frost-free period: 131 to 178 days
Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 95 percent
Minor components: 5 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Setting
Parent material: Surface covered by pavement, concrete, buildings, and other structures underlain by disturbed and natural soil material

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 5 percent
Landform: Low hills
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E. T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service.

U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service. U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084>

Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service.
National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service.
2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service, 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

RUNOFF CURVE NUMBER (CN) CALCULATIONS- EXISTING



DYNAMIC ENGINEERING

EXISTING DRAINAGE AREA SUMMARY AND AVERAGE CURVE NUMBER(CN) CALCULATIONS

Project: Surfside Crossing Multi-Family Development
 Job #: 2241-99-002
 Location: Memorial Drive & 8th Ave, Neptune, Nj

Computed By: SMM
 Checked By: SRC
 Date: 6/14/2022

Drainage Area	Impervious Area (acre)	Impervious Area (sf)	Curve Number (CN) Used	HSG A - Open Space Area (acre)	HSG A - Open Space Area (sf)	Curve Number (CN) Used	HSG A - Wooded Area (acre)	HSG A - Wooded Area (sf)	Curve Number (CN) Used	Avg. Perv. Curve Number	Total Pervious Area (acres)	Total Area (acres)	TC (Min.)
DA 1 - 8TH AVE	0.04	1,631	98	0.04	1,655	39	0.01	463	30	37	0.05	0.09	10
DA 2 - LOW POINT	0.08	3,695	98	0.58	25,241	39	0.28	12,397	30	36	0.86	0.95	12
DA 3 - 9TH AVE	0.00	-	98	0.62	26,997	39	0.13	5,783	30	37	0.75	0.75	10
BYPASS - MEMORIAL	0.00	-	98	0.03	1,176	39	0.00	-	30	39	0.03	0.03	10
Total	0.12	5326.00		1.26	55069.00		0.43	18643.00			1.67	1.81	

Per County Soil Survey -	Soil Abbr	HSG	A	Soil	Fort mott
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Description	Runoff Curve Number (CN) (HSG A)	Runoff Curve Number (CN) (HSG B)	Runoff Curve Number (CN) (HSG C)	Runoff Curve Number (CN) (HSG D)
Impervious Surface	98	98	98	98
Open Space (lawn) (good)	39	61	74	80
Woods (good)	30	55	70	77

RUNOFF CURVE NUMBER (CN) CALCULATIONS- PROPOSED



DYNAMIC ENGINEERING

PROPOSED DRAINAGE AREA SUMMARY AND AVERAGE CURVE NUMBER(CN) CALCULATIONS

Project: Surfside Crossing Multi-Family Development
 Job #: 2241-99-002
 Location: Memorial Drive & 8th Ave, Neptune, NJ

Computed By: SMM
 Checked By: SRC
 Date: 6/14/2022
 Last Revised: 11/22/2022

Drainage Area	Impervious Area (acre)	Impervious Area (sf)	Curve Number (CN) Used	HSG A - Open Space Area (acre)	HSG A - Open Space Area (sf)	Curve Number (CN) Used	HSG A - Wooded Area (acre)	HSG A - Wooded Area (sf)	Curve Number (CN) Used	Avg. Perv. Curve Number	Total Pervious Area (acres)	Total Area (acres)	TC (Min.)
DA 1 - BASIN 1	0.09	3,981	98	0.04	1,791	39	0.00	-	30	39	0.04	0.13	6
DA 2 - BASIN 2	0.50	21,700	98	0.06	2,778	39	0.00	-	30	39	0.06	0.56	6
DA 3 - BASIN 3	0.76	33,163	98	0.11	4,964	39	0.00	-	30	39	0.11	0.88	6
BYPASS MEMORIAL	0.00	103	98	0.07	2,926	39	0.00	-	30	39	0.07	0.07	6
BYPASS 8TH AVE	0.04	1,929	98	0.08	3,520	39	0.00	-	30	39	0.08	0.13	6
BYPASS 8TH AVE	0.03	1,294	98	0.02	710	39	0.00	-	30	39	0.02	0.05	6
Total	1.43	62,170		0.38	16,689.00		0.00	0.00			0.29	1.81	

Per County Soil Survey -	Soil Abbr	HSG	A	Soil	Fort Mott
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Description	Runoff Curve Number (CN) (HSG A)	Runoff Curve Number (CN) (HSG B)	Runoff Curve Number (CN) (HSG C)	Runoff Curve Number (CN) (HSG D)
Impervious Surface	98	98	98	98
Open Space (lawn) (good)	39	61	74	80
Woods (good)	30	55	70	77

**EXISTING TIME OF CONCENTRATION (T_c)
CALCULATIONS**



1904 Main Street, Lake Como, NJ 07719
(732) 974-0198

Date: 6/14/2022
Project: SURFSIDE CROSSING
Project No: 2241-99-002

Calculated By: SMM
Checked By: SRC

Worksheet 3: Time of Concentration (T_c) Calculations

Land Condition: Existing
Drainage Area: DA - 2

• Sheet Flow :

1. Surface Description
2. Manning's Roughness Coefficient, n
3. Flow Length, L { total $L \leq 100$ ft }
4. Two-Year 24-hour Rainfall, p_2 for ... Monmouth County
5. Land Slope, s (ft/ft)
6. Travel Time, $T_t = \frac{0.007 (n L)^{0.8}}{p_2^{0.5} s^{0.4}}$

AB				
Range (natural)				
0.13				
100.0 ft				
3.38 in	3.38 in		3.38 in	
0.020 ft/ft				
0.142 hr	+	0.000 hr	+	0.000 hr
				=
				0.142 hr

• Shallow Concentrated Flow :

7. Surface Description
8. Flow Length, L
9. Watercourse Slope, s
10. Average velocity, V { see Figure 3.1 }
11. Travel Time, $T_t = \frac{L}{3600 V}$

BC				
Unpaved				
290.0 ft				
0.013 ft/ft				
1.83 ft/s				
0.044 hr	+	0.000 hr	+	0.000 hr
				=
				0.044 hr

• Channel Flow :

12. Pipe Diameter, D
13. Cross-Sectional Flow Area, A
14. Wetted Perimeter, p_w
15. Hydraulic Radius, $r = A / p_w$
16. Channel Slope, s
17. Pipe Material
18. Manning's Roughness Coefficient, n
19. Velocity, $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
20. Flow Length, L
21. Travel Time, $T_t = \frac{L}{3600 V}$
22. Watershed or subarea Time of Concentration, T_c { add T_t in steps 6, 11 and 21 }

0.000 hr	+	0.000 hr	+	0.000 hr
				=
				0.000 hr
				0.186 hr
				11.1 min



1904 Main Street, Lake Como, NJ 07719
(732) 974-0198

Date: 6/14/2022
Project: SURFSIDE CROSSING
Project No: 2241-99-002

Calculated By: SMM
Checked By: SRC

Worksheet 3: Time of Concentration (T_c) Calculations

Land Condition: Existing
Drainage Area: DA - 3

• Sheet Flow :

1. Surface Description
2. Manning's Roughness Coefficient, n
3. Flow Length, L { total $L \leq 100$ ft }
4. Two-Year 24-hour Rainfall, p_2 for . . . Monmouth County
5. Land Slope, s (ft/ft)
6. Travel Time, $T_t = \frac{0.007 (n L)^{0.8}}{p_2^{0.5} s^{0.4}}$

AB				
Range (natural)				
0.13				
100.0 ft				
3.38 in	3.38 in		3.38 in	
0.025 ft/ft				
0.130 hr	+	0.000 hr	+	0.000 hr
				=
				0.130 hr

• Shallow Concentrated Flow :

7. Surface Description
8. Flow Length, L
9. Watercourse Slope, s
10. Average velocity, V { see Figure 3.1 }
11. Travel Time, $T_t = \frac{L}{3600 V}$

BC				
Unpaved				
233.0 ft				
0.009 ft/ft				
1.56 ft/s				
0.041 hr	+	0.000 hr	+	0.000 hr
				=
				0.041 hr

• Channel Flow :

12. Pipe Diameter, D
13. Cross-Sectional Flow Area, A
14. Wetted Perimeter, p_w
15. Hydraulic Radius, $r = A / p_w$
16. Channel Slope, s
17. Pipe Material
18. Manning's Roughness Coefficient, n
19. Velocity, $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$
20. Flow Length, L
21. Travel Time, $T_t = \frac{L}{3600 V}$
22. Watershed or subarea Time of Concentration, T_c { add T_t in steps 6, 11 and 21 }

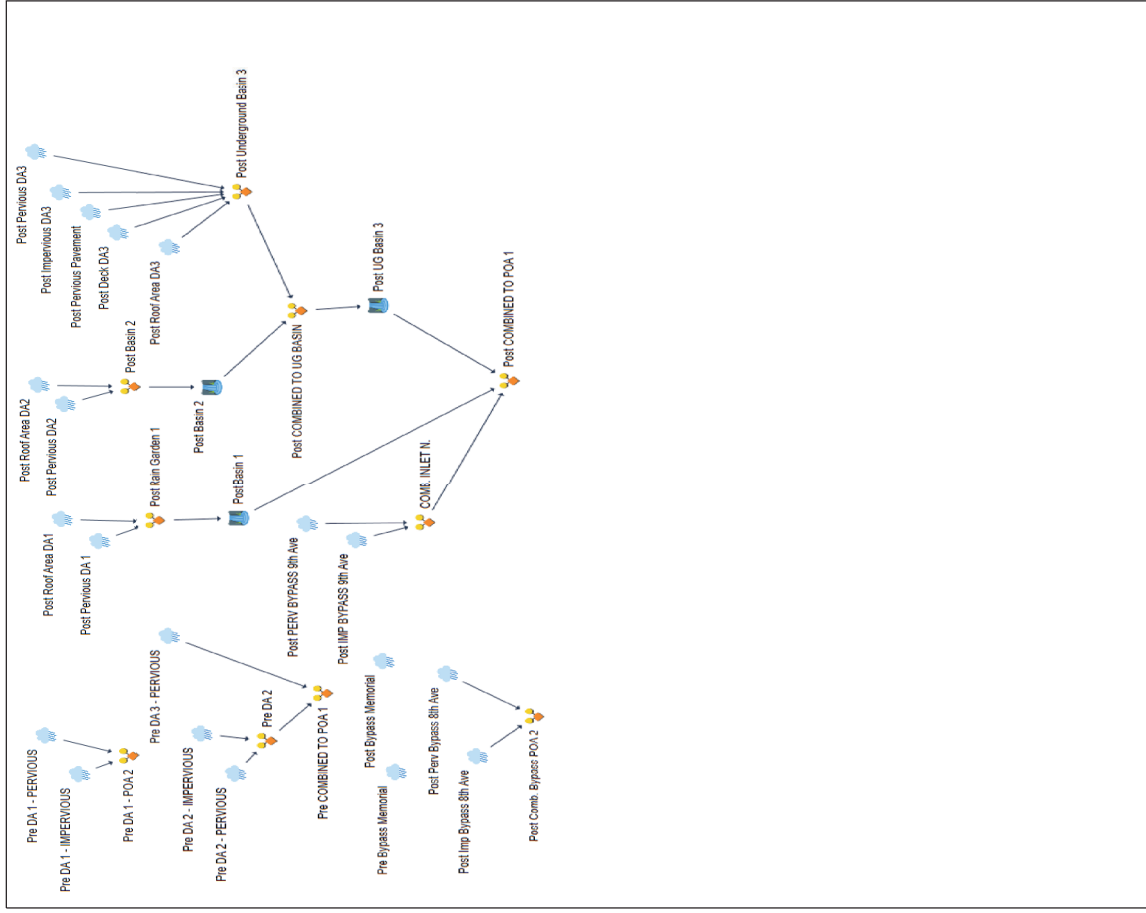
0.000 hr	+	0.000 hr	+	0.000 hr
				=
				0.000 hr
				0.171 hr
				10.3 min

**HYDROGRAPH SUMMARY REPORTS –
EXISTING & PROPOSED
2 YR., 10 YR., 25 YR. & 100 YR.**

Basin Model

Hydrology Studio v 3.0.0.26

Project Name:
11-22-2022



Hydrograph 2-yr Summary

Hydrology Studio v 3.0.0.26

Project Name:
11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.120	12.13	474	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.000	24.00	0.095	----	----	----
3	NRCS Runoff	Post Roof Area DA3	1.132	12.10	3.891	----	----	----
4	NRCS Runoff	Post Deck DA3	0.608	12.17	2.690	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.414	12.17	1.834	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.000	24.00	1.18	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.291	12.10	1.000	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.000	24.00	1.76	----	----	----
9	NRCS Runoff	Post Roof Area DA2	1.617	12.10	5.558	----	----	----
10	Junction	Pre DA 1 - POA 2	0.120	12.13	474	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.000	24.00	3.55	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.193	12.17	856	----	----	----
13	Junction	Post Underground Basin 3	2.217	12.13	9.274	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.291	12.10	1.002	6, 7	----	----
15	Junction	Post Basin 2	1.617	12.10	5.560	8, 9	----	----
16	Pond Route	Post Basin 2	1.046	12.17	1.616	15	14.22	851
17	Pond Route	Post Basin 1	0.000	13.90	0.000	14	14.09	346
18	Junction	Post COMBINED TO UG BASIN 3	3.244	12.13	10.890	13, 16	----	----
19	Pond Route	Post UG Basin 3	0.000	12.23	0.000	18	12.43	2,540
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.221	12.17	978	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.000	0.00	0.000	----	----	----
22	Junction	Pre DA 2	0.221	12.17	978	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.000	24.00	1.47	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.221	12.17	980	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.097	12.10	333	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.000	24.00	2.35	----	----	----
27	Junction	COMB. INLET N	0.097	12.10	336	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	0.097	12.10	336	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.000	24.00	0.969	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.000	24.00	2.26	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.129	12.10	445	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.000	24.00	0.588	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.129	12.10	445	31, 32	----	----

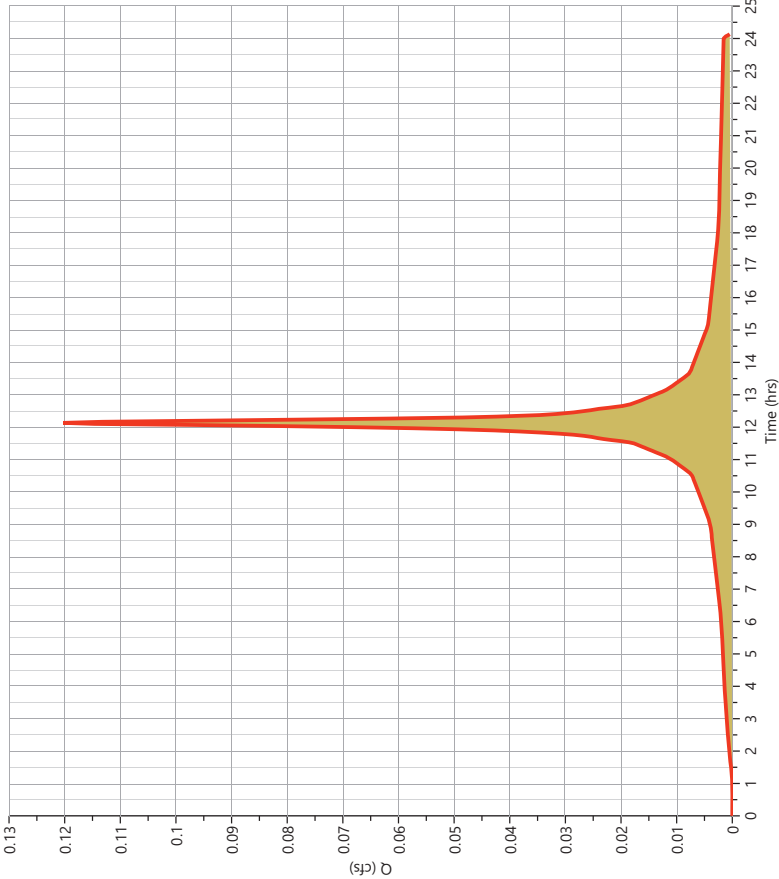
Hydrograph Report

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.120 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 474 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.12 cfs



Hydrograph Report

Pre DA 1 - PERVIOUS

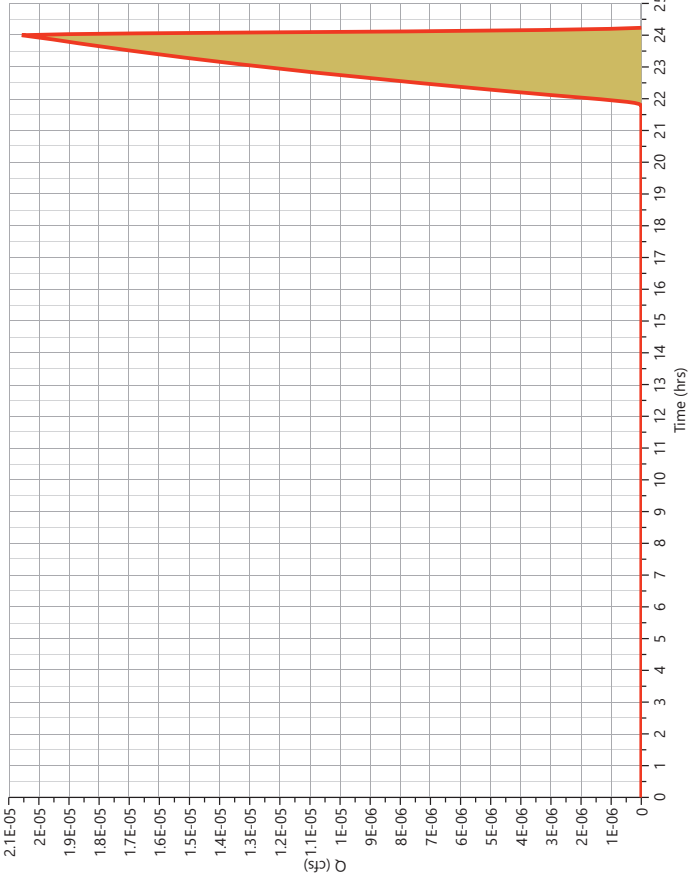
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.095 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.04	39	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.00 cfs



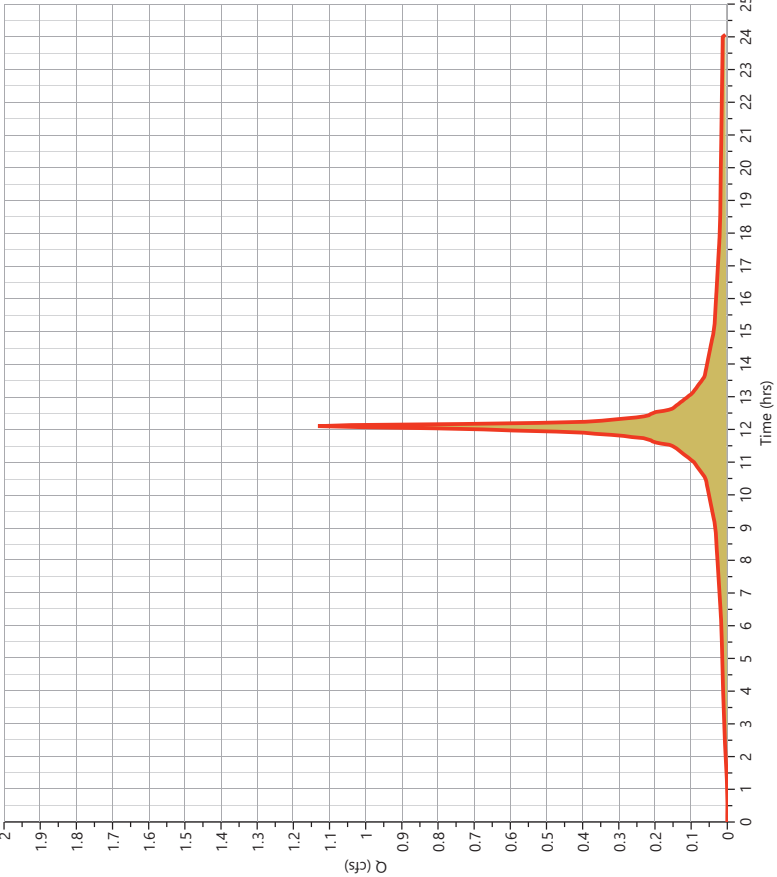
Hydrograph Report

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.132 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 3.891 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.13 cfs



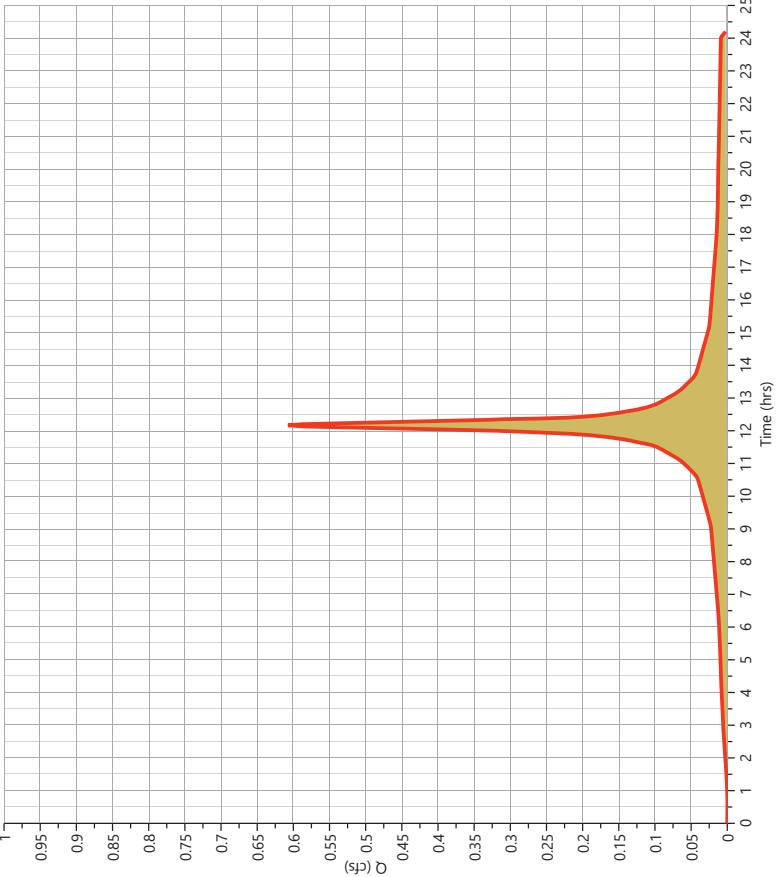
Hydrograph Report

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.608 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2.690 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.61 cfs



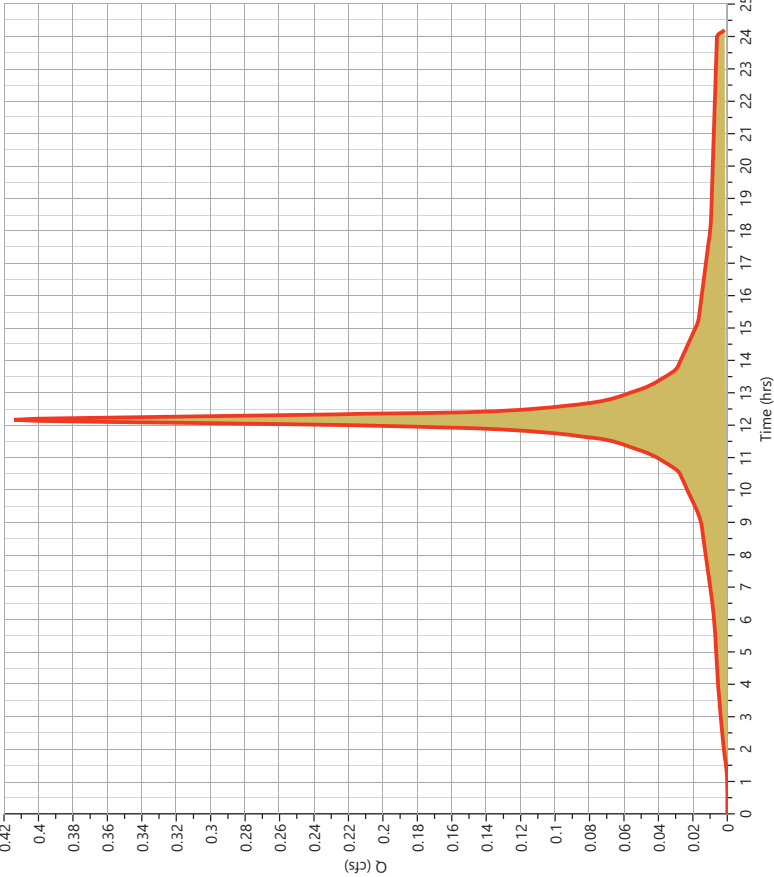
Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.414 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,834 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.41 cfs



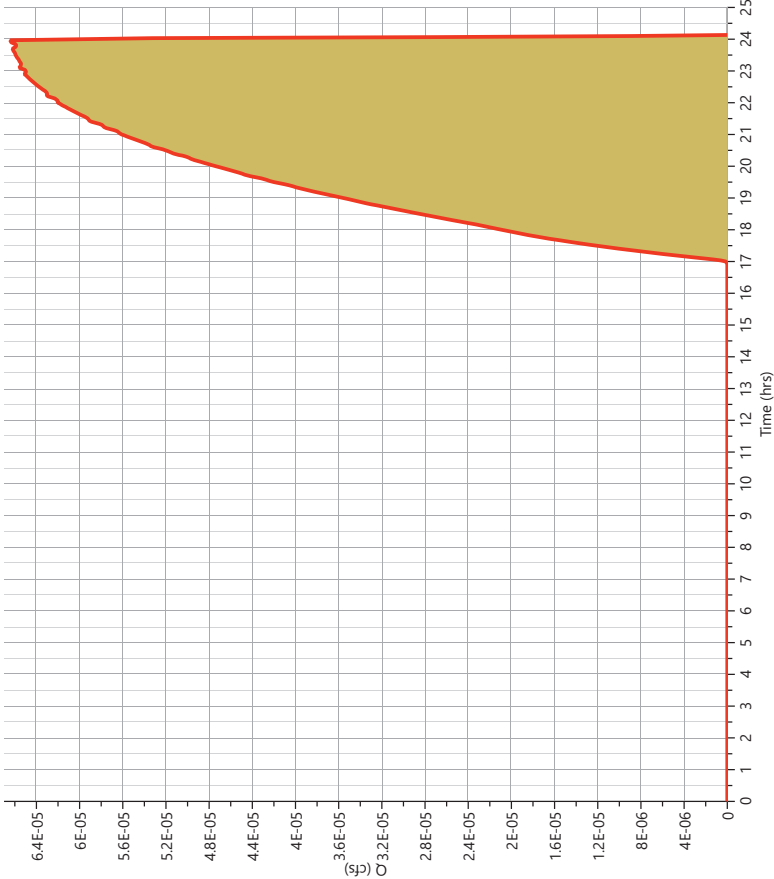
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.18 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



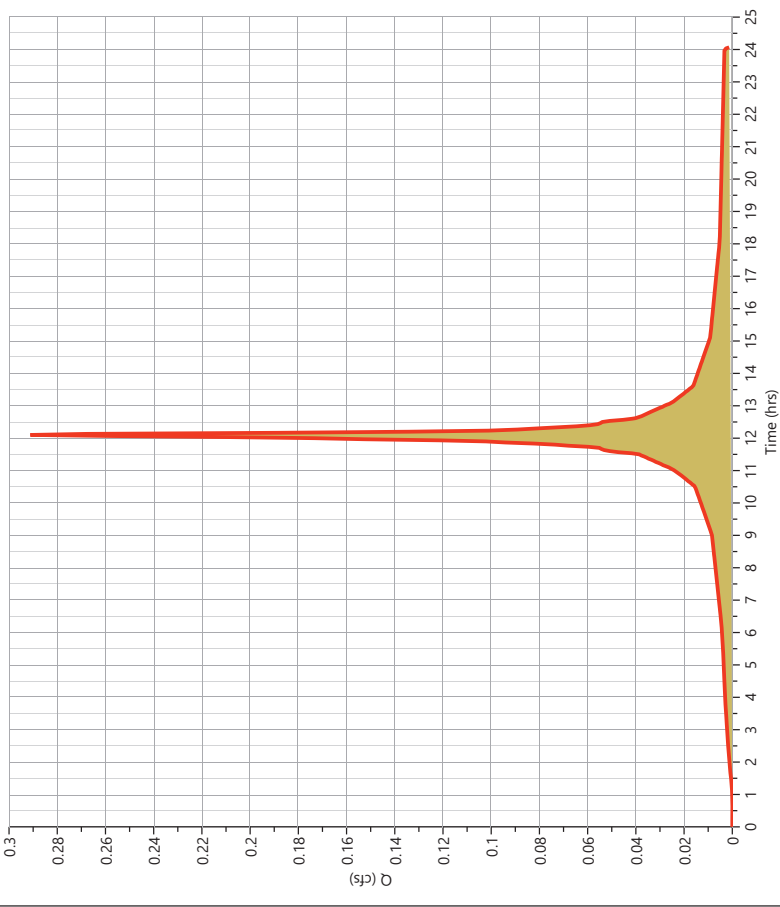
Hydrograph Report

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.291 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,000 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.29 cfs



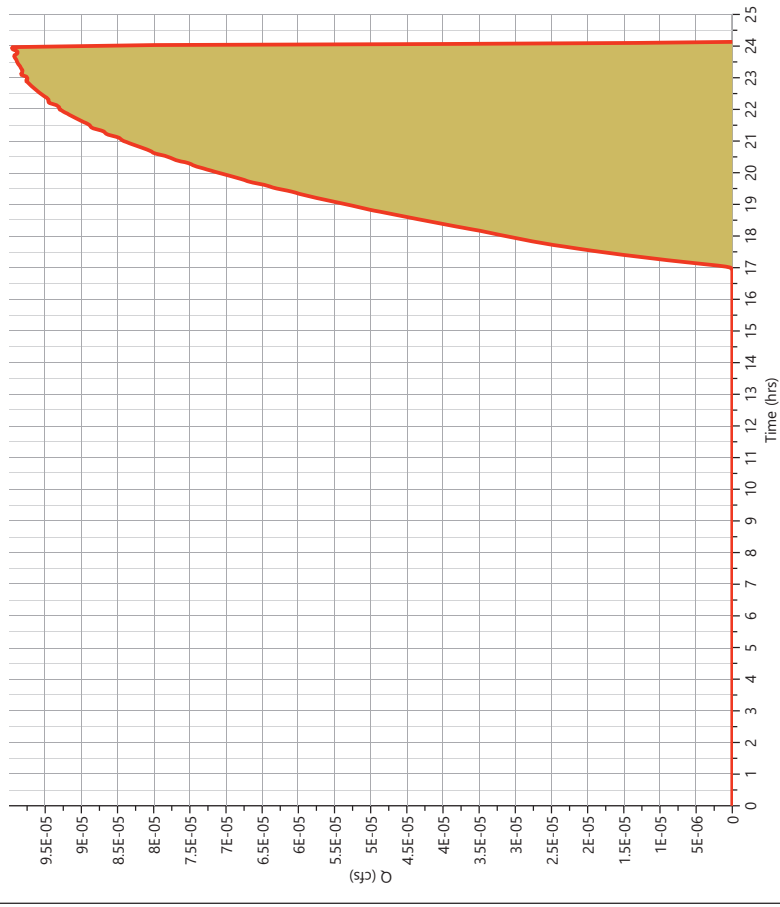
Hydrograph Report

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.76 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

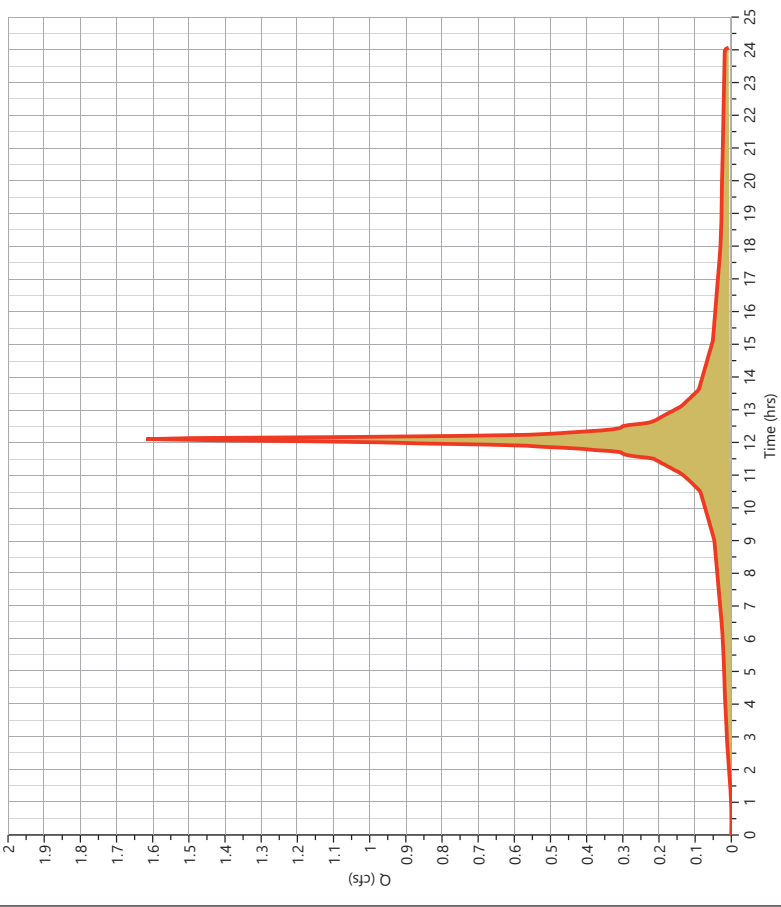
11-22-2022

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.617 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 5,558 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.62 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

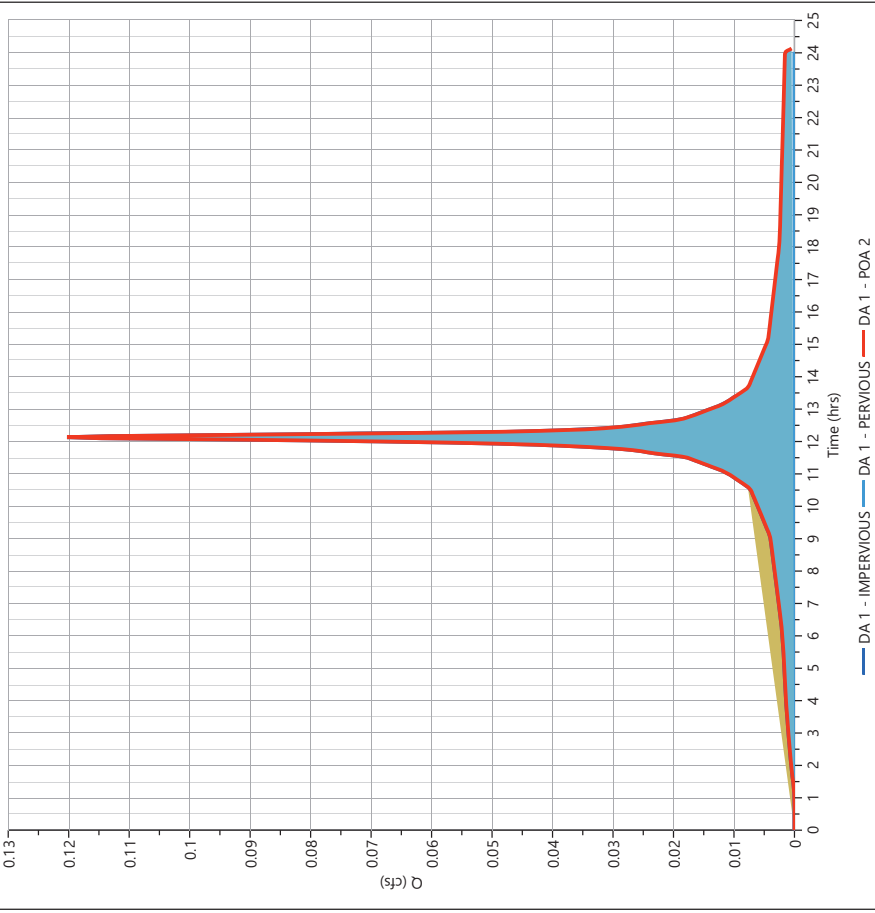
11-22-2022

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.120 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 474 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.12 cfs



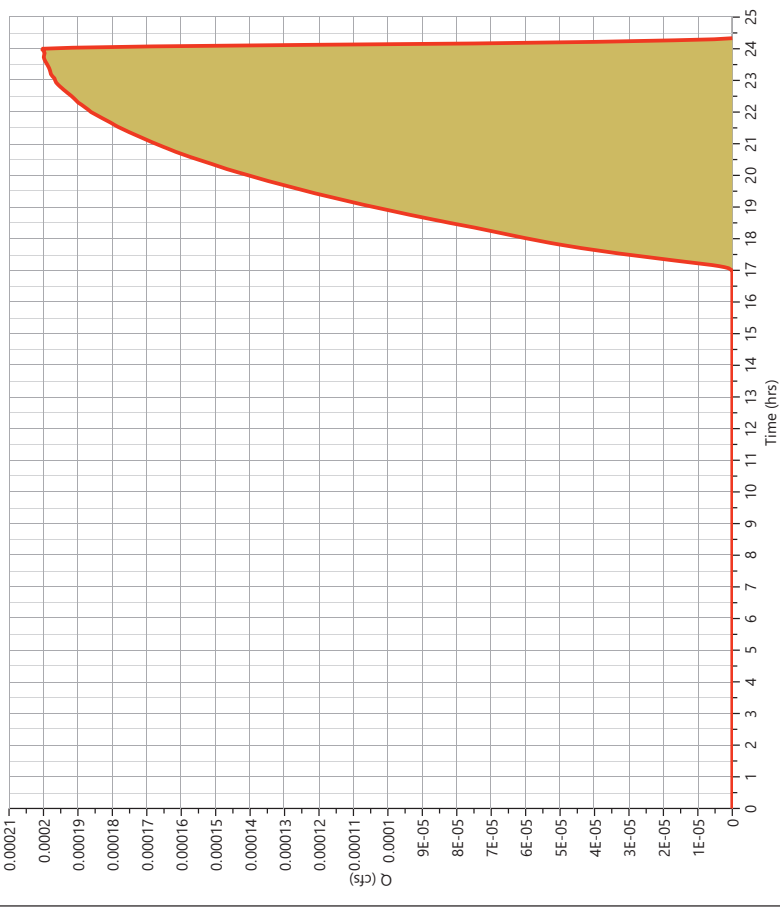
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 3.55 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



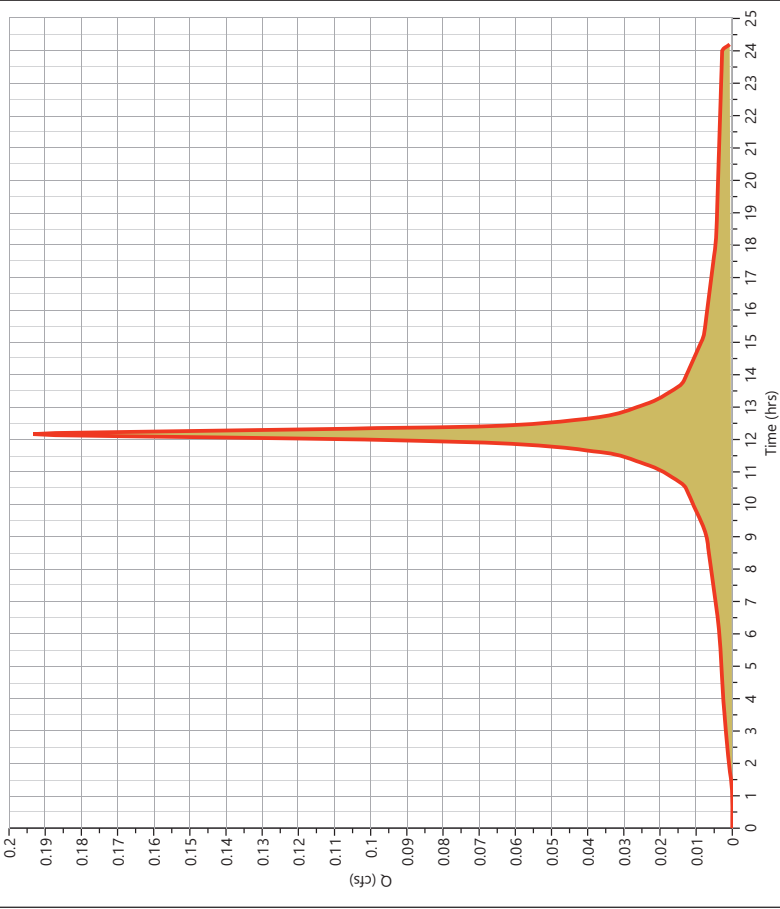
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.193 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 856 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.19 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

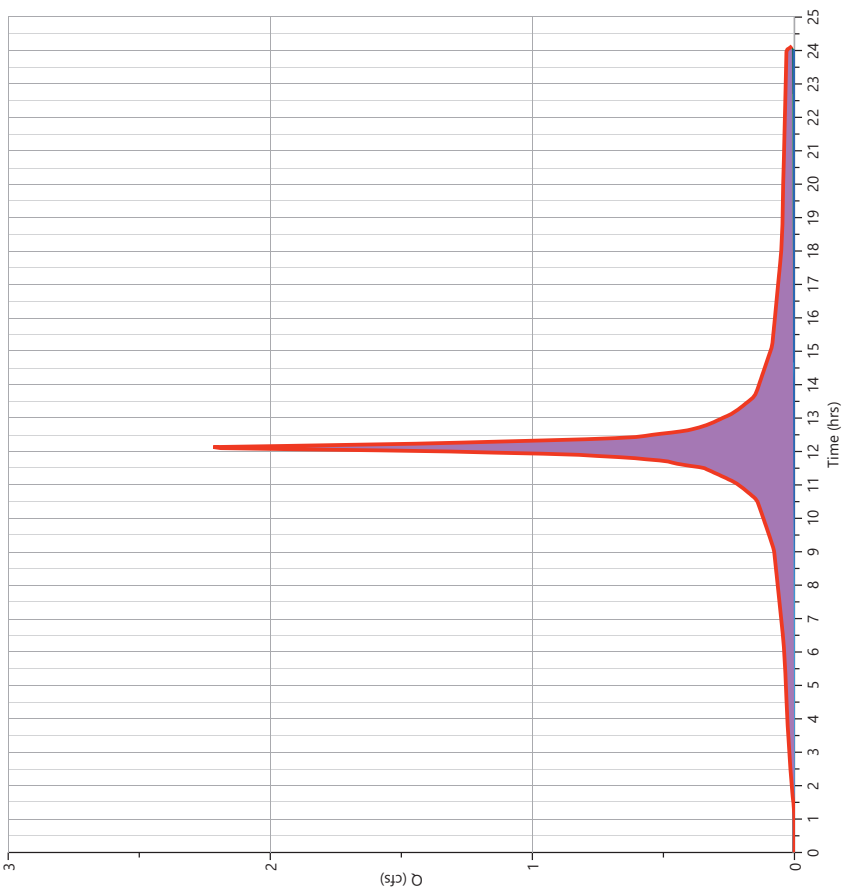
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 2.217 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 9,274 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 2.22 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

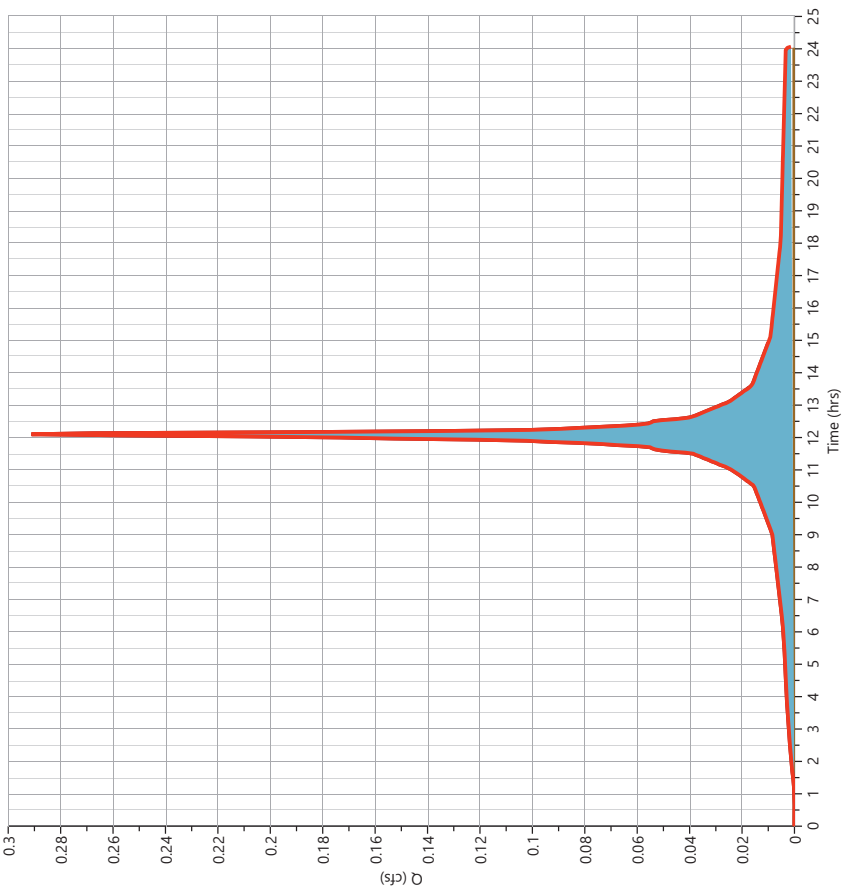
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.291 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,002 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.29 cfs



Hydrograph Report

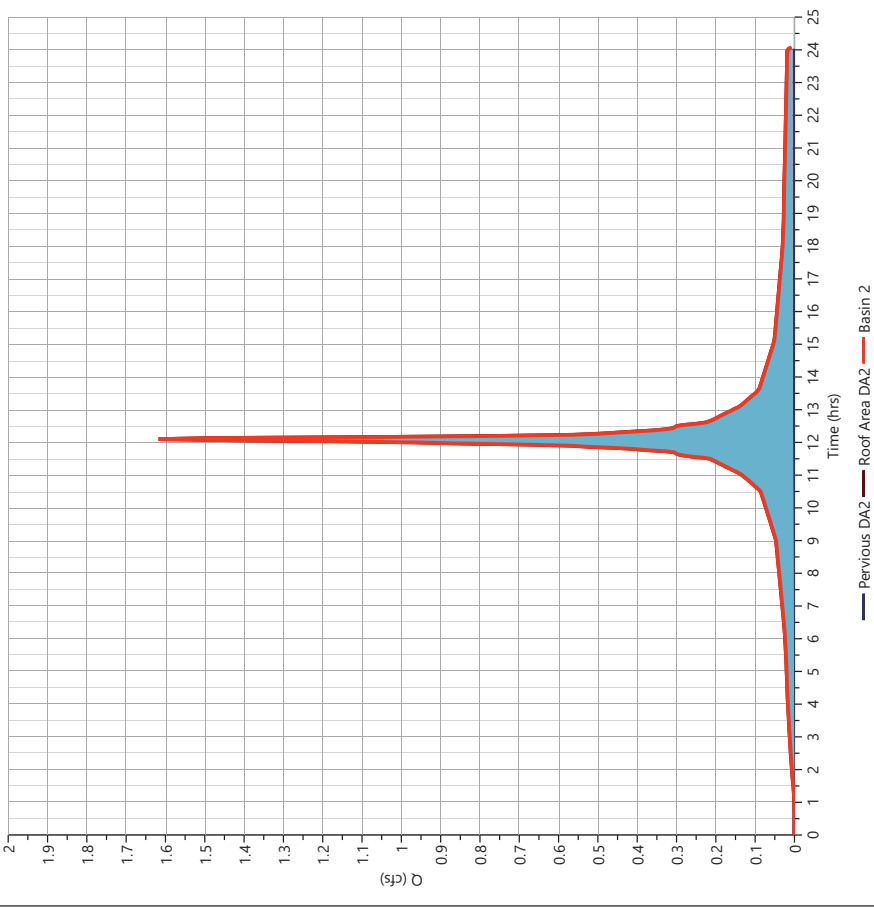
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 1.617 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 5,560 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 1.62 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

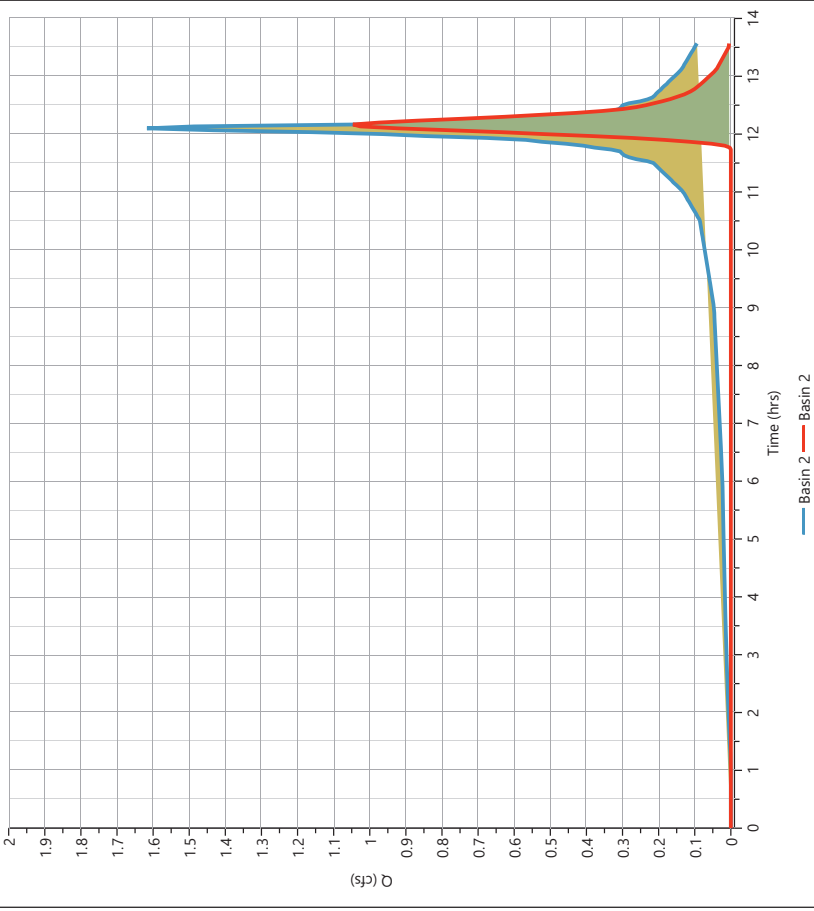
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 1.046 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,616 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.22 ft
Pond Name	= Basin 2	Max. Storage	= 851 cuft

Pond Routing by Storage Indication Method

Qp = 1.05 cfs



Pond Report

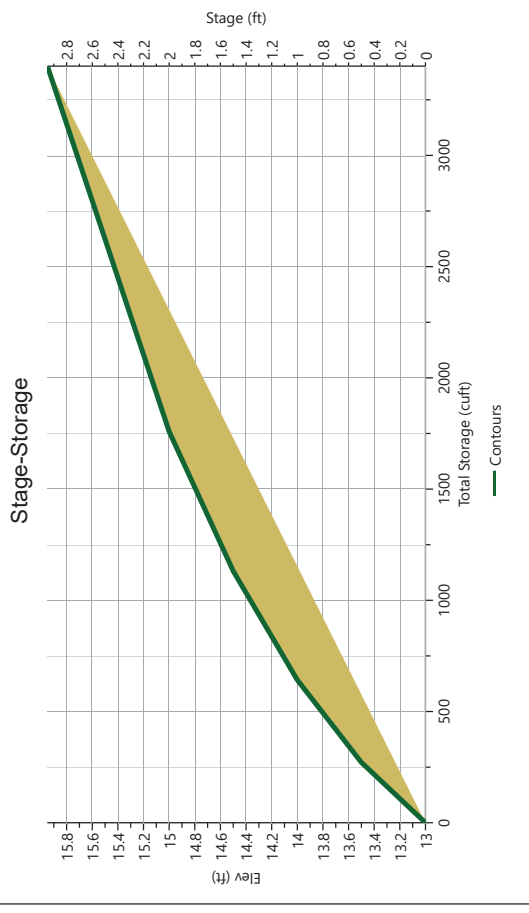
Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Storage

[illegible]

Pond Report

Project Name:

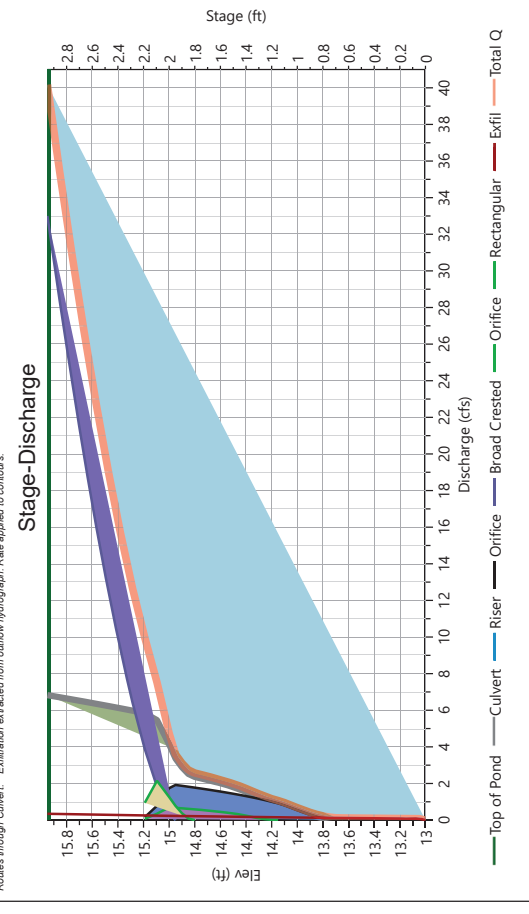
Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2*	3	
Rise, in	12	6	4		Orifice Dia, in
Span, in	12	6	4		No. Orifices
No. Barrels	1	2	2		Invert Elevation, ft
Invert Elevation, ft	12.19	13.67	14.15		Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60		Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
Shape / Type	Box	1	2*	3	
Crest Elevation, ft	14.95	Broad Crested	Rectangular		Exfiltration, in/hr
Crest Length, ft	12	14.95	14.82		7.20**
Angle, deg		10	5		
Weir Coefficient, Cw	3.3	3.3	3.3		



Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000
0.50	13.50	271	0.000 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.107		0.107
1.00	14.00	642	0.540 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.140		0.680
1.50	14.50	1,132	1.800 oc	1.440	0.360	0.000	0.000	0.000	0.000	0.000	0.000	0.186		1.986
2.00	15.00	1,759	4,141 oc	1.744	0.695	0.000	0.443	0.369	1.260	0.000	0.232	0.232		4.742
2.95	15.95	3,401	6,826 oc	0.000	0.000	0.000	0.000	33.00	0.000	0.000	0.344	0.344		40.17

Suffix: ey, ic = inlet control, oc = outlet control, s = submerged weir

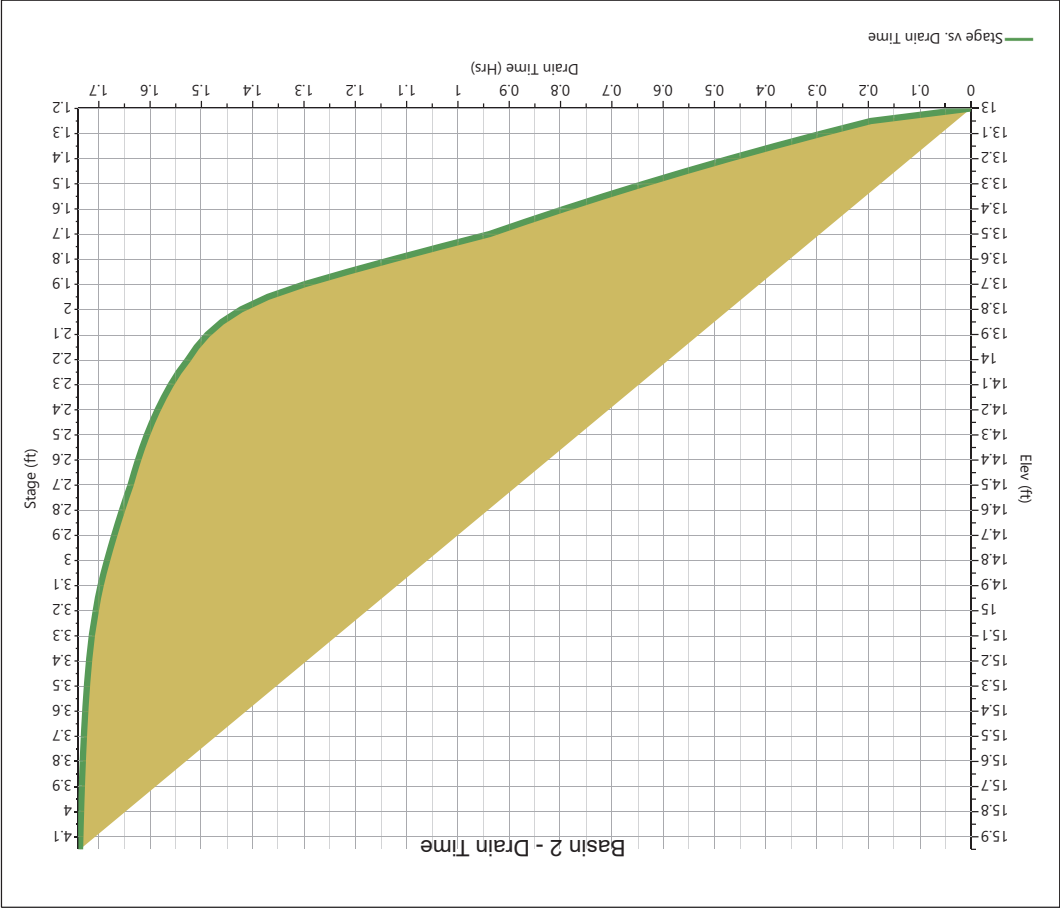
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Pond Drawdown



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

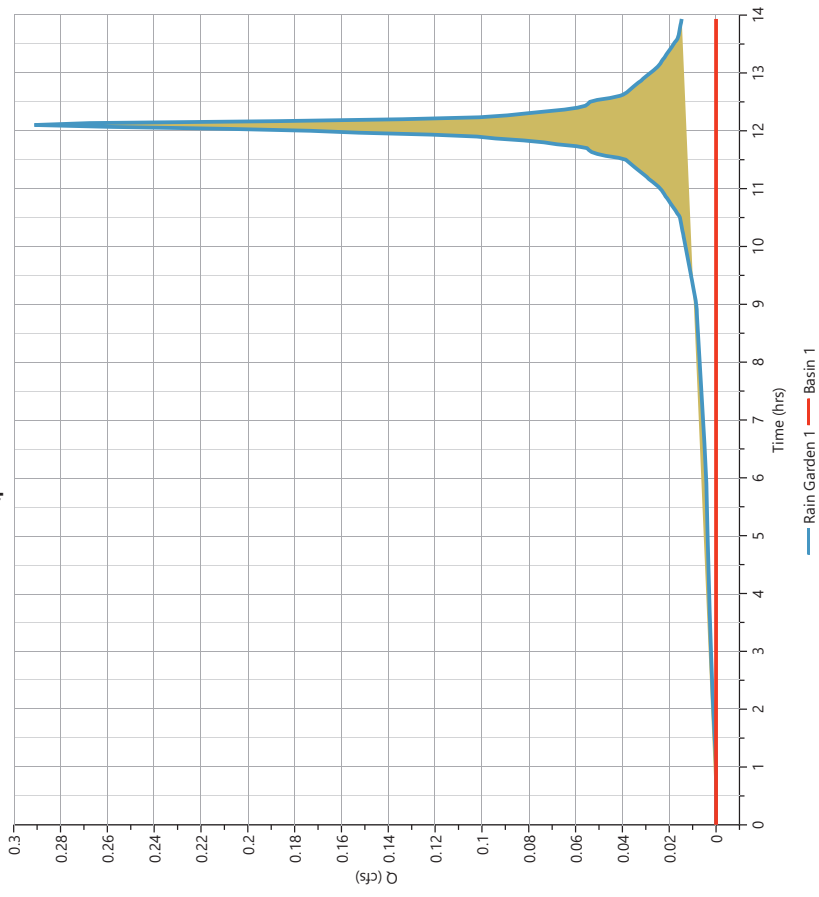
Post Basin 1

Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 13.90 hrs
Time Interval	= 2 min	Hydrograph Volume	= 0.000 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.09 ft
Pond Name	= BASIN 1	Max. Storage	= 346 cuft

Pond Routing by Storage Indication Method

Qp = 0.00 cfs



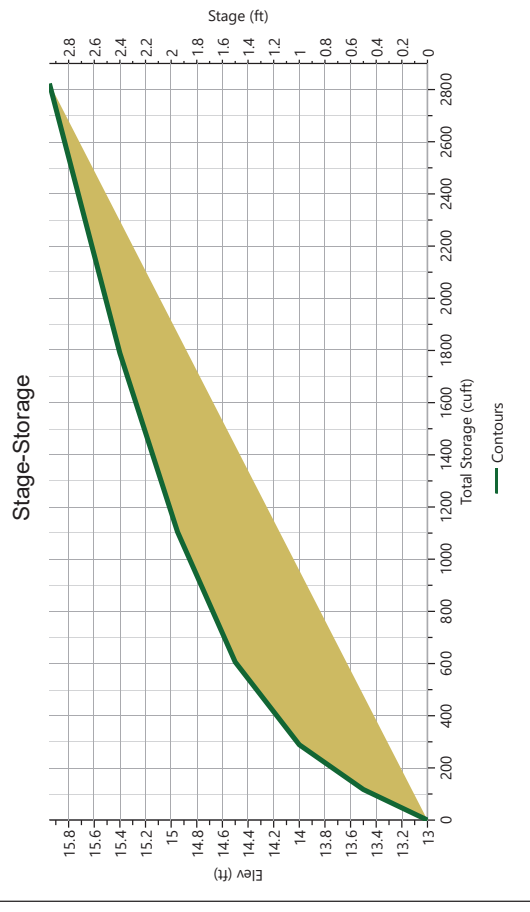
Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

BASIN 1

Stage-Storage

[illegible]

Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

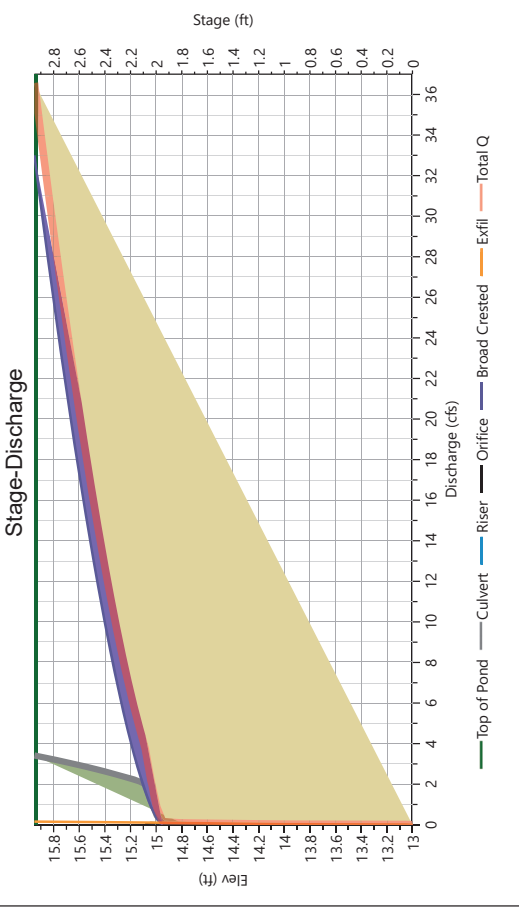
11-22-2022

BASIN 1

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2	3	
Rise, in	12	3			Orifice Dia, in
Span, in	12	3			No. Orifices
No. Barrels	1	1			Invert Elevation, ft
Invert Elevation, ft	13.90	14.77			Height, ft
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co
Length, ft	100				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
		1	2	3	
Shape / Type	Box	Broad Crested			Exfiltration, in/hr
Crest Elevation, ft	14.95	14.95			3.25**
Crest Length, ft	12	10			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to contours.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

BASIN 1

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000			0.000	0.000				0.000		0.000
0.50	13.50	117	0.000	0.000			0.000	0.000				0.022		0.022
1.00	14.00	289	0.000	0.000			0.000	0.000				0.030		0.030
1.50	14.50	606	0.000	0.000			0.000	0.000				0.066		0.066
1.95	14.95	1,104	0.055 oc	0.055			0.000	0.000				0.101		0.156
2.40	15.40	1,790	2.629 oc	0.000			0.000	9.962				0.128		12.72
2.95	15.95	2,823	3.415 oc	0.000			0.000	33.00				0.155		36.57

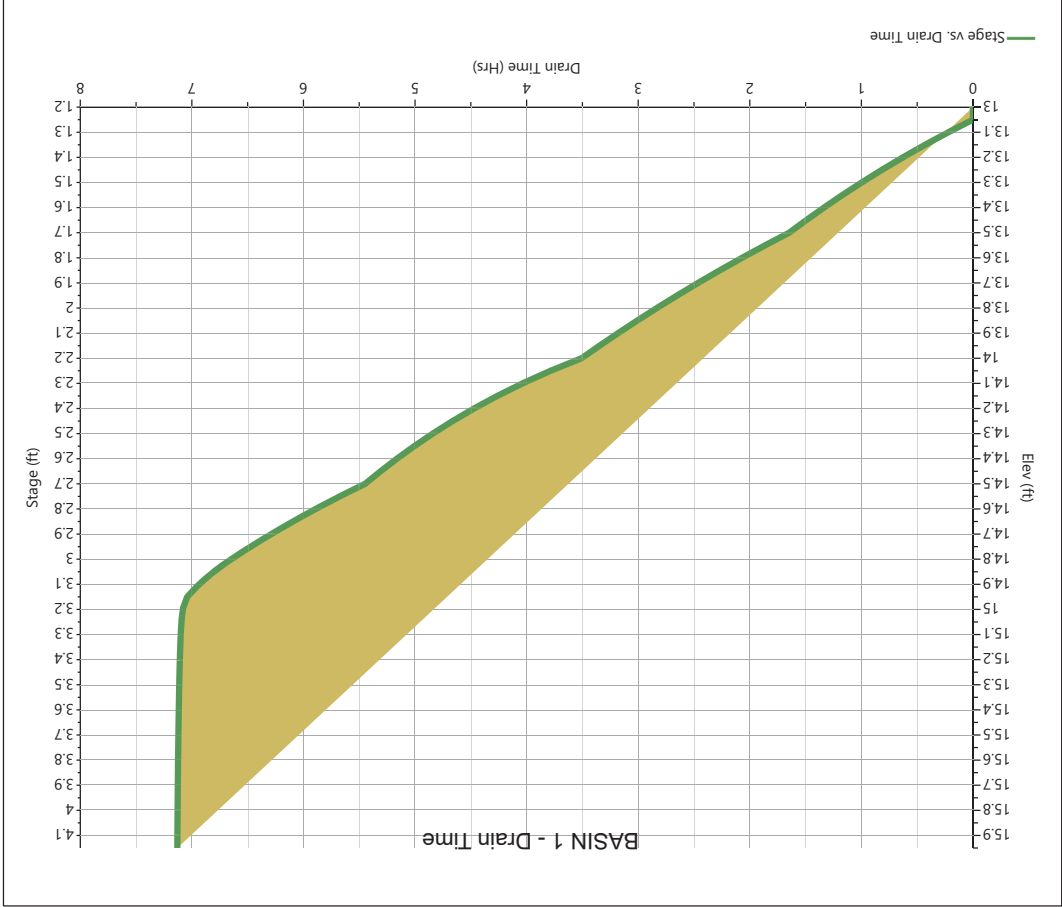
Suffix key: /c = inlet control, oc = outlet control, s = submerged weir

Pond Report

Project Name:
Hydrology Studio v 3.0.0.26

BASIN 1

Pond Drawdown

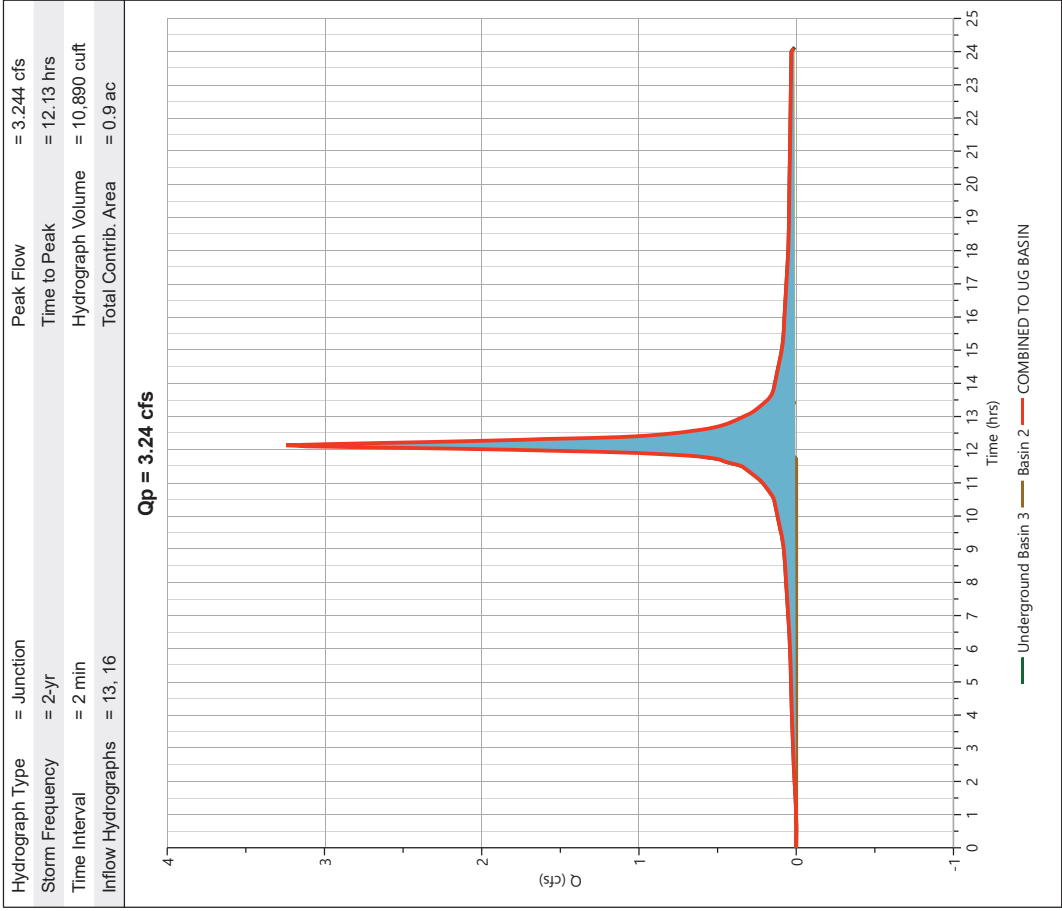


Hydrograph Report

Project Name:
Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18



Hydrograph Report

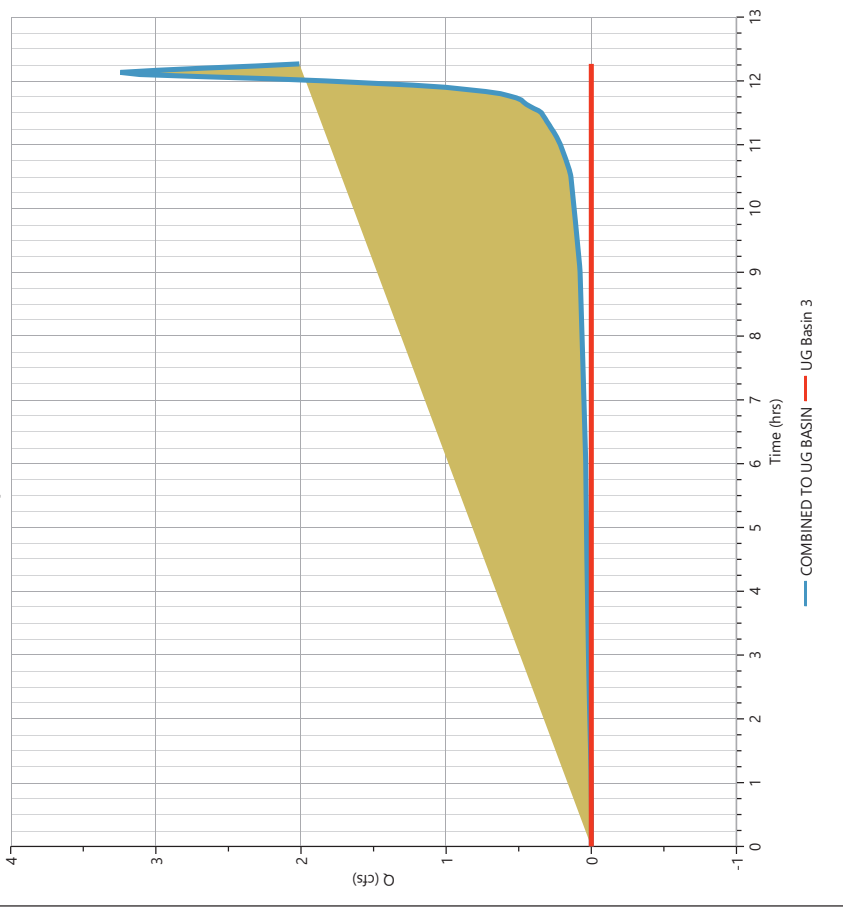
Post UG Basin 3

Hyd. No. 19

Hydrograph Type	= Pond Route	
Storm Frequency	= 2-yr	Peak Flow
Time Interval	= 2 min	Time to Peak
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Hydrograph Volume
Pond Name	= UG BASIN	Max. Elevation
		Max. Storage

Pond Routing by Storage Indication Method

Qp = 0.00 cfs



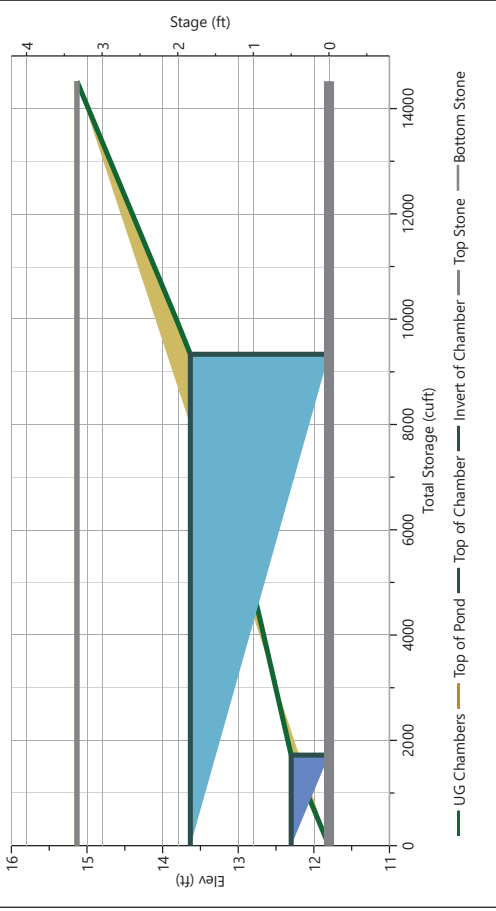
Pond Report

UG BASIN

Stage-Storage

StormTech® SC-310™ Chamber			Stage / Storage Table			
	Description	Input	Stage (in)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)
	Chamber Height, in	16	0.0	11.80	8,601	0.000
	Chamber Shape	Arch	2.0	11.97	8,601	573
	Chamber Width, in	34	4.0	12.13	8,601	573
	Installed Length, ft	7.12	6.0	12.30	8,601	573
	No. Chambers	346	10.0	12.63	8,601	1,043
	Bare Chamber Stor, cuft	5,086	12.0	12.80	8,601	1,032
	No. Rows	20	14.0	12.97	8,601	1,008
	Space Between Rows, in	6	16.0	13.13	8,601	973
	Stone Above, in	18	18.0	13.30	8,601	924
	Stone Below, in	6	20.0	13.47	8,601	854
	Stone Sides, in	12	22.0	13.63	8,601	732
	Stone Ends, in	12	24.0	13.80	8,601	599
	Encasement Voids, %	40.00	26.0	13.97	8,601	573
	Encasement Bottom Elevation, ft	11.80	28.0	14.13	8,601	573
			30.0	14.30	8,601	573
			32.0	14.47	8,601	573
			34.0	14.63	8,601	573
			36.0	14.80	8,601	573
			38.0	14.97	8,601	573
			40.0	15.13	8,601	573

Stage-Storage



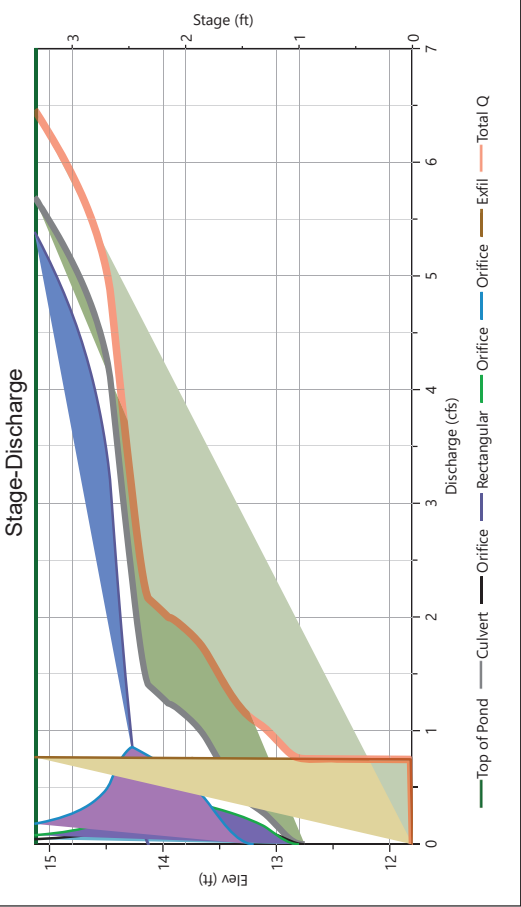
Pond Report

UG BASIN

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Perforated Riser
		1*	2*	3*	
Rise, in	12	3	4	6	Hole Diameter, in
Span, in	12	3	4	6	No. holes
No. Barrels	1	1	1	1	Invert Elevation, ft
Invert Elevation, ft	12.20	12.75	12.80	13.20	Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60	0.60	Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs		Ancillary	
		1*	2		
Shape / Type		Rectangular			3.75**
Crest Elevation, ft		14.13			
Crest Length, ft		5			
Angle, deg					
Weir Coefficient, Cw		3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to corrours.



Pond Report

UG BASIN

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	11.80	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000		0.000
0.17	11.97	573	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.748		0.748
0.33	12.13	1,147	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.749		0.749
0.50	12.30	1,720	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.750		0.750
0.67	12.47	2,763	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.751		0.751
0.83	12.63	3,811	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.752		0.752
1.00	12.80	4,844	0.008 oc	0.006	0.000	0.000	0.000	0.000	0.000	0.000		0.753		0.758
1.17	12.97	5,852	0.134 oc	0.072	0.062	0.000	0.000	0.000	0.000	0.000		0.754		0.887
1.33	13.13	6,825	0.292 oc	0.120	0.172	0.000	0.000	0.000	0.000	0.000		0.755		1.046
1.50	13.30	7,748	0.428 oc	0.154	0.243	0.031	0.000	0.000	0.000	0.000		0.756		1.184
1.67	13.47	8,602	0.669 oc	0.182	0.297	0.190	0.000	0.000	0.000	0.000		0.757		1.425
1.83	13.63	9,334	0.952 oc	0.203	0.343	0.406	0.000	0.000	0.000	0.000		0.758		1.710
2.00	13.80	9,933	1.135 oc	0.207	0.368	0.559	0.000	0.000	0.000	0.000		0.759		1.893
2.17	13.97	10,506	1.252 oc	0.206	0.366	0.679	0.000	0.000	0.000	0.000		0.760		2.012
2.33	14.13	11,079	1.408 oc	0.224	0.399	0.781	0.000	0.000	0.000	0.000		0.761		2.169
2.50	14.30	11,653	2.575 oc	0.209	0.372	0.837	0.000	0.000	0.000	0.000		0.762		3.337
2.67	14.47	12,226	4.116 oc	0.134	0.238	0.535	0.000	0.000	0.000	0.000		0.763		4.879
2.83	14.63	12,800	4.747 oc	0.090	0.160	0.360	0.000	0.000	0.000	0.000		0.764		5.510
3.00	14.80	13,373	5.125 oc	0.068	0.121	0.271	0.000	0.000	0.000	0.000		0.765		5.889
3.17	14.97	13,946	5.430 oc	0.054	0.096	0.217	0.000	0.000	0.000	0.000		0.766		6.195
3.33	15.13	14,520	5.693 oc	0.045	0.080	0.180	0.000	0.000	0.000	0.000		0.767		6.460

Suffix key: /c = inlet control, oc = outlet control, s = submerged weir

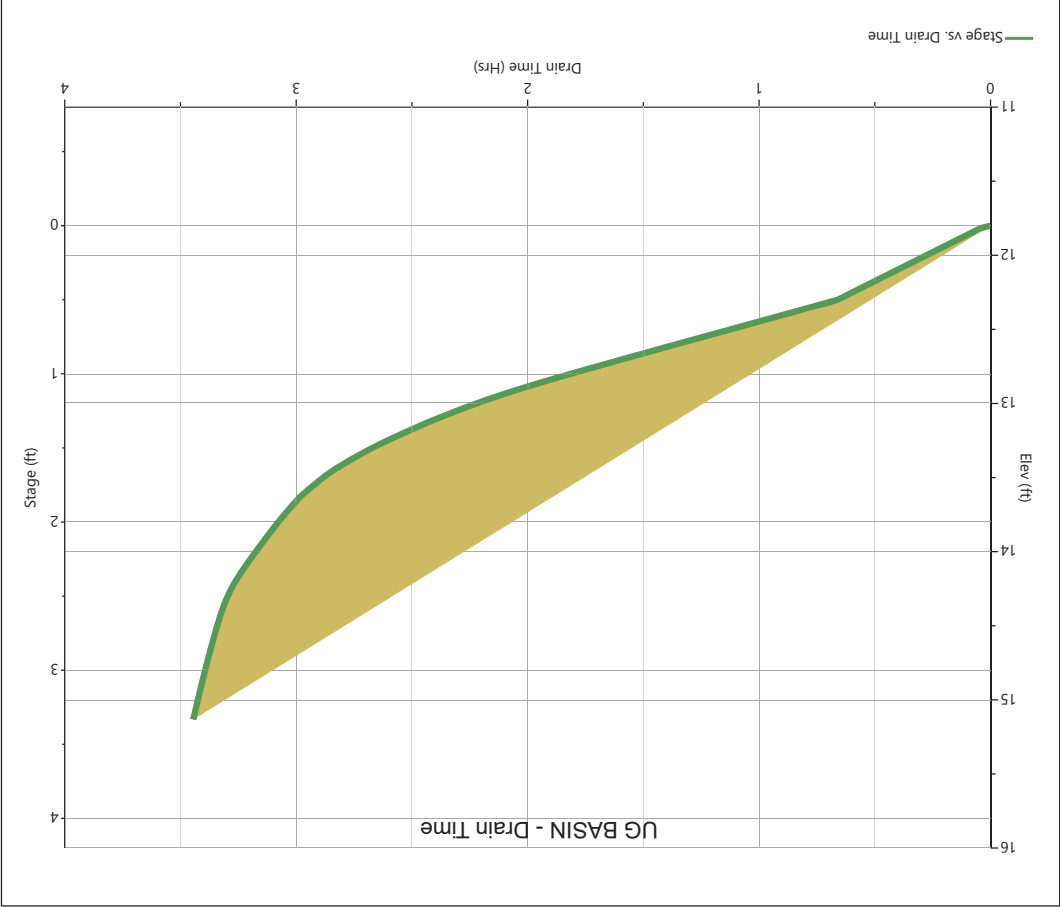
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

UG BASIN

Pond Drawdown



Hydrograph Report

Project Name:
11-22-2022

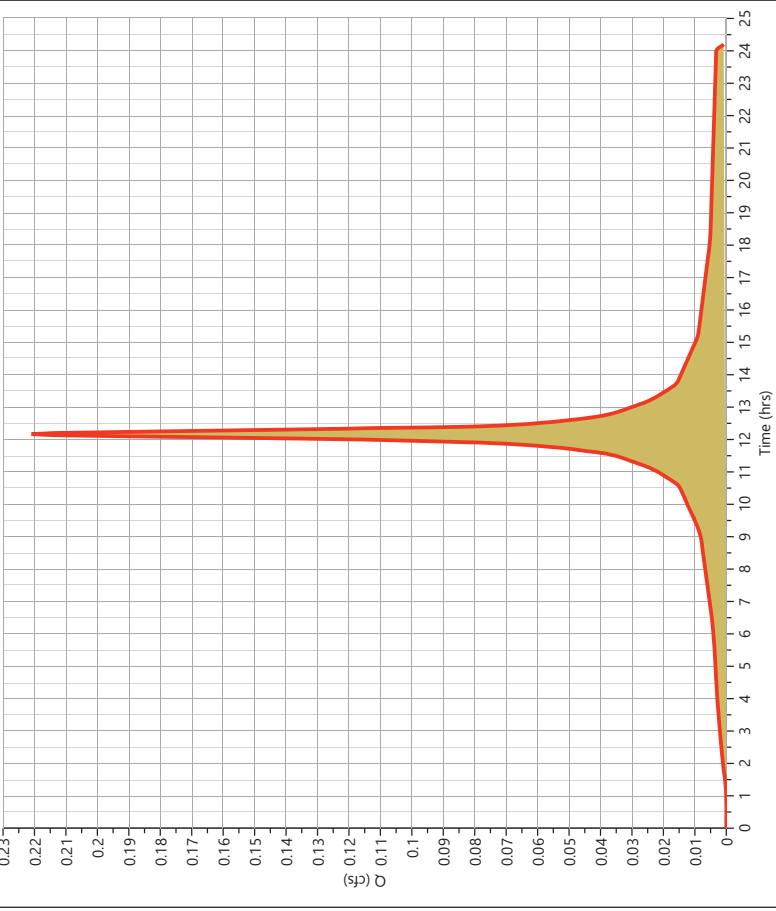
Hydrology Studio v 3.0.0.26

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 978 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.22 cfs



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.58	39	OPEN SPACE
0.28	30	WOODS
0.86	36	Weighted CN Method Employed

Qp = 0.00 cfs

Hydrograph Report

Project Name:
11-22-2022

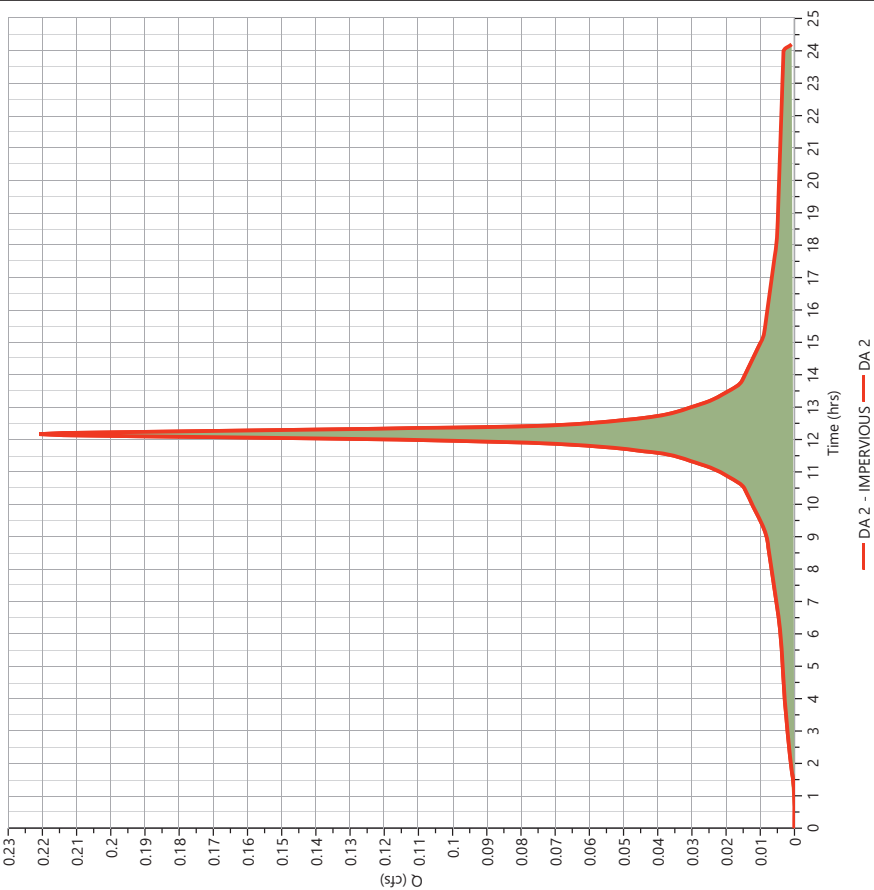
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 978 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.22 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

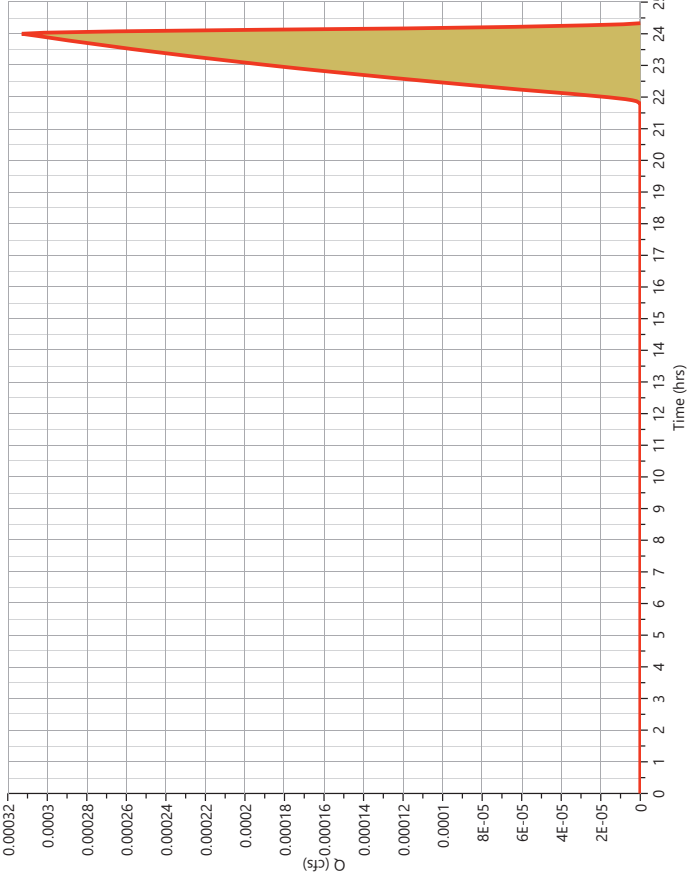
Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.47 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.62	39	OPEN SPACE
0.13	30	WOODS
0.75	37	Weighted CN Method Employed

Qp = 0.00 cfs



Hydrograph Report

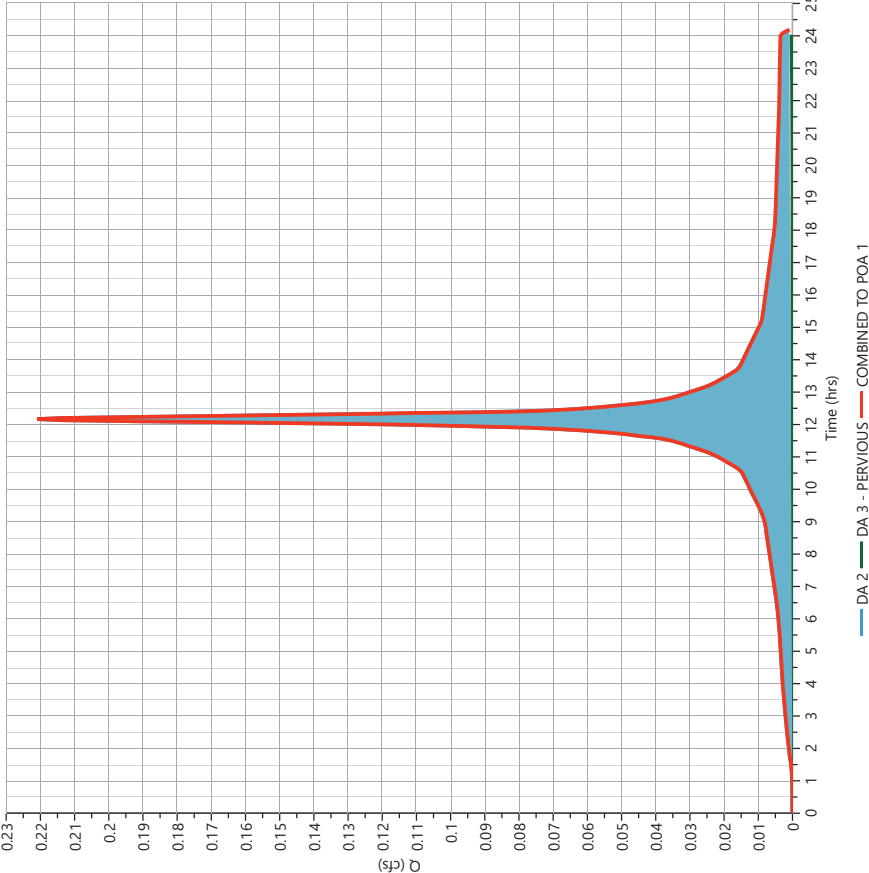
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 980 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac

Qp = 0.22 cfs



Hydrograph Report

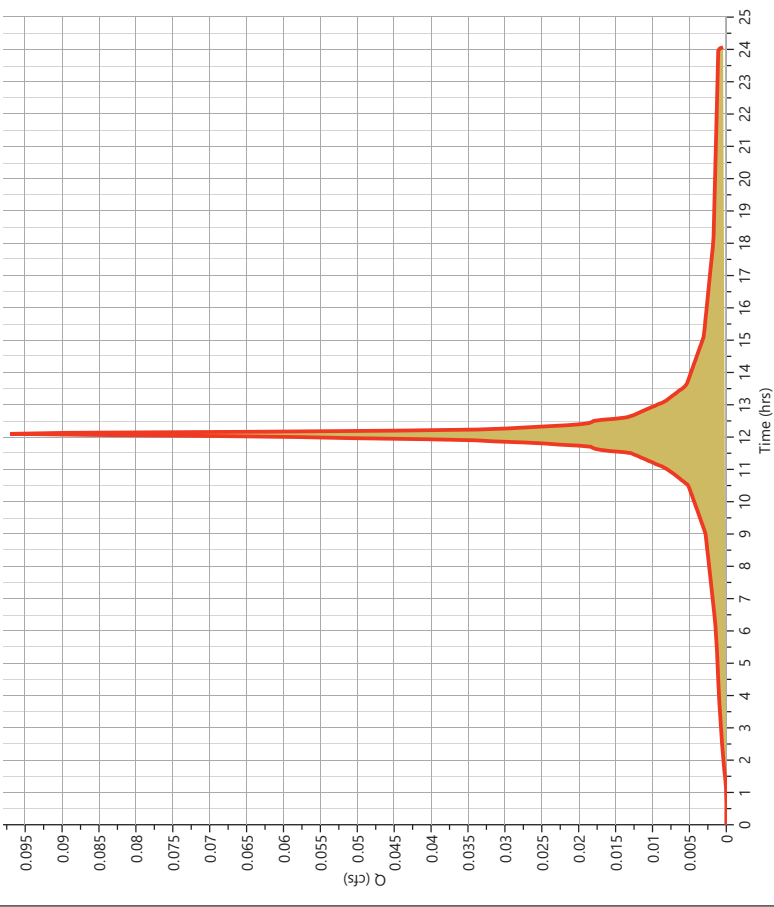
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.097 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 333 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.10 cfs



Hydrograph Report

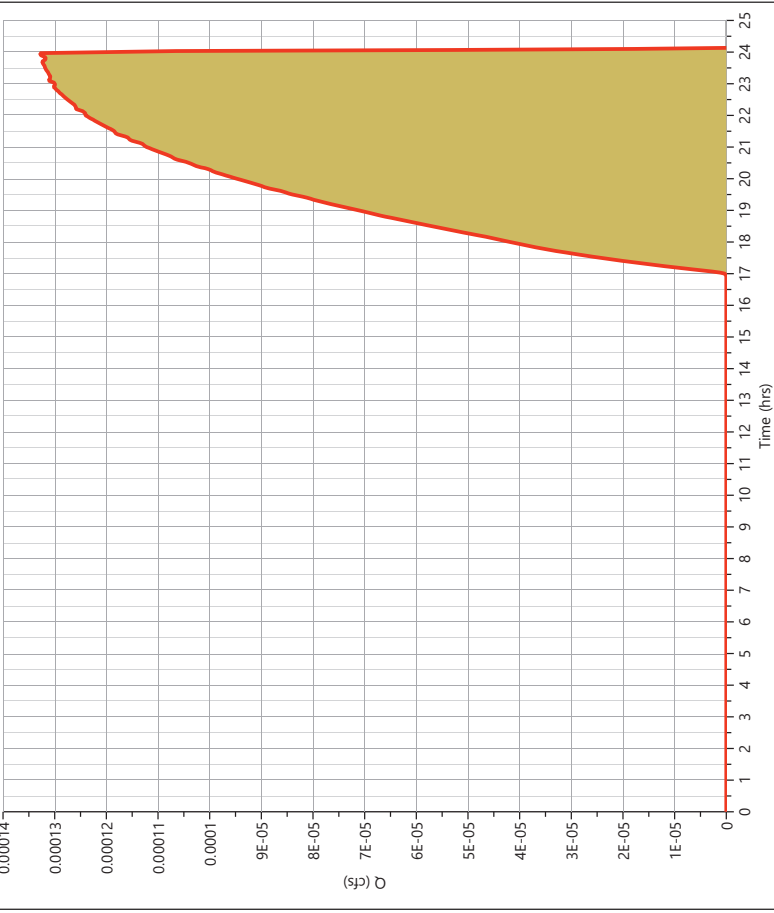
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 2.35 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



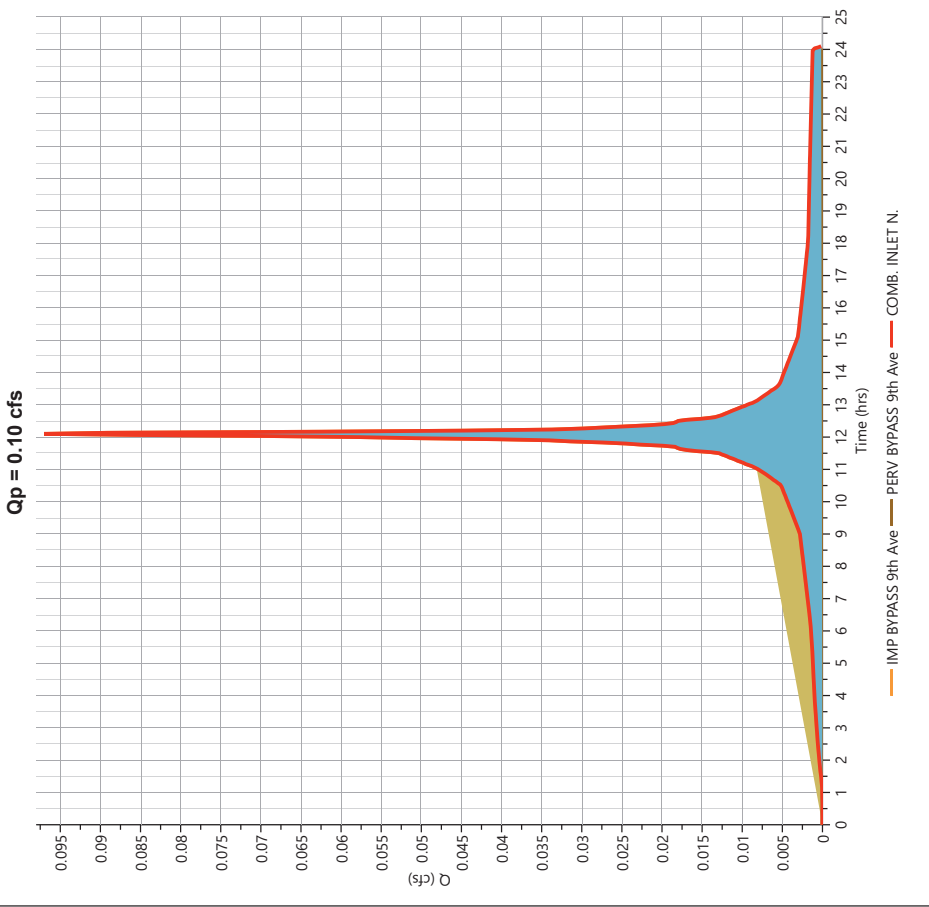
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.097 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 336 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



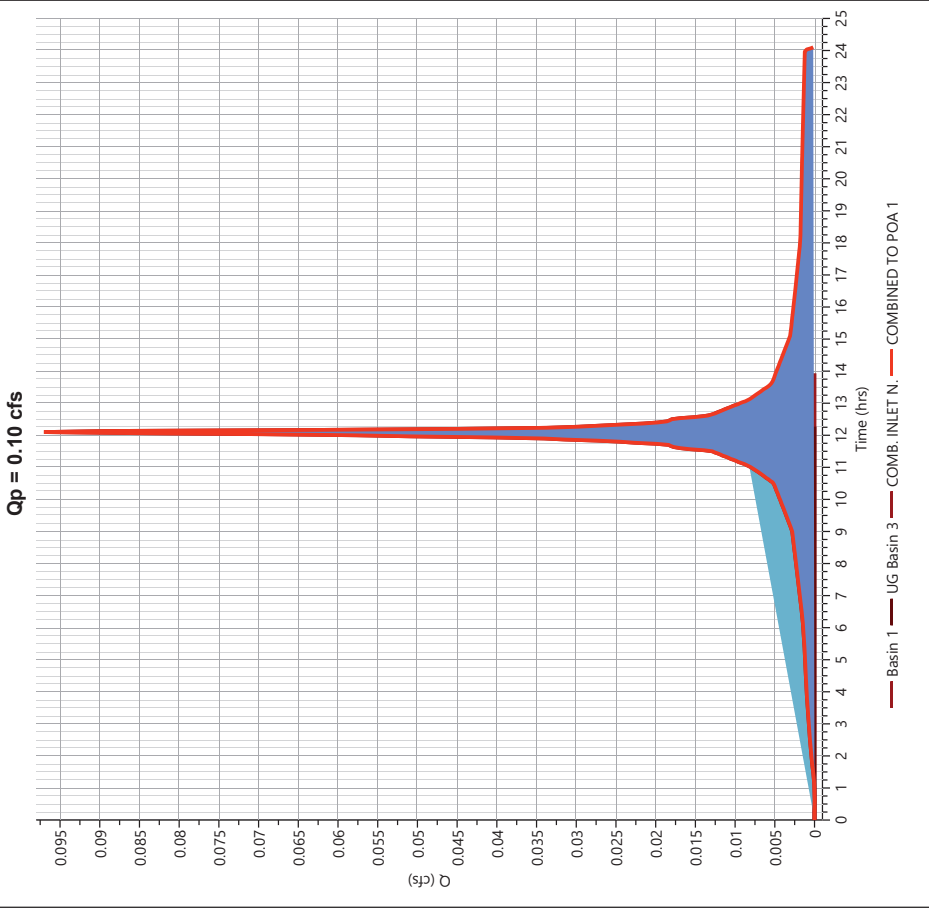
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 0.097 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 336 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



Hydrograph Report

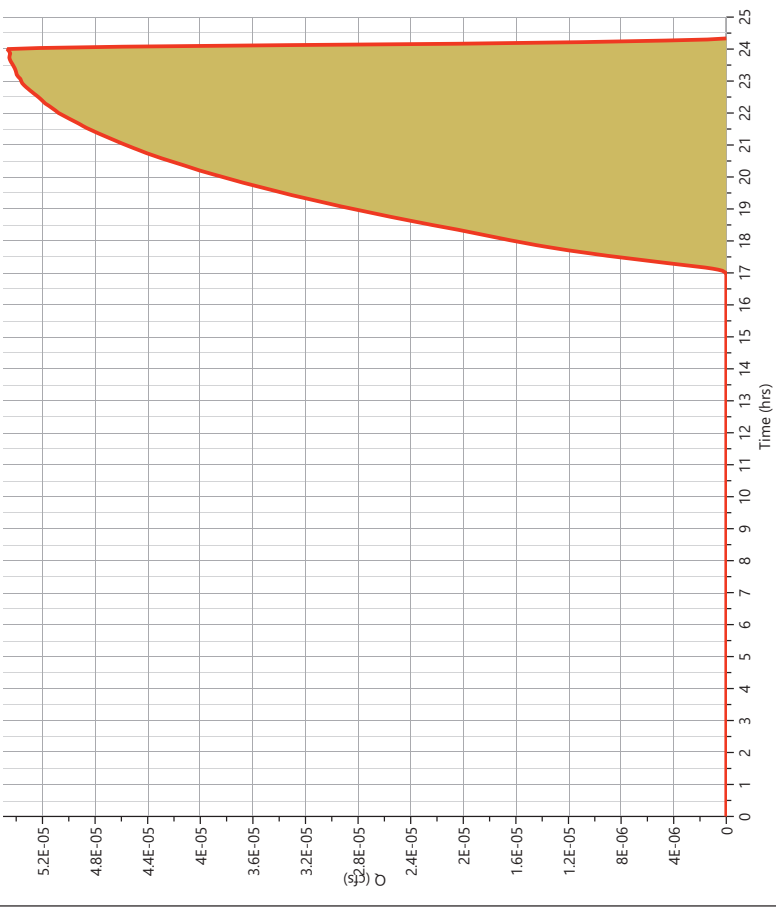
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.969 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

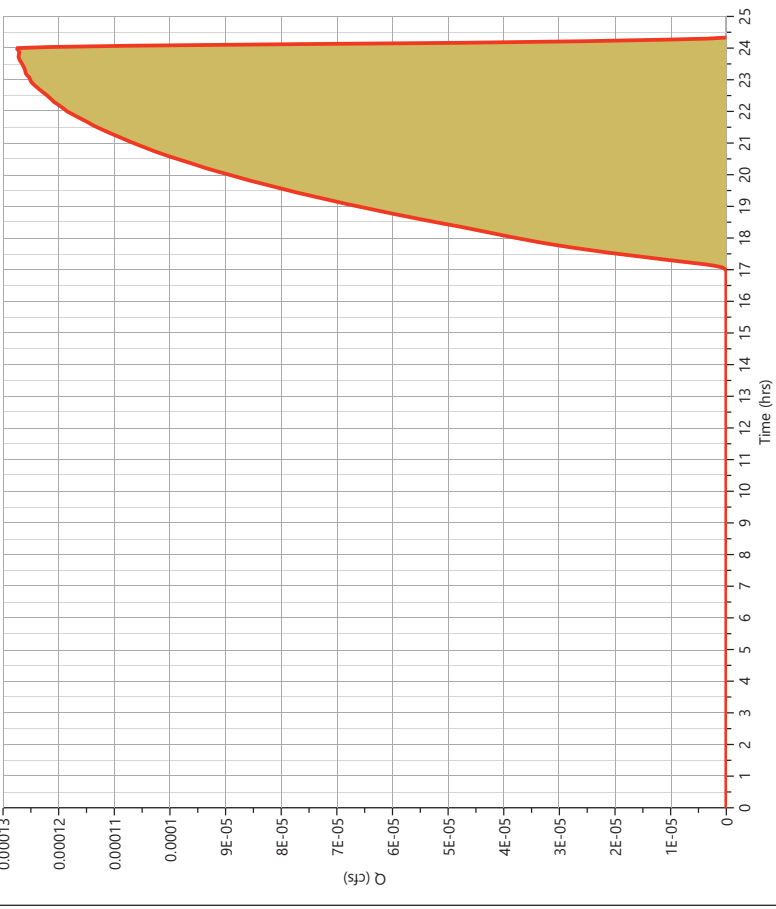
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 2.26 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



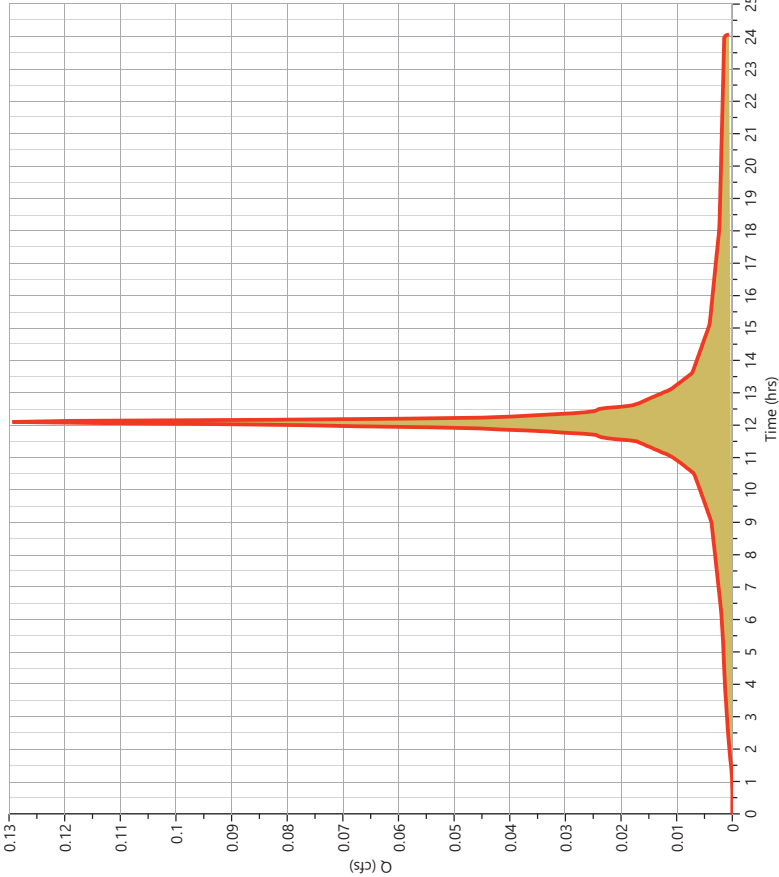
Hydrograph Report

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.129 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 445 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.13 cfs



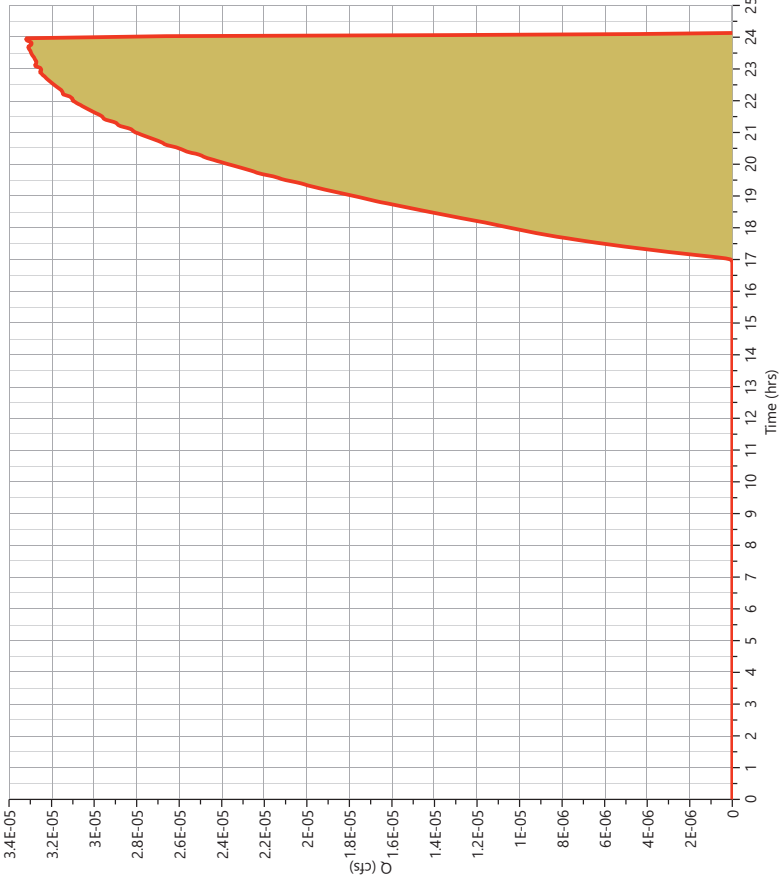
Hydrograph Report

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.588 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

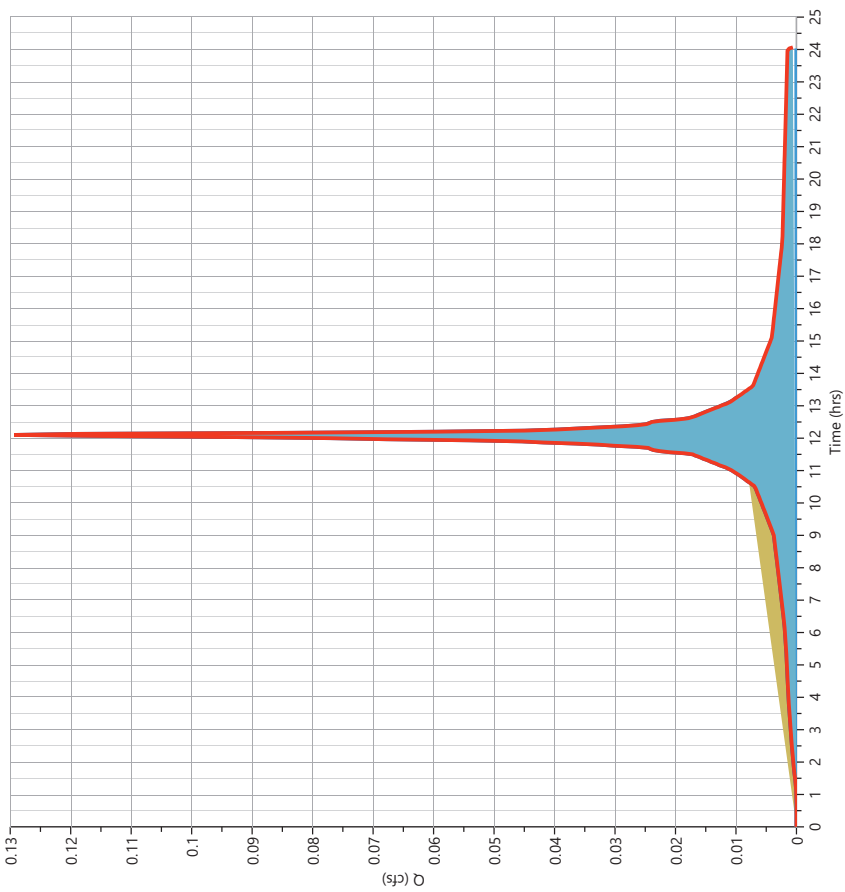
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.129 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 445 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.13 cfs



Imp Bypass 8th Ave Perv Bypass 8th Ave Comb. Bypass POA 2

Hydrograph 10-yr Summary

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.187	12.13	753	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.002	12.97	38.7	----	----	----
3	NRCS Runoff	Post Roof Area DA3	1.763	12.10	6,173	----	----	----
4	NRCS Runoff	Post Deck DA3	0.947	12.17	4,268	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.646	12.17	2,910	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.002	12.50	39.9	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.453	12.10	1,587	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.004	12.50	59.8	----	----	----
9	NRCS Runoff	Post Roof Area DA2	2.519	12.10	8,818	----	----	----
10	Junction	Pre DA 1 - POA 2	0.187	12.13	791	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.007	12.60	121	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.301	12.17	1,358	----	----	----
13	Junction	Post Underground Basin 3	3.455	12.13	14,830	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.453	12.10	1,627	6, 7	----	----
15	Junction	Post Basin 2	2.519	12.10	8,878	8, 9	----	----
16	Pond Route	Post Basin 2	1.739	12.17	3,439	15	14.49	1,105
17	Pond Route	Post Basin 1	0.000	12.17	0.000	14	14.46	581
18	Junction	Post COMBINED TO UG BASIN 5	5.179	12.13	18,268	13, 16	12.93	5,656
19	Pond Route	Post UG Basin 3	0.099	12.70	258	18	----	----
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.344	12.17	1,552	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.021	13.30	570	----	----	----
22	Junction	Pre DA 2	0.344	12.17	2,122	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.026	13.00	598	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.344	12.17	2,720	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.151	12.10	529	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.005	12.50	79.7	----	----	----
27	Junction	COMB. INLET N	0.151	12.10	609	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	0.151	12.10	866	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.002	12.60	32.9	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.004	12.60	76.7	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.202	12.10	705	----	----	----
32	NRCS Runoff	Post Post Perv Bypass 8th Ave	0.001	12.50	19.9	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.202	12.10	725	31, 32	----	----

Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.187 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 753 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.19 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - PERVIOUS

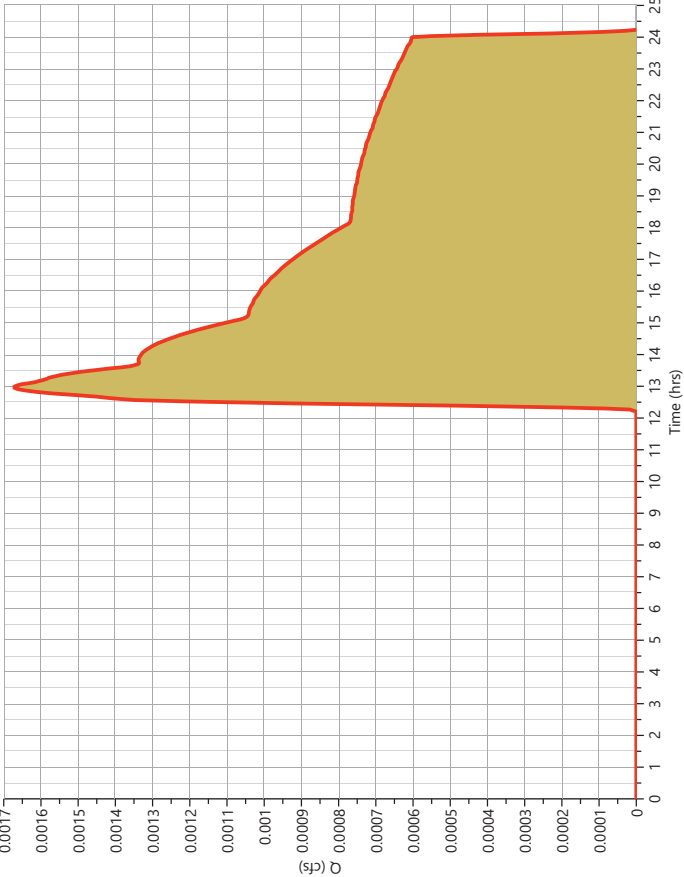
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.97 hrs
Time Interval	= 2 min	Runoff Volume	= 38.7 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.04	39	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.00 cfs



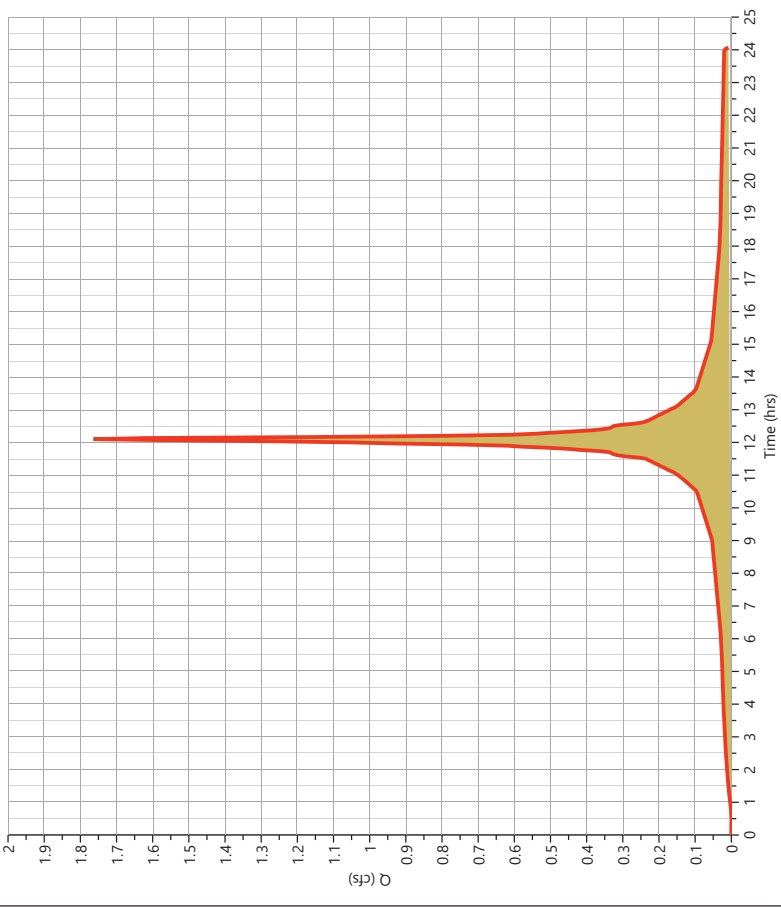
Hydrograph Report

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.763 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 6,173 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.76 cfs



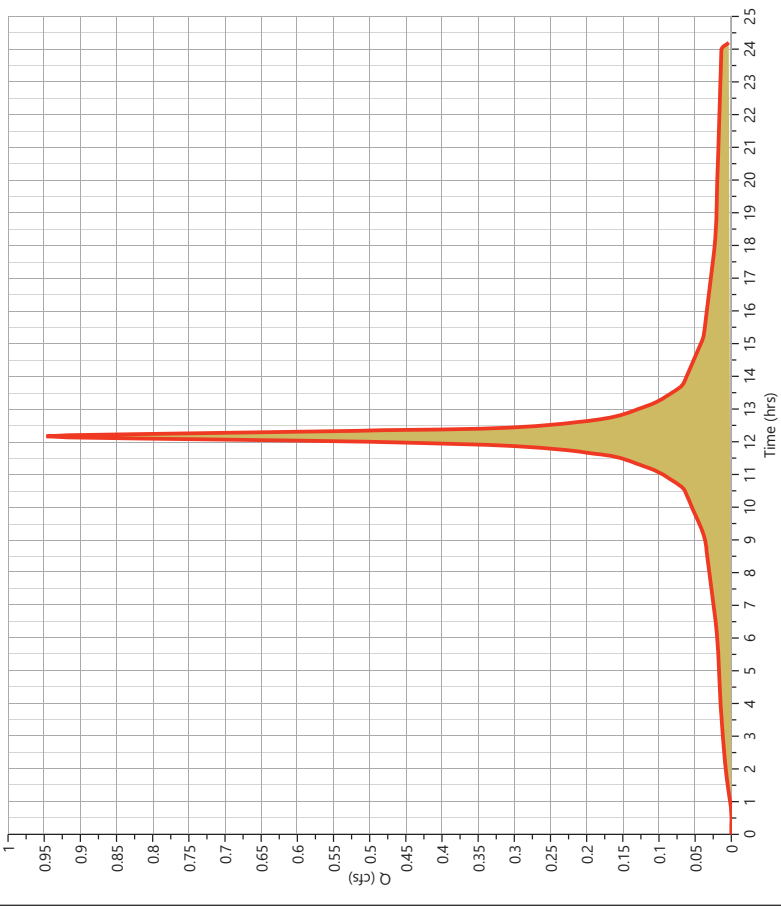
Hydrograph Report

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.947 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 4,268 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.95 cfs



Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.646 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,910 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.65 cfs



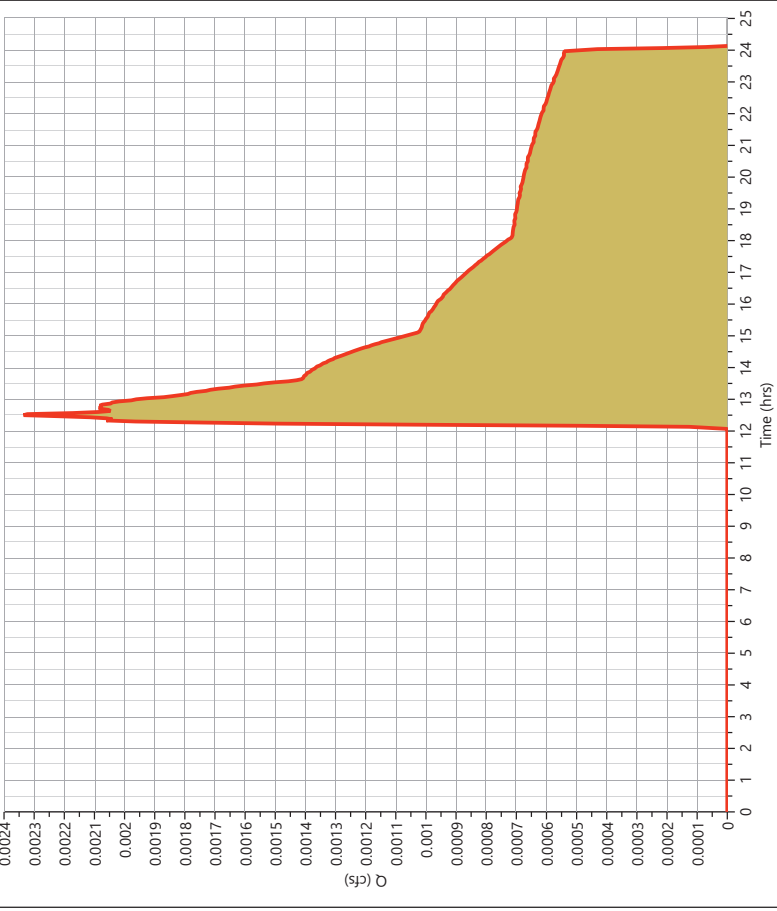
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 39.9 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



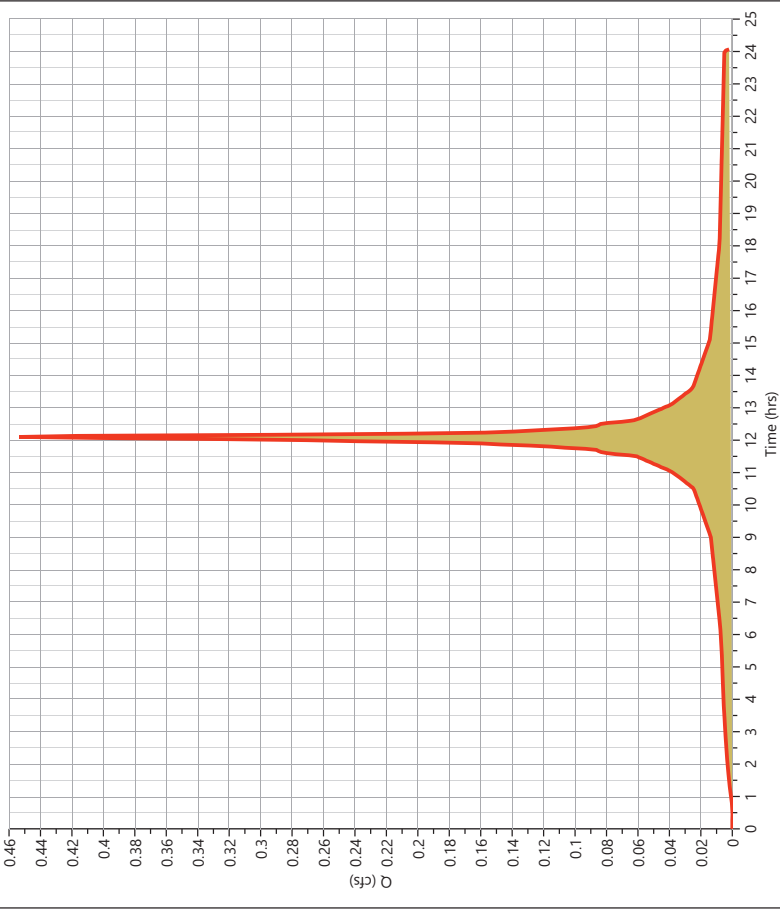
Hydrograph Report

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.453 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,587 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.45 cfs



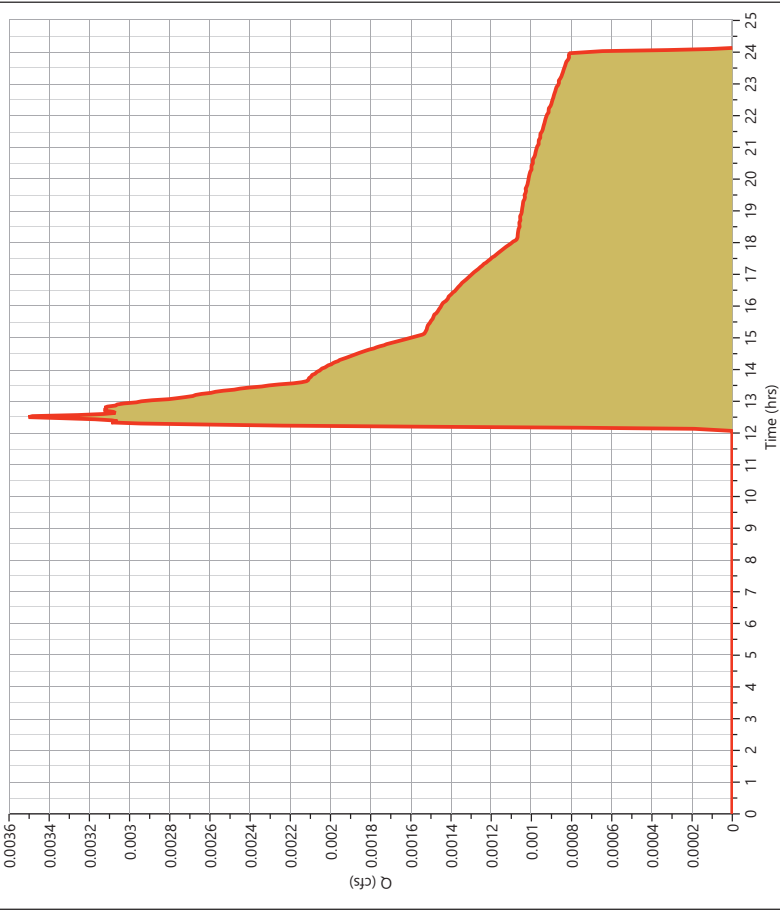
Hydrograph Report

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.004 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 59.8 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



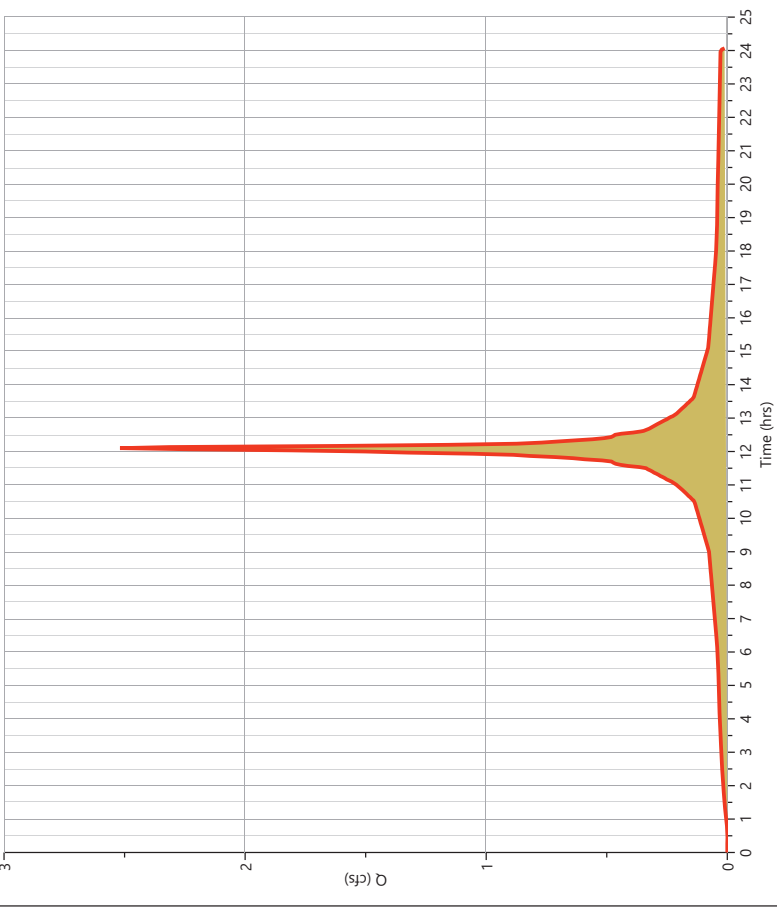
Hydrograph Report

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.519 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 8,818 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 2.52 cfs



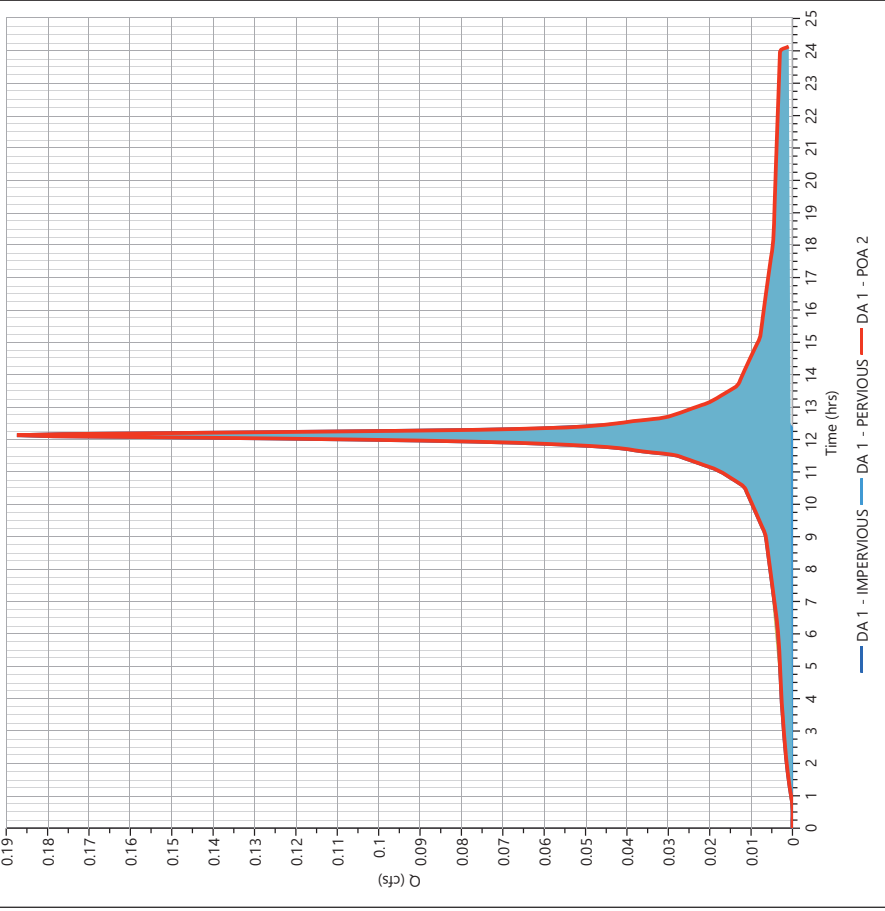
Hydrograph Report

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.187 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 791 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.19 cfs



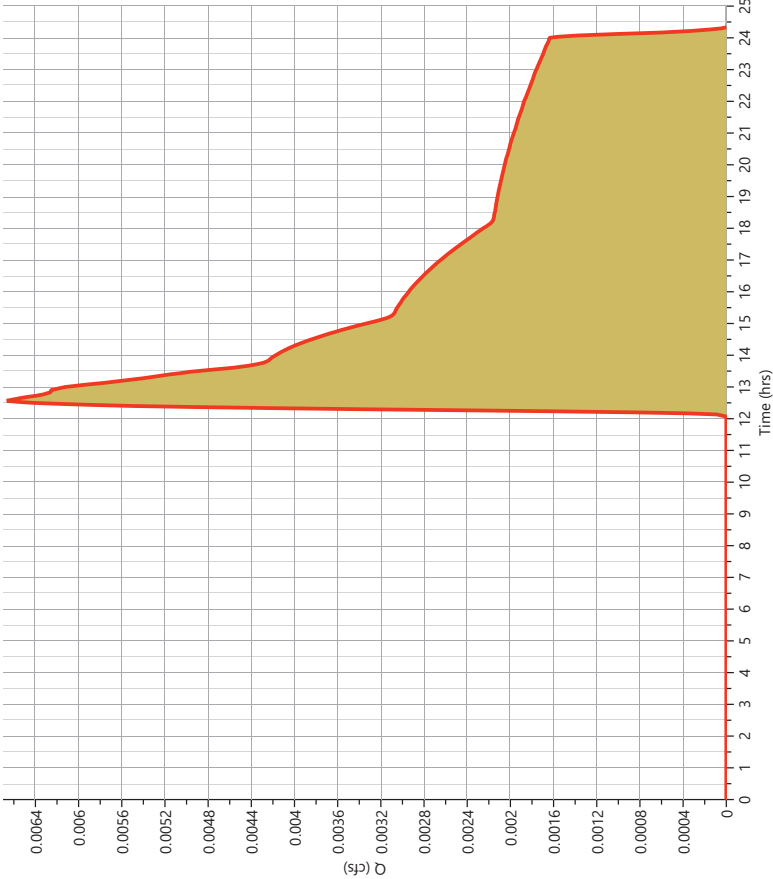
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.007 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 121 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.01 cfs



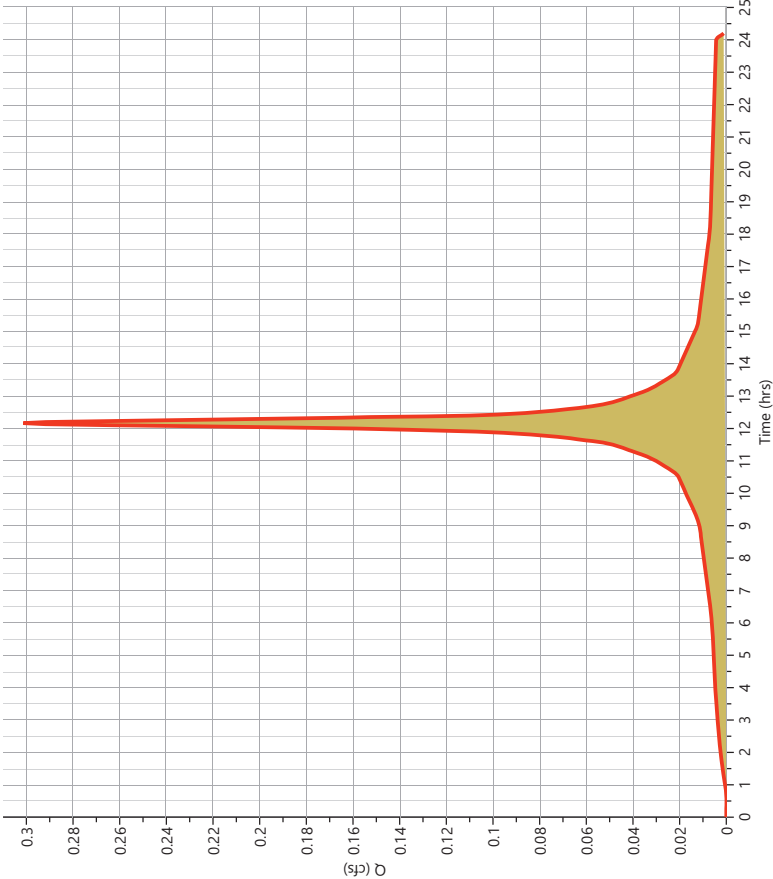
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.301 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,358 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.30 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

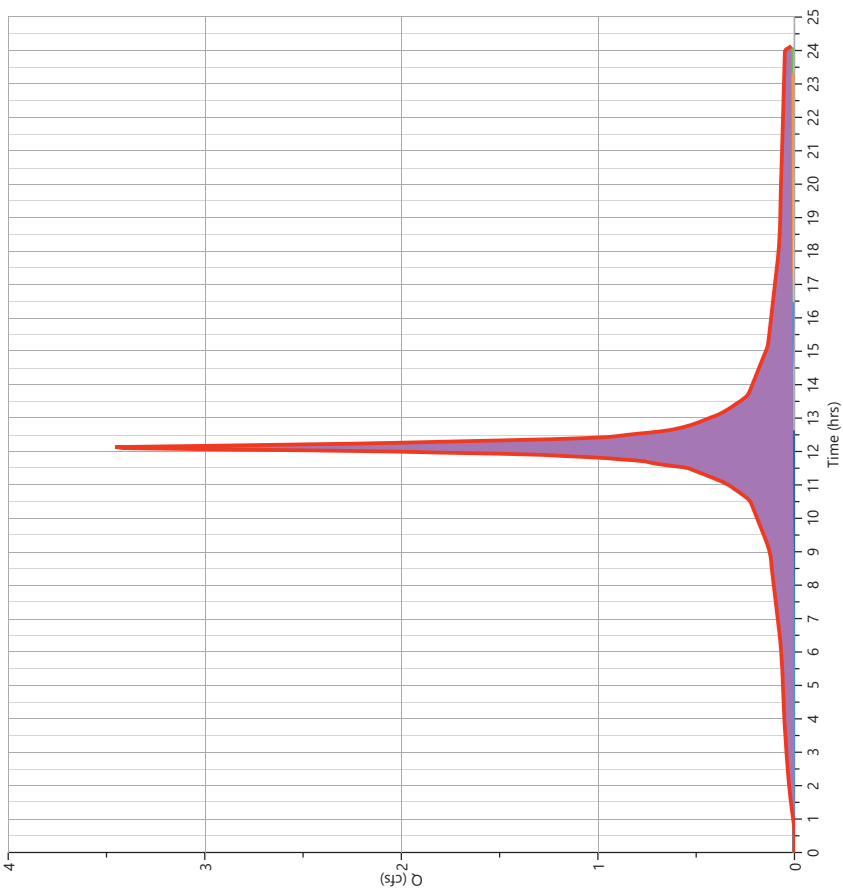
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 3.455 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 14,830 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 3.45 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

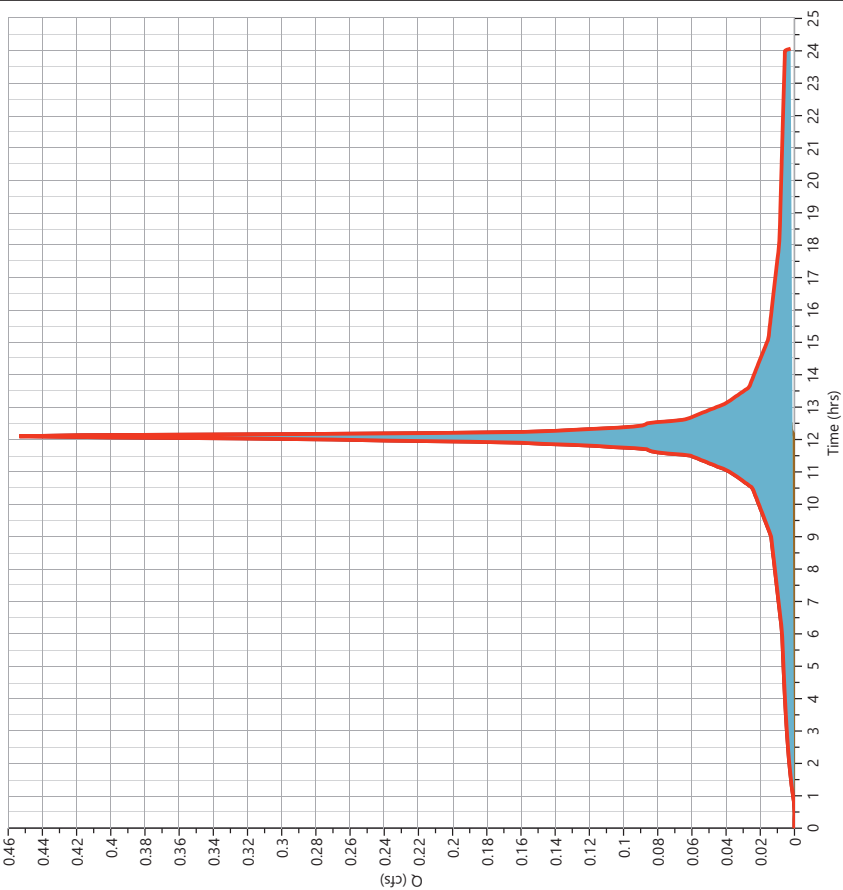
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.453 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,627 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.45 cfs



Hydrograph Report

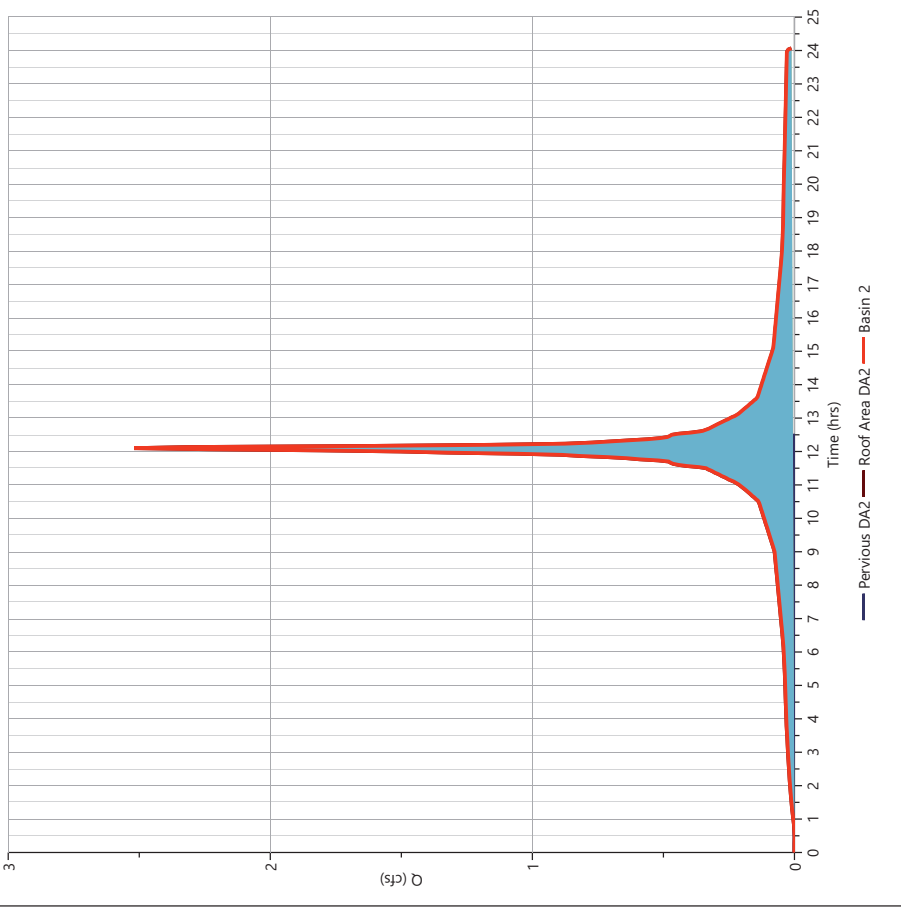
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 2,519 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 8,878 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 2.52 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

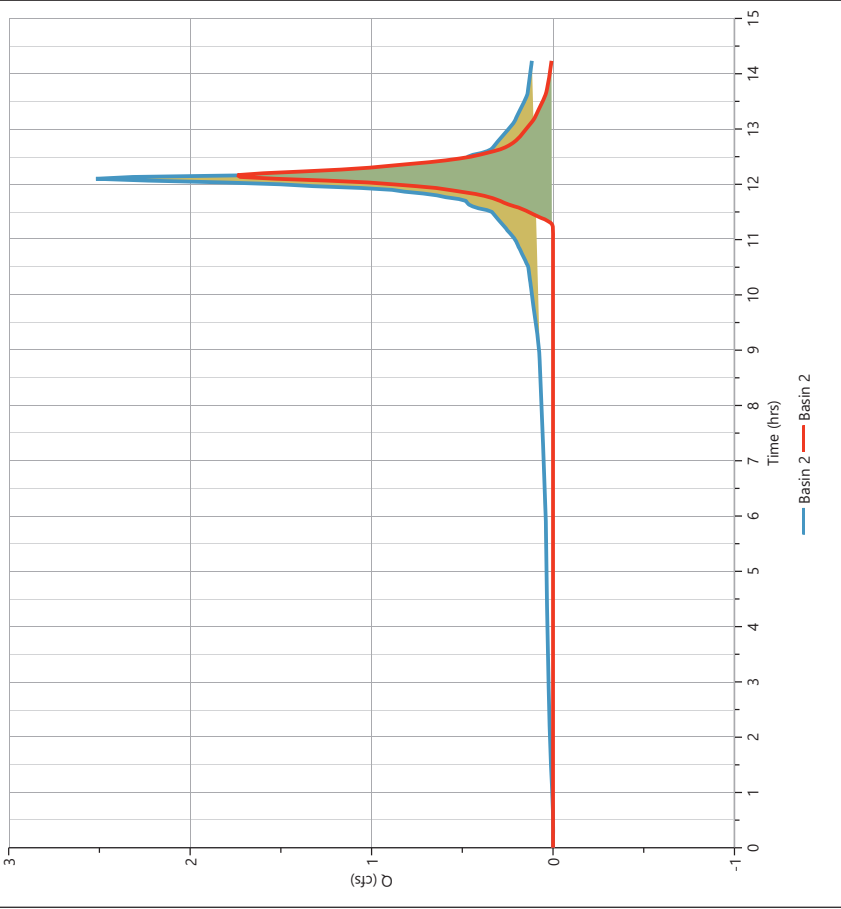
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 1,739 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 3,439 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.49 ft
Pond Name	= Basin 2	Max. Storage	= 1,105 cuft

Pond Routing by Storage Indication Method

Qp = 1.74 cfs



Hydrograph Report

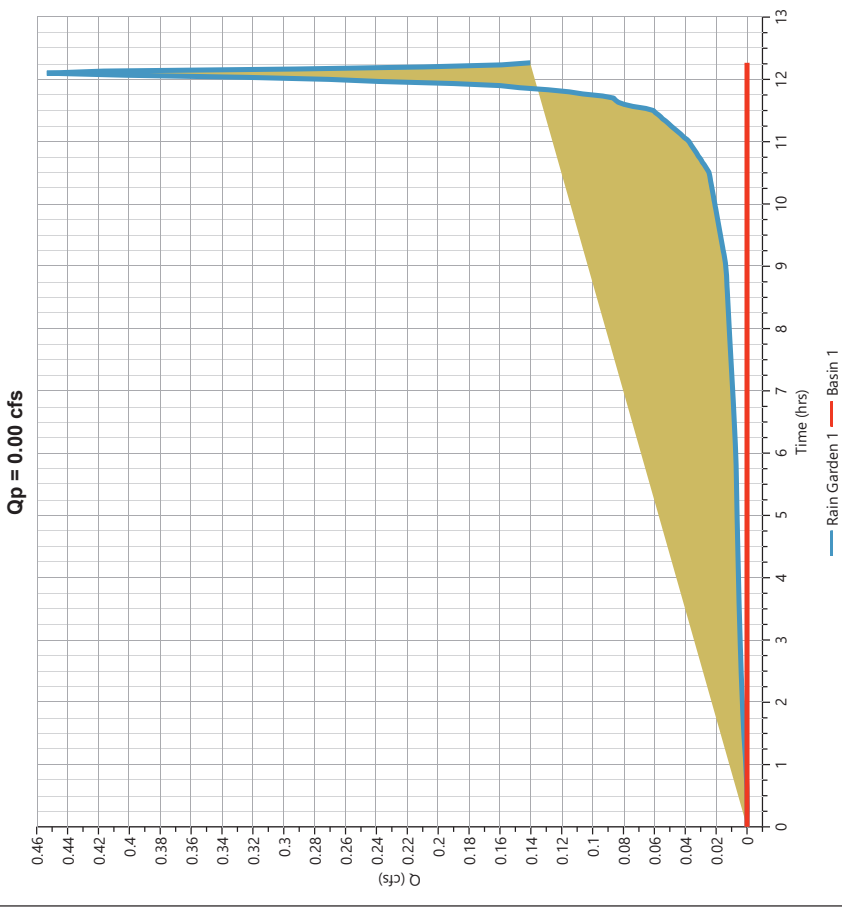
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 1

Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.000 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 0.000 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.46 ft
Pond Name	= BASIN 1	Max. Storage	= 581 cuft

Pond Routing by Storage Indication Method



Hydrograph Report

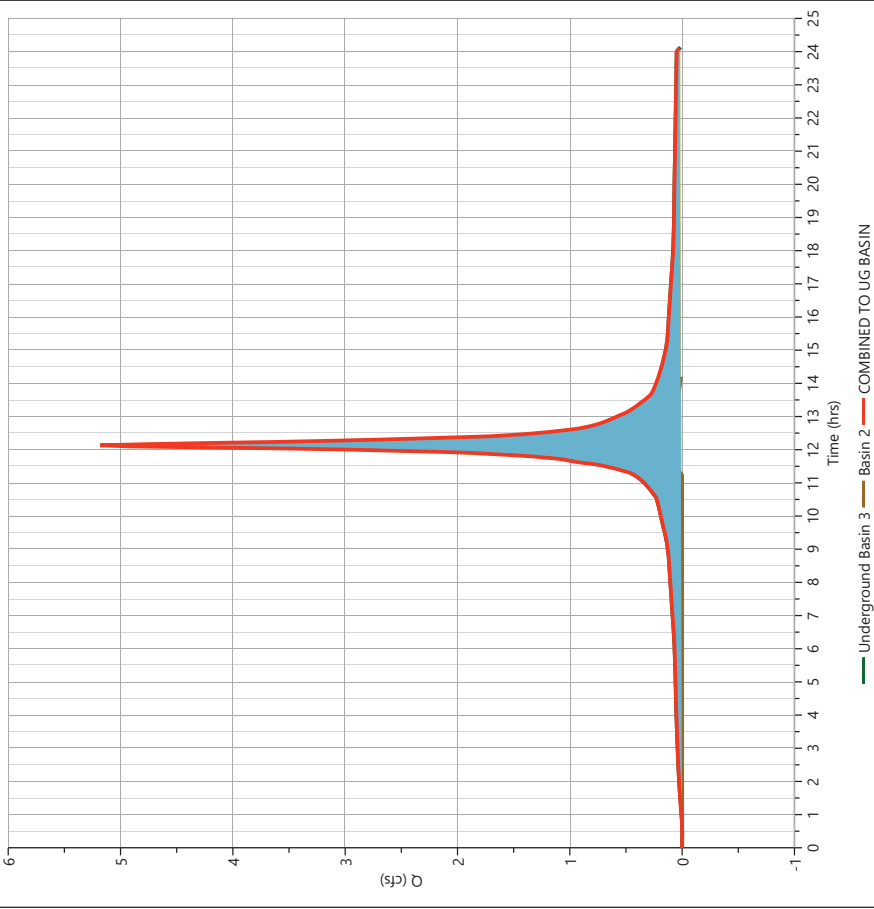
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 5.179 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 18,268 cuft
Inflow Hydrographs	= 13, 16	Total Contrib. Area	= 0.9 ac

Qp = 5.18 cfs



Hydrograph Report

Post UG Basin 3

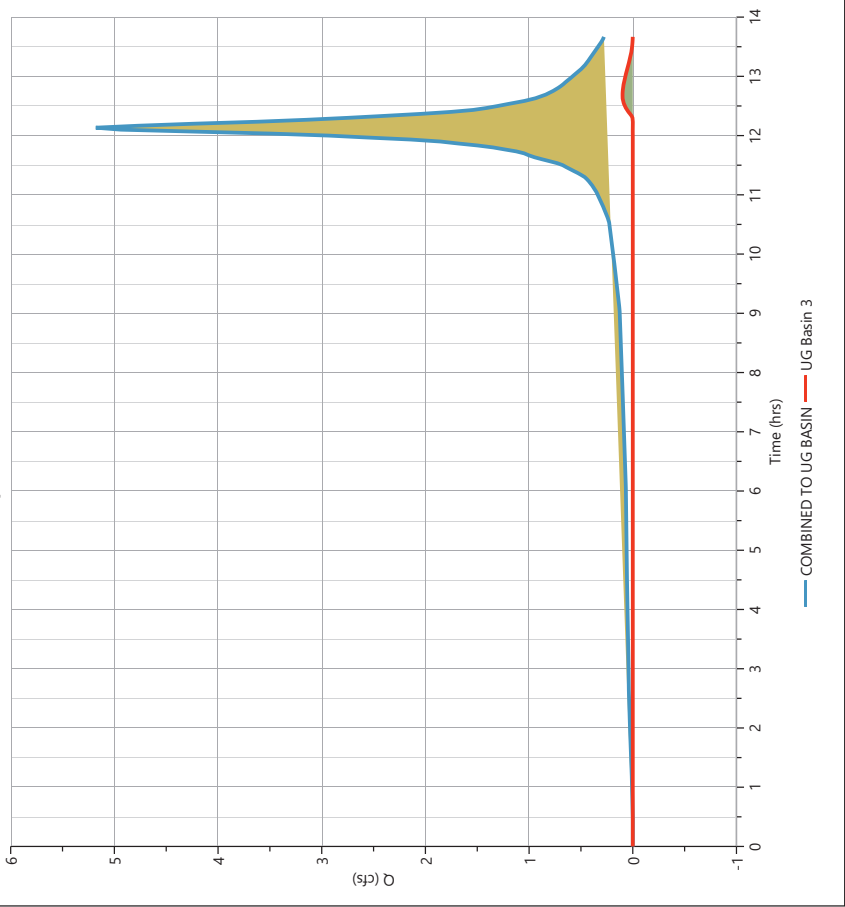
Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.099 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.70 hrs
Time Interval	= 2 min	Hydrograph Volume	= 258 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 12.93 ft
Pond Name	= UG BASIN	Max. Storage	= 5,656 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 26 min

Qp = 0.10 cfs



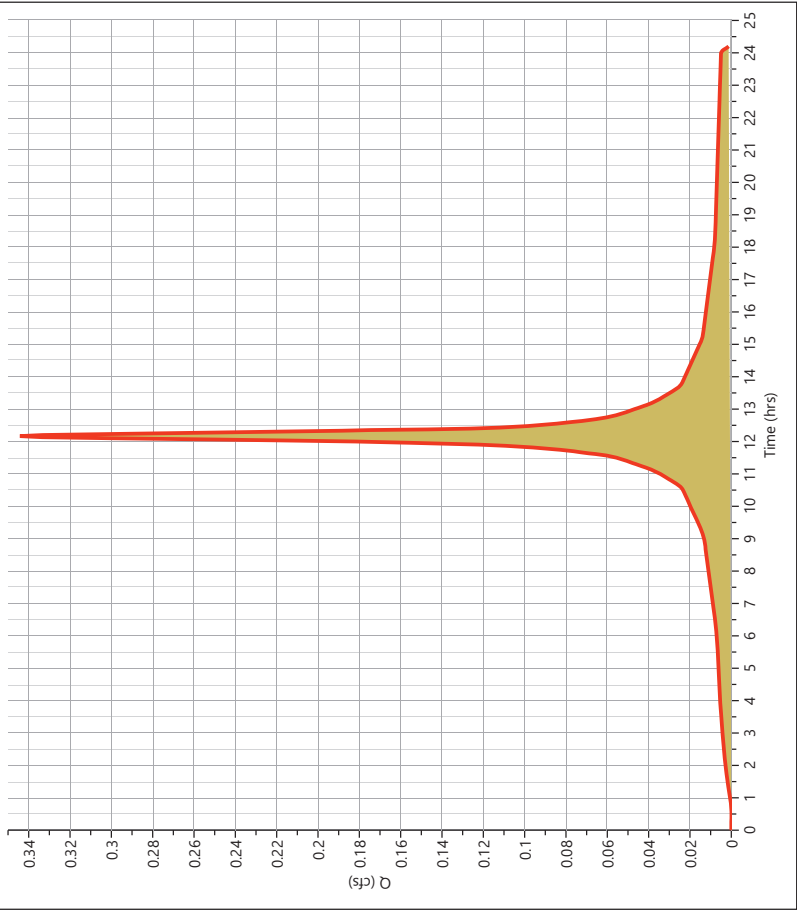
Hydrograph Report

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,552 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.34 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

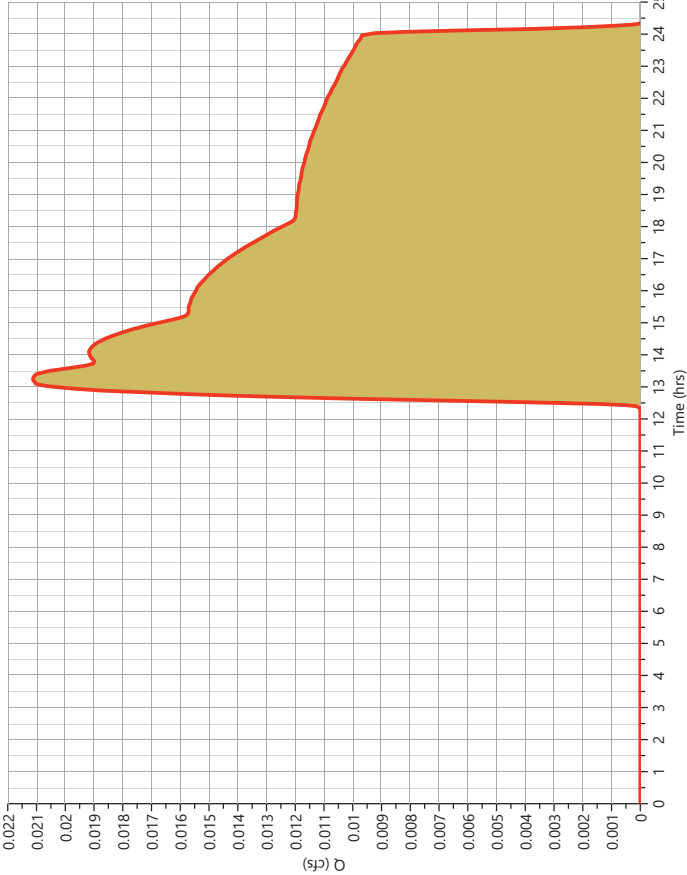
Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.021 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.30 hrs
Time Interval	= 2 min	Runoff Volume	= 570 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.58	39	OPEN SPACE
0.28	30	WOODS
0.86	36	Weighted CN Method Employed

Qp = 0.02 cfs



Hydrograph Report

Project Name:

11-22-2022

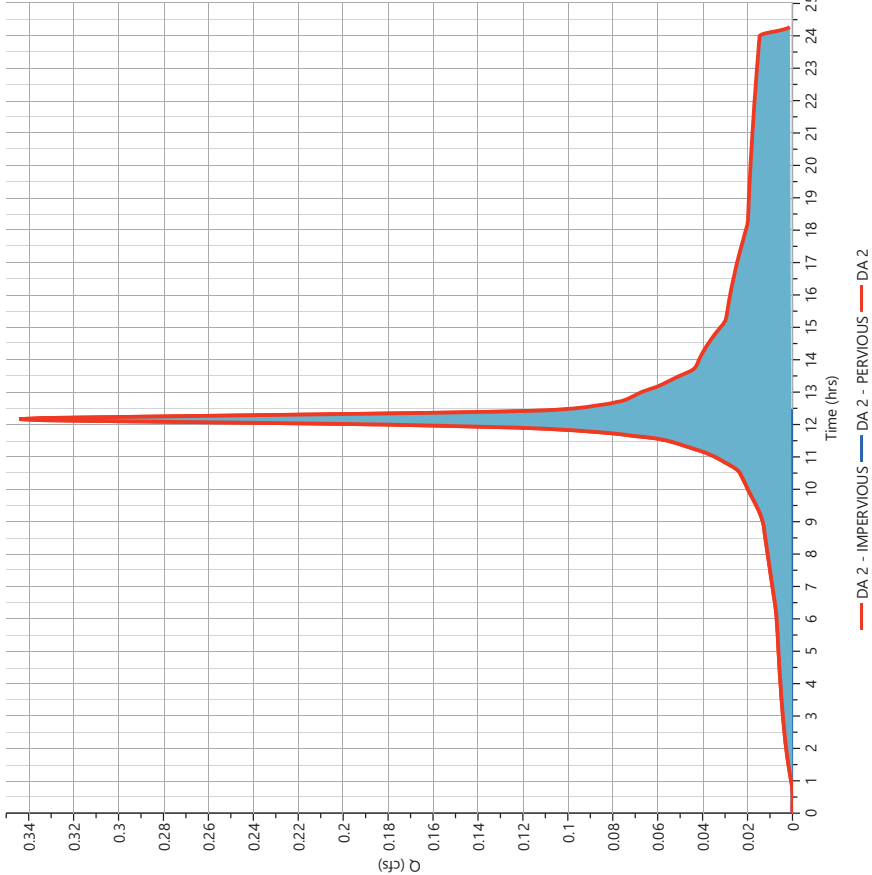
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,122 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.34 cfs



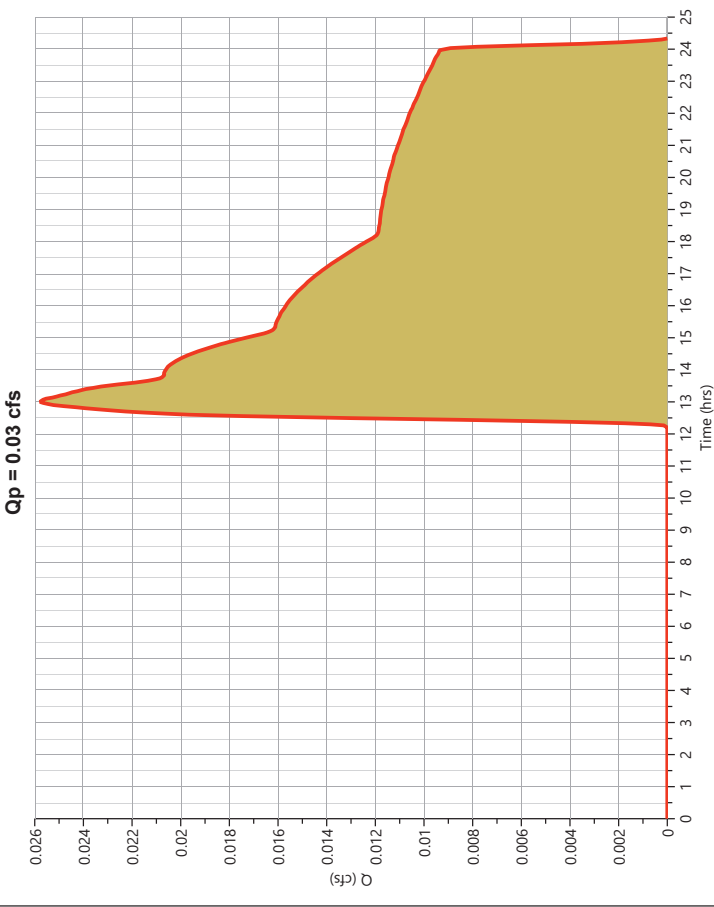
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.026 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.00 hrs
Time Interval	= 2 min	Runoff Volume	= 598 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	38	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	



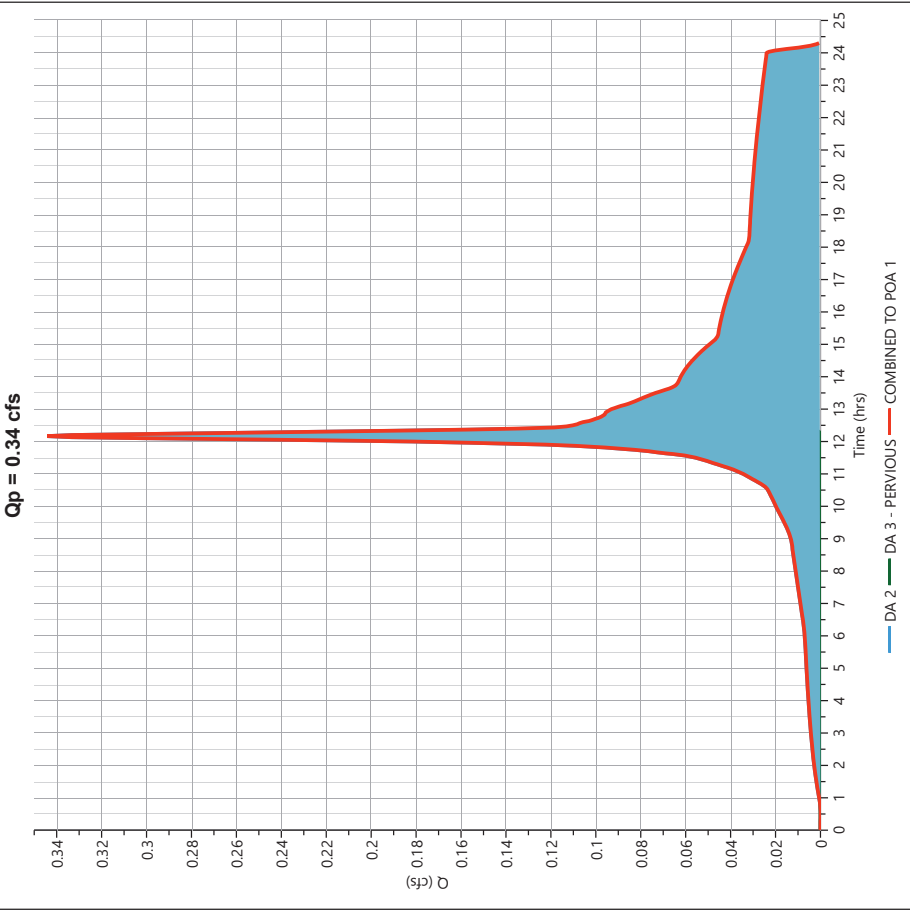
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,720 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac



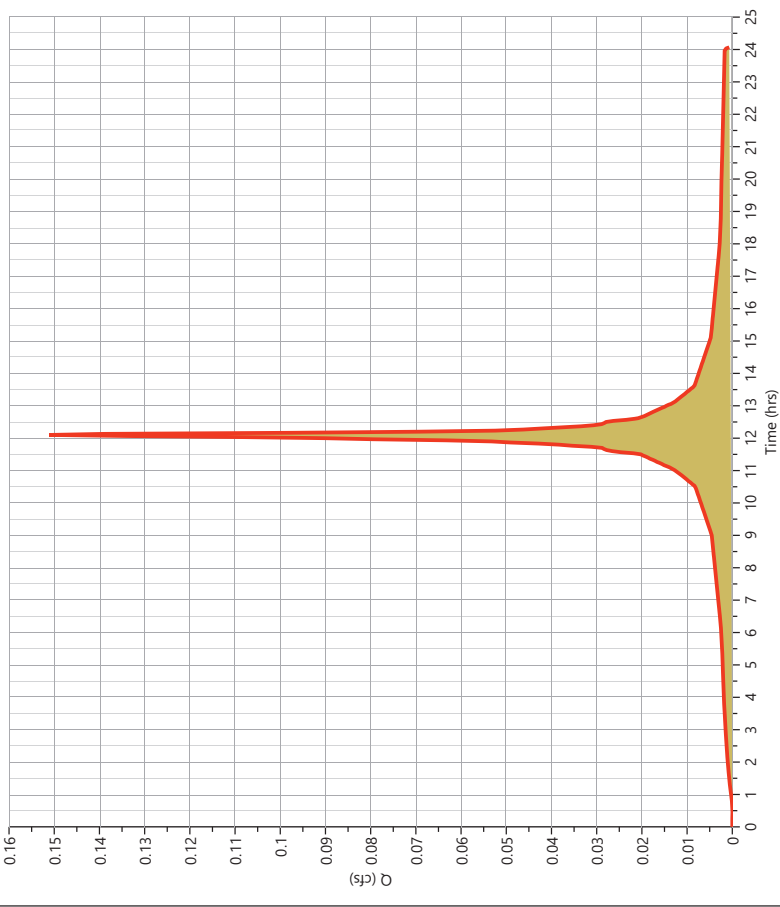
Hydrograph Report

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 529 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.15 cfs



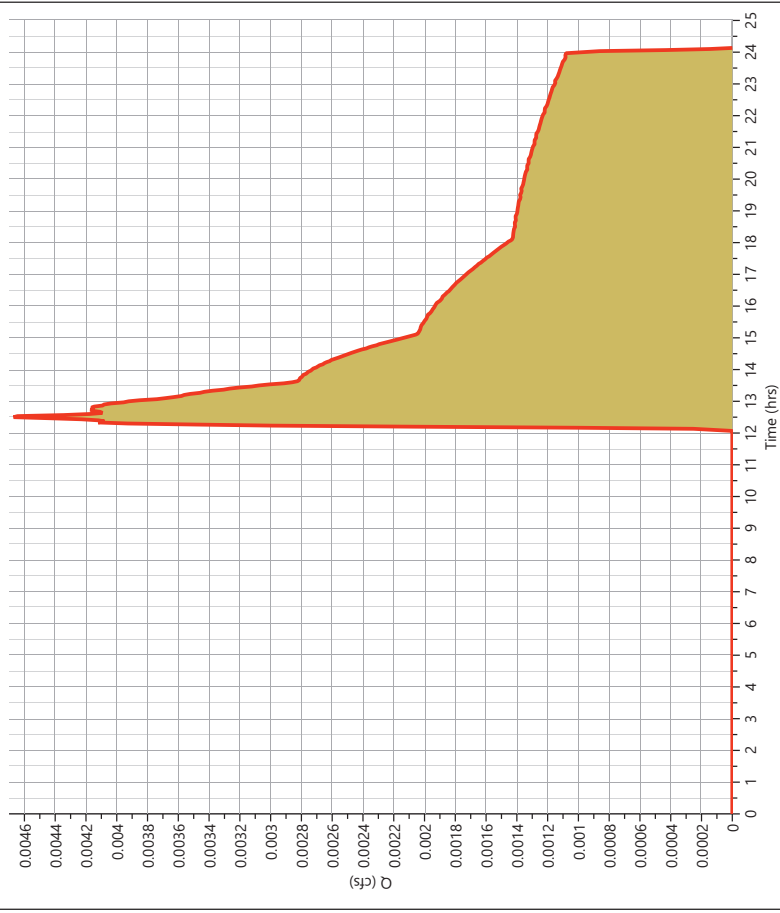
Hydrograph Report

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.005 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 79.7 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

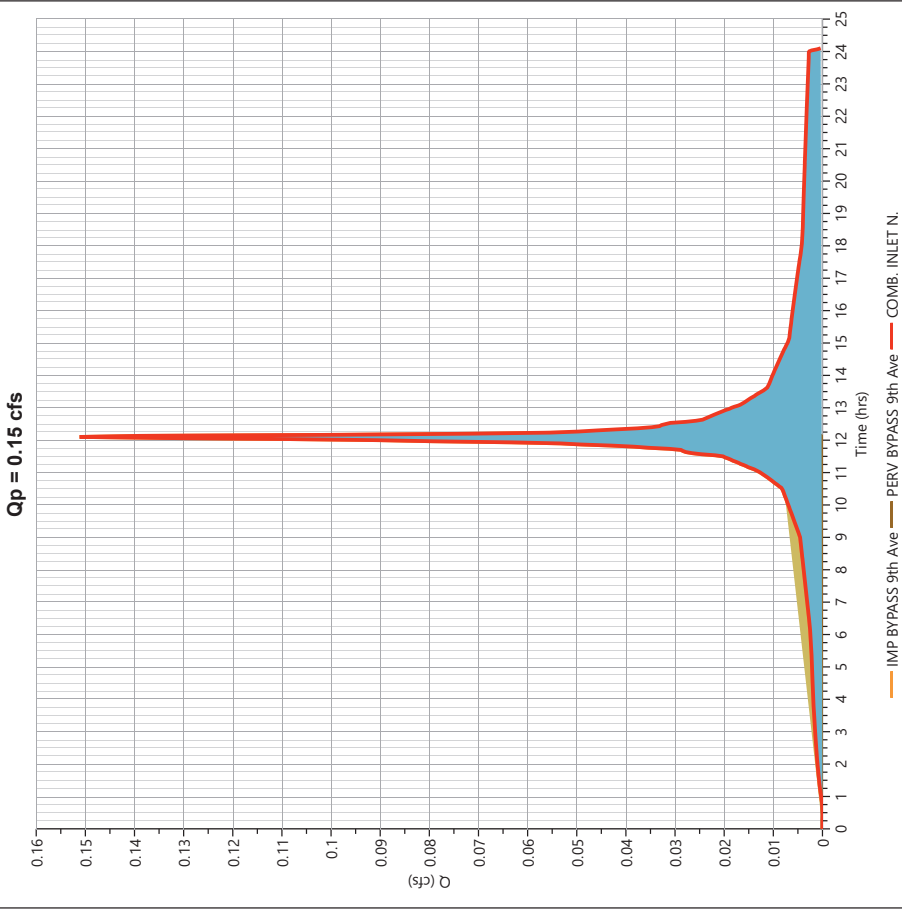
Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 609 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



Hydrograph Report

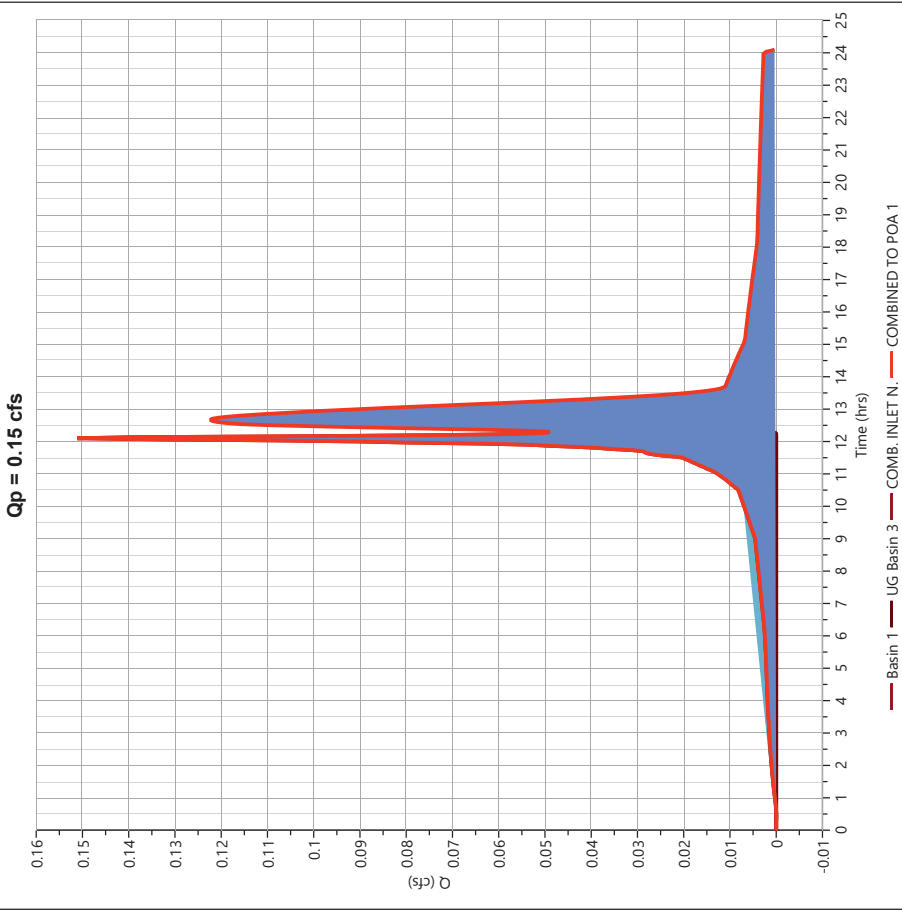
Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 866 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



Hydrograph Report

Project Name:
11-22-2022

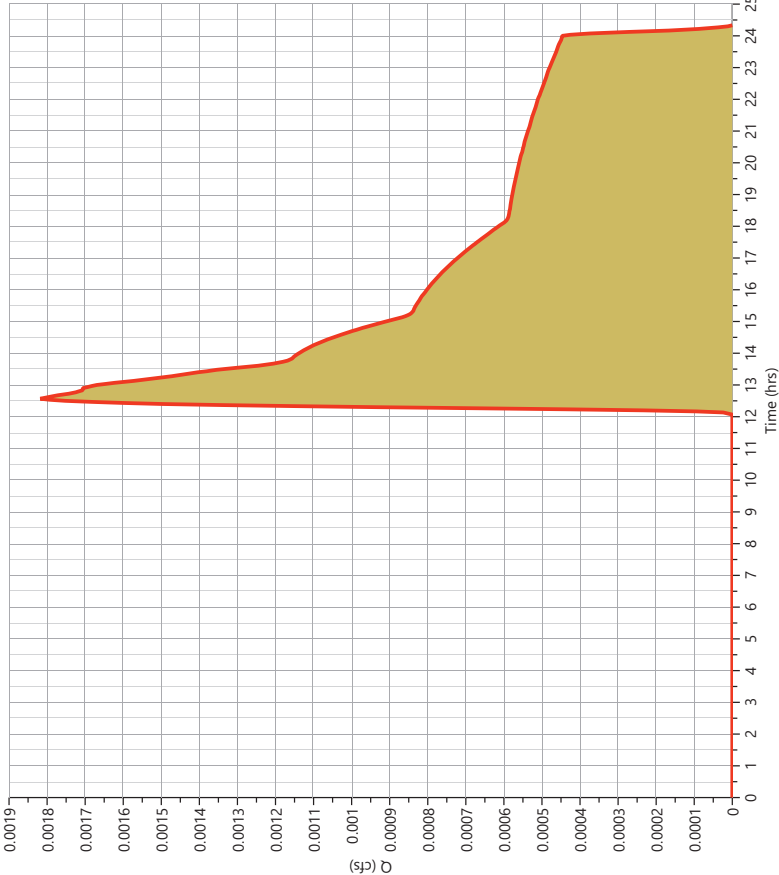
Hydrology Studio v 3.0.0.26

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 32.9 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:
11-22-2022

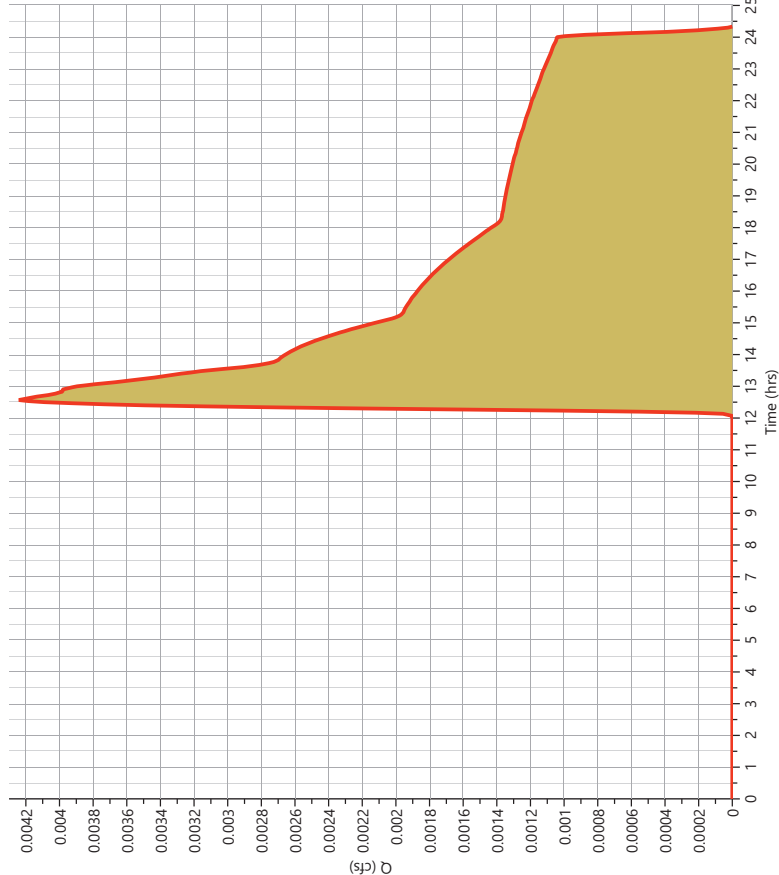
Hydrology Studio v 3.0.0.26

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.004 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 76.7 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

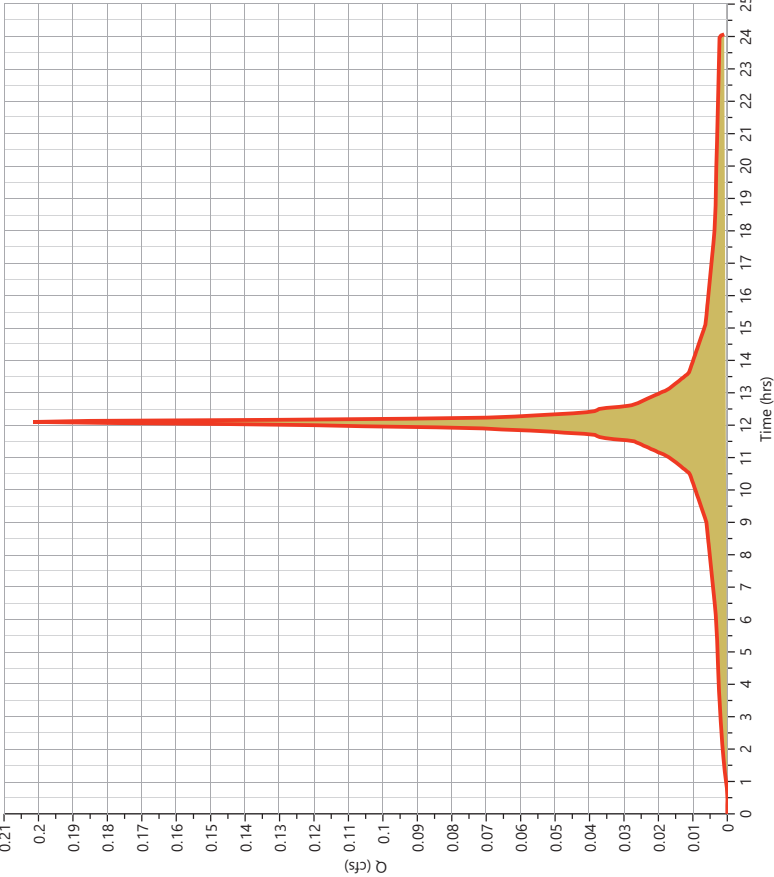
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.202 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 705 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.20 cfs



Hydrograph Report

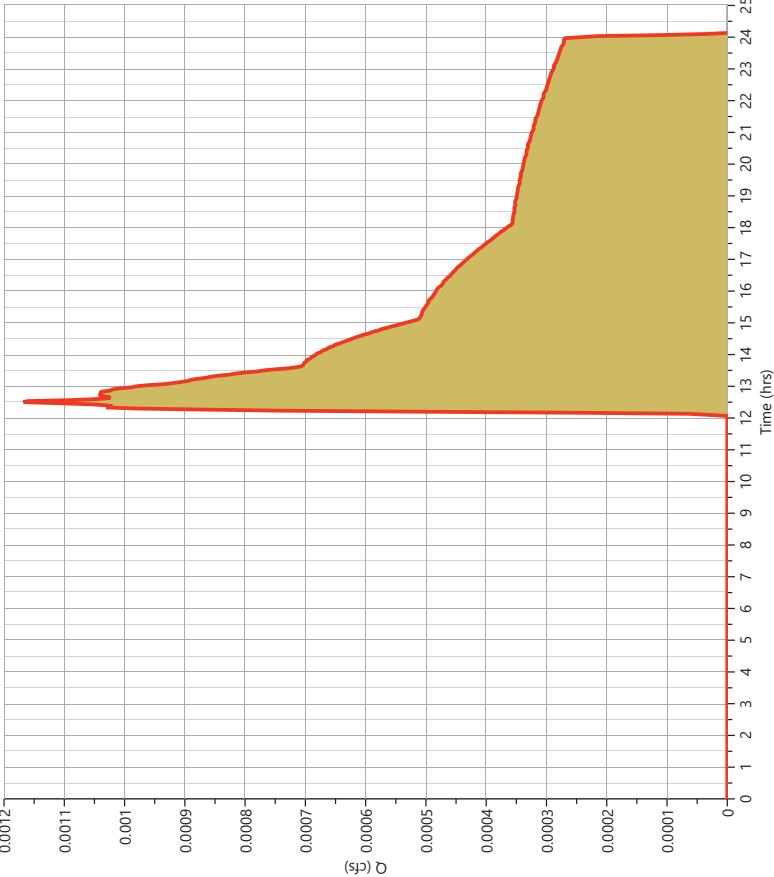
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.001 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 19.9 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

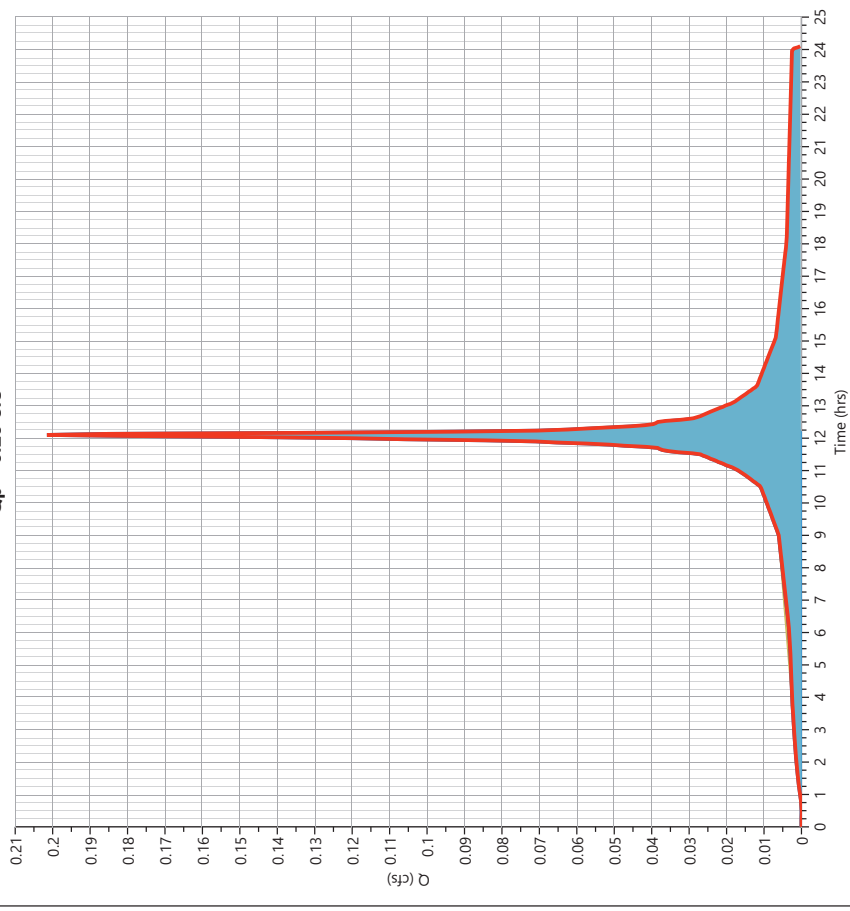
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.202 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 725 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.20 cfs



Hydrograph 25-yr Summary

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.234	12.13	947	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.010	12.27	100	----	----	----
3	NRCS Runoff	Post Roof Area DA3	2.203	12.10	7,767	----	----	----
4	NRCS Runoff	Post Deck DA3	1.183	12.17	5,371	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.806	12.17	3,662	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.016	12.13	93.2	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.566	12.10	1,997	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.024	12.13	140	----	----	----
9	NRCS Runoff	Post Roof Area DA2	3.147	12.10	11,096	----	----	----
10	Junction	Pre DA 1 - POA 2	0.240	12.13	1,047	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.034	12.30	282	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.376	12.17	1,709	----	----	----
13	Junction	Post Underground Basin 3	4.334	12.13	18,790	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.579	12.10	2,090	6, 7	----	----
15	Junction	Post Basin 2	3.166	12.10	11,236	8, 9	----	----
16	Pond Route	Post Basin 2	2.093	12.17	4,890	15	14.66	1,315
17	Pond Route	Post Basin 1	0.000	13.87	0.000	14	14.65	769
18	Junction	Post COMBINED TO UG BASIN	6.390	12.13	23,680	13, 16	----	----
19	Pond Route	Post UG Basin 3	0.413	12.67	1,929	18	13.29	7,673
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.430	12.17	1,953	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.120	12.37	1,574	----	----	----
22	Junction	Pre DA 2	0.486	12.20	3,527	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.140	12.33	1,549	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.588	12.20	5,077	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.189	12.10	666	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.032	12.13	186	----	----	----
27	Junction	COMB. INLET N	0.214	12.10	852	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	0.449	12.63	2,781	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.009	12.30	76.9	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.021	12.30	179	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.252	12.10	888	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.008	12.13	46.6	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.258	12.10	934	31, 32	----	----

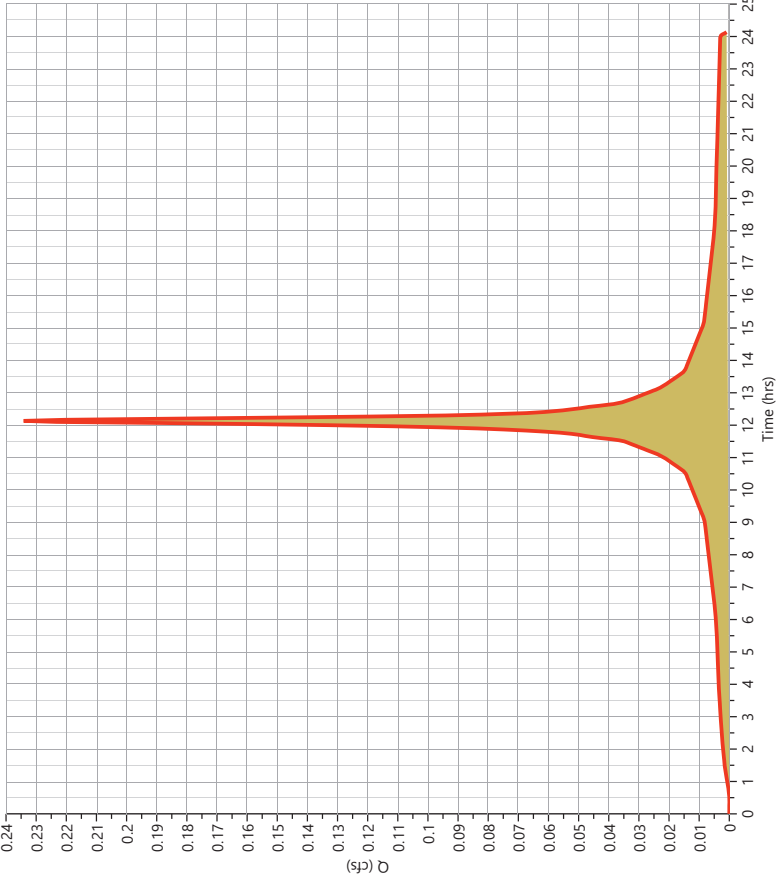
Hydrograph Report

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.234 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 947 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.23 cfs



Hydrograph Report

Pre DA 1 - PERVIOUS

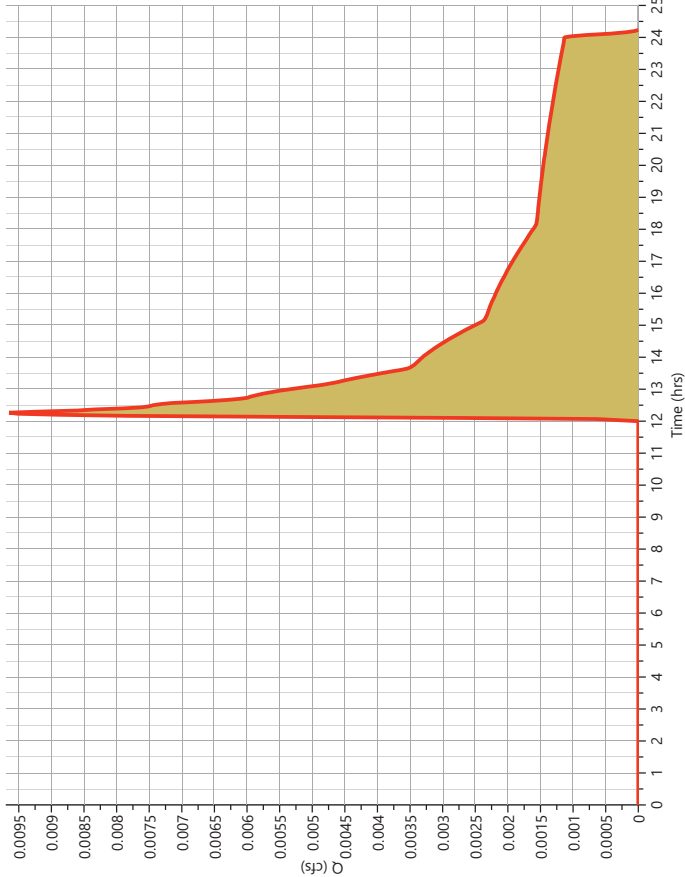
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.010 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.27 hrs
Time Interval	= 2 min	Runoff Volume	= 100 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.04	39	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.01 cfs



Hydrograph Report

Project Name:
11-22-2022

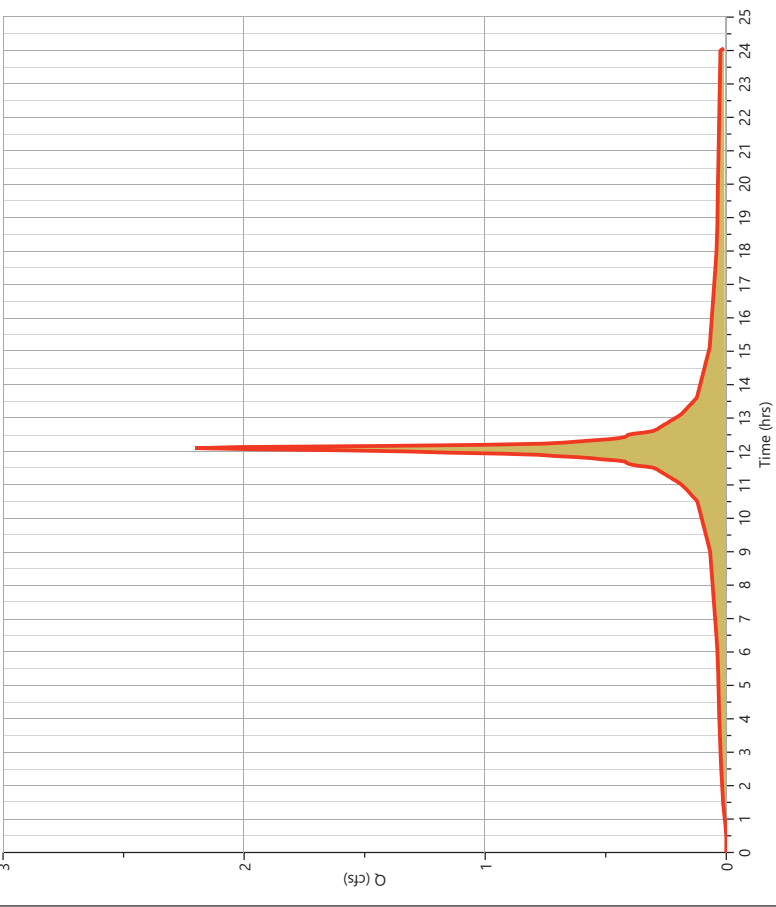
Hydrology Studio v 3.0.0.26

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.203 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 7,767 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 2.20 cfs



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.183 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 5,371 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.18 cfs



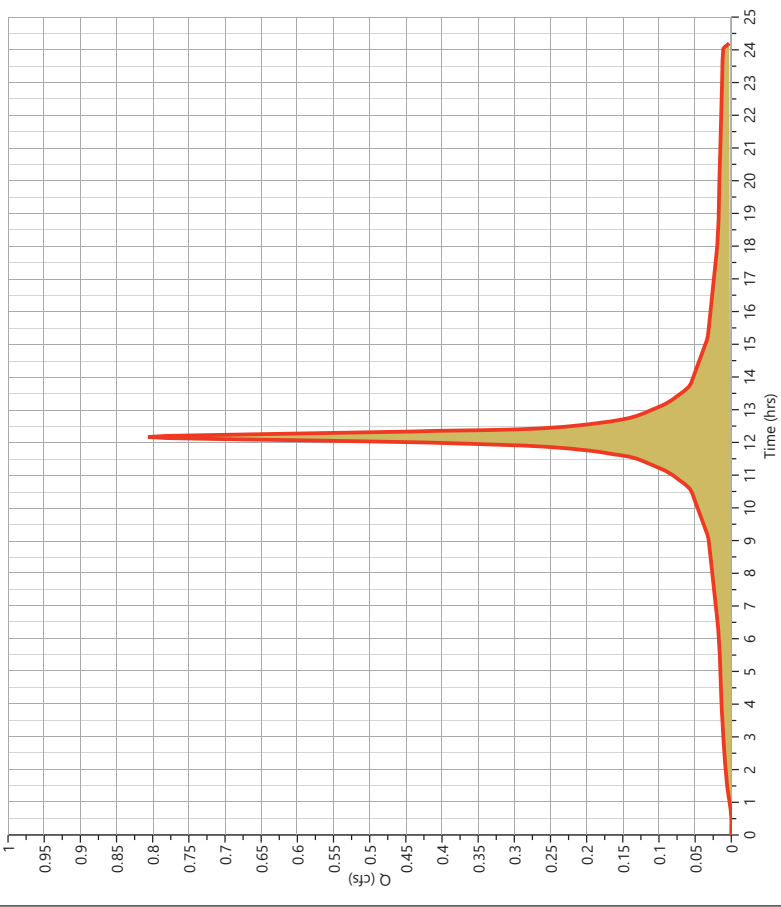
Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.806 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 3.662 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.81 cfs



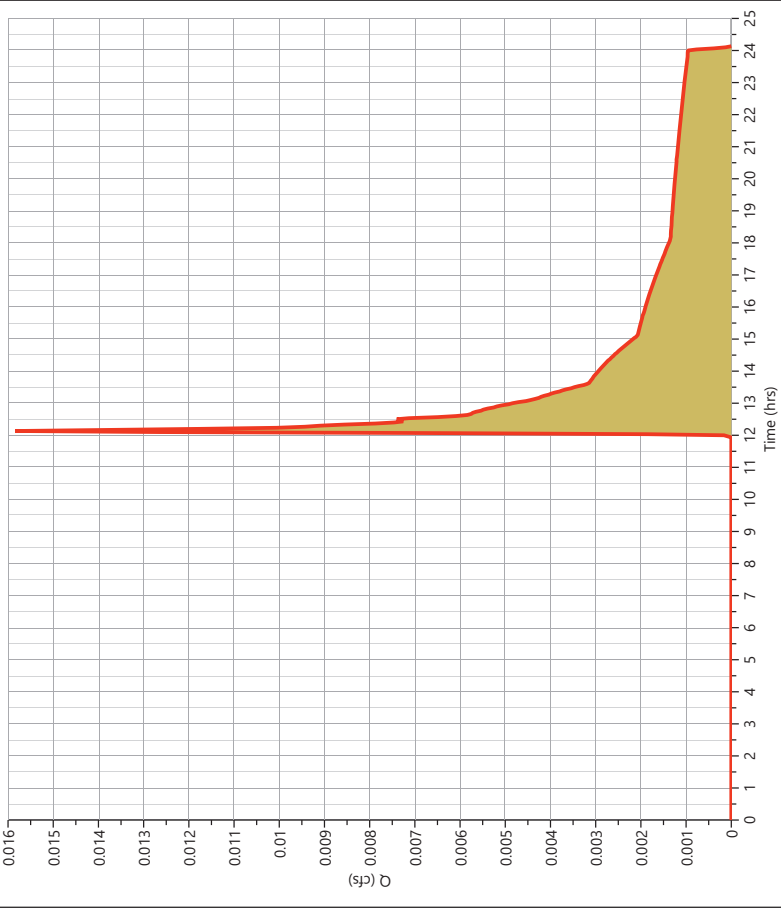
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.016 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 93.2 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.02 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

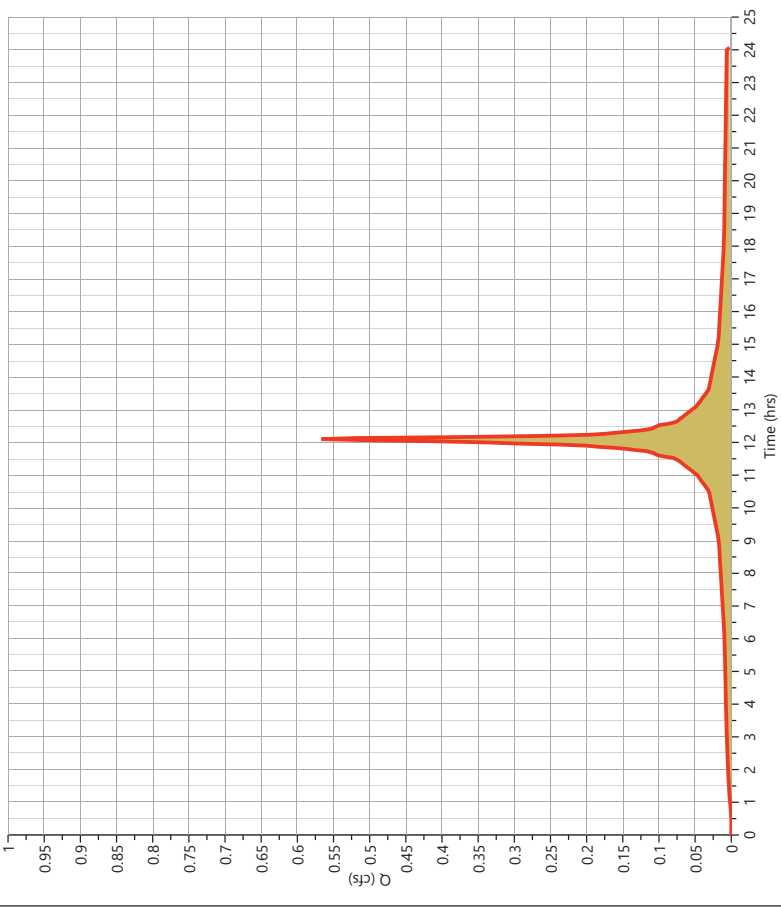
11-22-2022

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.566 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1.997 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.57 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

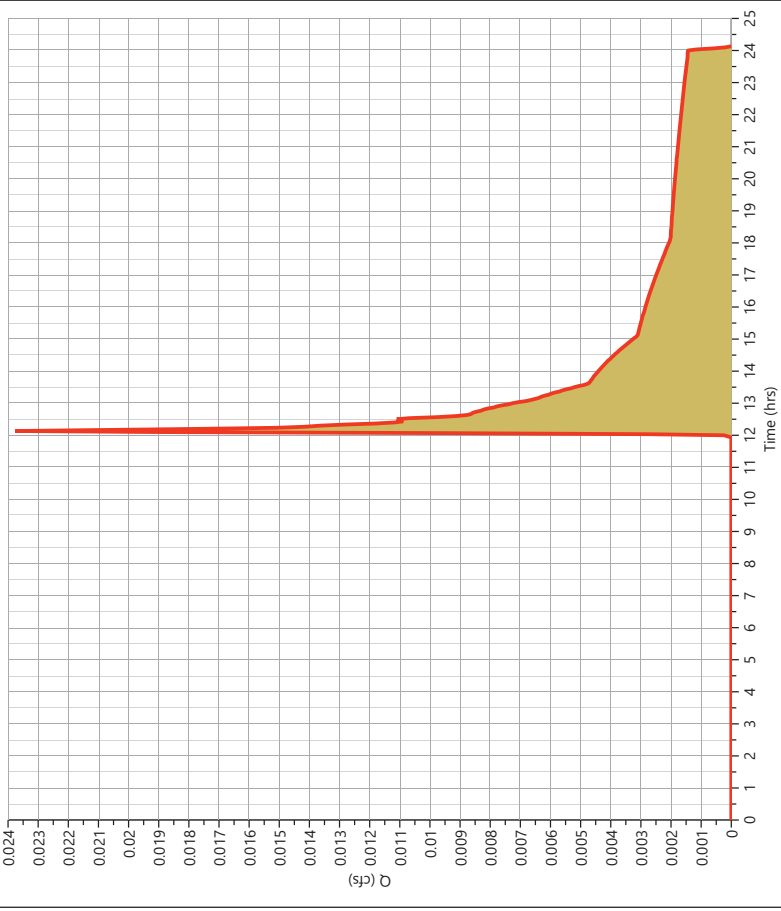
11-22-2022

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.024 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 140 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.02 cfs



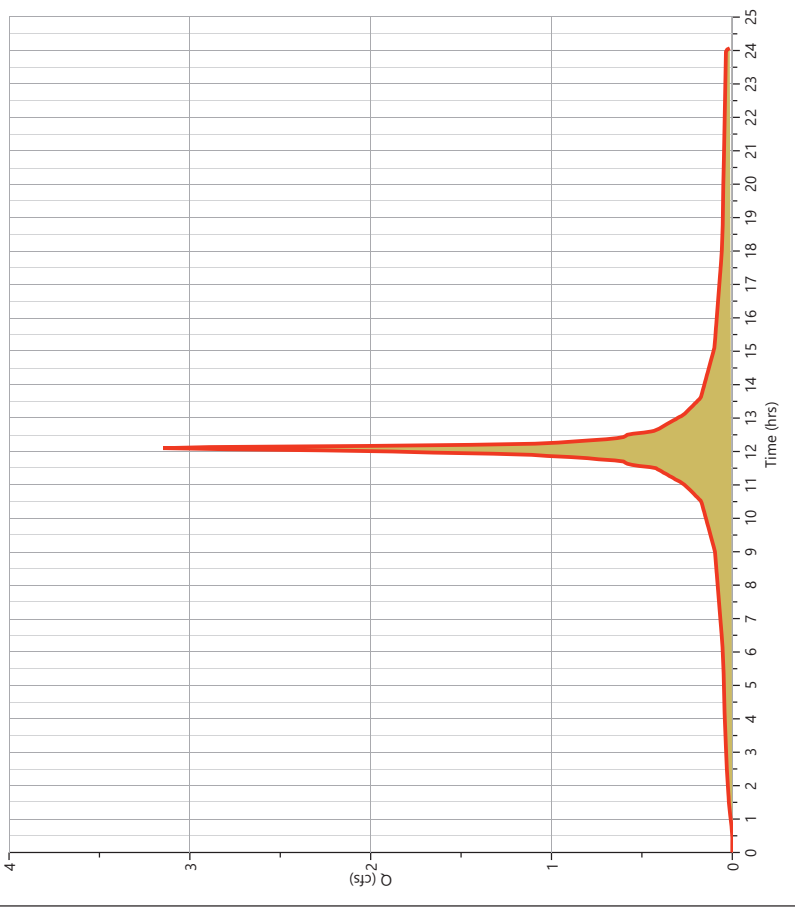
Hydrograph Report

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.147 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 11,096 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 3.15 cfs



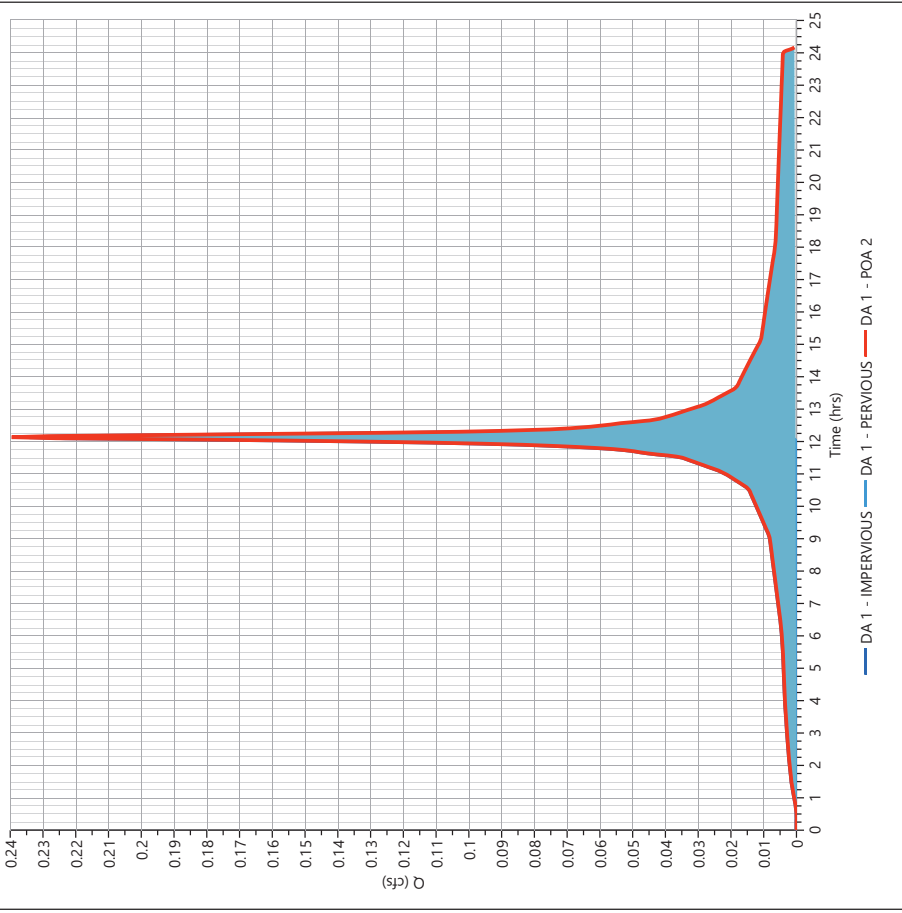
Hydrograph Report

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.240 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,047 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.24 cfs



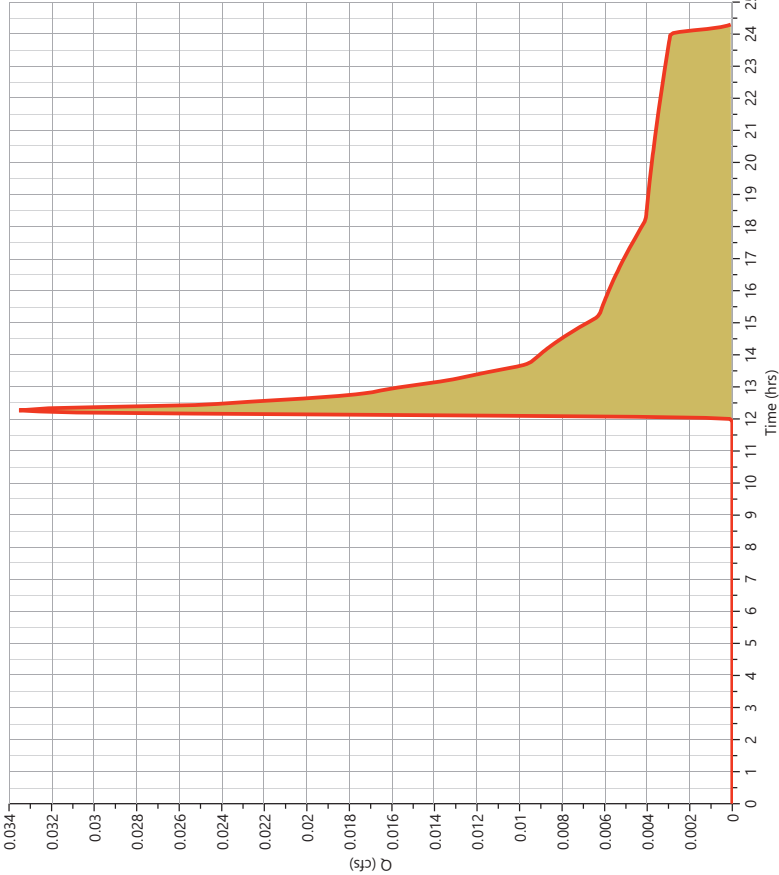
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.034 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.30 hrs
Time Interval	= 2 min	Runoff Volume	= 282 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.03 cfs



Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.376 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,709 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.38 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

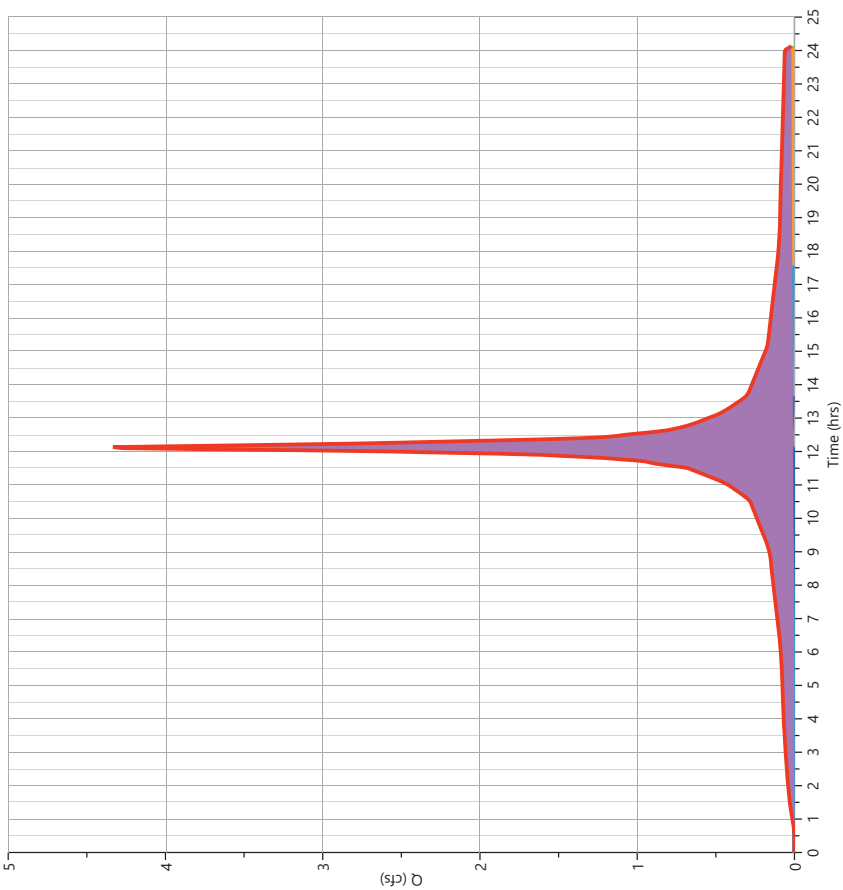
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 4.334 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 18,790 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 4.33 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

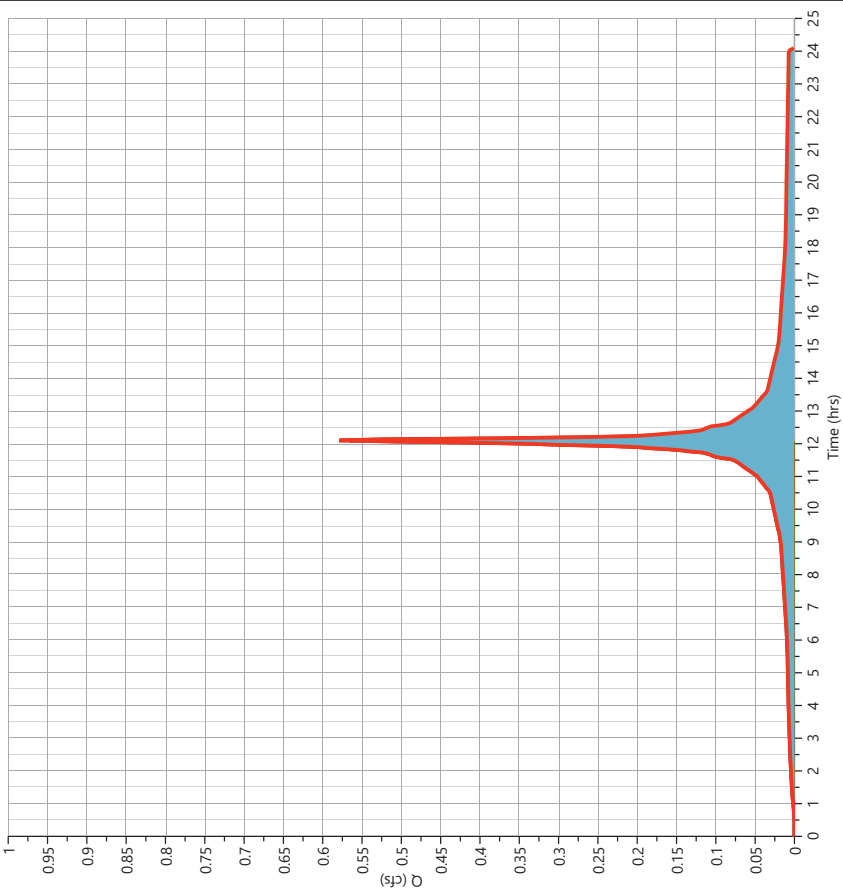
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.579 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,090 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.58 cfs



Hydrograph Report

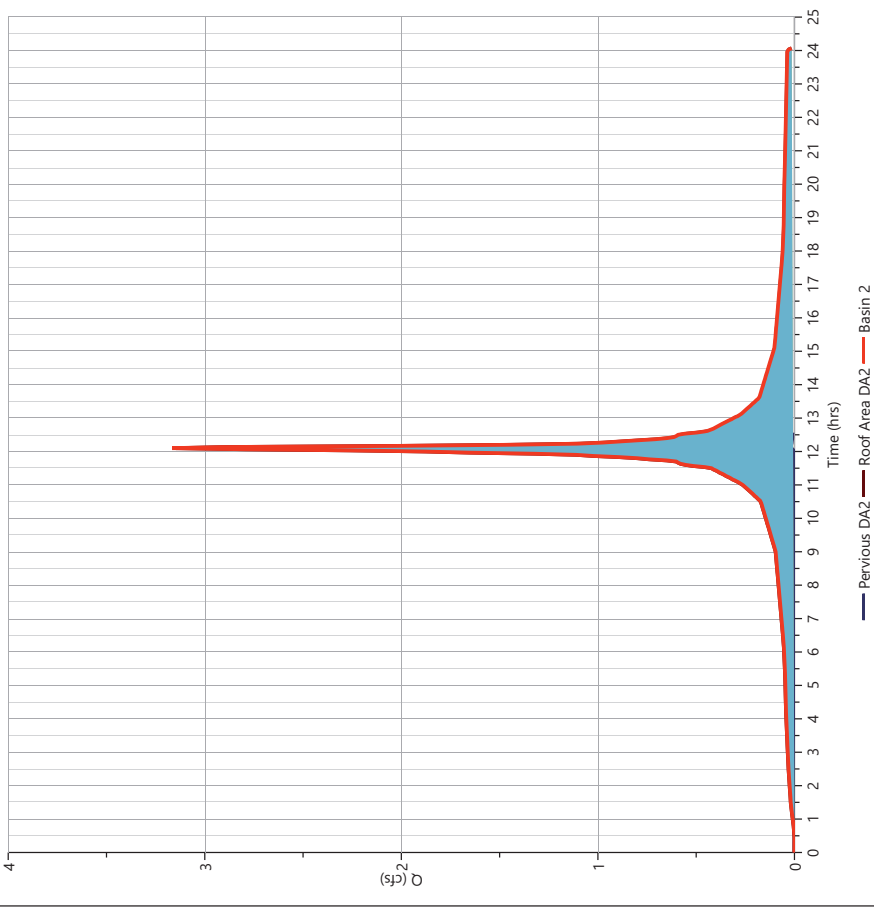
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 3.166 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 11,236 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 3.17 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

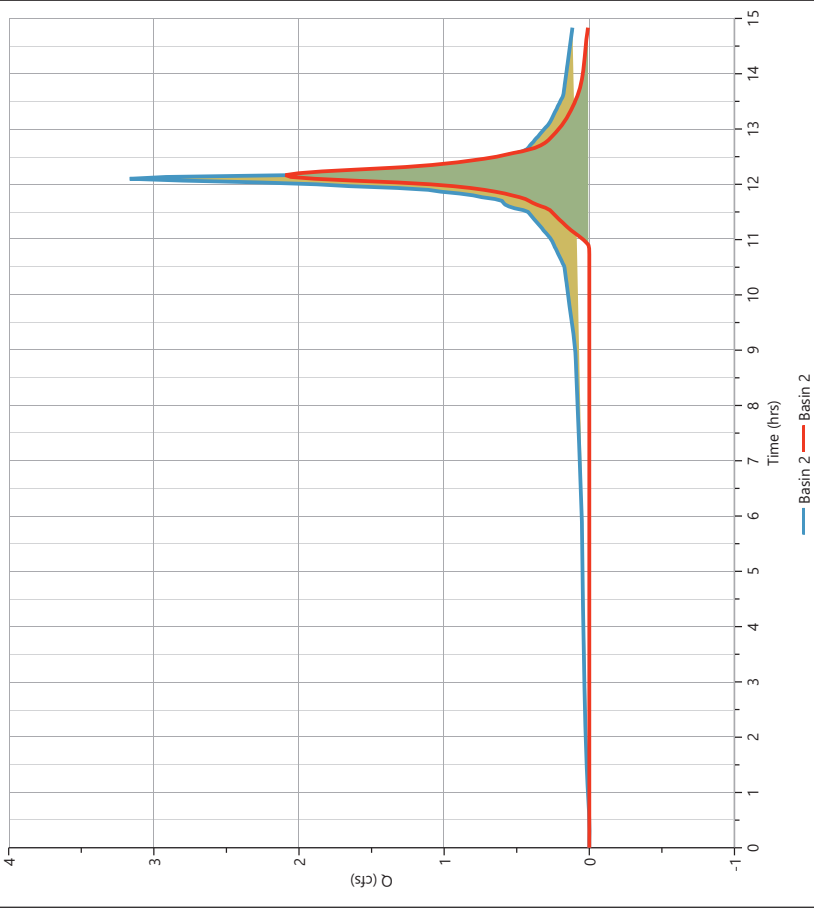
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 2.093 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 4,890 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.66 ft
Pond Name	= Basin 2	Max. Storage	= 1,315 cuft

Pond Routing by Storage Indication Method

Qp = 2.09 cfs



Hydrograph Report

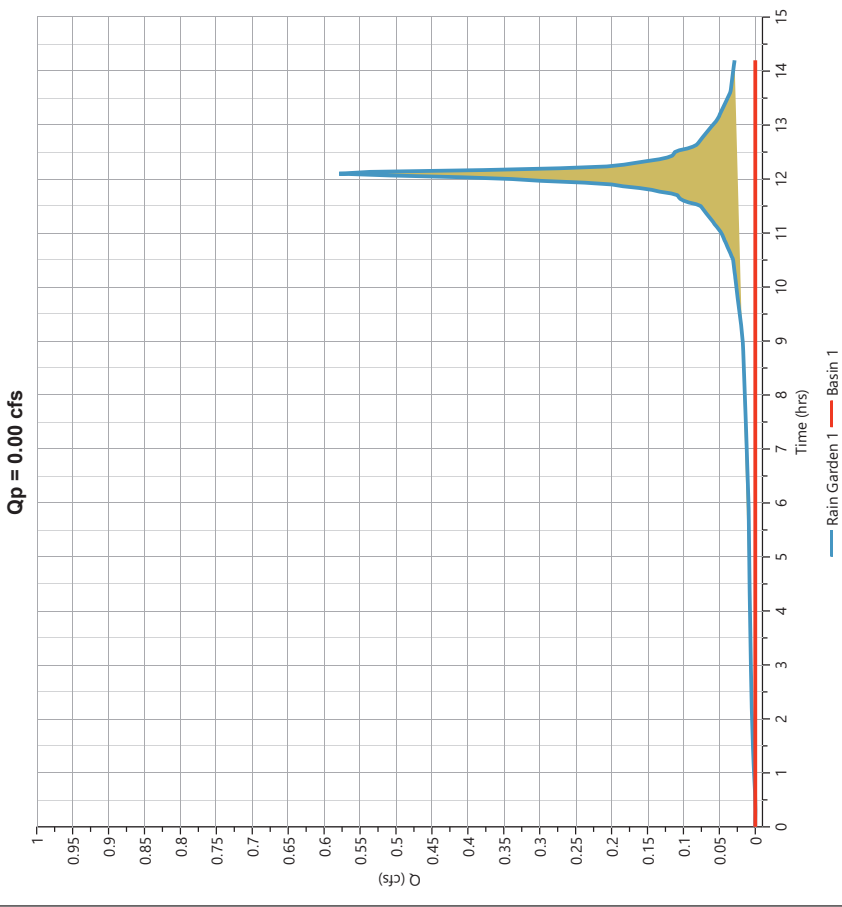
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 1

Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.000 cfs
Storm Frequency	= 25-yr	Time to Peak	= 13.87 hrs
Time Interval	= 2 min	Hydrograph Volume	= 0.000 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.65 ft
Pond Name	= BASIN 1	Max. Storage	= 769 cuft

Pond Routing by Storage Indication Method
Center of mass detention time = 43 min



Hydrograph Report

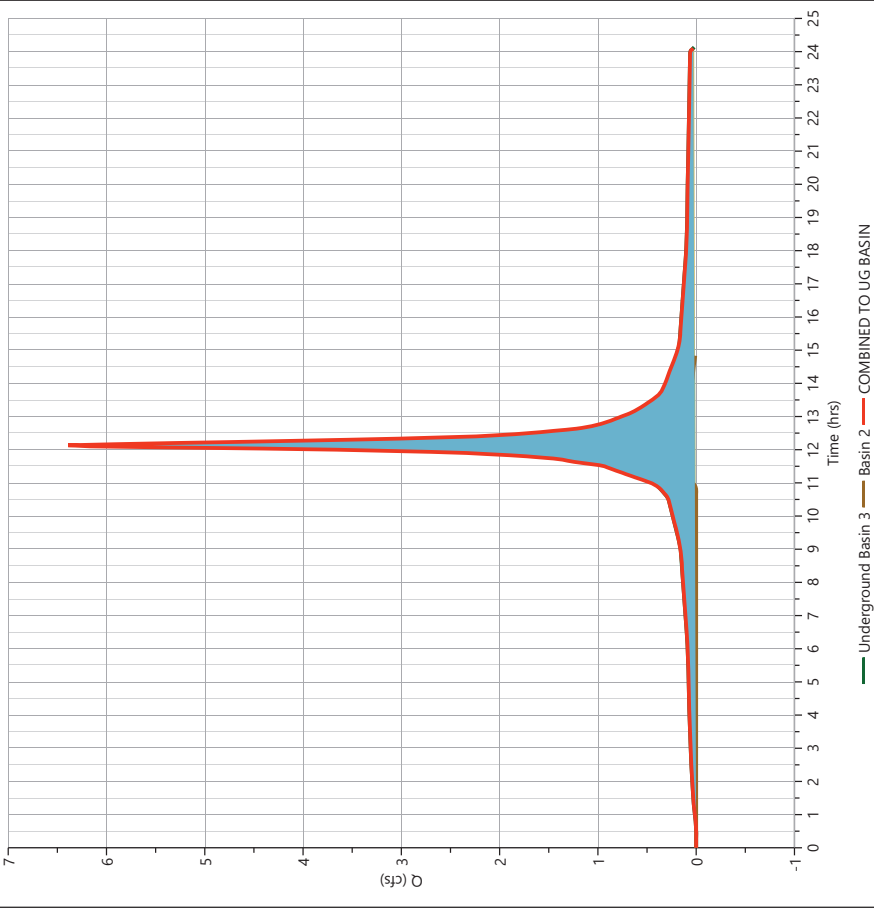
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 6.390 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 23,680 cuft
Inflow Hydrographs	= 13, 16	Total Contrib. Area	= 0.9 ac

Qp = 6.39 cfs



Hydrograph Report

Post UG Basin 3

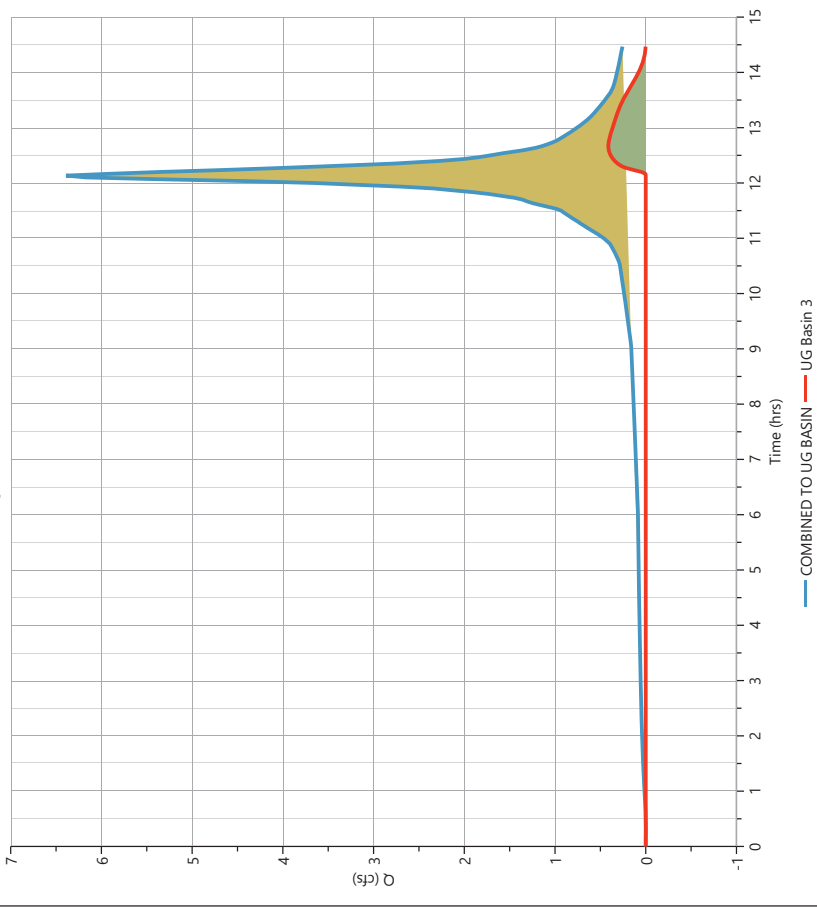
Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.413 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.67 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,929 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 13.29 ft
Pond Name	= UG BASIN	Max. Storage	= 7,673 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 38 min

Qp = 0.41 cfs



Hydrograph Report

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.430 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,953 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.43 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

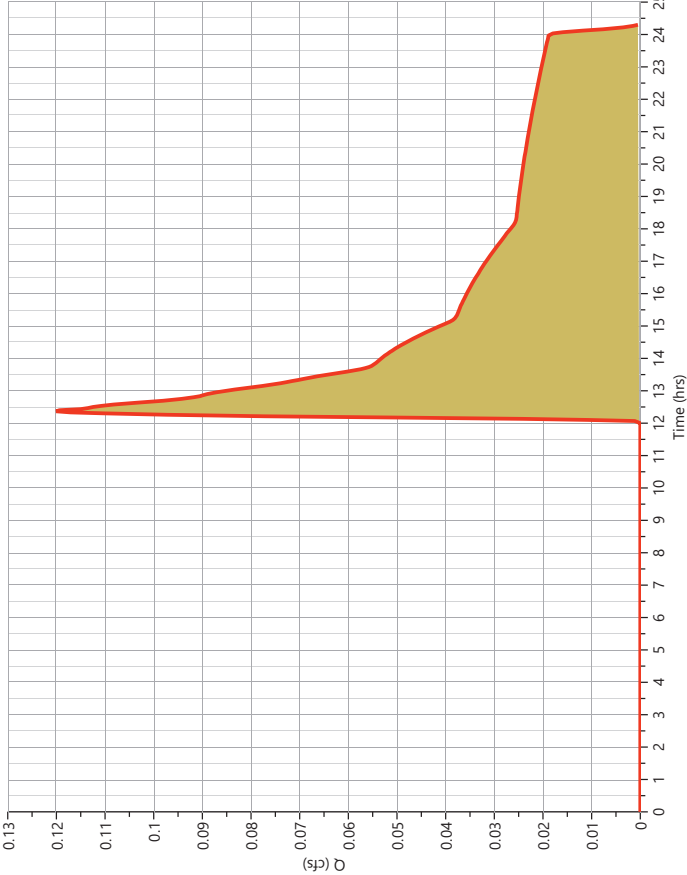
Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.120 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.37 hrs
Time Interval	= 2 min	Runoff Volume	= 1,574 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.58	39	OPEN SPACE
0.28	30	WOODS
0.86	36	Weighted CN Method Employed

Qp = 0.12 cfs



Hydrograph Report

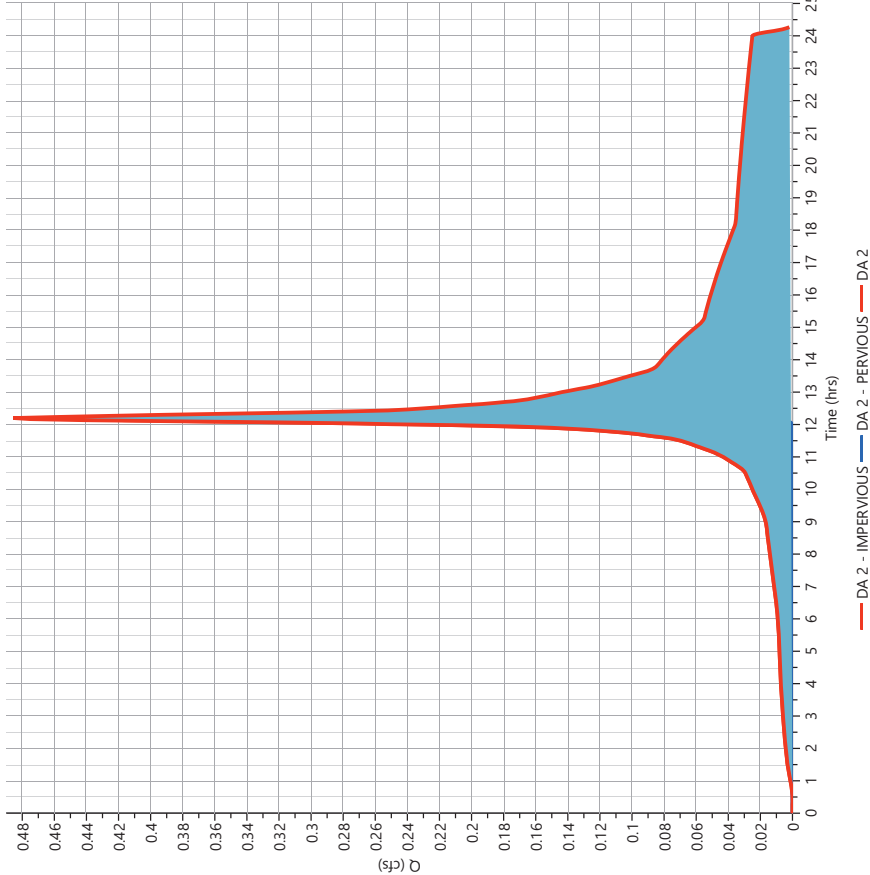
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.486 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 3,527 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.49 cfs



Hydrograph Report

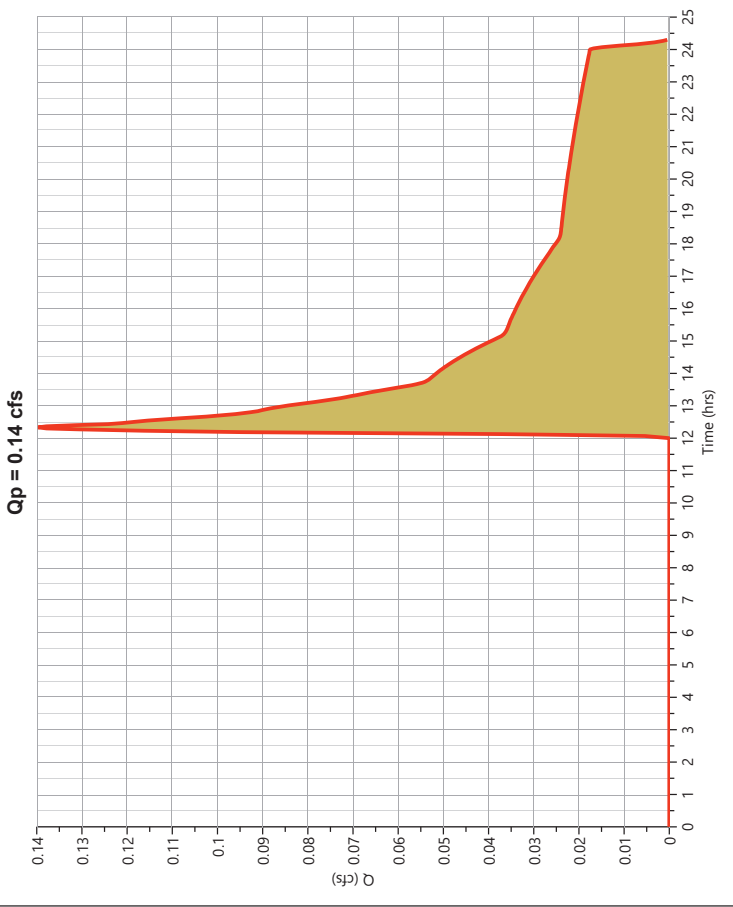
Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.140 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.33 hrs
Time Interval	= 2 min	Runoff Volume	= 1,549 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	38	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	



Hydrograph Report

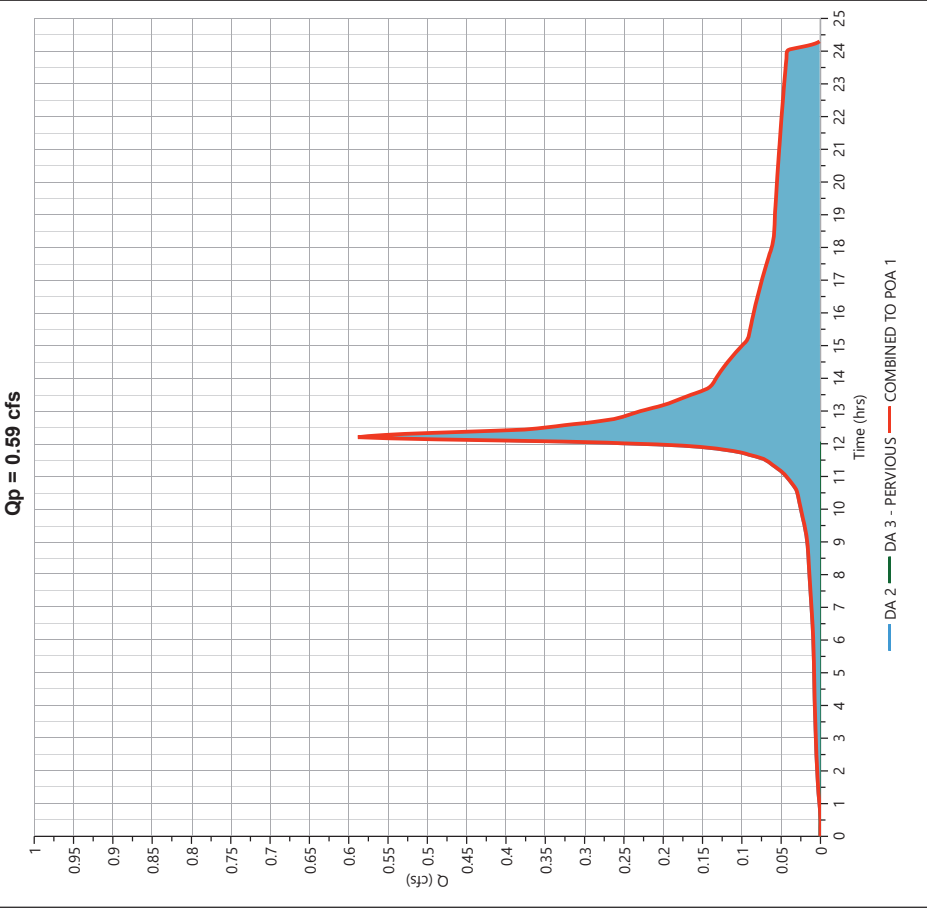
Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.588 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 5,077 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac



Hydrograph Report

Project Name:
11-22-2022

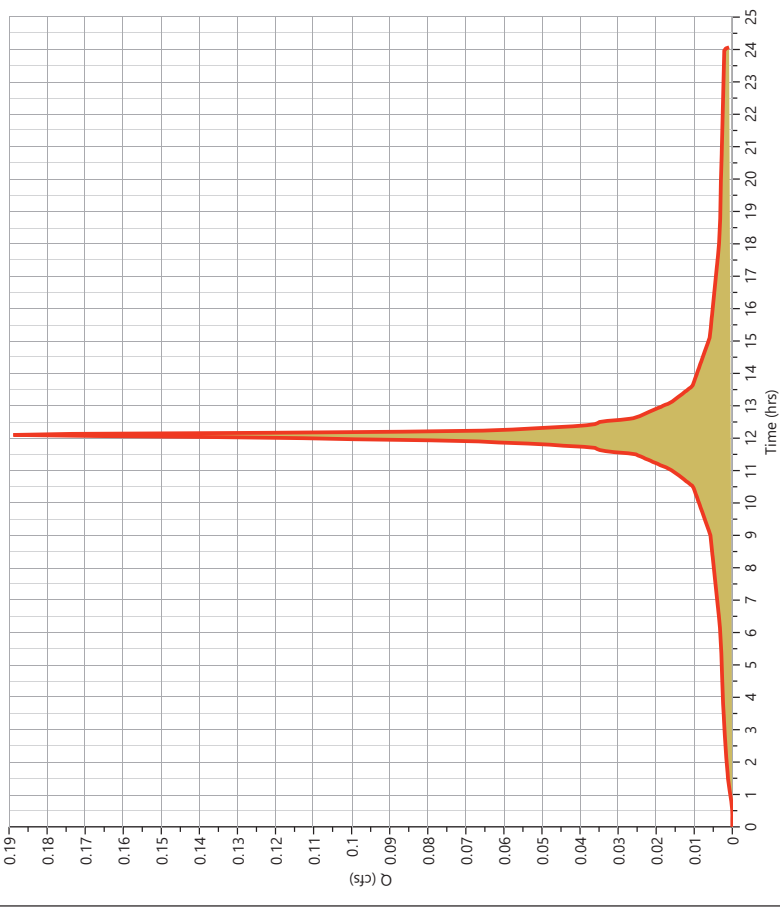
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.189 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 666 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.19 cfs



Hydrograph Report

Project Name:
11-22-2022

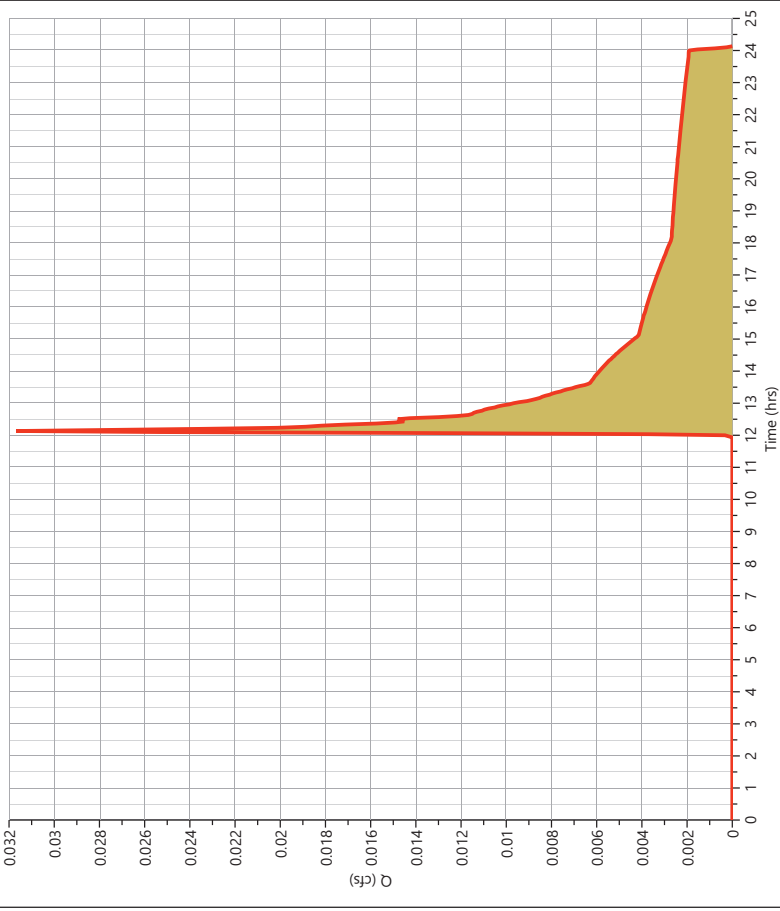
Hydrology Studio v 3.0.0.26

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.032 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 186 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.03 cfs



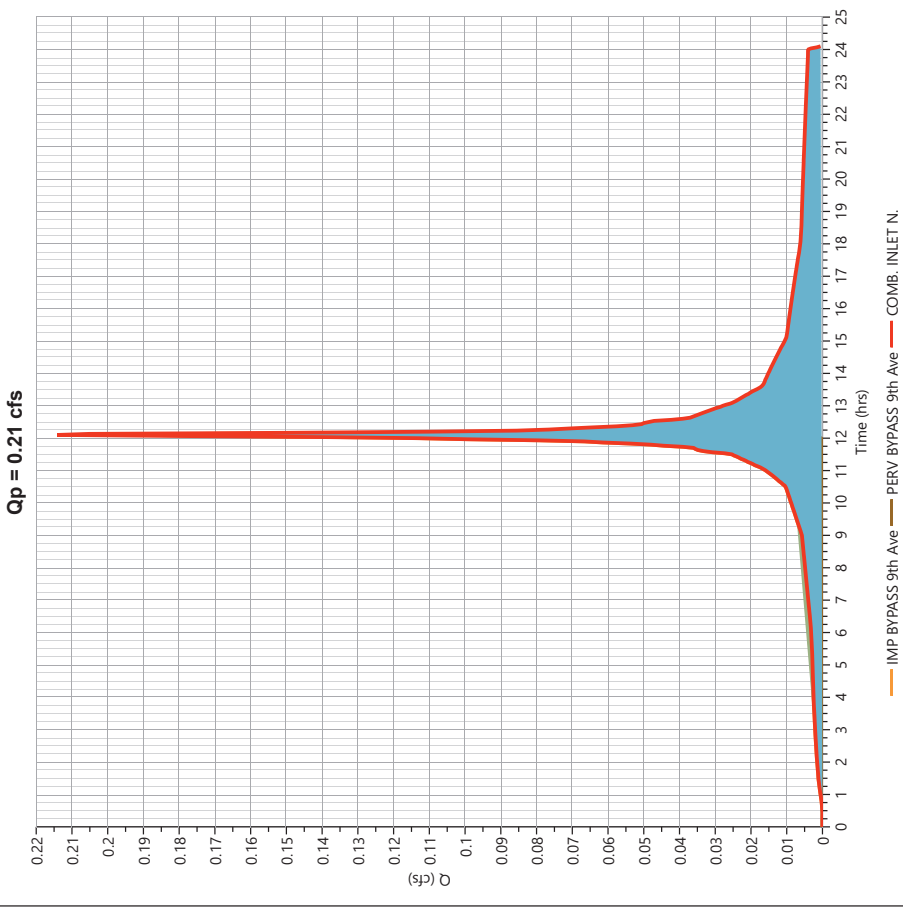
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.214 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 852 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



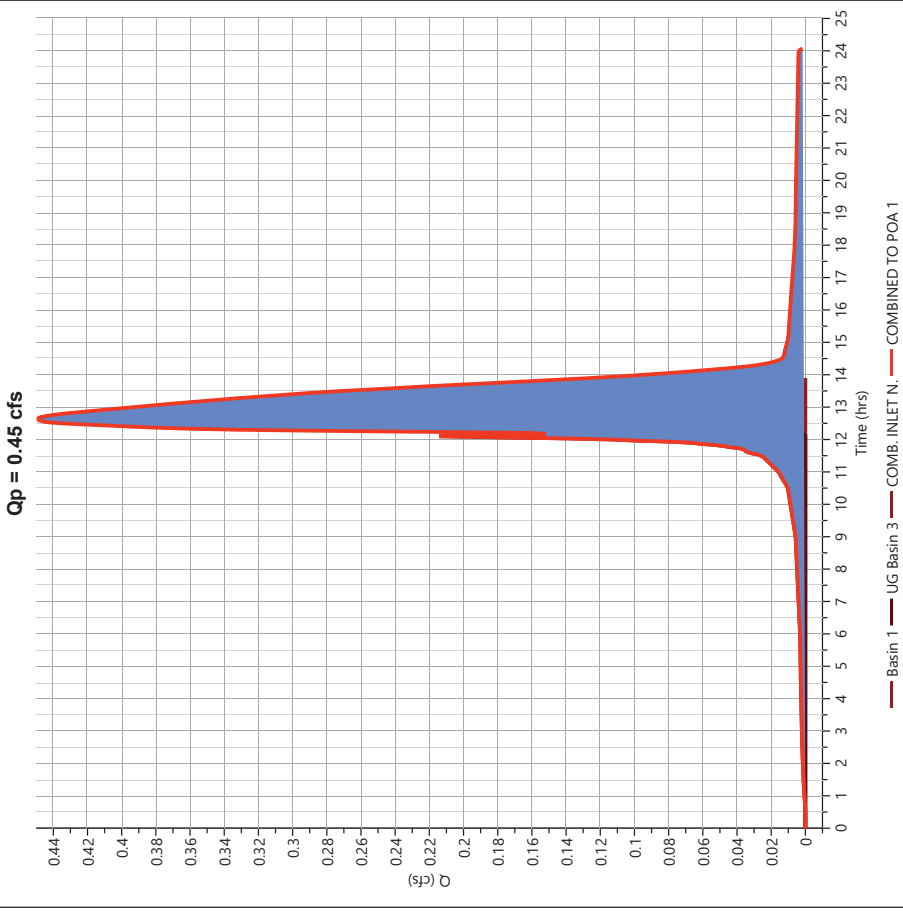
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 0.449 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.63 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,781 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



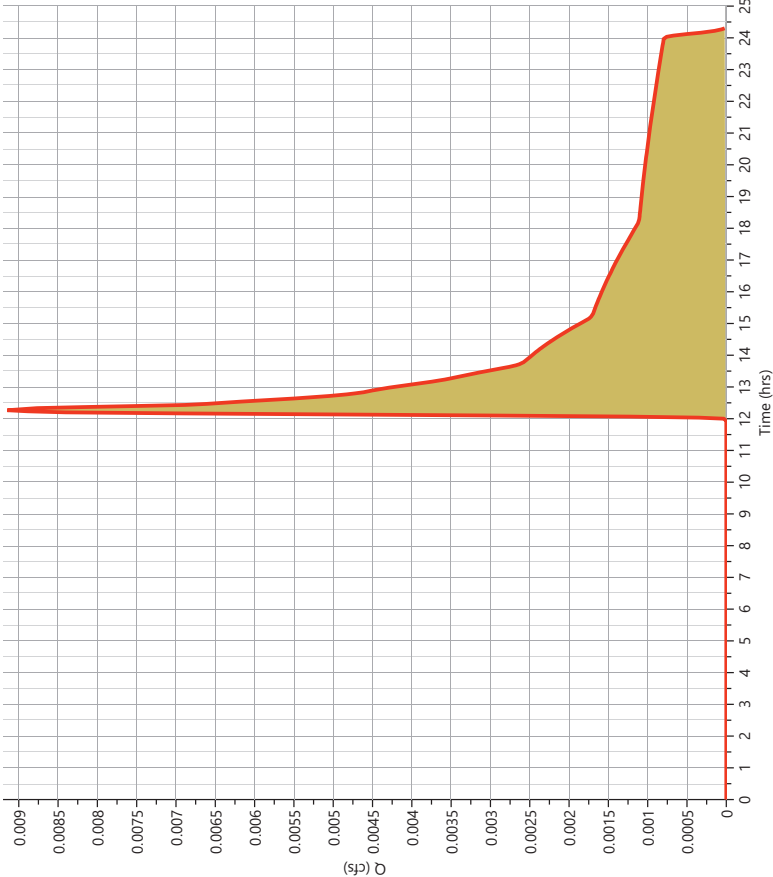
Hydrograph Report

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.009 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.30 hrs
Time Interval	= 2 min	Runoff Volume	= 76.9 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.01 cfs



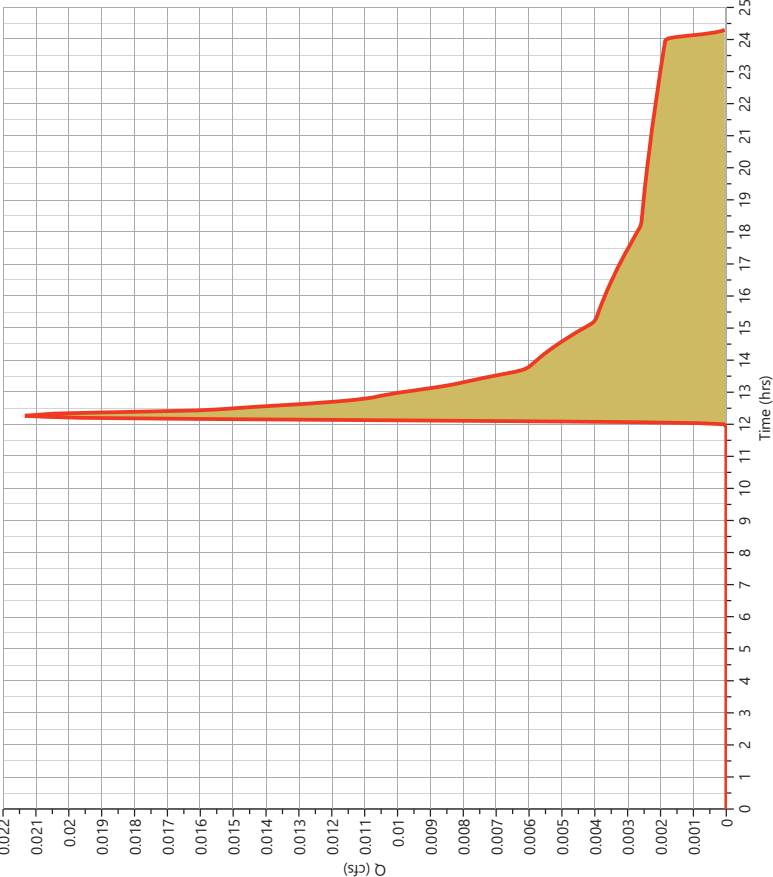
Hydrograph Report

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.021 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.30 hrs
Time Interval	= 2 min	Runoff Volume	= 179 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.02 cfs



Hydrograph Report

Project Name:
11-22-2022

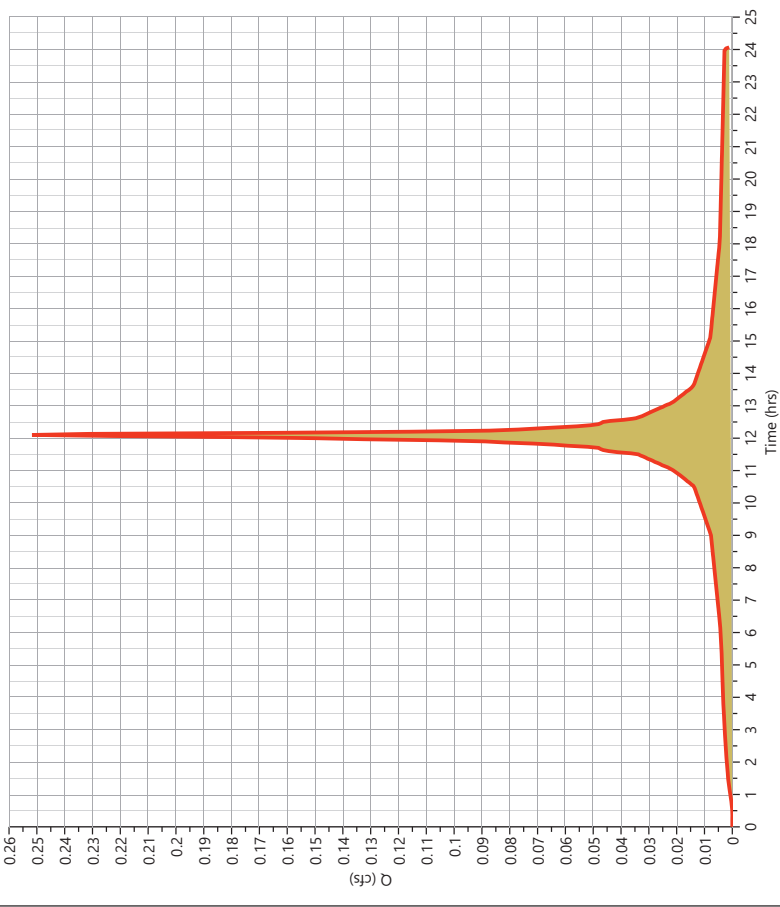
Hydrology Studio v 3.0.0.26

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.252 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 888 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.25 cfs



Hydrograph Report

Project Name:
11-22-2022

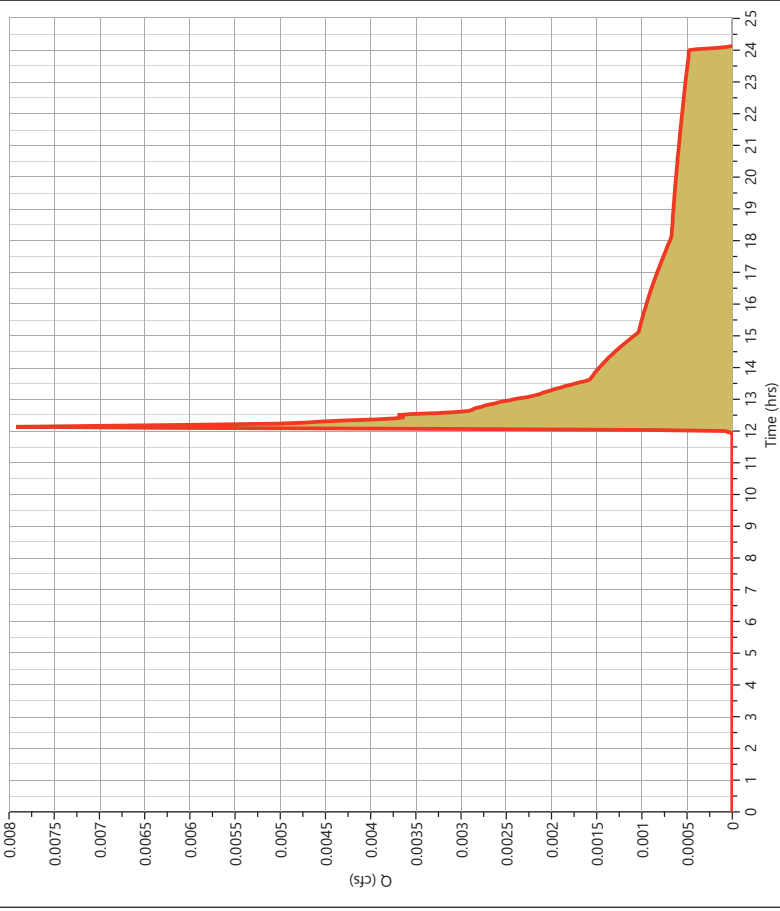
Hydrology Studio v 3.0.0.26

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.008 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 46.6 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 6.76 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.01 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

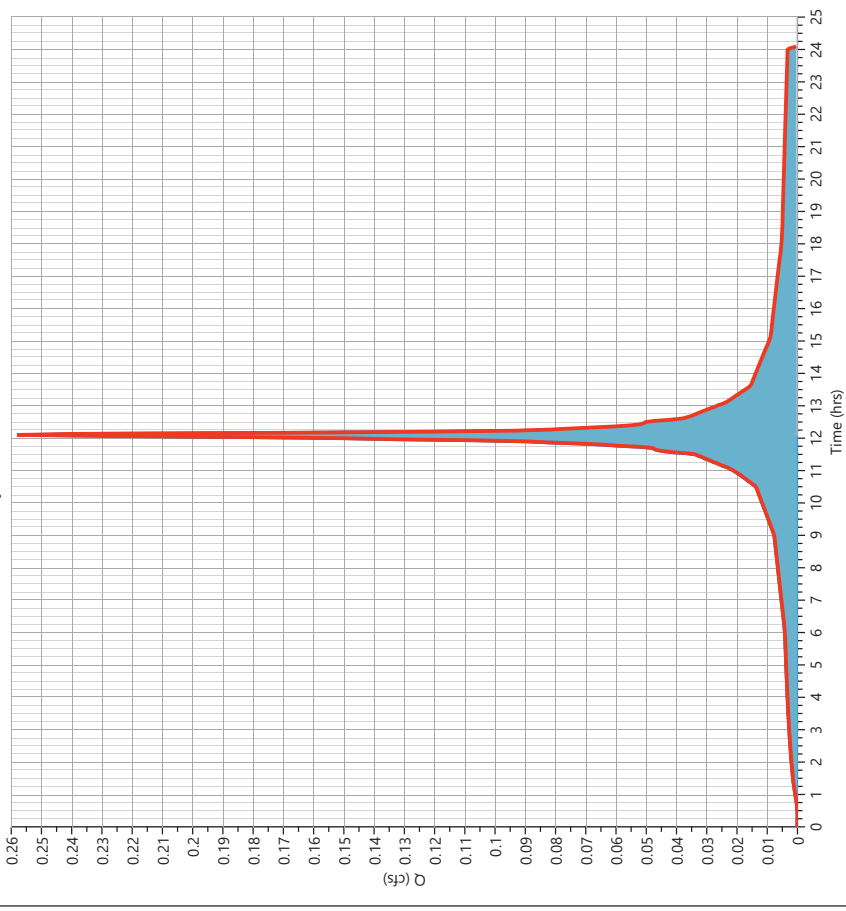
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.258 cfs
Storm Frequency	= 25-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 934 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.26 cfs



Hydrograph 100-yr Summary

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.322	12.13	1,311	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.060	12.17	273	----	----	----
3	NRCS Runoff	Post Roof Area DA3	3.025	12.10	10,755	----	----	----
4	NRCS Runoff	Post Deck DA3	1.624	12.17	7,436	----	----	----
5	NRCS Runoff	Post Pervious Pavement	1.108	12.17	5,070	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.066	12.13	236	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.778	12.10	2,766	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.099	12.13	354	----	----	----
9	NRCS Runoff	Post Roof Area DA2	4.321	12.10	15,364	----	----	----
10	Junction	Pre DA 1 - POA 2	0.379	12.13	1,584	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.149	12.20	713	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.517	12.17	2,366	----	----	----
13	Junction	Post Underground Basin 3	6.052	12.13	26,341	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.843	12.10	3,001	6, 7	----	----
15	Junction	Post Basin 2	4.419	12.10	15,718	8, 9	----	----
16	Pond Route	Post Basin 2	3.234	12.17	7,926	15	14.97	1,679
17	Pond Route	Post Basin 1	0.052	12.53	159	14	14.94	1,096
18	Junction	Post COMBINED TO UG BASIN 9 285	34.267	12.13	34,267	13, 16	----	----
19	Pond Route	Post UG Basin 3	1.432	12.57	7,206	18	14.14	11,104
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.591	12.17	2,704	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.810	12.20	4,475	----	----	----
22	Junction	Pre DA 2	1.381	12.20	7,179	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.509	12.20	4,218	----	----	----
24	Junction	Pre COMBINED TO POA 1	2.190	12.20	11,397	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.259	12.10	922	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.132	12.13	471	----	----	----
27	Junction	COMB. INLET N	0.390	12.10	1,393	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	1.564	12.53	8,759	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.041	12.20	194	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.095	12.20	454	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.346	12.10	1,229	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.033	12.13	118	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.378	12.10	1,347	31, 32	----	----

Hydrograph Report

Project Name:

11-22-2022

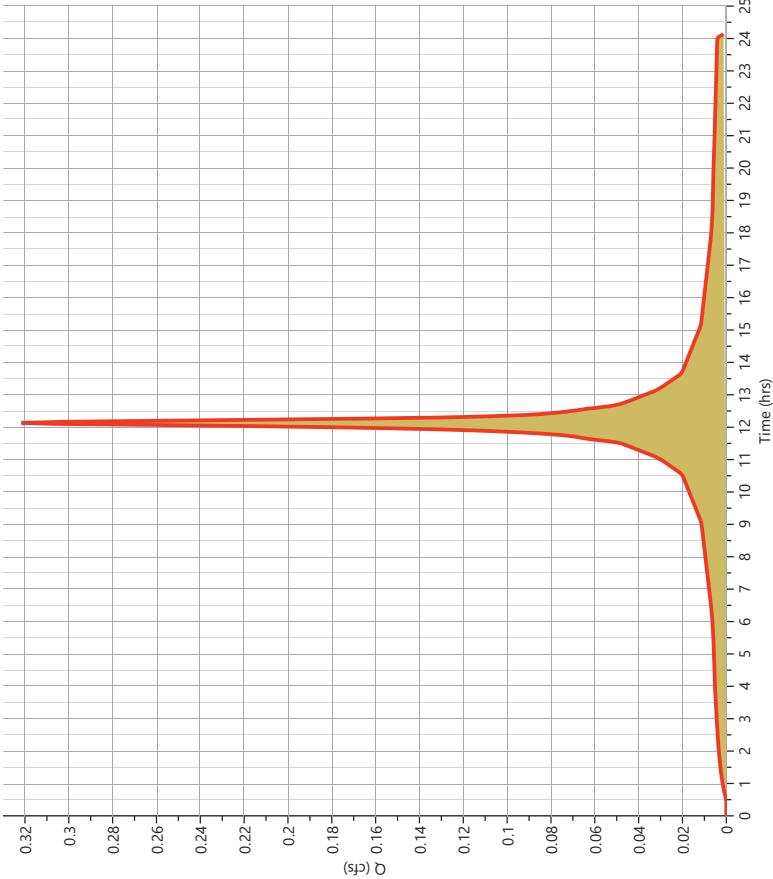
Hydrology Studio v 3.0.0.26

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.322 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 1,311 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.32 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

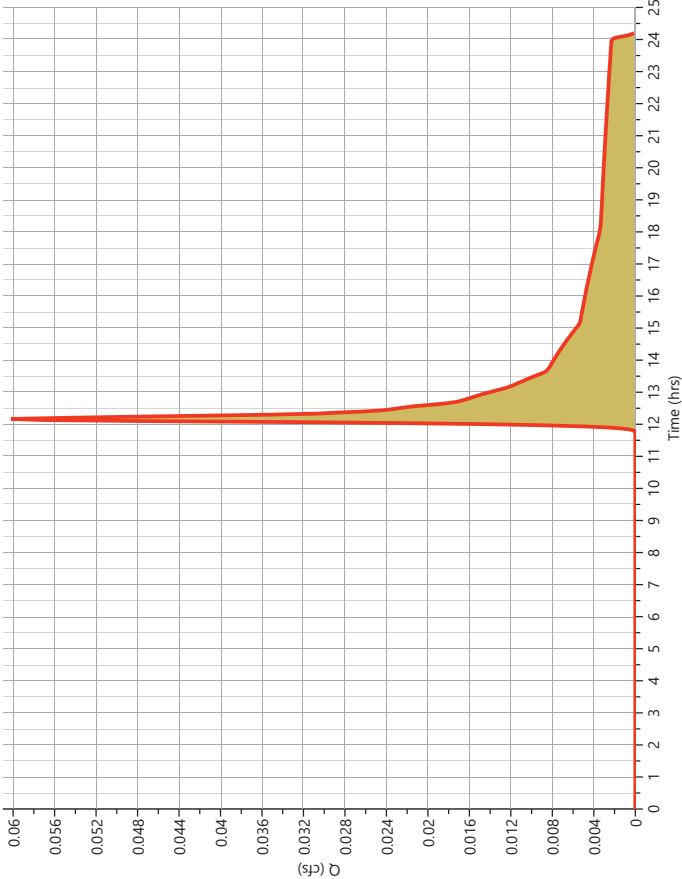
Pre DA 1 - PERVIOUS

Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.060 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 273 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet		
AREA (ac)	CN	DESCRIPTION
0.04	98	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.06 cfs



Hydrograph Report

Project Name:
11-22-2022

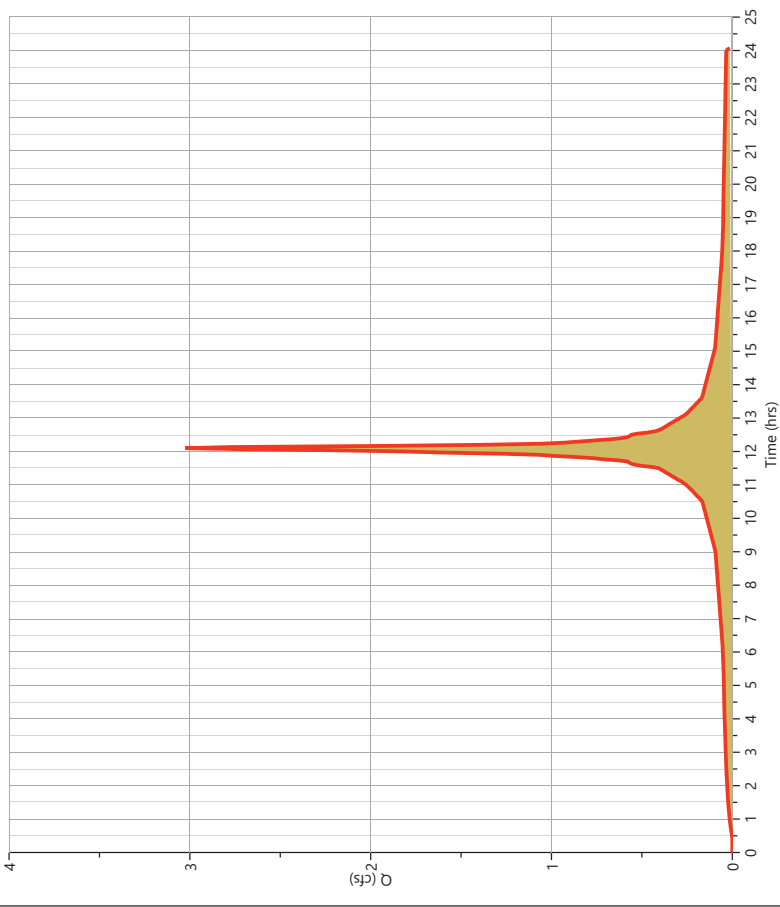
Hydrology Studio v 3.0.0.26

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.025 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 10,755 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 3.03 cfs



Hydrograph Report

Project Name:
11-22-2022

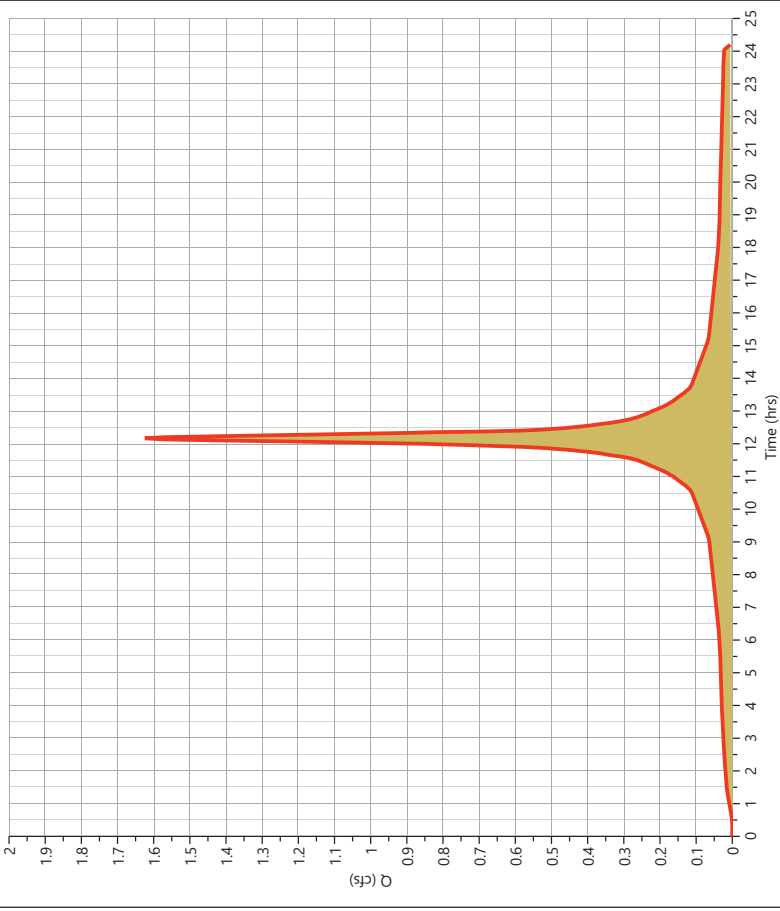
Hydrology Studio v 3.0.0.26

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.624 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 7,436 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.62 cfs



Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1,108 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 5,070 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.11 cfs



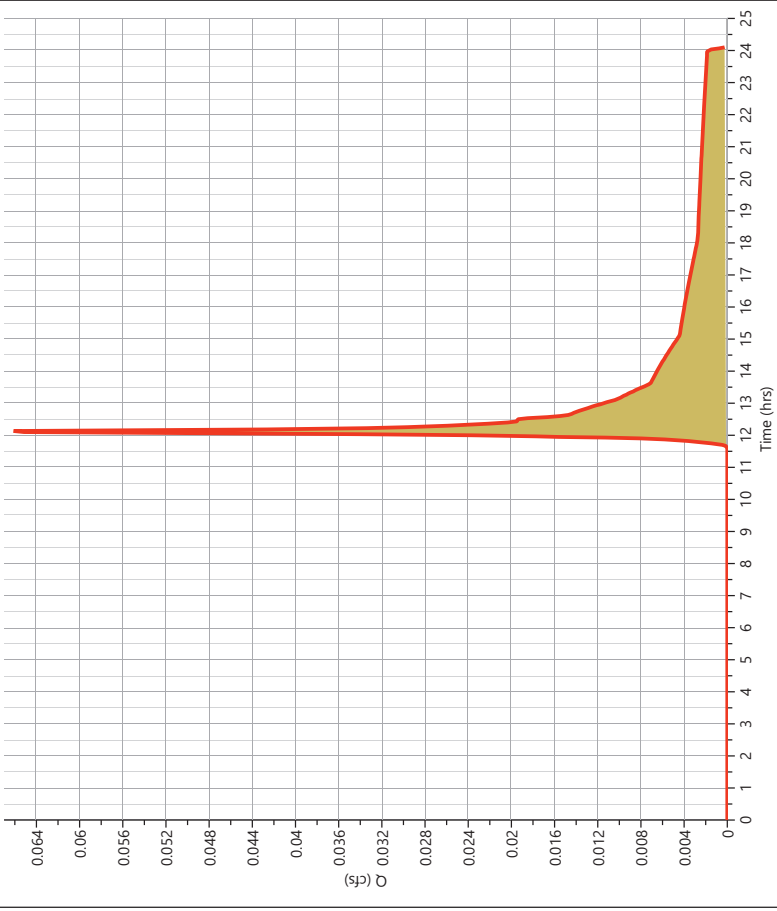
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.066 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 236 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.07 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

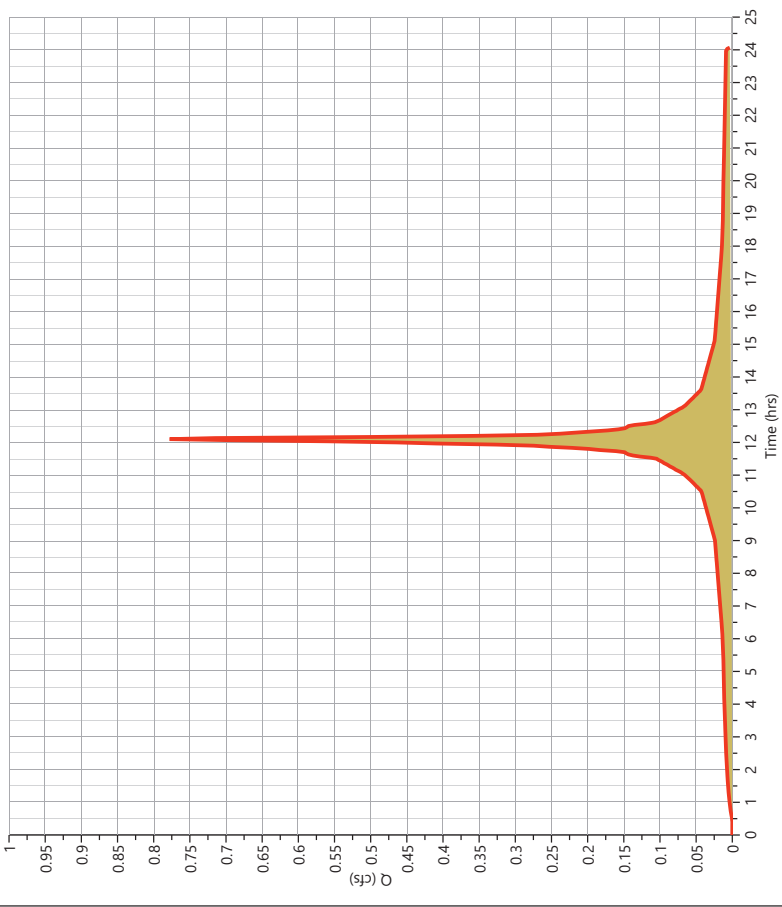
11-22-2022

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.778 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 2,766 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.78 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

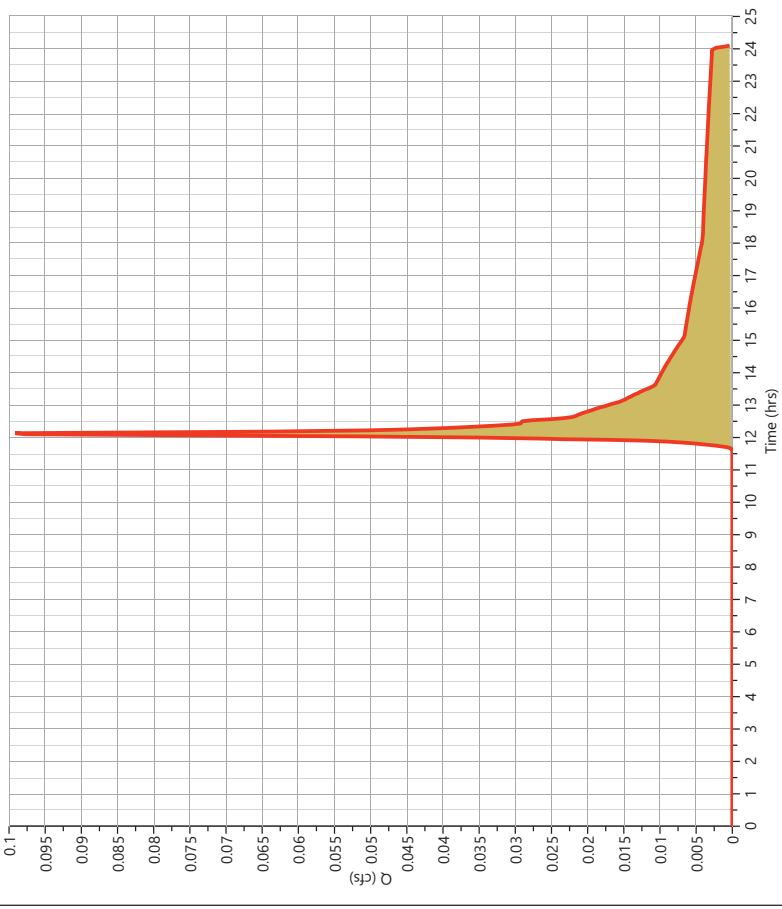
11-22-2022

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.099 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 354 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.10 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

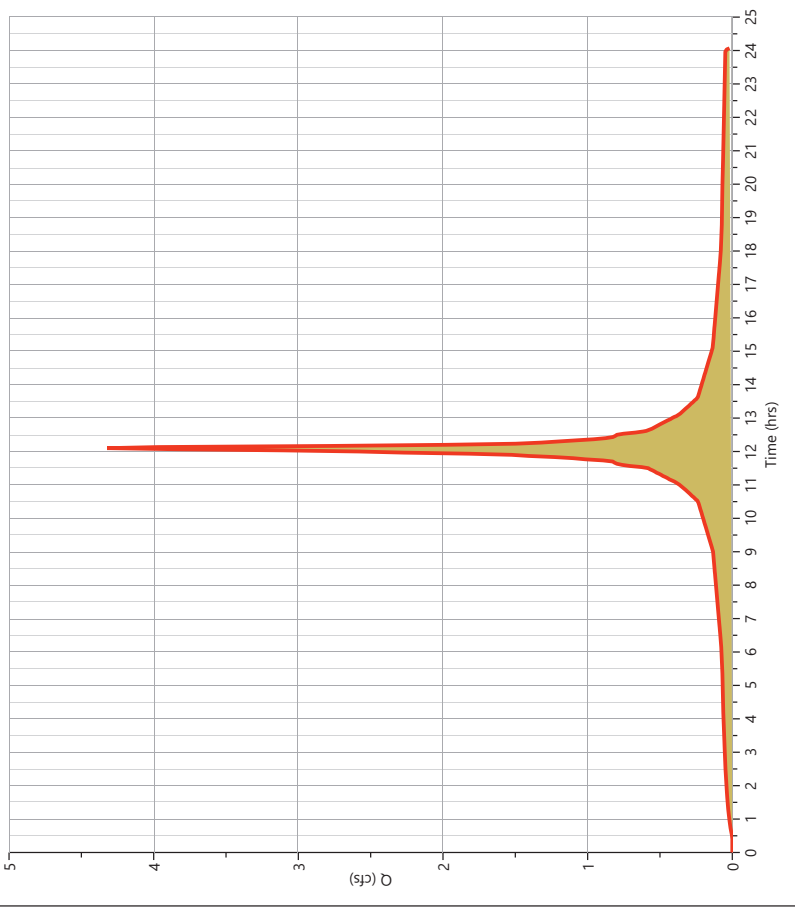
11-22-2022

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.321 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 15,364 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 4.32 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

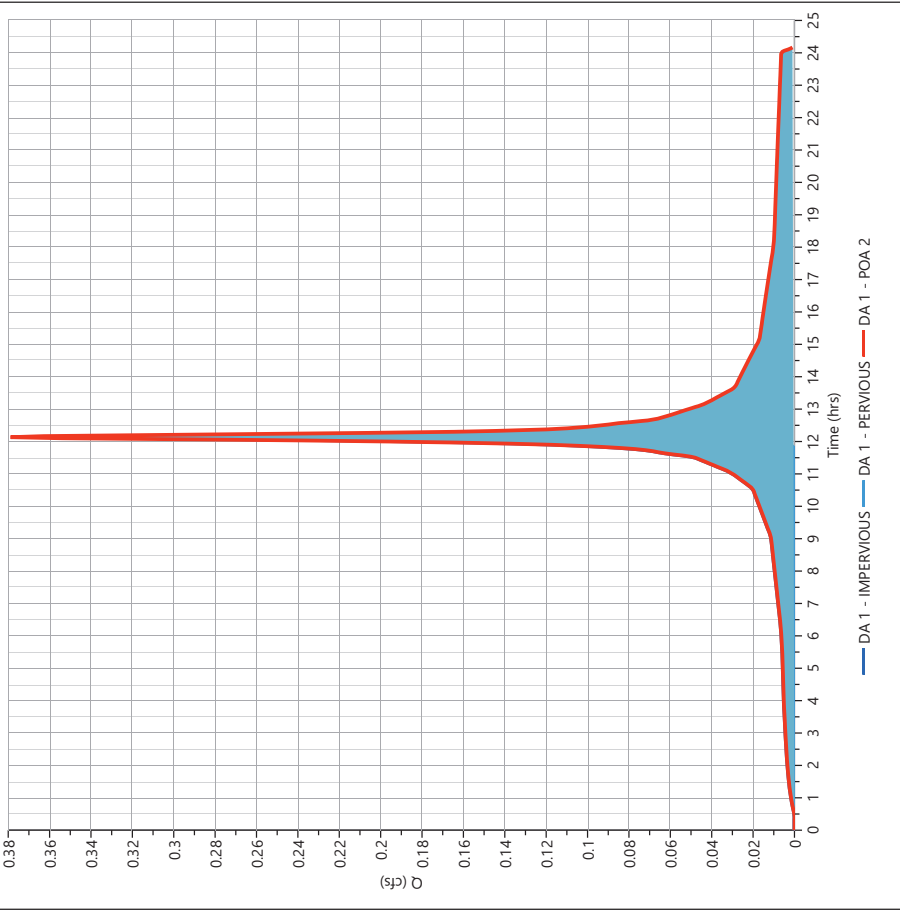
11-22-2022

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.379 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,584 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.38 cfs



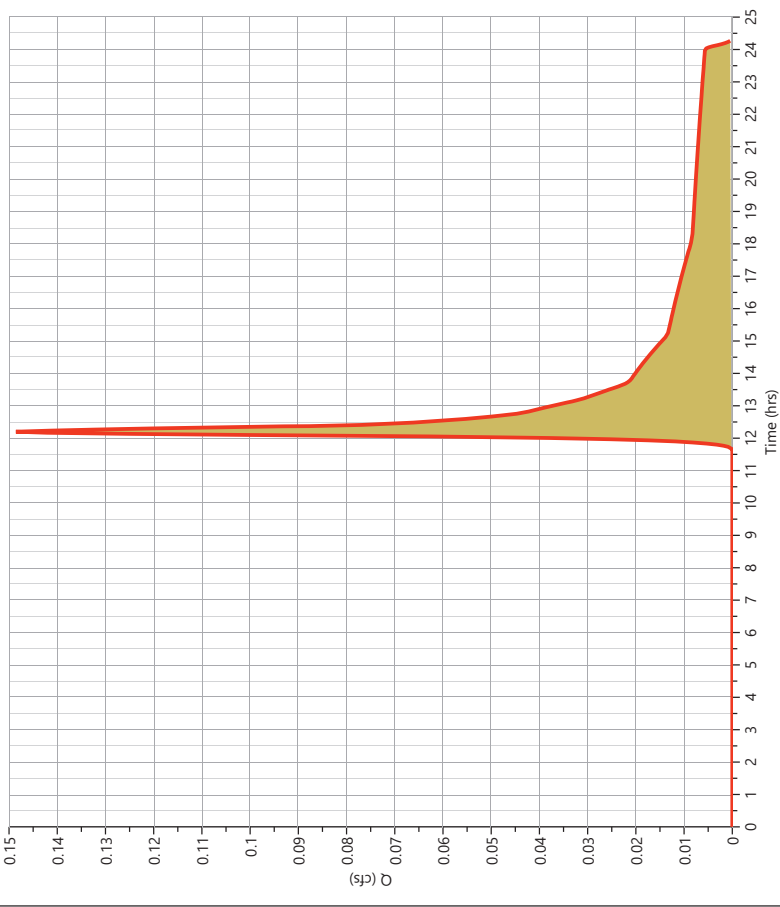
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.149 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 713 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.15 cfs



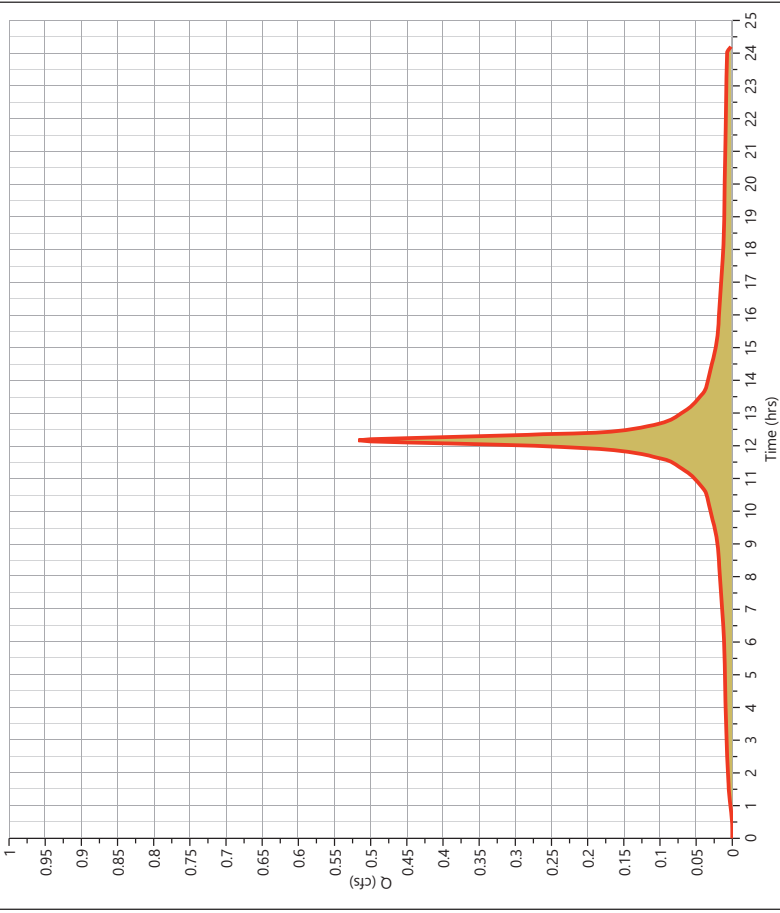
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.517 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,366 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.52 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

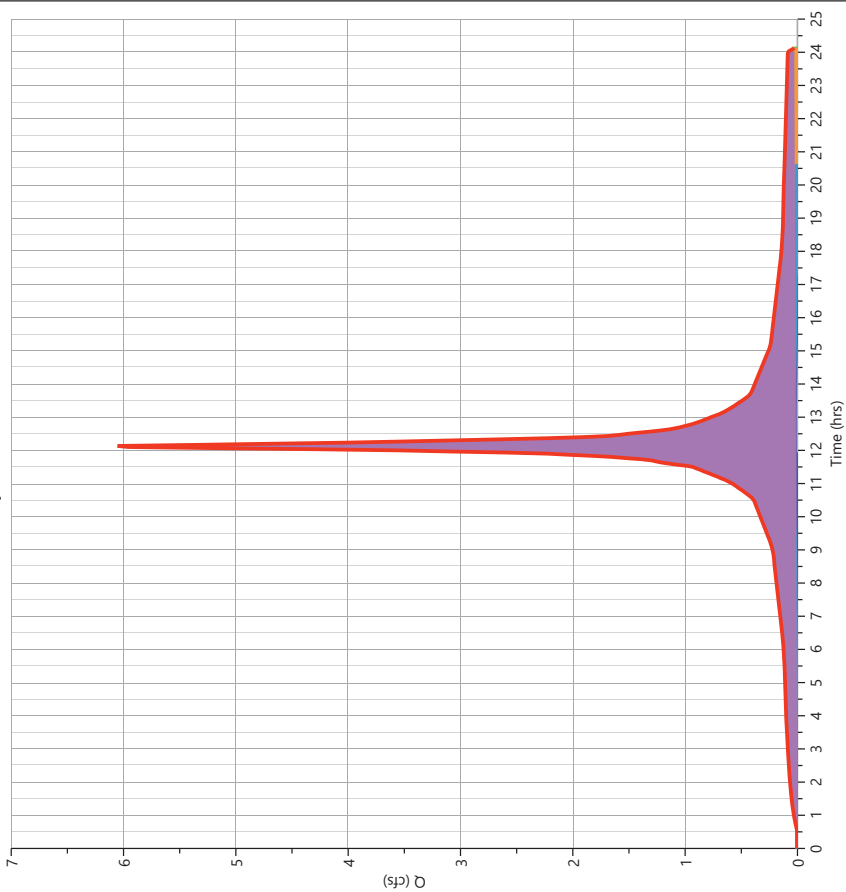
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 6.052 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 26,341 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 6.05 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

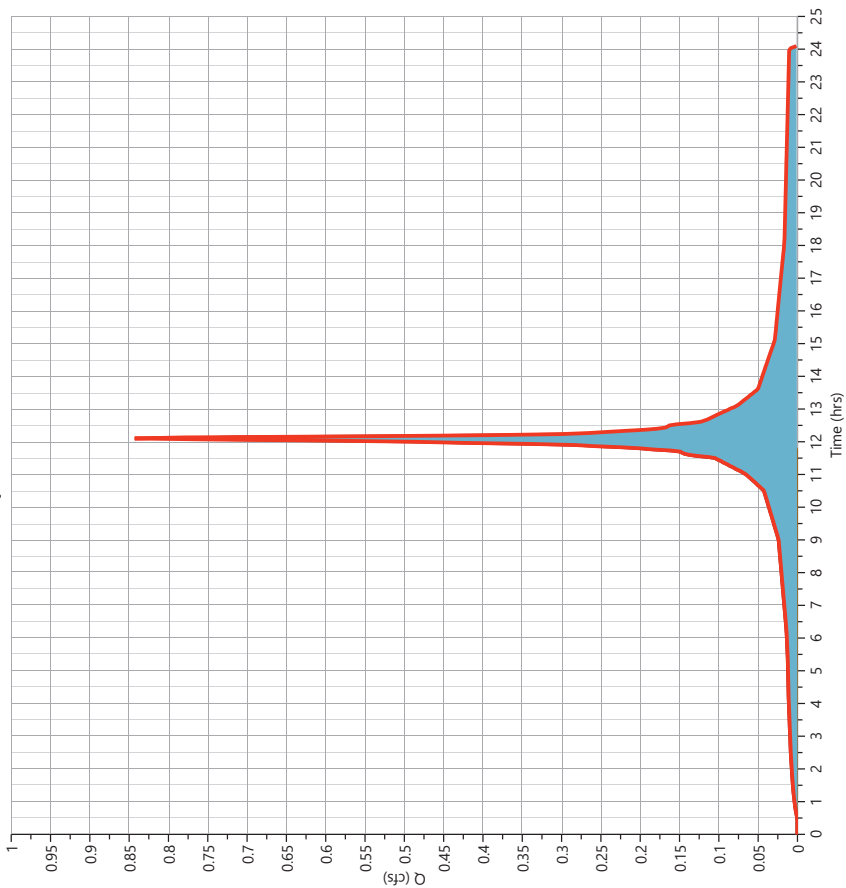
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.843 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 3,001 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.84 cfs



Hydrograph Report

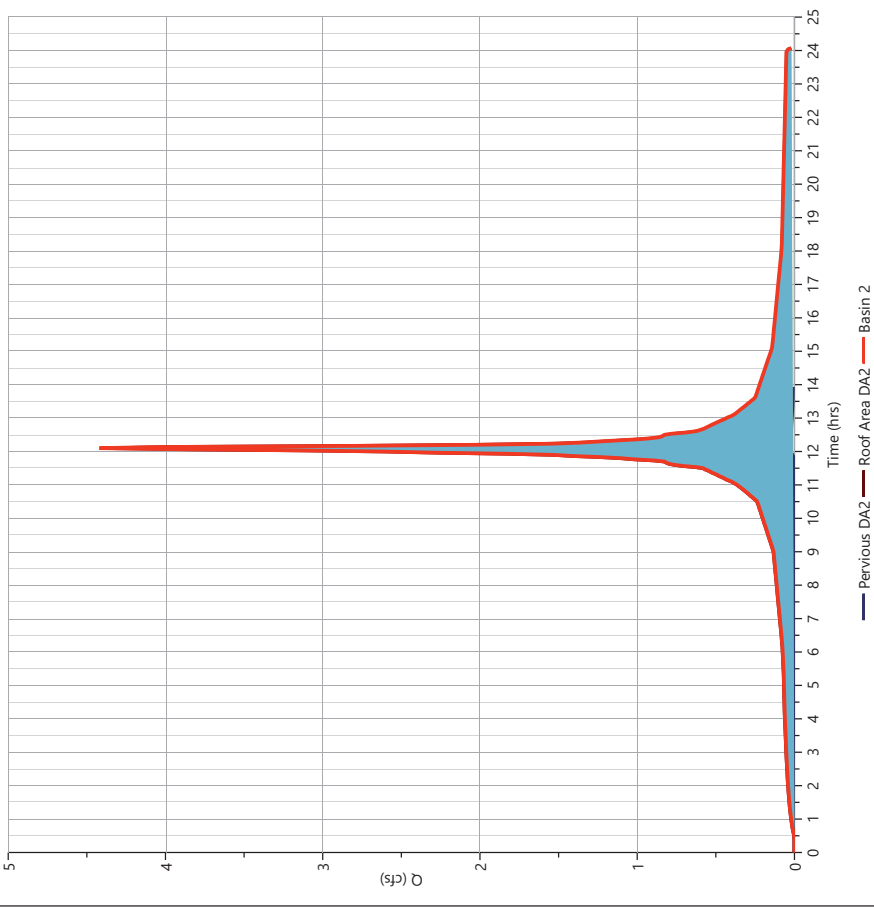
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 4.419 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 15,718 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 4.42 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

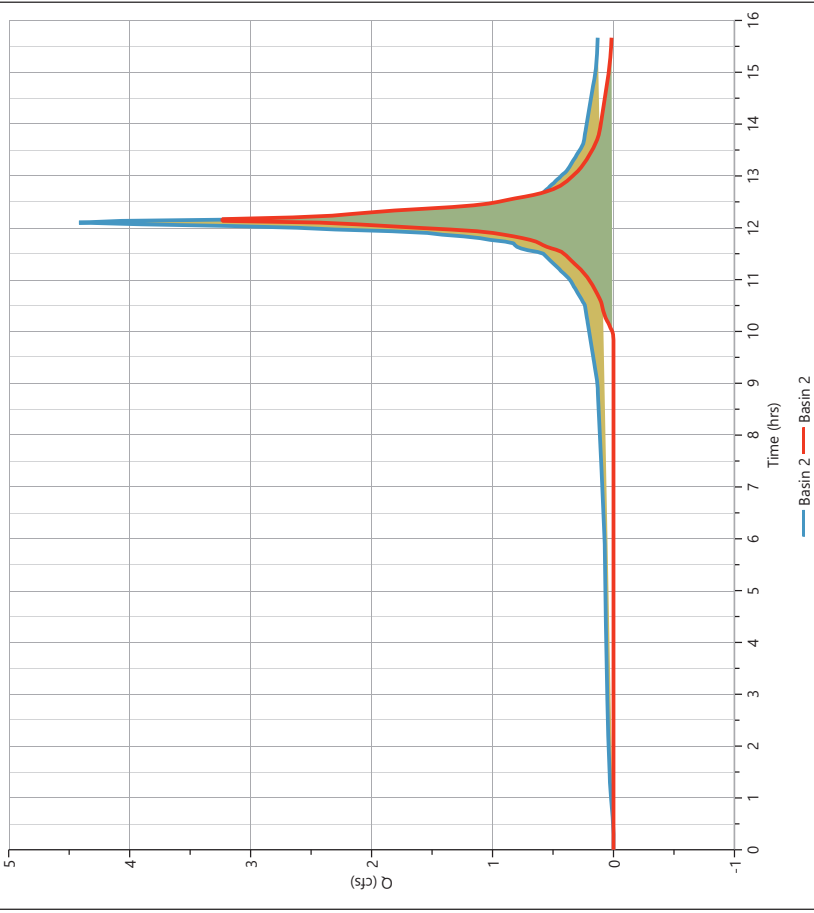
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 3.234 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 7,926 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.97 ft
Pond Name	= Basin 2	Max. Storage	= 1,679 cuft

Pond Routing by Storage Indication Method

Qp = 3.23 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 1

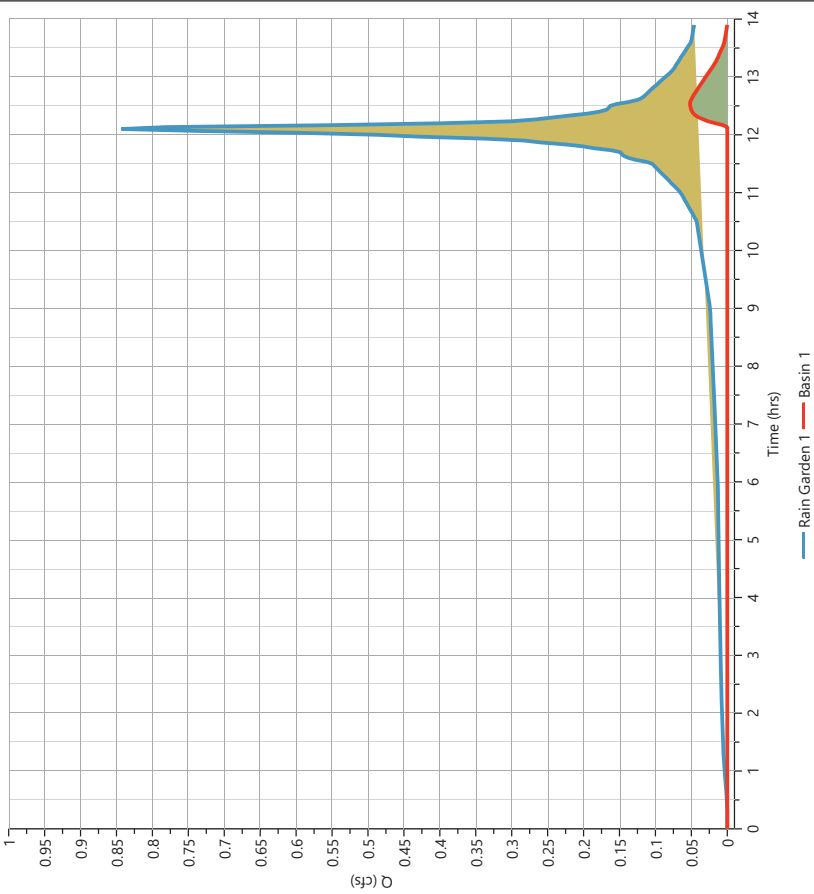
Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.052 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.53 hrs
Time Interval	= 2 min	Hydrograph Volume	= 159 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.94 ft
Pond Name	= BASIN 1	Max. Storage	= 1,096 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 13 min

Qp = 0.05 cfs



Hydrograph Report

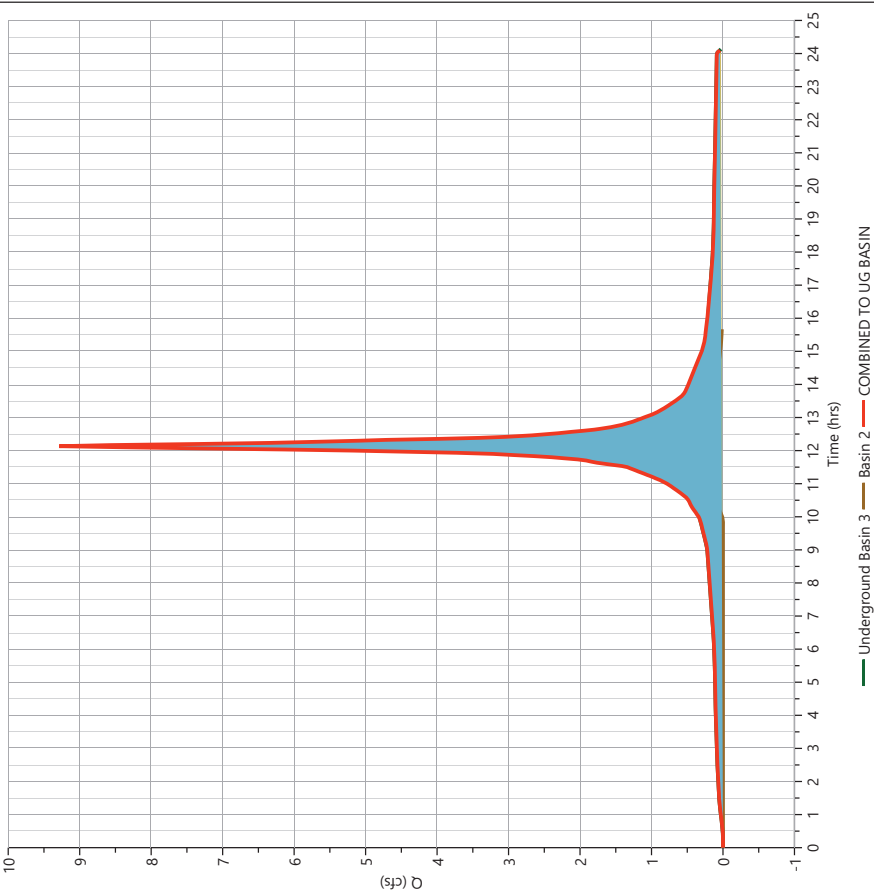
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 9.285 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 34,267 cuft
Inflow Hydrographs	= 13, 16	Total Contrib. Area	= 0.9 ac

Qp = 9.28 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

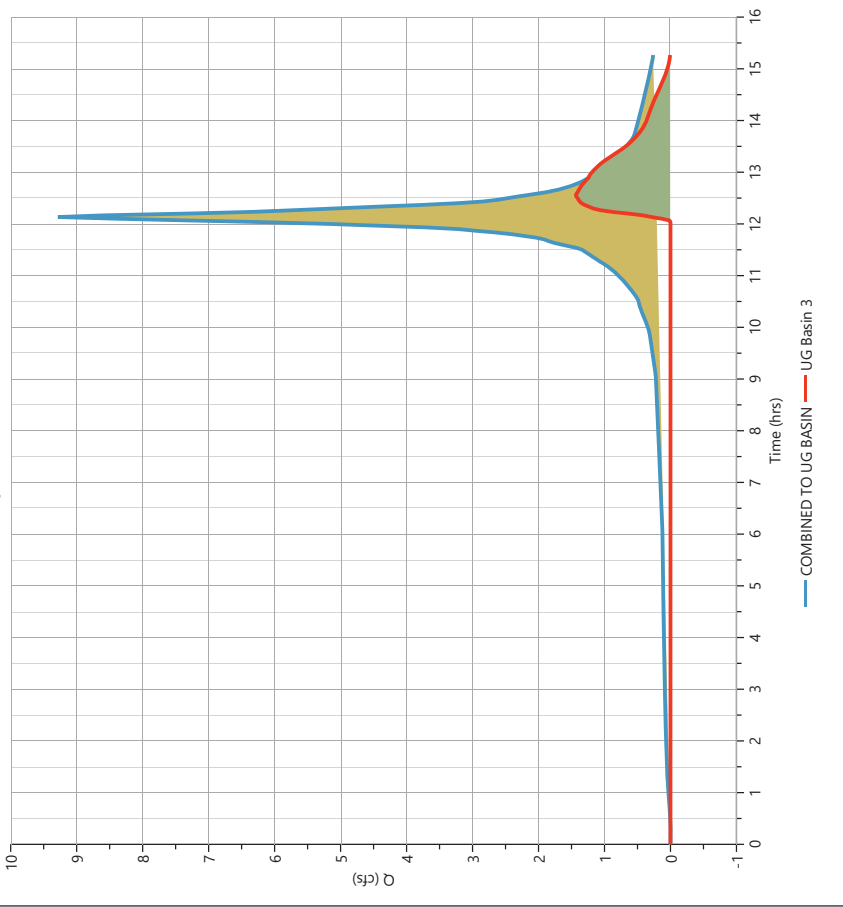
Post UG Basin 3

Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 1,432 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.57 hrs
Time Interval	= 2 min	Hydrograph Volume	= 7,206 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 14.14 ft
Pond Name	= UG BASIN	Max. Storage	= 11,104 cuft

Pond Routing by Storage Indication Method

Qp = 1.43 cfs



Hydrograph Report

Project Name:

11-22-2022

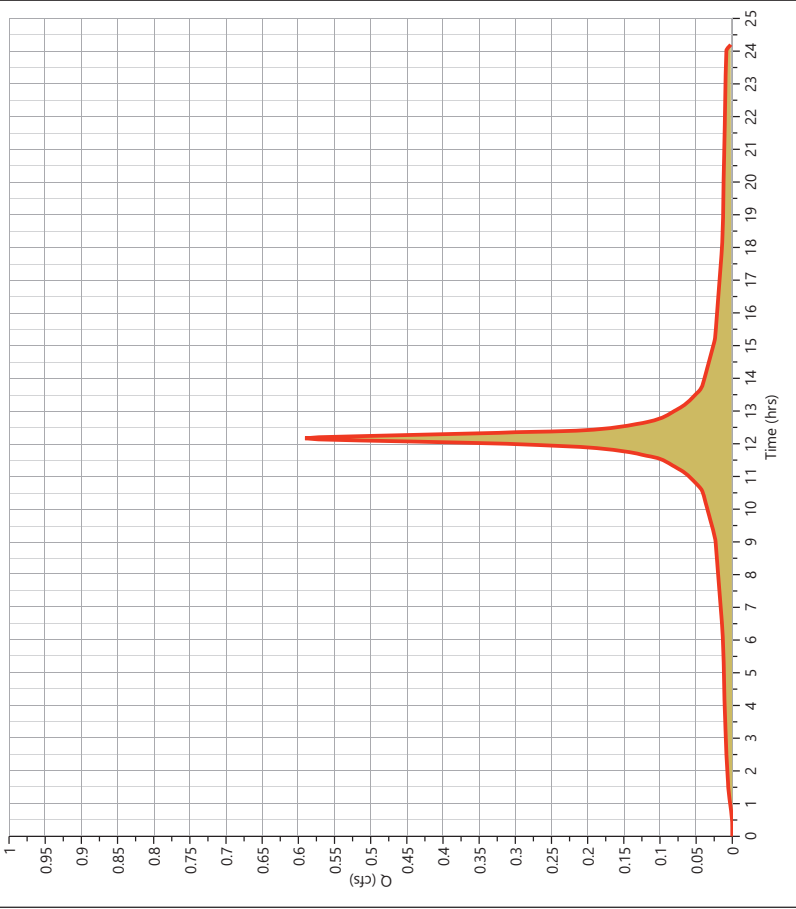
Hydrology Studio v 3.0.0.26

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.591 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,704 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.59 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

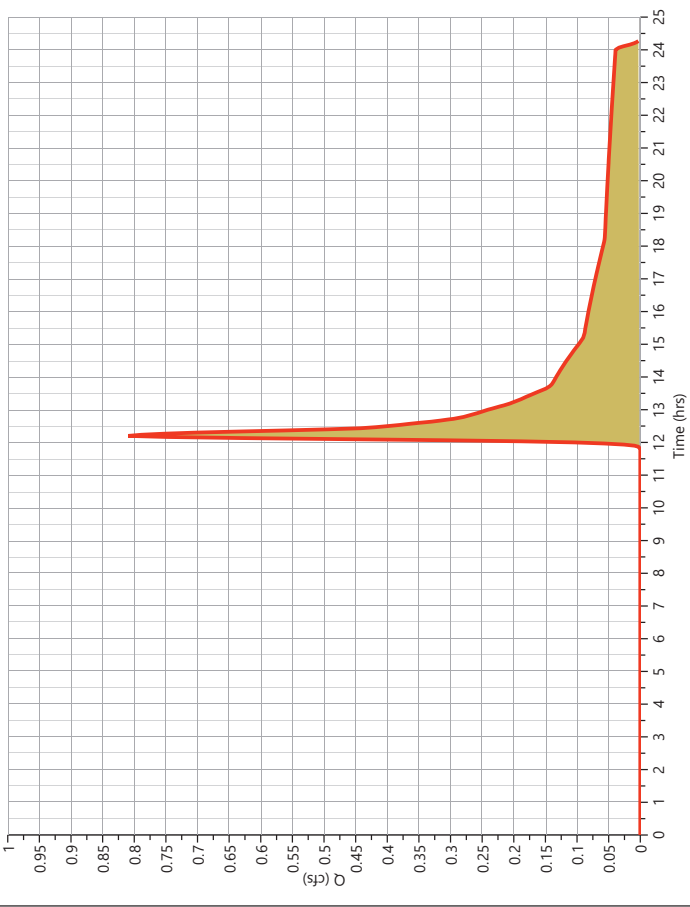
11-22-2022

Pre DA 2 - PERVIOUS

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.810 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 4.475 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.58	39	OPEN SPACE	
0.28	30	WOODS	
0.86	36	Weighted CN Method Employed	

Qp = 0.81 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

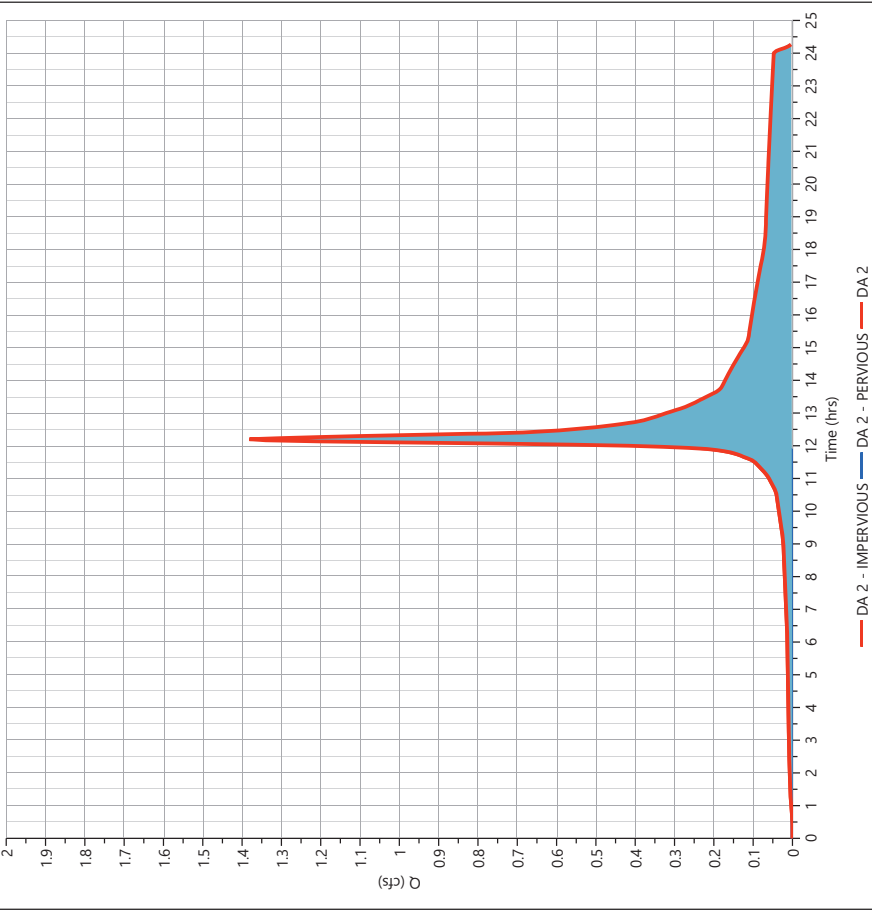
11-22-2022

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 1.381 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 7,179 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 1.38 cfs



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

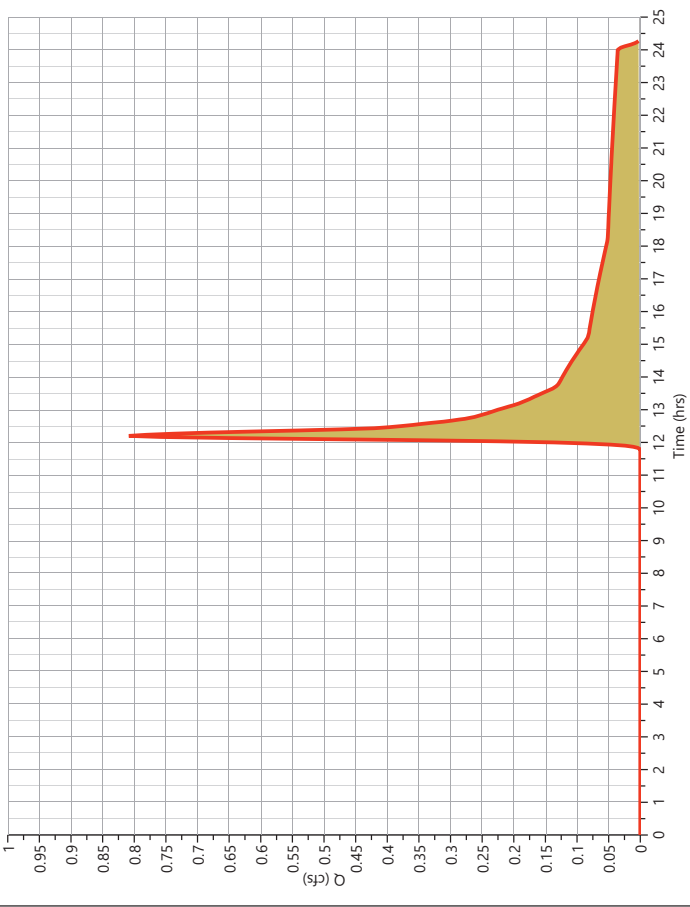
Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.809 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 4,218 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.62	38	OPEN SPACE
0.13	30	WOODS
0.75	37	Weighted CN Method Employed

Qp = 0.81 cfs



Hydrograph Report

Project Name:
11-22-2022

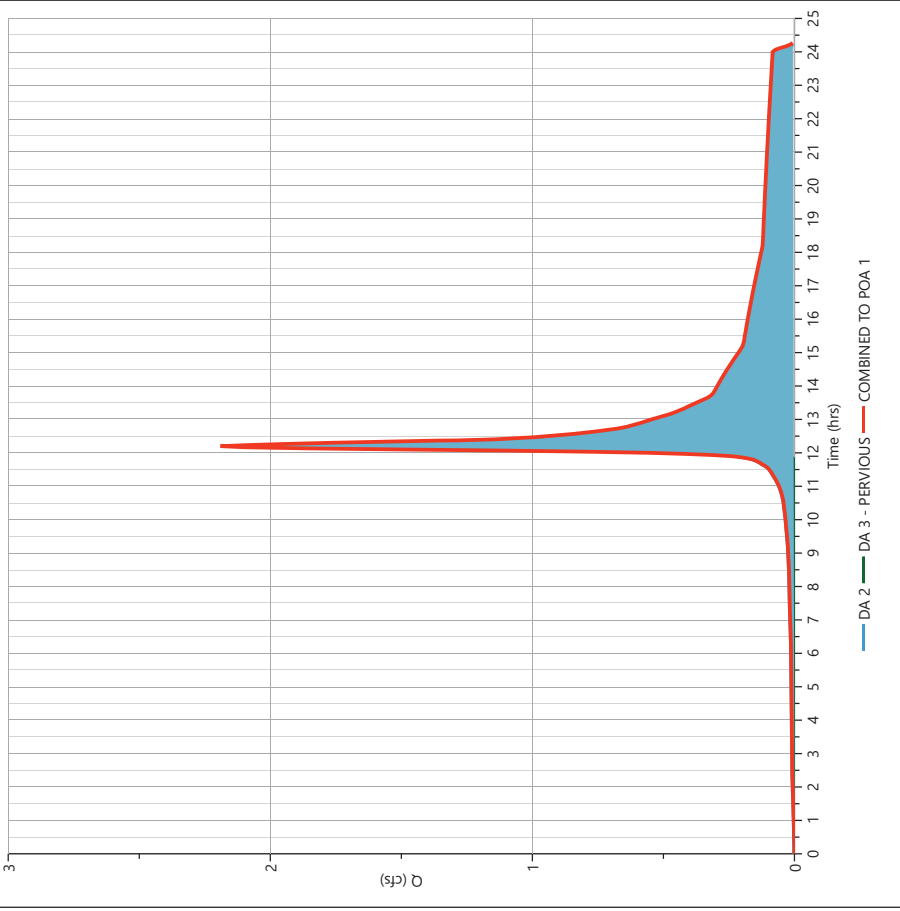
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 2,190 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 11,397 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac

Qp = 2.19 cfs



Hydrograph Report

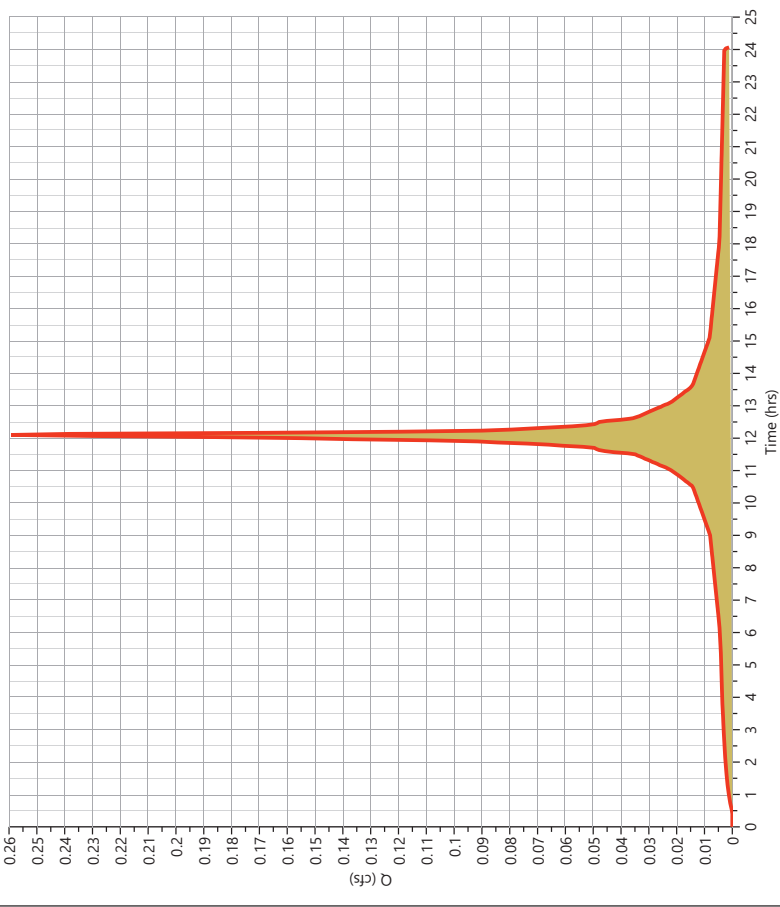
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.259 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 922 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.26 cfs



Hydrograph Report

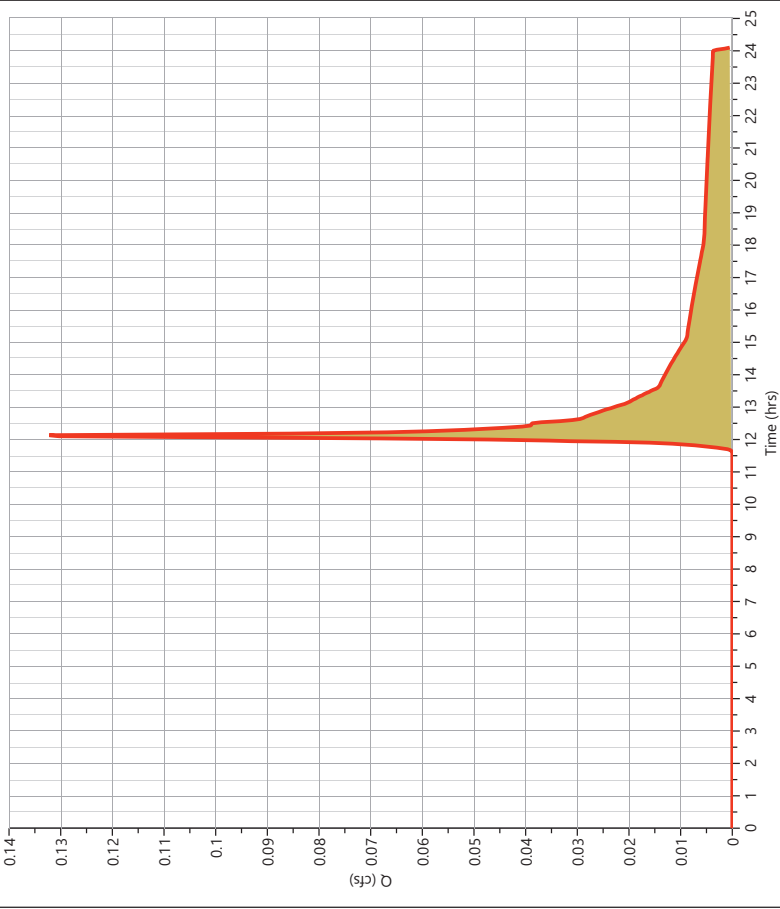
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.132 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 471 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.13 cfs



Hydrograph Report

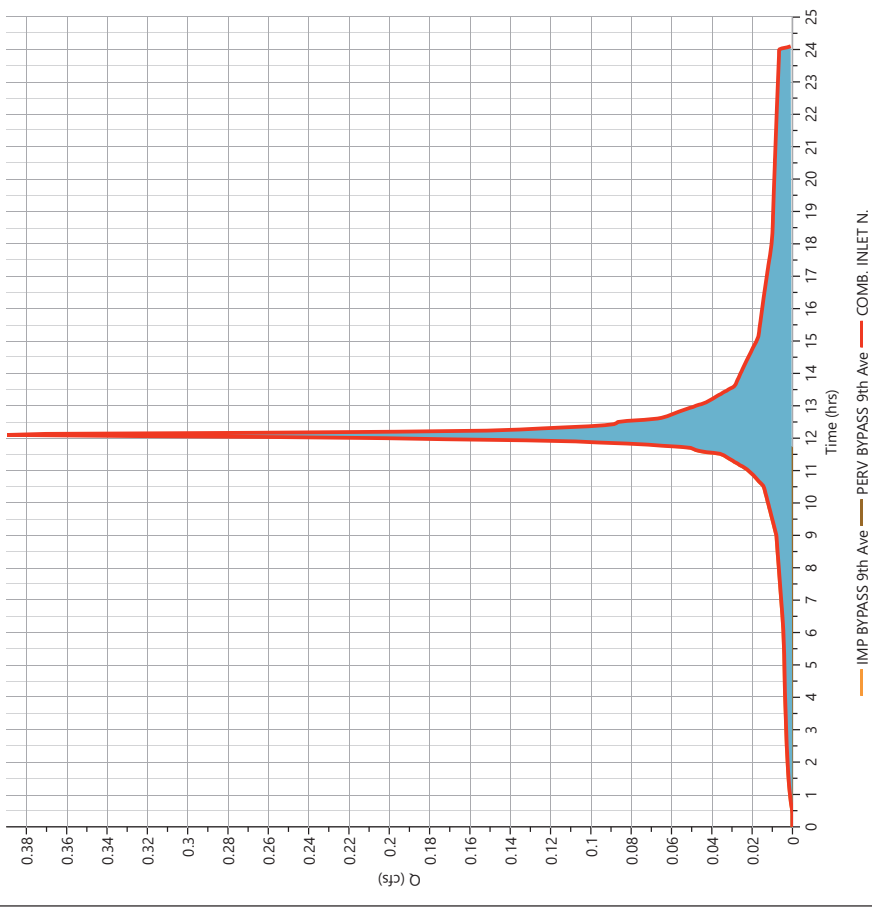
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.390 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,393 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac

Qp = 0.39 cfs



Hydrograph Report

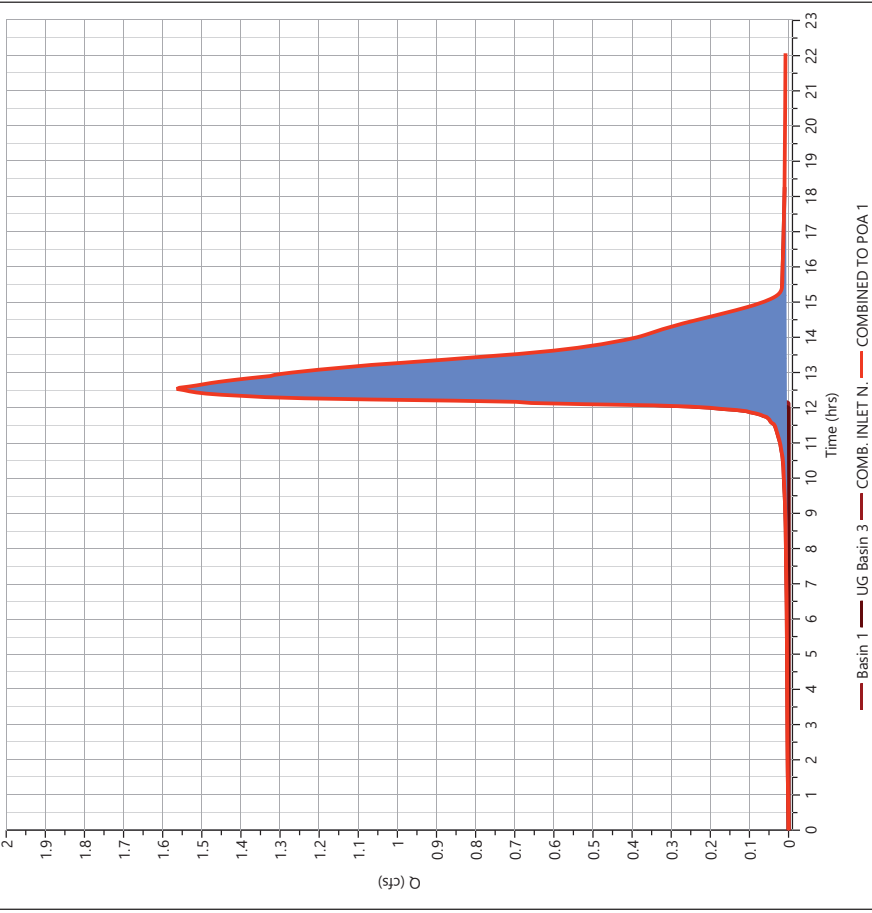
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 1.564 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.53 hrs
Time Interval	= 2 min	Hydrograph Volume	= 8,759 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac

Qp = 1.56 cfs



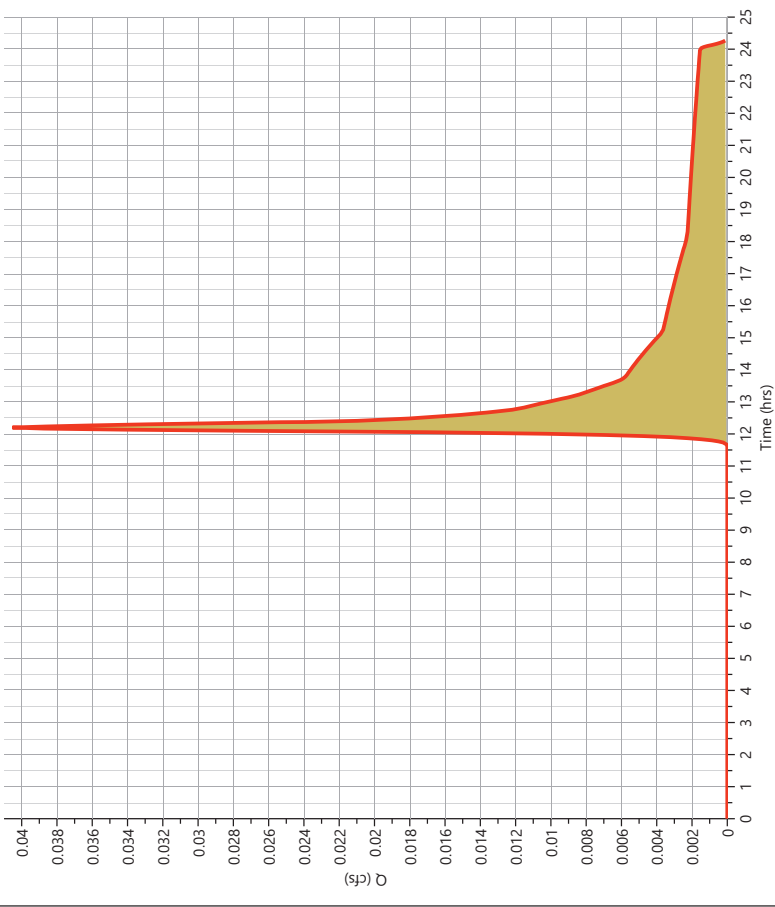
Hydrograph Report

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.041 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 194 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.04 cfs



Hydrograph Report

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.095 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 454 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.09 cfs



Hydrograph Report

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.346 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,229 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.35 cfs



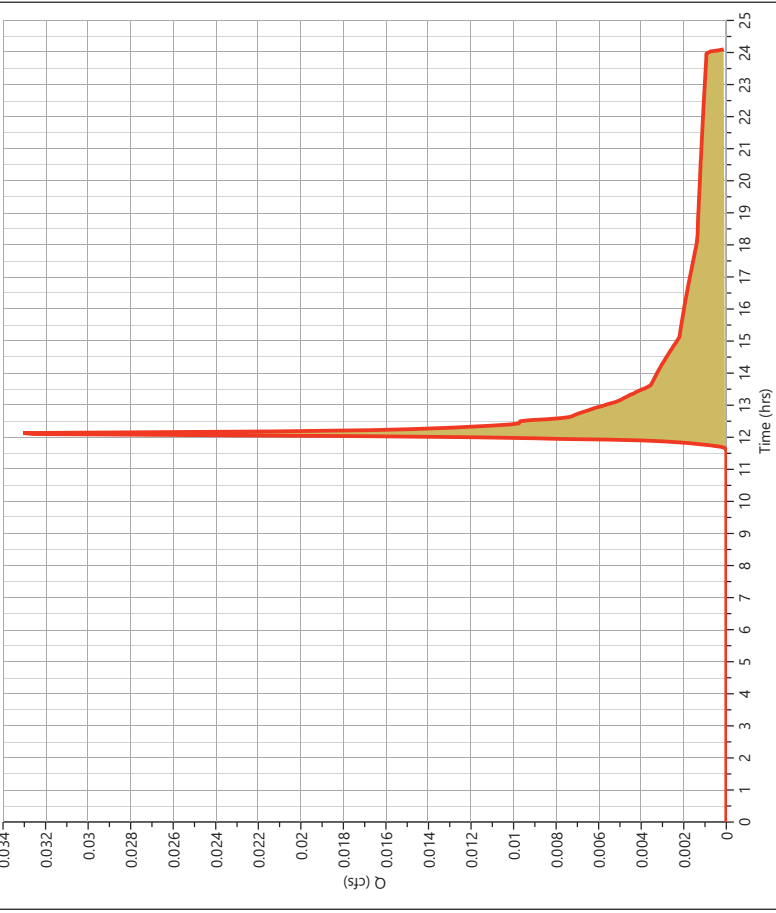
Hydrograph Report

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.033 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 118 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.03 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

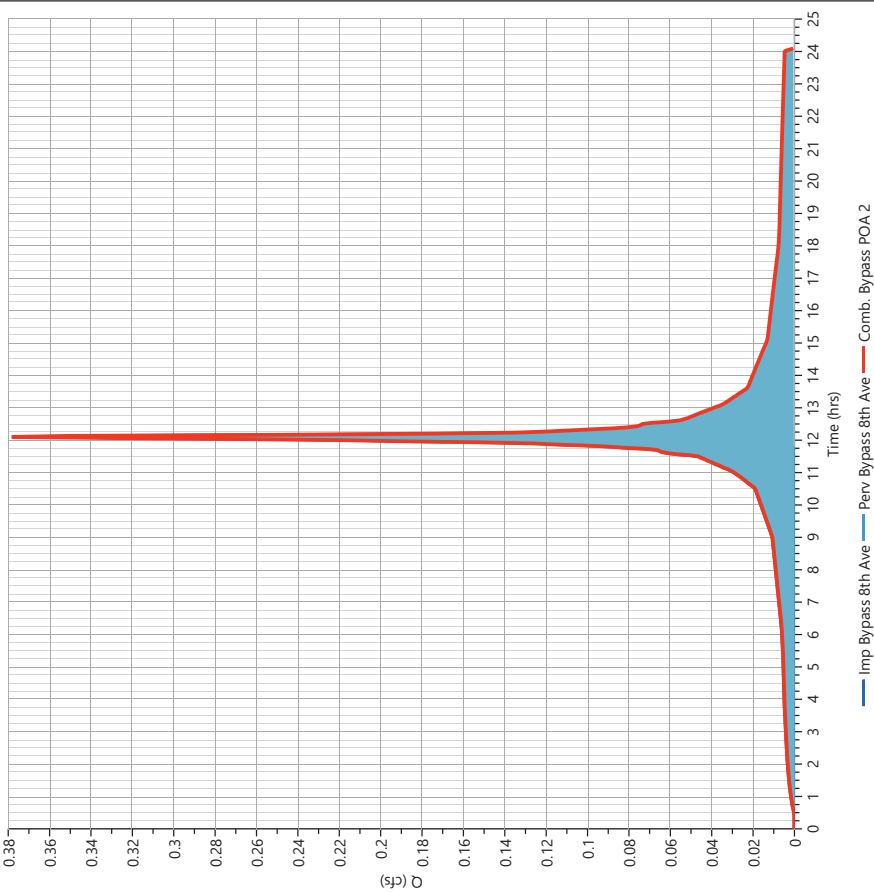
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.378 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,347 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.38 cfs

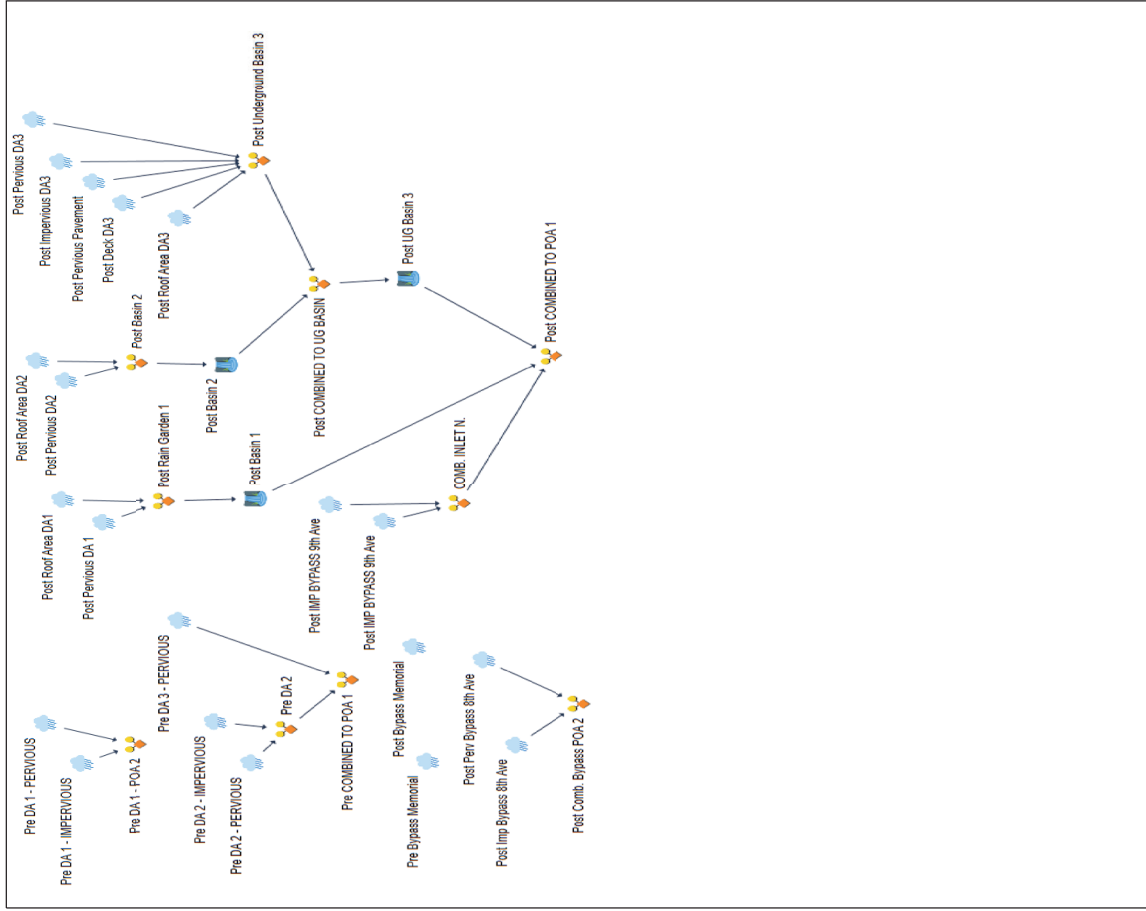


**HYDROGRAPH SUMMARY REPORTS –
EXISTING & PROPOSED
WATER QUALITY STORM**

Basin Model

Hydrology Studio v 3.0.0.26

Project Name:
11-22-2022



Hydrograph 1-yr Summary

Hydrology Studio v 3.0.0.26

Project Name:
11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.041	12.13	150	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.000	0.00	0.000	----	----	----
3	NRCS Runoff	Post Roof Area DA3	0.383	12.10	1,232	----	----	----
4	NRCS Runoff	Post Deck DA3	0.205	12.17	852	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.140	12.17	581	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.000	0.00	0.000	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.098	12.10	317	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.000	0.00	0.000	----	----	----
9	NRCS Runoff	Post Roof Area DA2	0.547	12.10	1,760	----	----	----
10	Junction	Pre DA 1 - POA 2	0.041	12.13	150	1, 2	13.67	387
11	NRCS Runoff	Post Pervious DA3	0.000	0.00	0.000	----	13.34	79.7
12	NRCS Runoff	Post Impervious DA3	0.065	12.17	271	----	----	----
13	Junction	Post Underground Basin 3	0.749	12.13	2,936	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.098	12.10	317	6, 7	----	----
15	Junction	Post Basin 2	0.547	12.10	1,760	8, 9	----	----
16	Pond Route	Post Basin 2	0.002	12.37	1.94	15	13.67	387
17	Pond Route	Post Basin 1	0.000	14.57	0.000	14	13.34	79.7
18	Junction	Post COMBINED TO UG BASIN	0.749	12.13	2,936	13, 16	----	----
19	Pond Route	Post UG Basin 3	0.000	11.83	0.000	18	11.81	48.3
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.075	12.17	310	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.000	0.00	0.000	----	----	----
22	Junction	Pre DA 2	0.075	12.17	310	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.000	0.00	0.000	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.075	12.17	310	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.033	12.10	106	----	----	----
26	NRCS Runoff	Post IMP BYPASS 9th Ave	0.000	0.00	0.000	----	----	----
27	Junction	COMB. INLET N	0.033	12.10	106	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	0.033	12.10	106	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.000	0.00	0.000	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.000	0.00	0.000	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.044	12.10	141	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.000	0.00	0.000	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.044	12.10	141	31, 32	----	----

Hydrograph Report

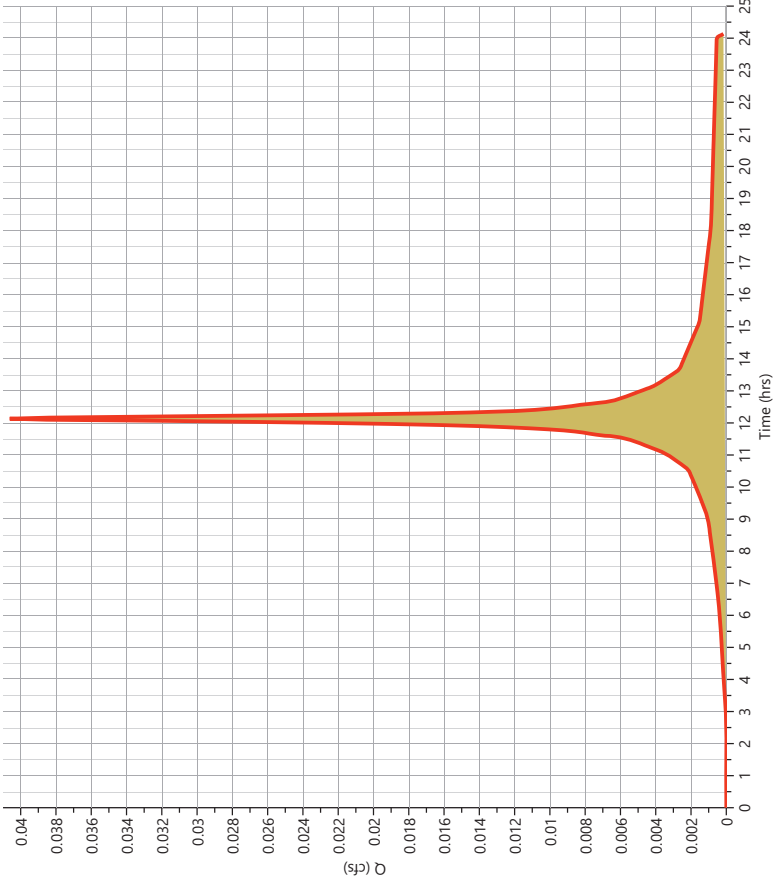
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.041 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 150 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.04 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - PERVIOUS

Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.04	39	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.00 cfs

Hydrograph Report

Project Name:
11-22-2022

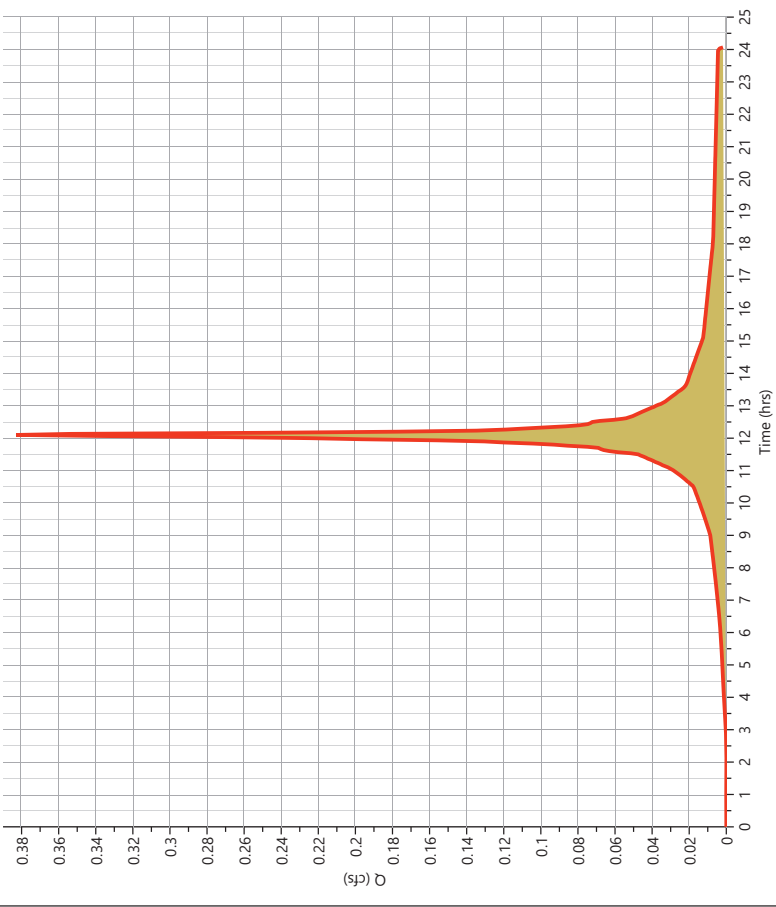
Hydrology Studio v 3.0.0.26

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.383 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,232 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.38 cfs



Hydrograph Report

Project Name:
11-22-2022

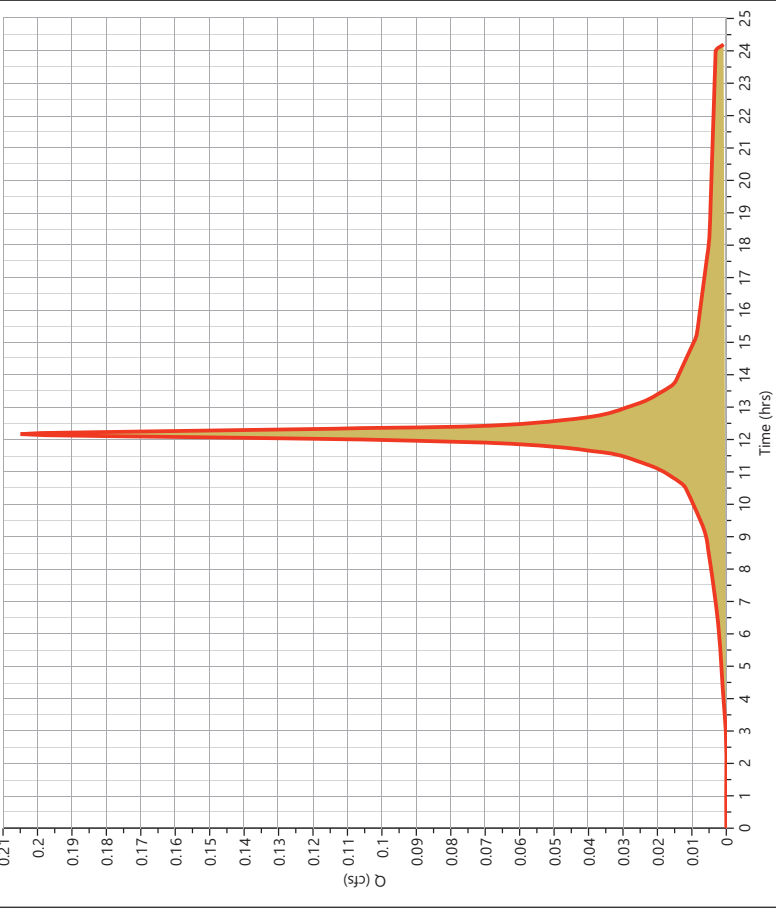
Hydrology Studio v 3.0.0.26

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.205 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 852 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.20 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

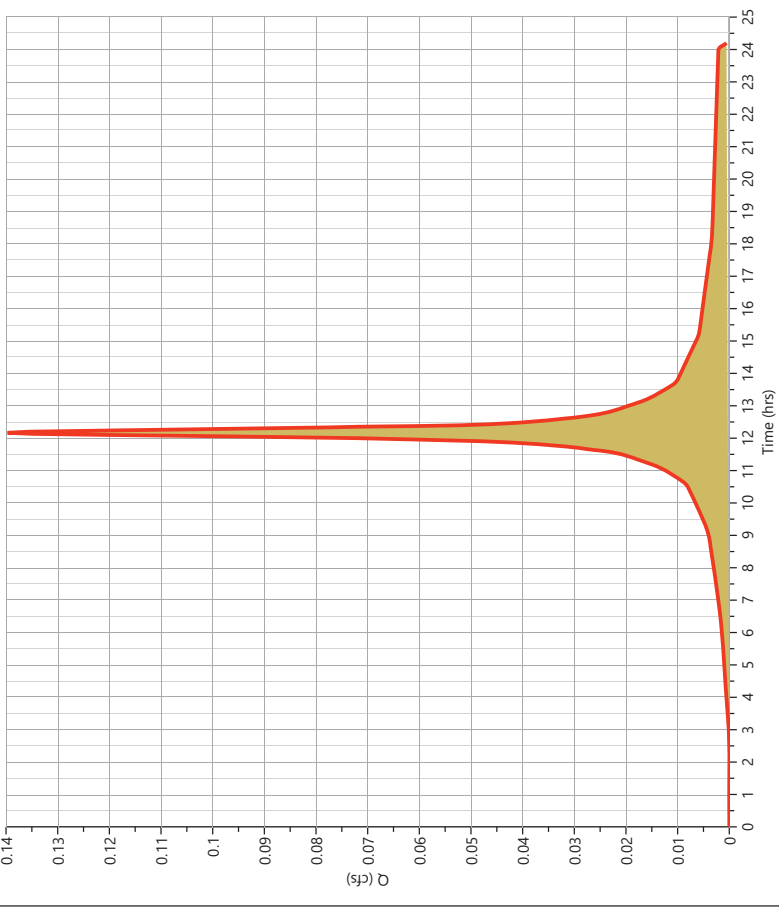
11-22-2022

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.140 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 581 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.14 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

11-22-2022

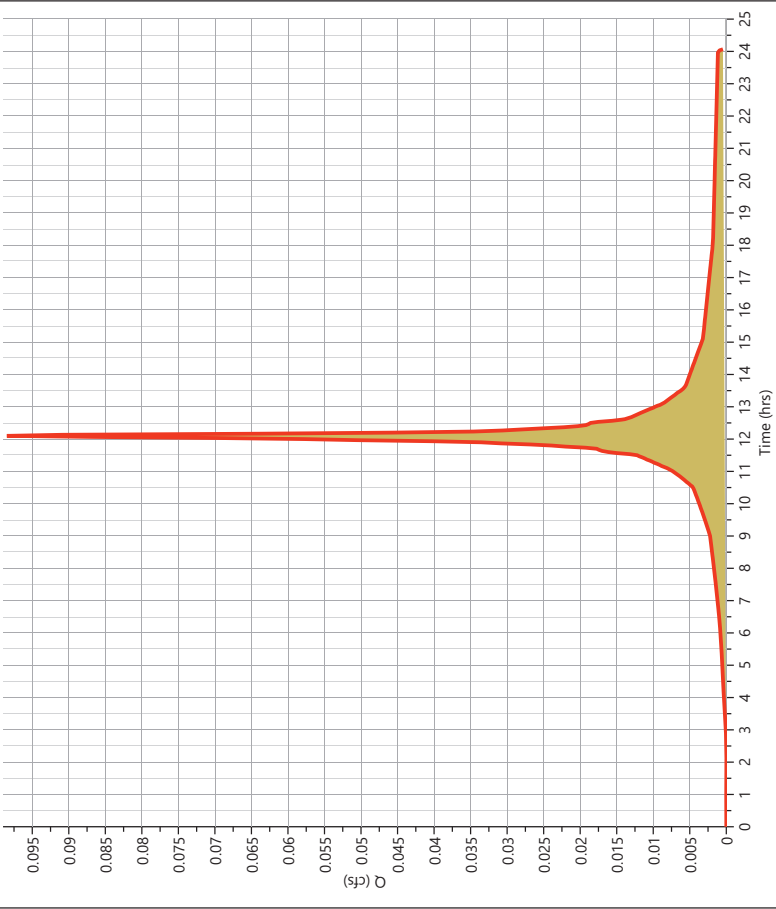
Hydrology Studio v 3.0.0.26

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.098 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 317 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.10 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

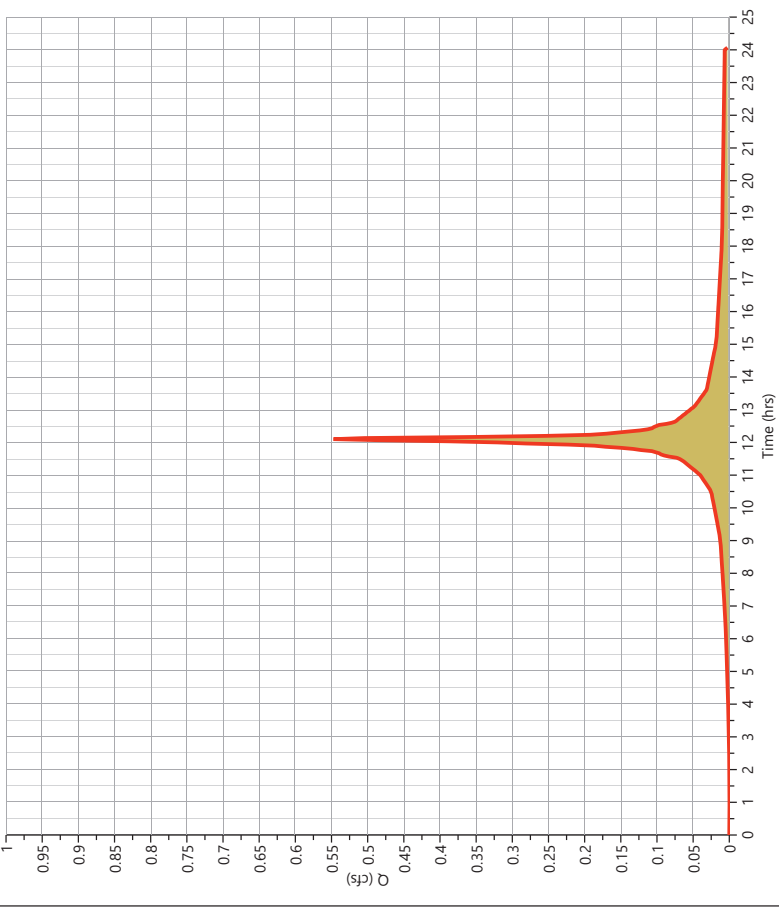
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.547 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,760 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.55 cfs



Hydrograph Report

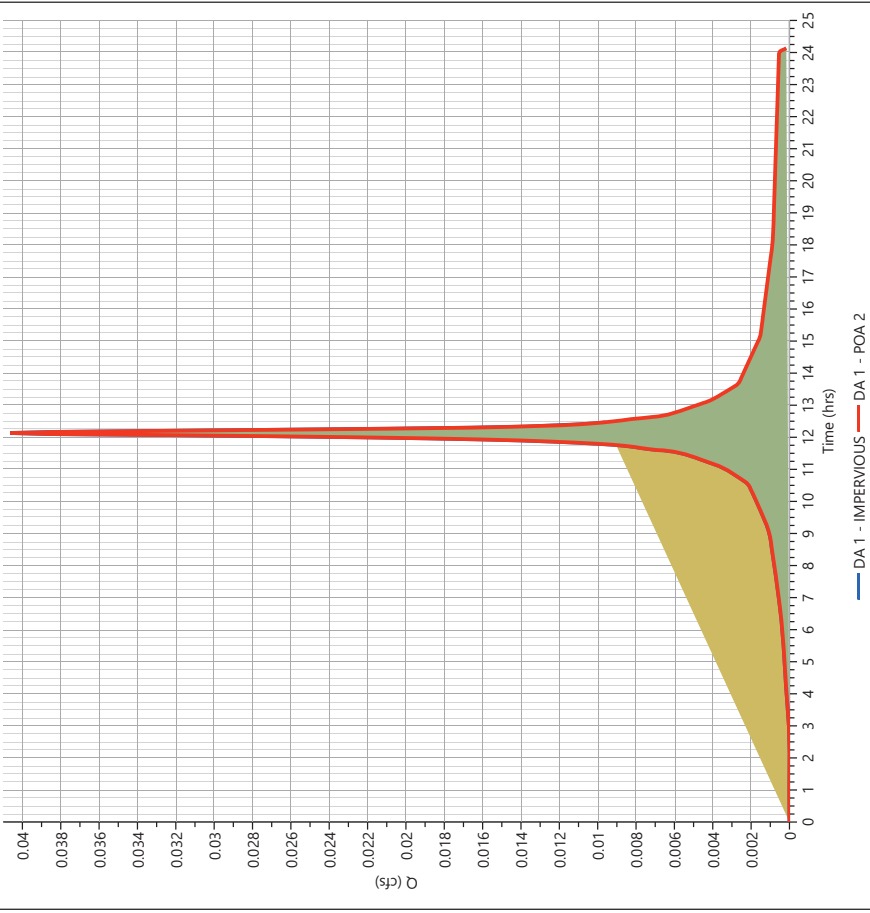
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.041 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 150 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.04 cfs



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs

Hydrograph Report

Project Name:
11-22-2022

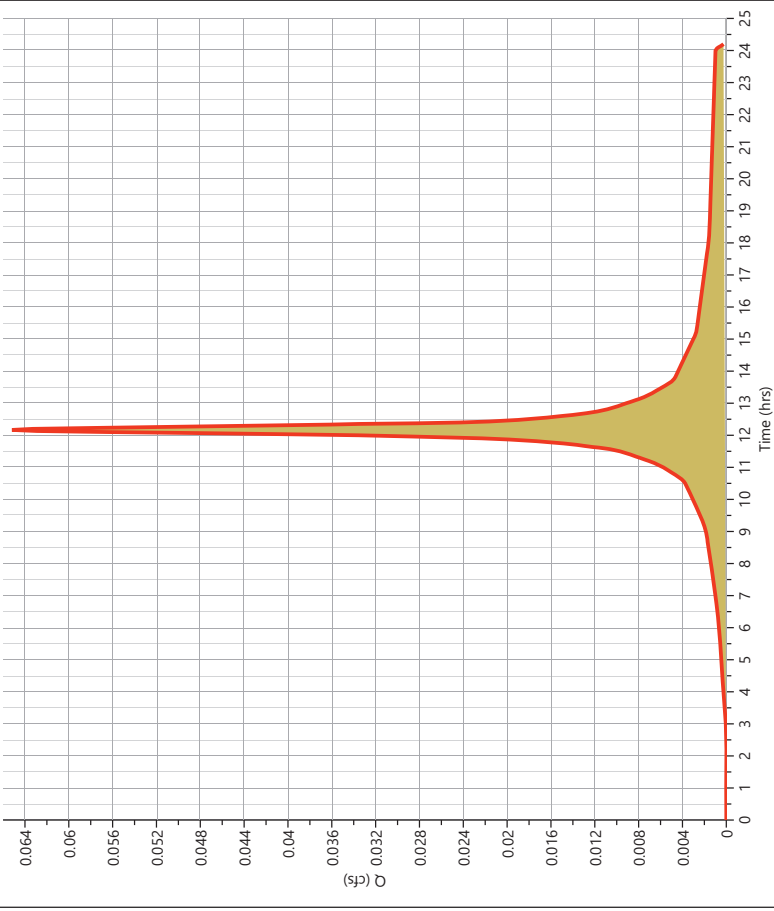
Hydrology Studio v 3.0.0.26

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.065 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 271 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.07 cfs



Hydrograph Report

Project Name:

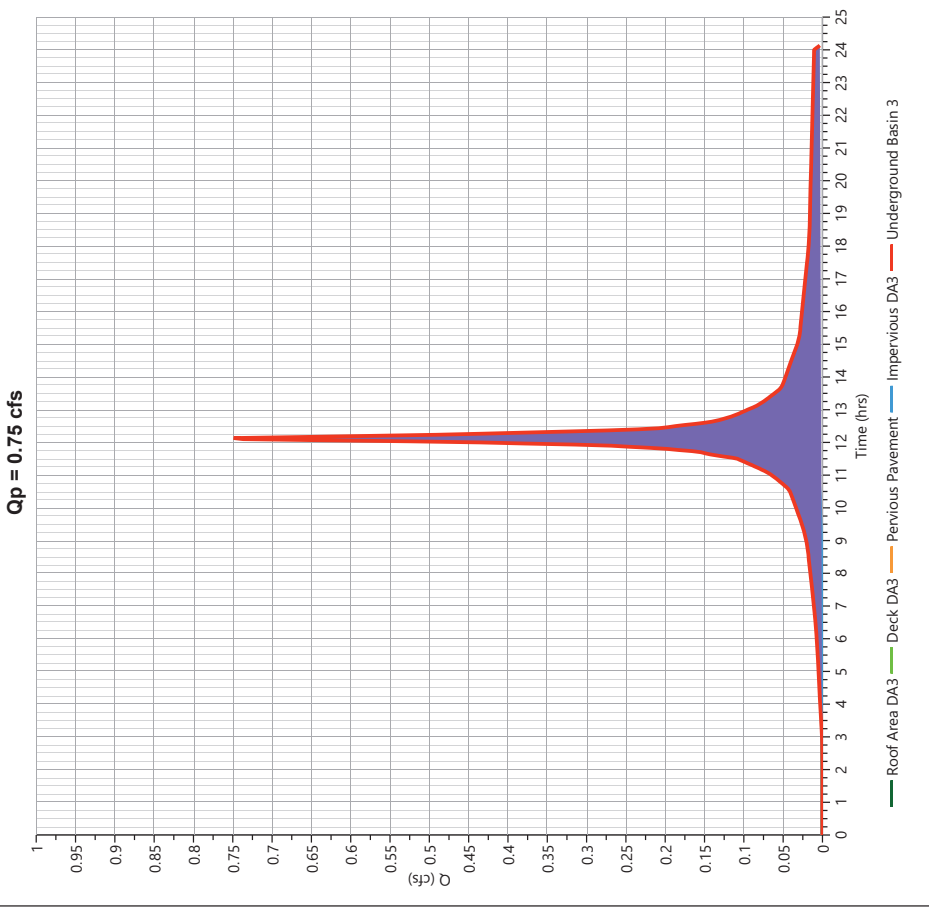
Hydrology Studio v 3.0.0.26

11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 0.749 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2.936 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac



Hydrograph Report

Project Name:

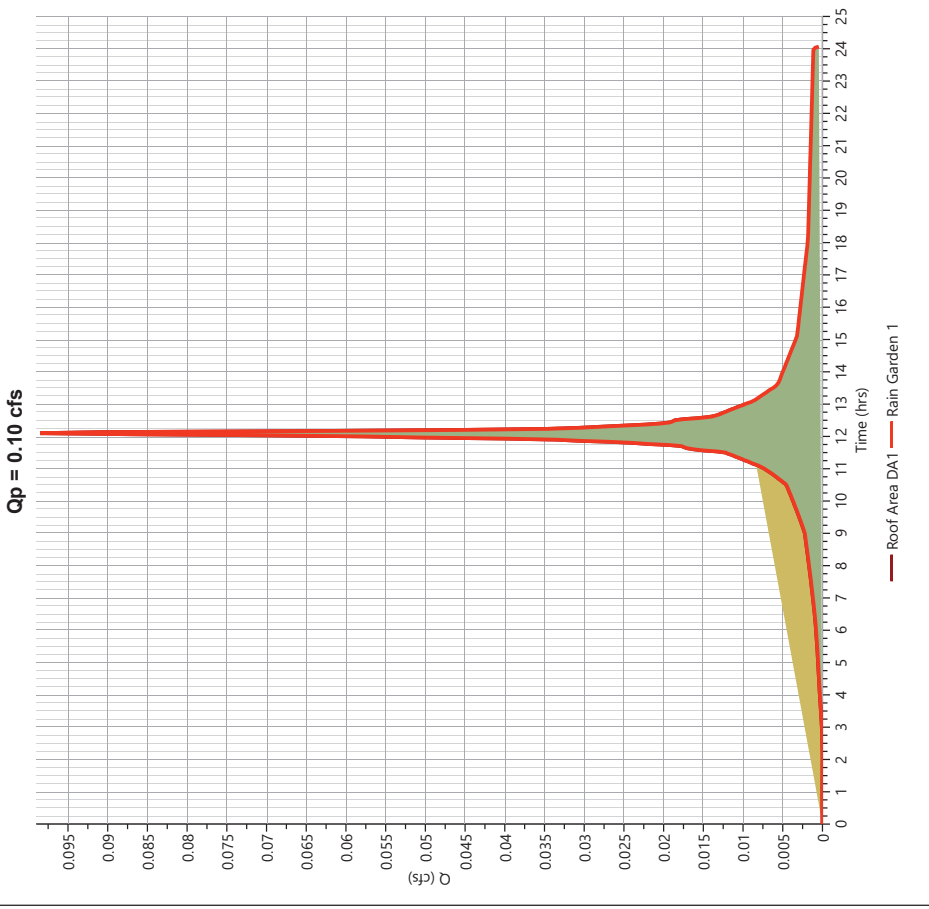
Hydrology Studio v 3.0.0.26

11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.098 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 317 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac



Hydrograph Report

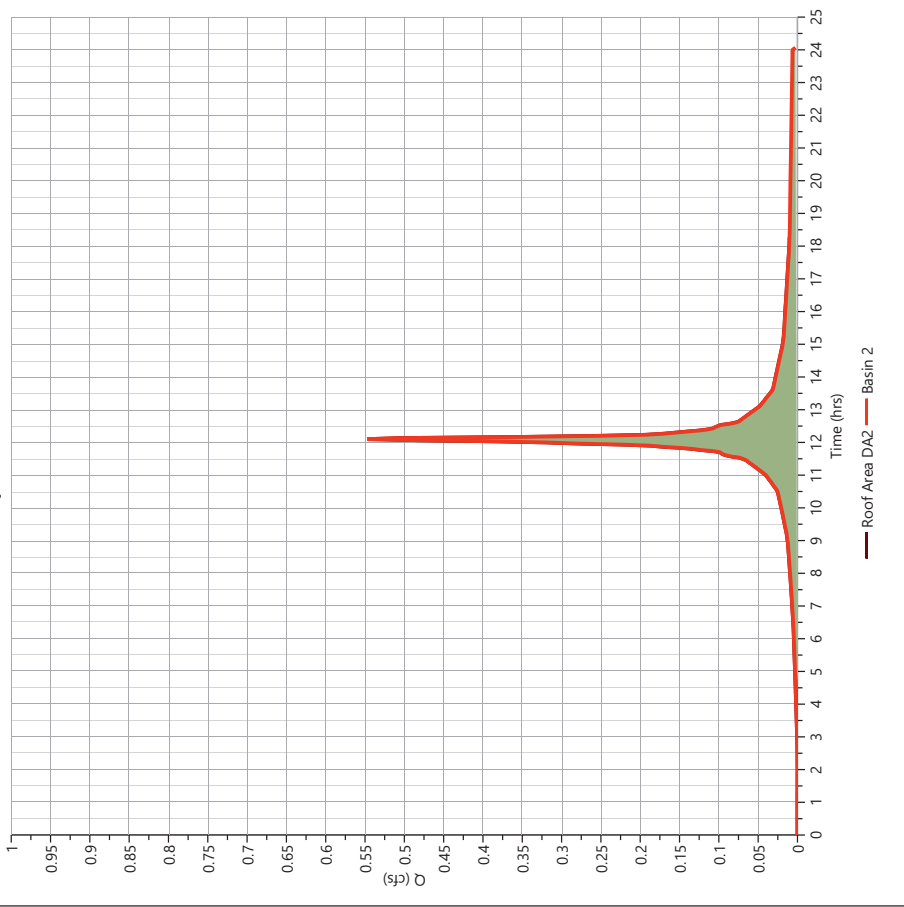
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 0.547 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,760 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 0.55 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

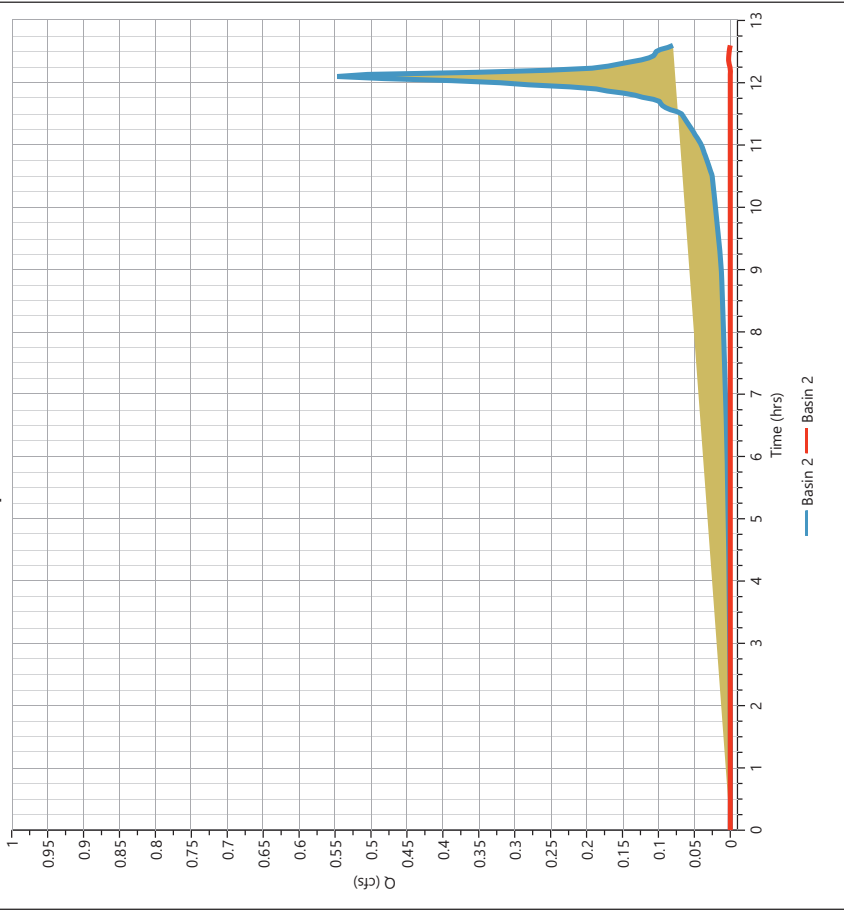
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 0.002 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.37 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1.94 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 13.67 ft
Pond Name	= Basin 2	Max. Storage	= 397 cuft

Pond Routing by Storage Indication Method

Qp = 0.00 cfs



Pond Report

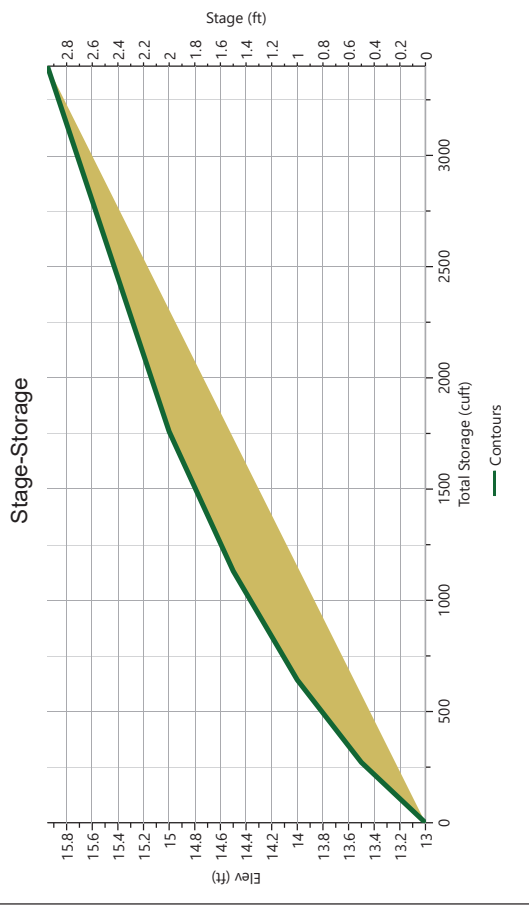
Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Storage

[illegible]

Pond Report

Project Name:

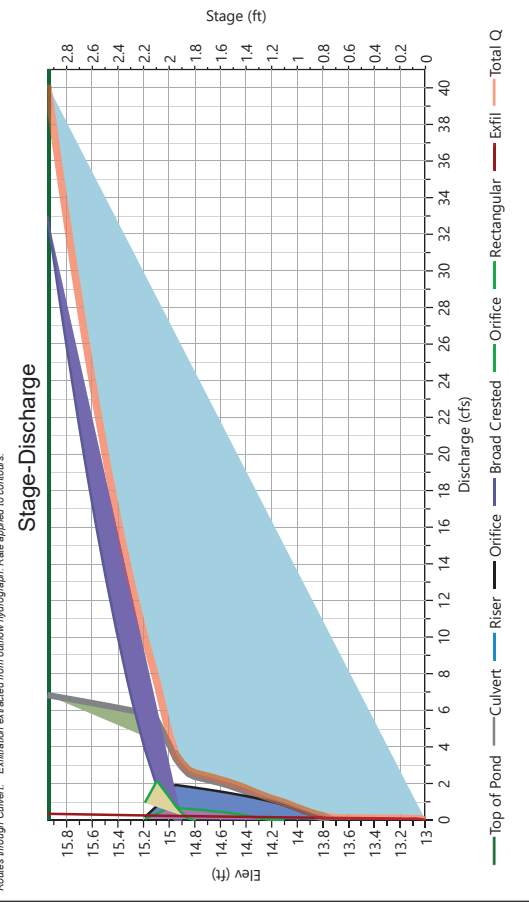
Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2*	3	
Rise, in	12	6	4		Orifice Dia, in
Span, in	12	6	4		No. Orifices
No. Barrels	1	2	2		Invert Elevation, ft
Invert Elevation, ft	12.19	13.67	14.15		Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60		Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
Shape / Type	Box	1	2*	3	
Crest Elevation, ft	14.95	Broad Crested	Rectangular		Exfiltration, in/hr
Crest Length, ft	12	14.95	14.82		
Angle, deg		10	5		
Weir Coefficient, Cw	3.3	3.3	3.3		



Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000
0.50	13.50	271	0.000 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.107		0.107
1.00	14.00	642	0.540 oc	0.540	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.140		0.680
1.50	14.50	1,132	1.800 oc	1.440	0.360	0.000	0.000	0.000	0.000	0.000	0.000	0.186		1.986
2.00	15.00	1,759	4,141 oc	1,744	0.695	0.000	0.443	0.369	1.260			0.232		4,742
2.95	15.95	3,401	6,826 oc	0.000	0.000	0.000	0.000	33.00	0.000			0.344		40.17

Suffix: ey, ic = inlet control, oc = outlet control, s = submerged weir

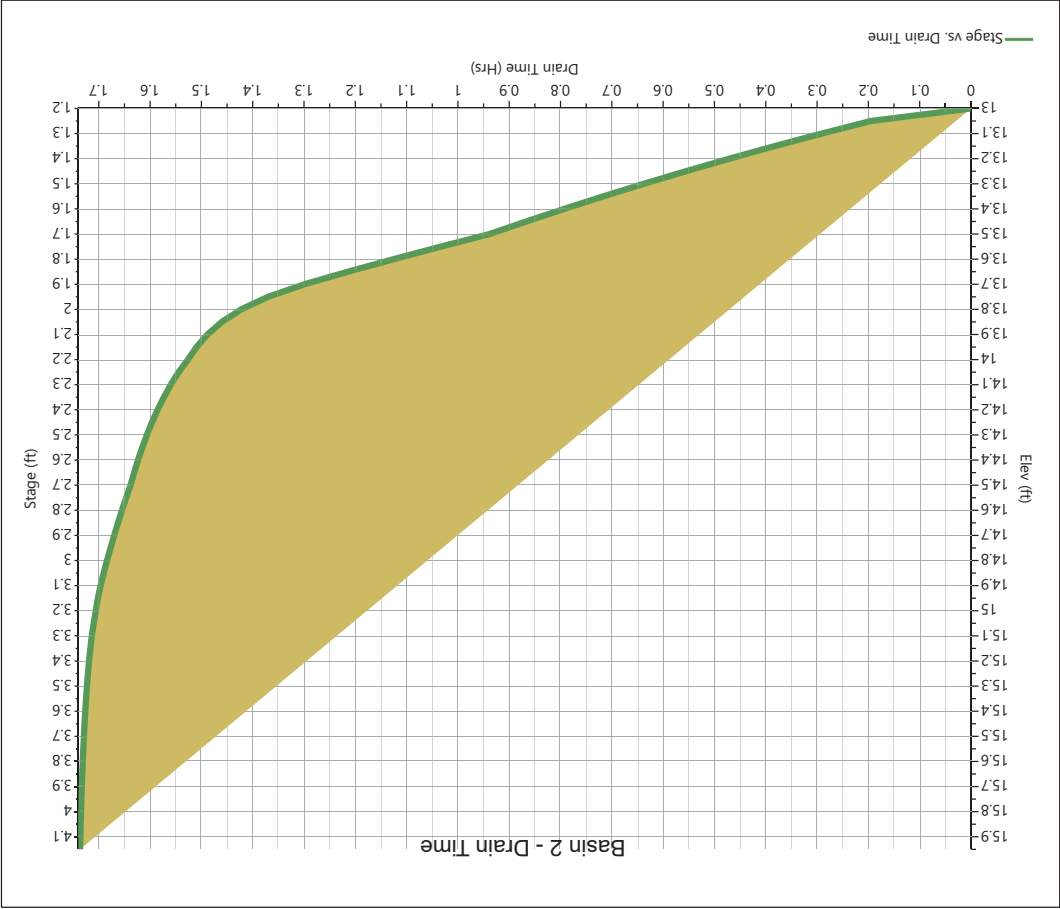
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Pond Drawdown



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Post Basin 1

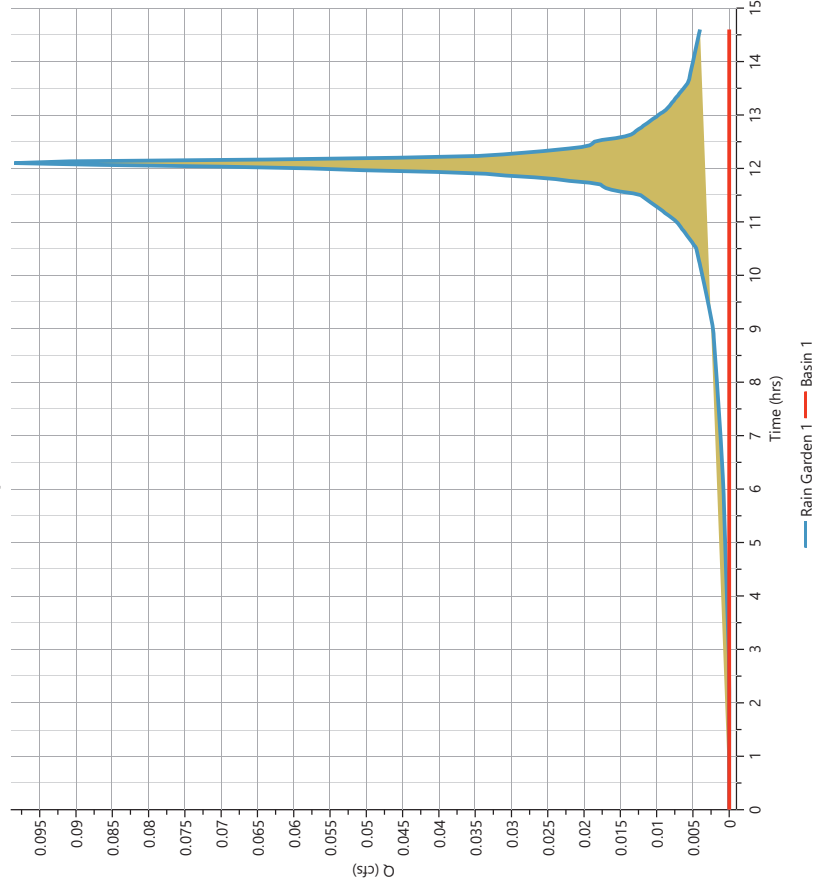
Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 14.57 hrs
Time Interval	= 2 min	Hydrograph Volume	= 0.000 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 13.34 ft
Pond Name	= BASIN 1	Max. Storage	= 79.7 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 1.47 hrs

Qp = 0.00 cfs



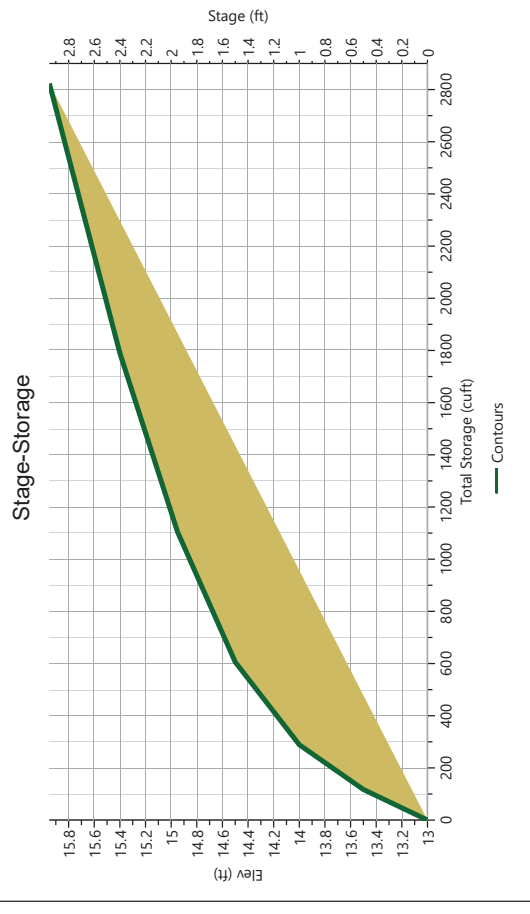
Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

BASIN 1

Stage-Storage

[illegible]

Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

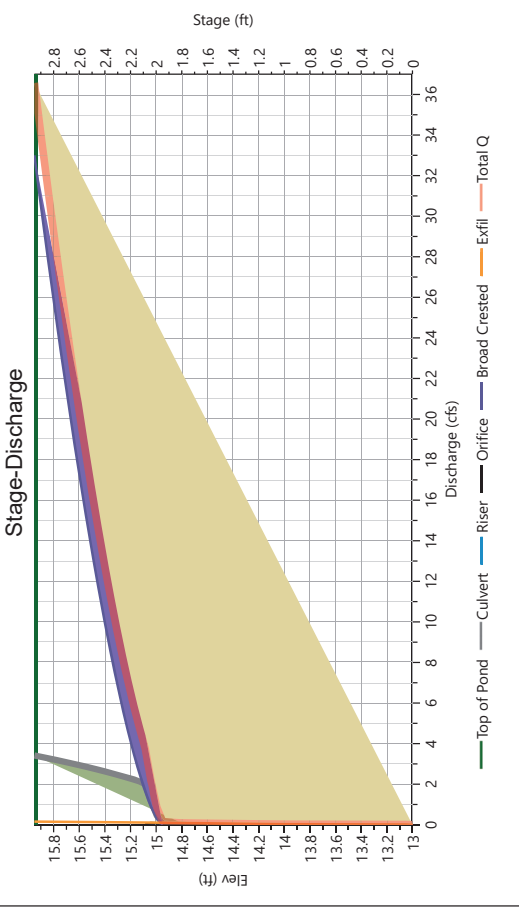
11-22-2022

BASIN 1

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2	3	
Rise, in	12	3			Orifice Dia, in
Span, in	12	3			No. Orifices
No. Barrels	1	1			Invert Elevation, ft
Invert Elevation, ft	13.90	14.77			Height, ft
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co
Length, ft	100				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
		1	2	3	
Shape / Type	Box	Broad Crested			Exfiltration, in/hr
Crest Elevation, ft	14.95	14.95			3.25**
Crest Length, ft	12	10			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to contours.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

BASIN 1

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000			0.000	0.000				0.000		0.000
0.50	13.50	117	0.000	0.000			0.000	0.000				0.022		0.022
1.00	14.00	289	0.000	0.000			0.000	0.000				0.030		0.030
1.50	14.50	606	0.000	0.000			0.000	0.000				0.066		0.066
1.95	14.95	1,104	0.055 oc	0.055			0.000	0.000				0.101		0.156
2.40	15.40	1,790	2.629 oc	0.000			0.000	9.962				0.128		12.72
2.95	15.95	2,823	3.415 oc	0.000			0.000	33.00				0.155		36.57

Suffix key: /c = inlet control, oc = outlet control, s = submerged weir

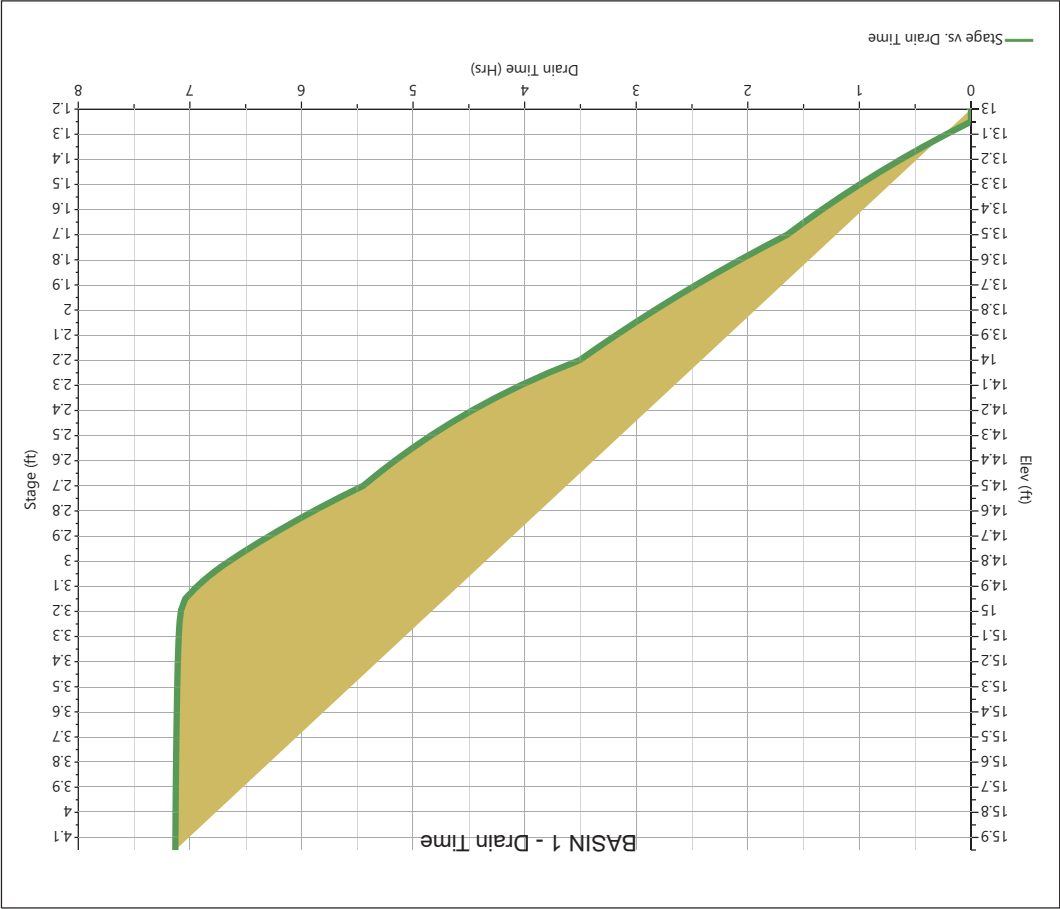
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

BASIN 1

Pond Drawdown

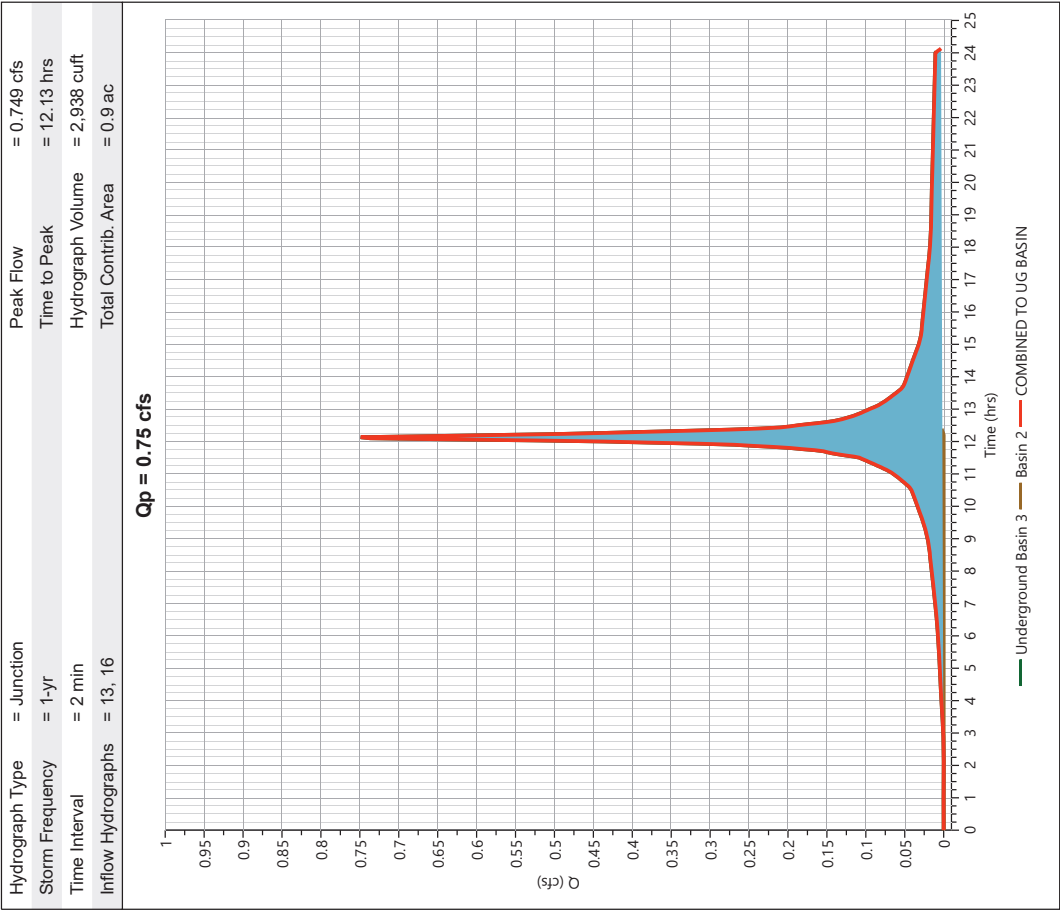


Hydrograph Report

Project Name:
11-22-2022

Post COMBINED TO UG BASIN

Hyd. No. 18



Hydrograph Report

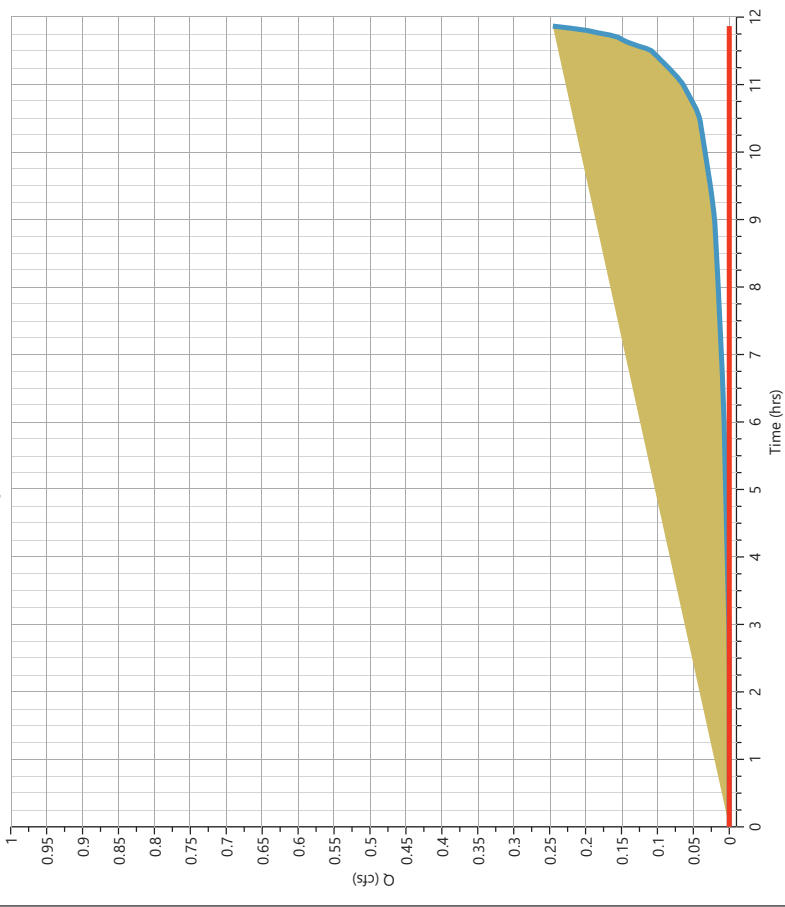
Post UG Basin 3

Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 11.83 hrs
Time Interval	= 2 min	Hydrograph Volume	= 0.000 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 11.81 ft
Pond Name	= UG BASIN	Max. Storage	= 48.3 cuft

Pond Routing by Storage Indication Method

Qp = 0.00 cfs



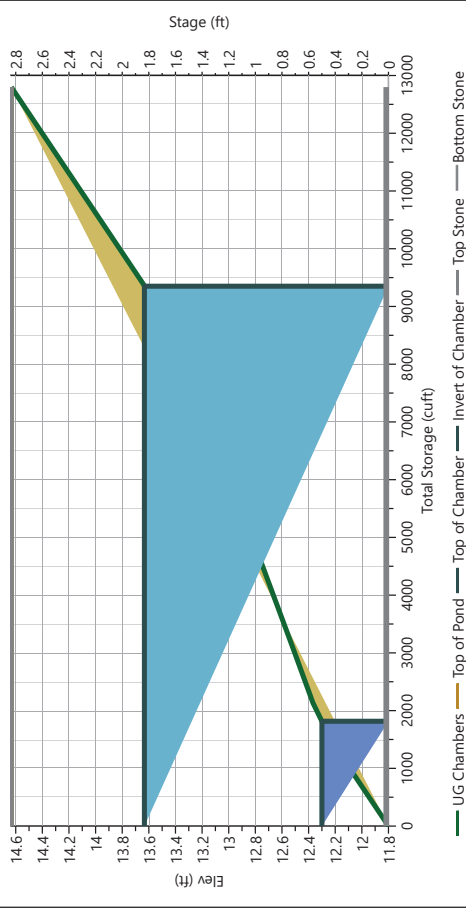
Pond Report

UG BASIN

Stage-Storage

StormTech® SC-310™ Chamber			Stage / Storage Table			
	Description	Input	Stage (in)	Elevation (ft)	Contour Area (sqft)	Total Storage (cuft)
	Chamber Height, in	16	0.0	11.80	8,601	0.000
	Chamber Shape	Arch	1.7	11.94	8,601	487
	Chamber Width, in	34	3.4	12.08	8,601	487
	Installed Length, ft	7.12	5.1	12.23	8,601	487
	No. Chambers	346	8.5	12.51	8,601	668
	Bare Chamber Stor, cuft	5,086	10.2	12.65	8,601	889
	No. Rows	20	11.9	12.79	8,601	877
	Space Between Rows, in	6	13.6	12.93	8,601	860
	Stone Above, in	12	15.3	13.08	8,601	836
	Stone Below, in	6	17.0	13.22	8,601	805
	Stone Sides, in	12	18.7	13.36	8,601	763
	Stone Ends, in	12	20.4	13.50	8,601	705
	Encasement Voids, %	40.00	22.1	13.64	8,601	626
	Encasement Bottom Elevation, ft	11.80	23.8	13.78	8,601	487
			25.5	13.93	8,601	487
			27.2	14.07	8,601	487
			28.9	14.21	8,601	487
			30.6	14.35	8,601	487
			32.3	14.49	8,601	487
			34.0	14.63	8,601	487

Stage-Storage



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

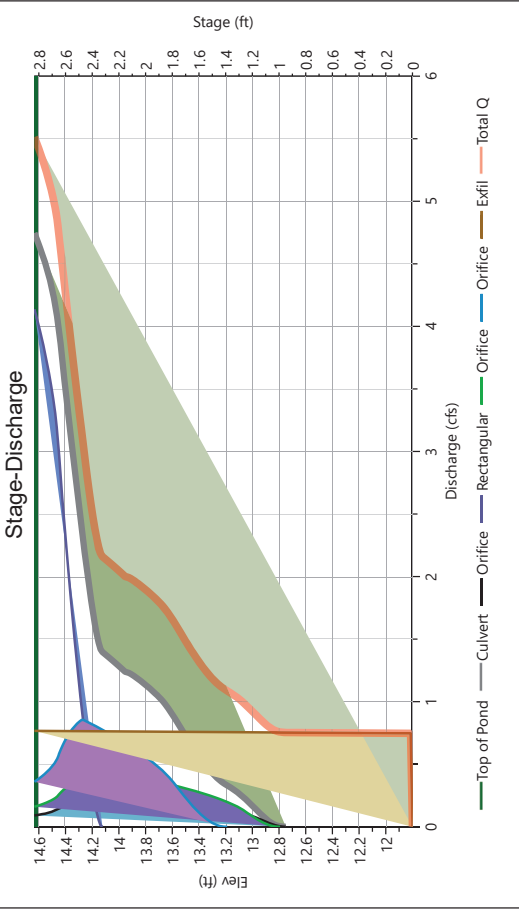
11-22-2022

UG BASIN

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Perforated Riser
		1*	2*	3*	
Rise, in	12	3	4	6	Hole Diameter, in
Span, in	12	3	4	6	No. holes
No. Barrels	1	1	1	1	Invert Elevation, ft
Invert Elevation, ft	12.20	12.75	12.80	13.20	Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60	0.60	Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs		Ancillary	
		1*	2		
Shape / Type		Rectangular			3.75**
Crest Elevation, ft		14.13			Exfiltration, in/hr
Crest Length, ft		5			
Angle, deg					
Weir Coefficient, Cw		3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to corrours.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

UG BASIN

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	11.80	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000		0.000
0.14	11.94	487	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.748		0.748
0.28	12.08	975	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.749		0.749
0.43	12.23	1,462	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.750		0.750
0.57	12.37	2,130	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.751		0.751
0.71	12.51	3,026	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.752		0.752
0.85	12.65	3,916	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.753		0.753
0.99	12.79	4,793	0.004 oc	0.004	0.000	0.000	0.000	0.000	0.000	0.000		0.754		0.757
1.13	12.93	5,653	0.098 oc	0.056	0.041	0.000	0.000	0.000	0.000	0.000		0.755		0.852
1.28	13.08	6,489	0.244 oc	0.106	0.138	0.000	0.000	0.000	0.000	0.000		0.756		1.000
1.42	13.22	7,294	0.349 oc	0.138	0.210	0.001	0.001	0.000	0.000	0.000		0.757		1.106
1.56	13.36	8,057	0.502 oc	0.164	0.263	0.074	0.000	0.000	0.000	0.000		0.758		1.259
1.70	13.50	8,761	0.724 oc	0.187	0.307	0.230	0.000	0.000	0.000	0.000		0.759		1.482
1.84	13.64	9,388	0.964 oc	0.203	0.345	0.416	0.000	0.000	0.000	0.000		0.760		1.724
1.98	13.78	9,875	1,120 oc	0.207	0.367	0.546	0.000	0.000	0.000	0.000		0.761		1.880
2.13	13.93	10,363	1,230 oc	0.208	0.370	0.652	0.000	0.000	0.000	0.000		0.762		1.991
2.27	14.07	10,850	1,346 oc	0.217	0.386	0.742	0.000	0.000	0.000	0.000		0.763		2.109
2.41	14.21	11,337	1,807 oc	0.224	0.398	0.823	0.362	0.000	0.000	0.000		0.764		2,571
2.55	14.35	11,825	3,024 oc	0.195	0.347	0.780	1,703	0.000	0.000	0.000		0.765		3,789
2.69	14.49	12,312	4,255 oc	0.124	0.220	0.495	3,417 s	0.000	0.000	0.000		0.766		5,021
2.83	14.63	12,800	4,747 oc	0.090	0.160	0.360	4,137 s	0.000	0.000	0.000		0.767		5,513

Suffix key: /c = inlet control, oc = outlet control, s = submerged weir

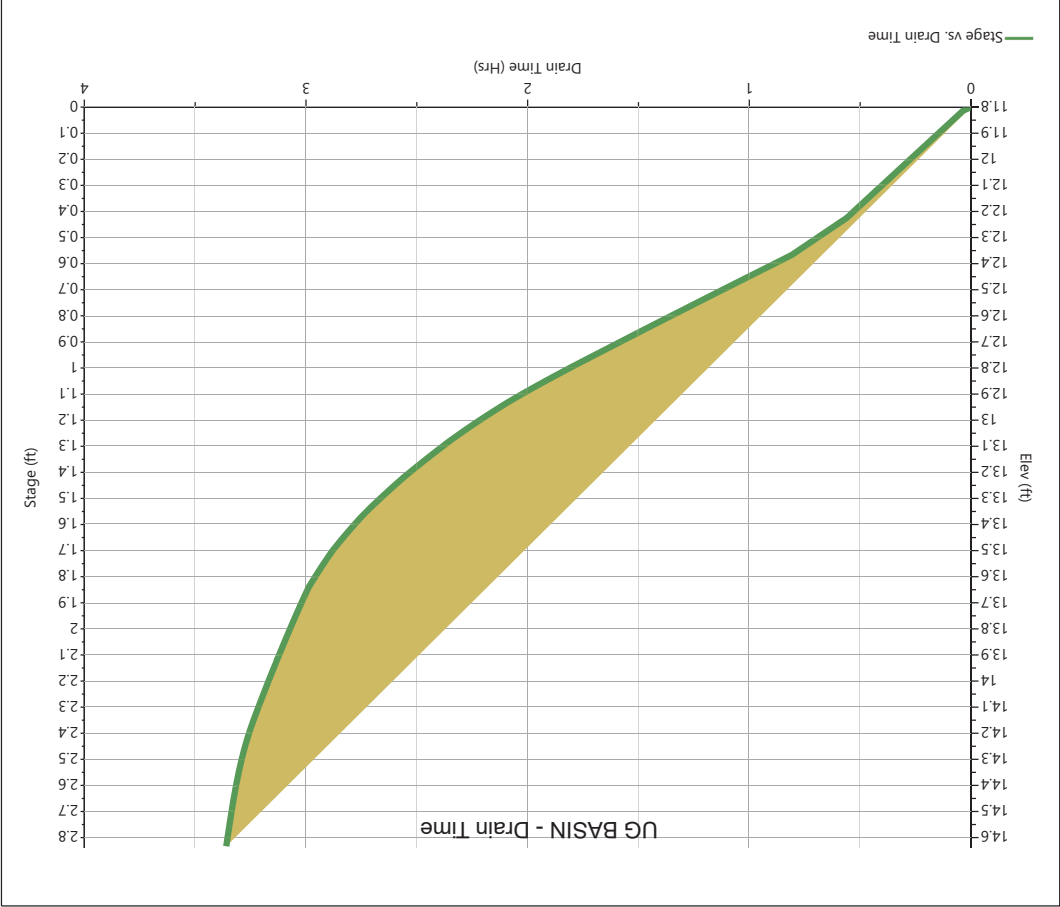
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

UG BASIN

Pond Drawdown



Hydrograph Report

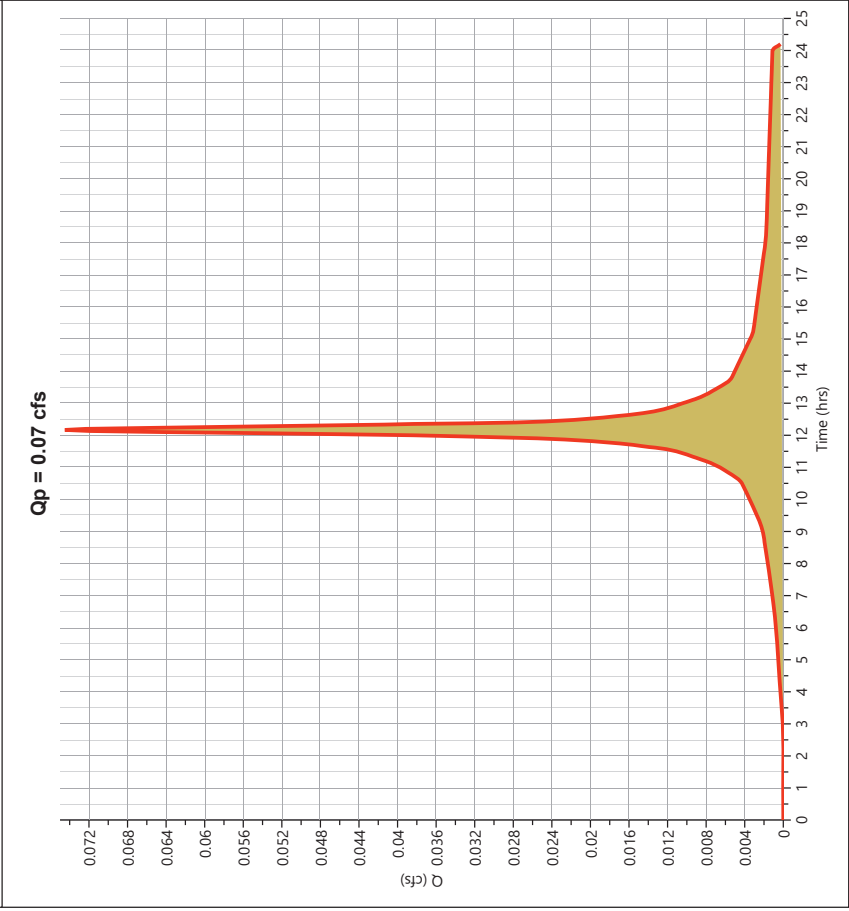
Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.075 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 310 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.58	39	OPEN SPACE	
0.28	30	WOODS	
0.86	36	Weighted CN Method Employed	

Qp = 0.00 cfs

Hydrograph Report

Project Name:
11-22-2022

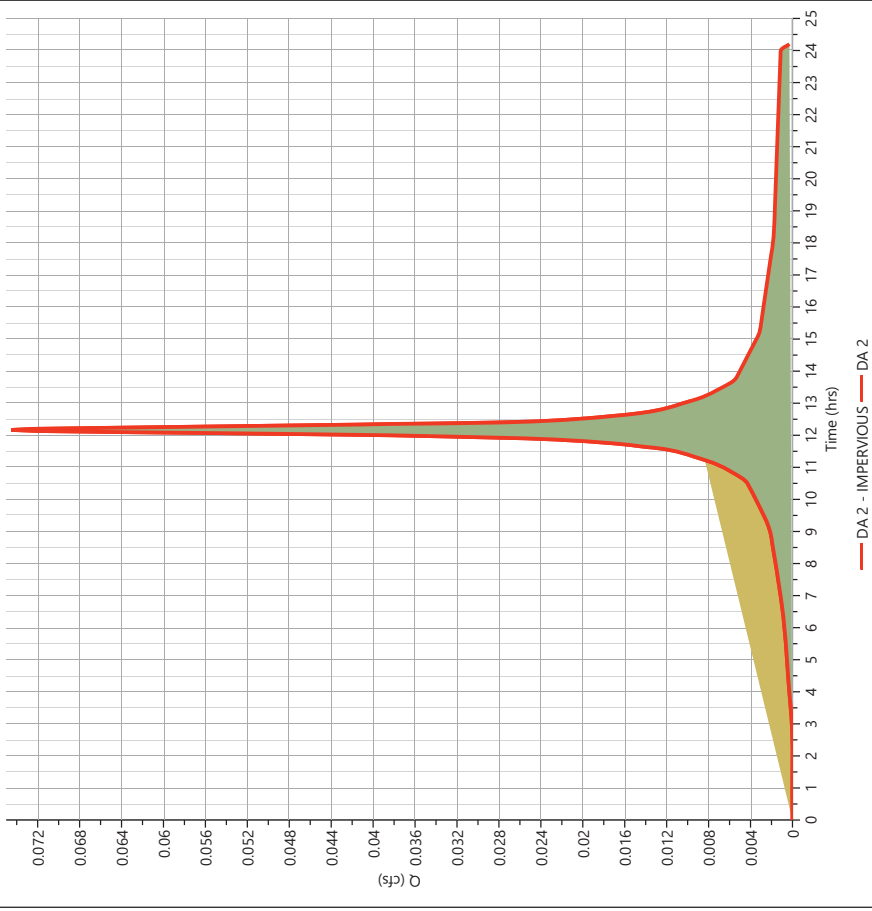
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.075 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 310 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.07 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	39	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	

Qp = 0.00 cfs

Hydrograph Report

Project Name:

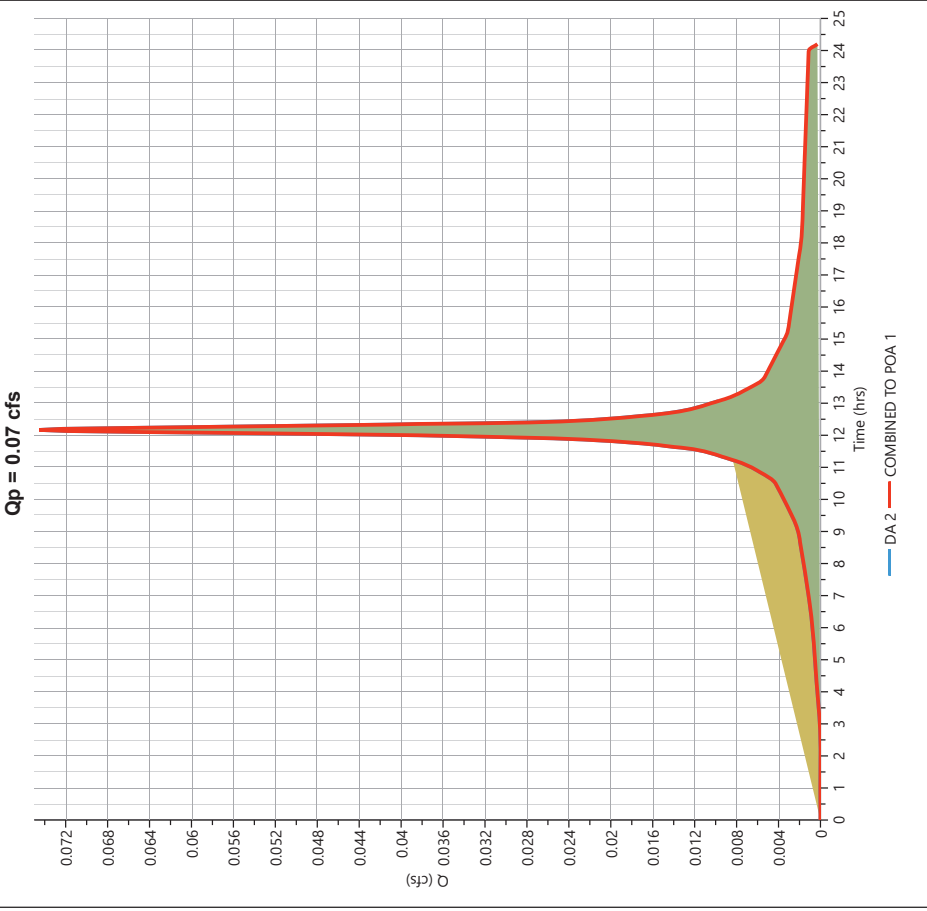
11-22-2022

Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.075 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 310 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac



Hydrograph Report

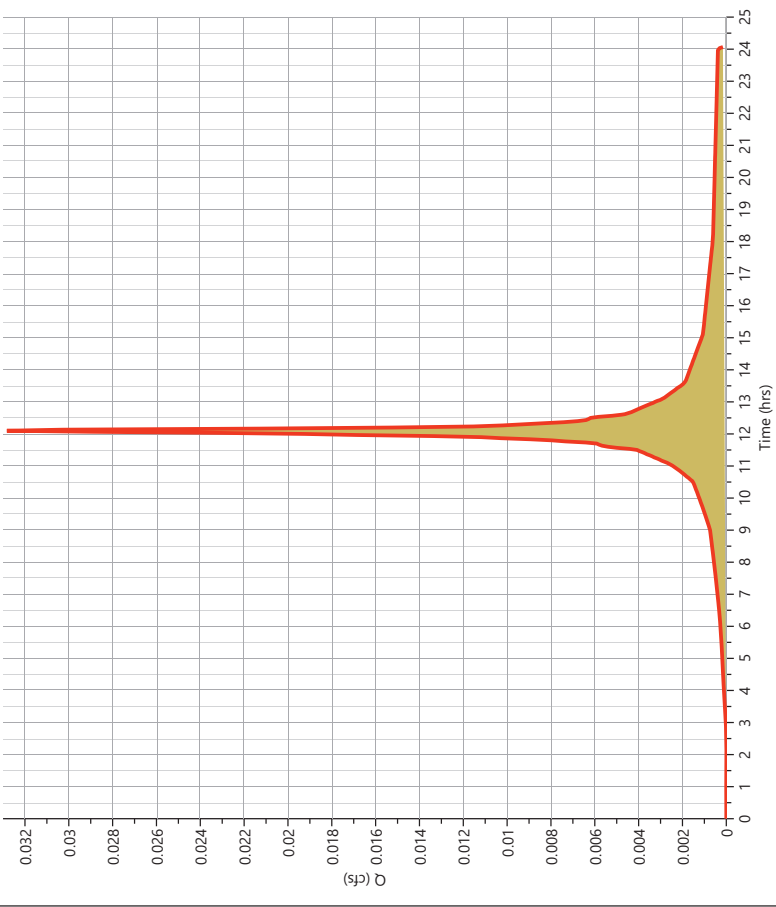
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.033 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 106 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.03 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



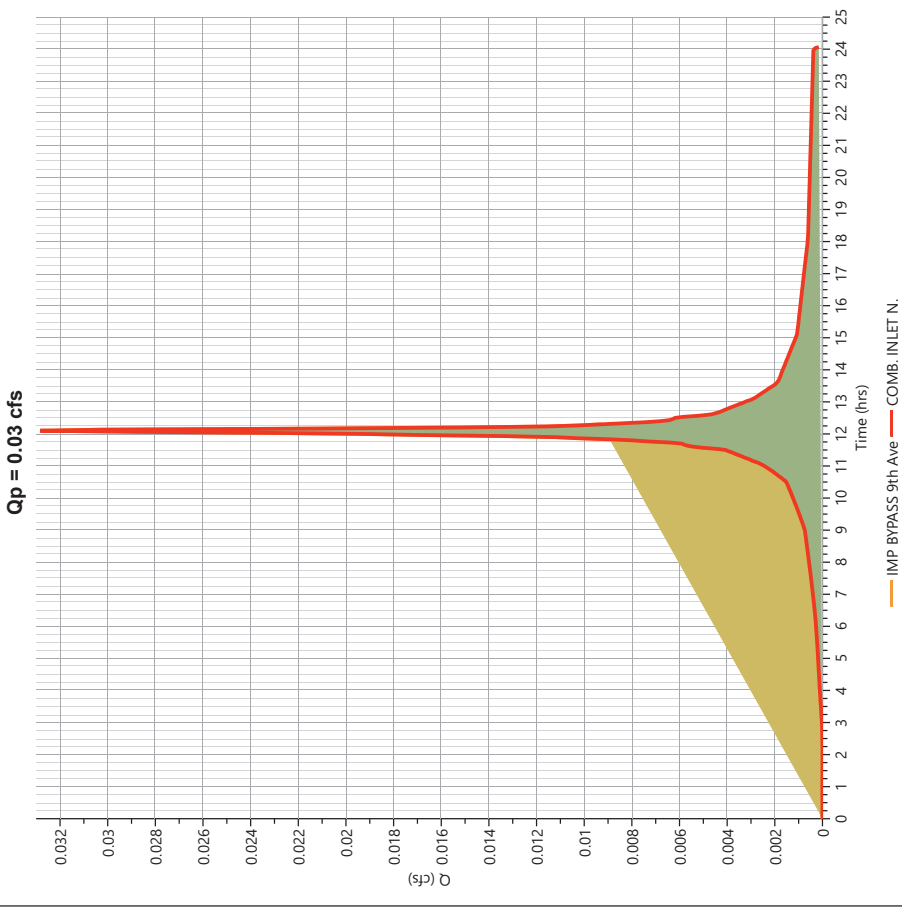
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.033 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 106 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



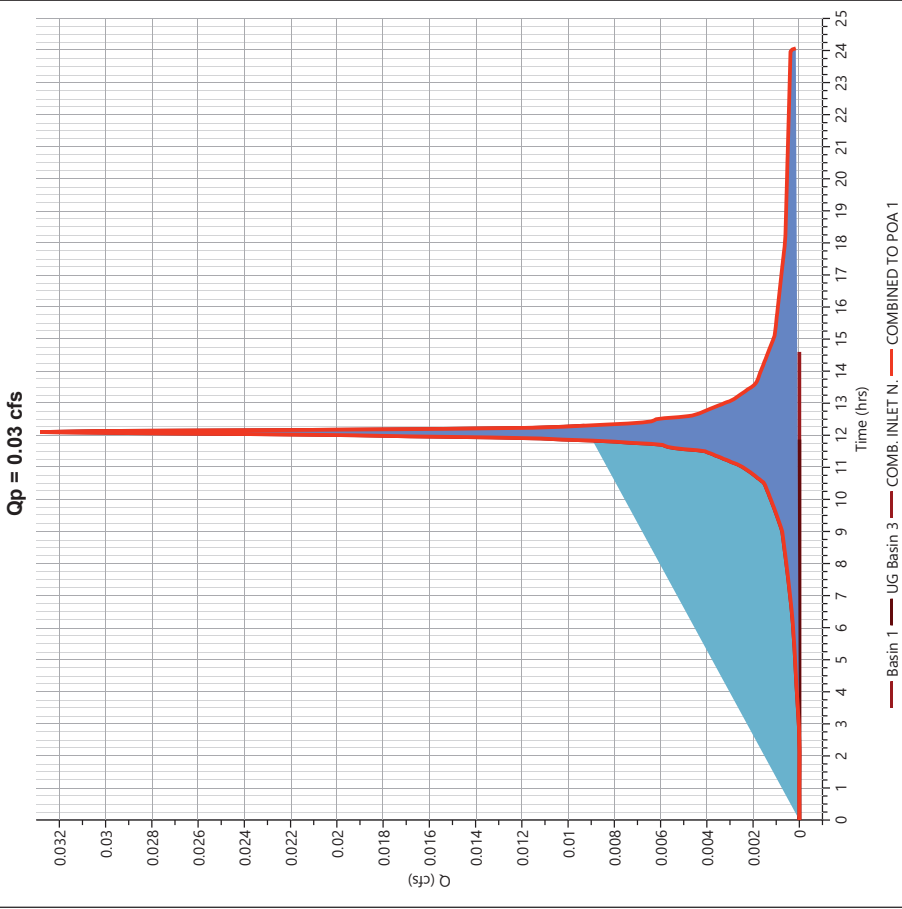
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 0.033 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 106 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs

Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs

Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

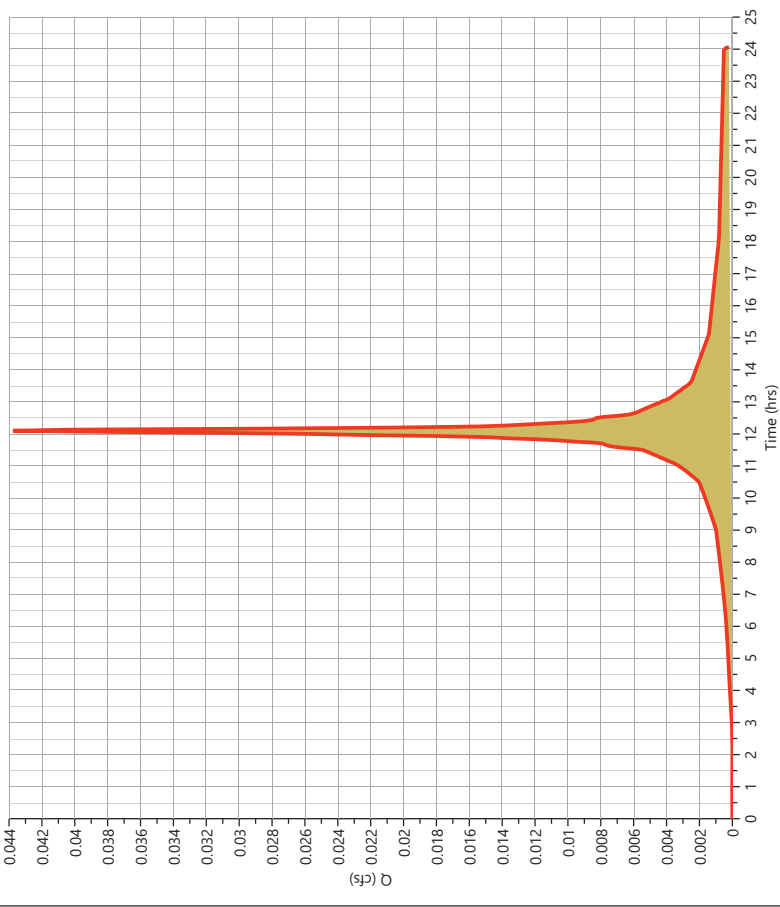
11-22-2022

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.044 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 141 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.04 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 1-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 1.25 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

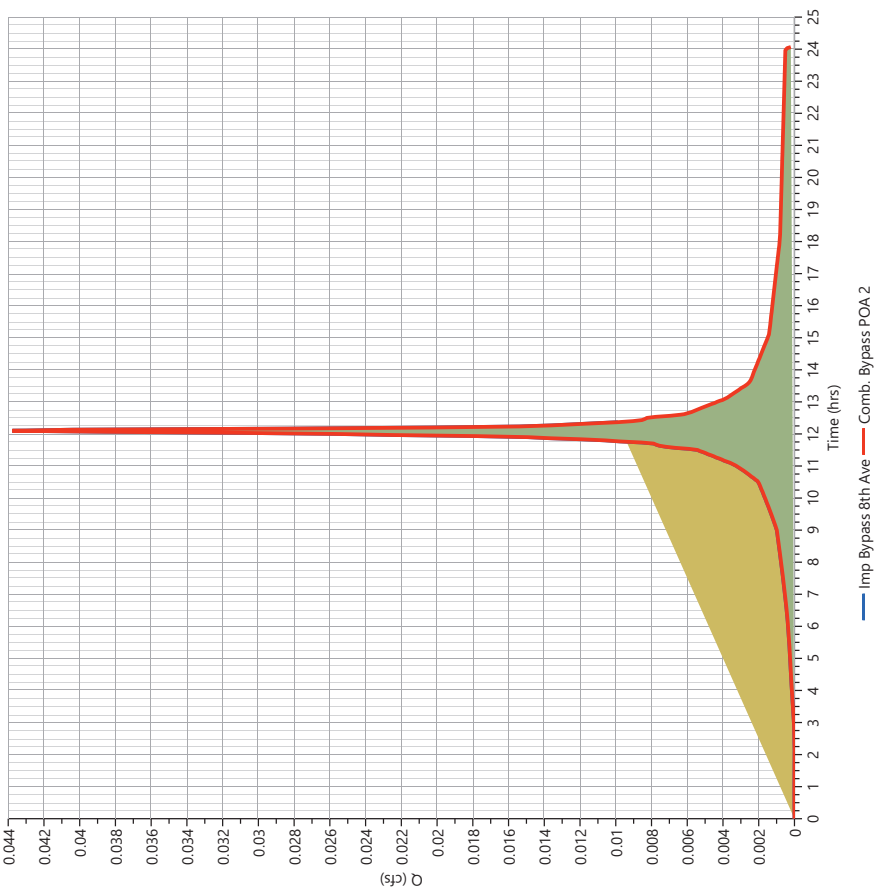
11-22-2022

Post Comb. Bypass POA 2

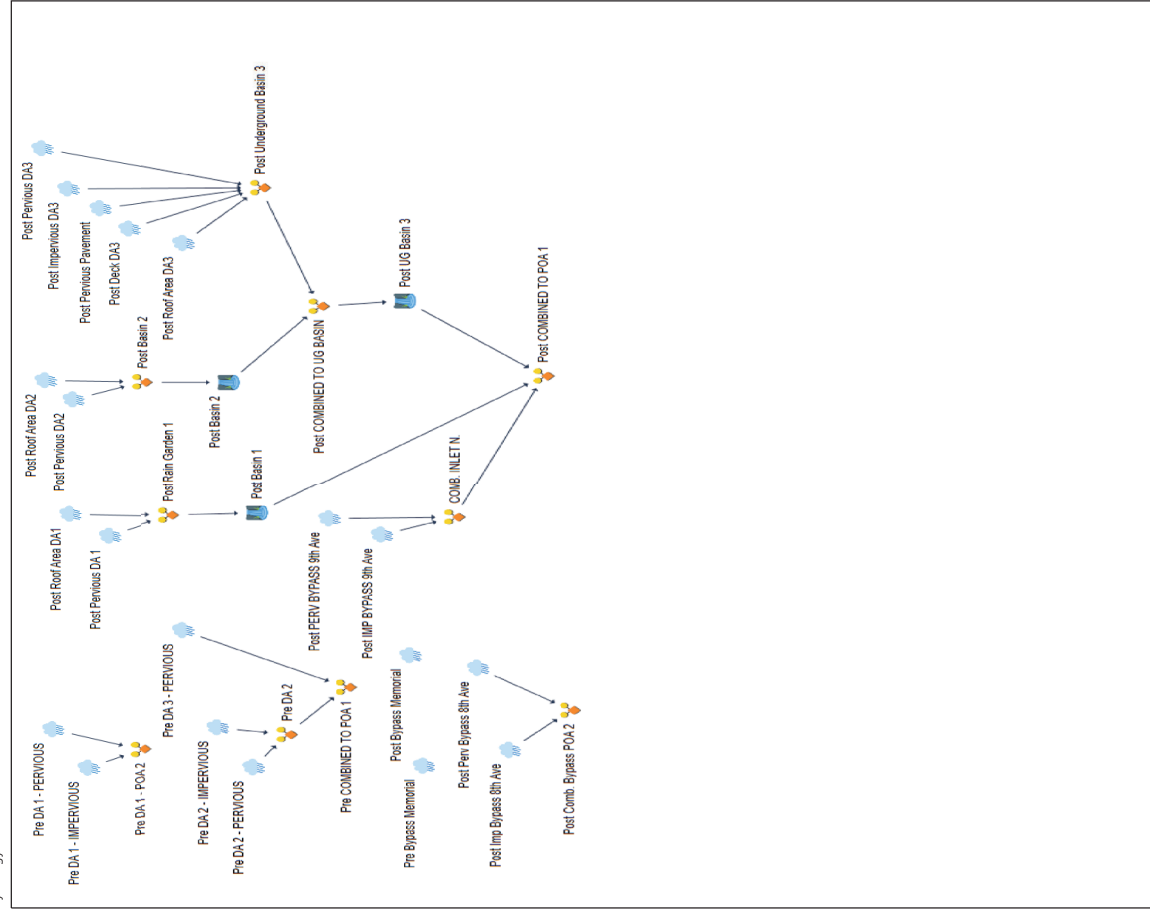
Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.044 cfs
Storm Frequency	= 1-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 141 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.04 cfs



**HYDROGRAPH SUMMARY REPORTS –
EMERGENCY SPILLWAY**



Hydrograph 100-yr Summary

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuf)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuf)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.322	12.13	1,311	---		
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.060	12.17	273	---		
3	NRCS Runoff	Post Roof Area DA3	3.025	12.10	10,755	---		
4	NRCS Runoff	Post Deck DA3	1.624	12.17	7,436	---		
5	NRCS Runoff	Post Pervious Pavement	1.108	12.17	5,070	---		
6	NRCS Runoff	Post Pervious DA 1	0.066	12.13	236	---		
7	NRCS Runoff	Post Roof Area DA1	0.778	12.10	2,766	---		
8	NRCS Runoff	Post Pervious DA2	0.099	12.13	354	---		
9	NRCS Runoff	Post Roof Area DA2	4.321	12.10	15,364	---		
10	Junction	Pre DA 1 - POA 2	0.379	12.13	1,584	1, 2		
11	NRCS Runoff	Post Pervious DA3	0.149	12.20	713	---		
12	NRCS Runoff	Post Impervious DA3	0.517	12.17	2,366	---		
13	Junction	Post Underground Basin 3	6.052	12.13	26,341	3, 4, 5, 11, 12		
14	Junction	Post Rain Garden 1	0.843	12.10	3,001	6, 7		
15	Junction	Post Basin 2	4.419	12.10	15,718	8, 9		
16	Pond Route	Post Basin 2	4.052	12.10	5,530	15	15.06	1,860
17	Pond Route	Post Basin 1	0.071	12.43	61.5	14	14.95	1,111
18	Junction	Post COMBINED TO UG BASIN	10.05	12.13	31,871	13, 16		
19	Pond Route	Post UG Basin_3	0.993	12.57	1,713	18	14.28	11,596
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.591	12.17	2,704	---		
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.810	12.20	4,475	---		
22	Junction	Pre DA 2	1.381	12.20	7,179	20, 21		
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.809	12.20	4,218	---		
24	Junction	Pre COMBINED TO POA 1	2.190	12.20	11,397	22, 23		
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.259	12.10	922	---		
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.132	12.13	471	---		
27	Junction	COMB. INLET N.	0.390	12.10	1,393	25, 26		
28	Junction	Post COMBINED TO POA 1	1.115	12.57	3,168	17, 19, 27		
29	NRCS Runoff	Pre Bypass Memorial	0.041	12.20	194	---		
30	NRCS Runoff	Post Bypass Memorial	0.095	12.20	454	---		
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.346	12.10	1,229	---		
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.033	12.13	118	---		
33	Junction	Post Comb. Bypass POA 2	0.378	12.10	1,347	31, 32		

Hydrograph Report

Project Name:

11-22-2022

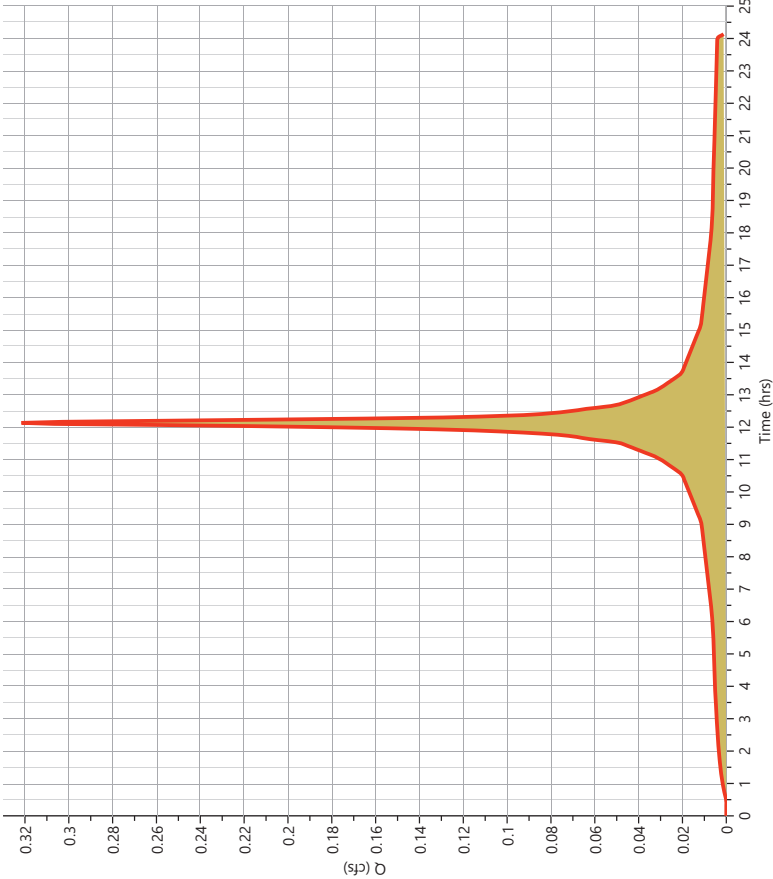
Hydrology Studio v 3.0.0.26

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.322 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 1,311 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.32 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

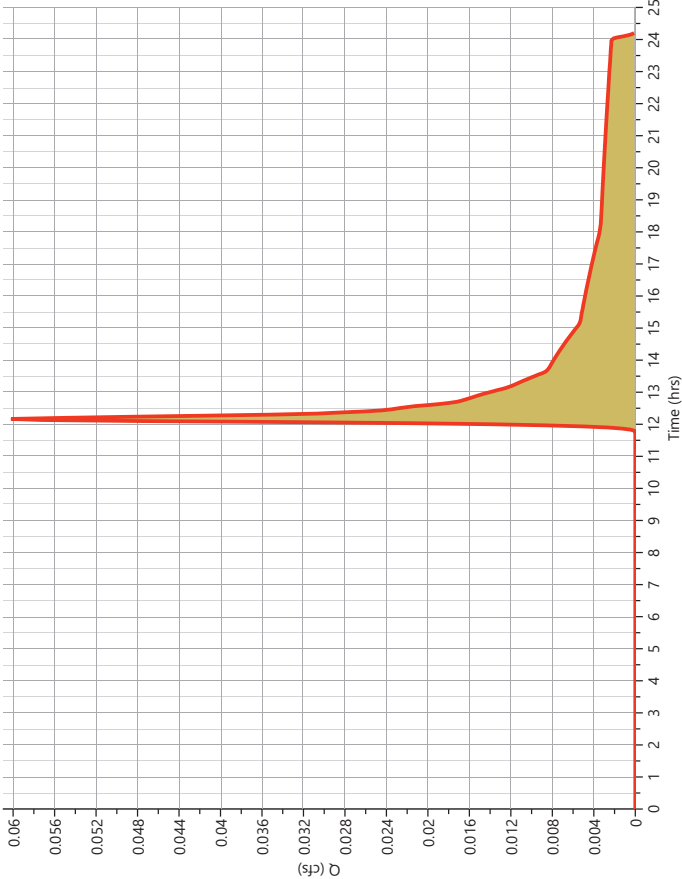
Pre DA 1 - PERVIOUS

Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.060 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 273 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet		
AREA (ac)	CN	DESCRIPTION
0.04	98	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.06 cfs



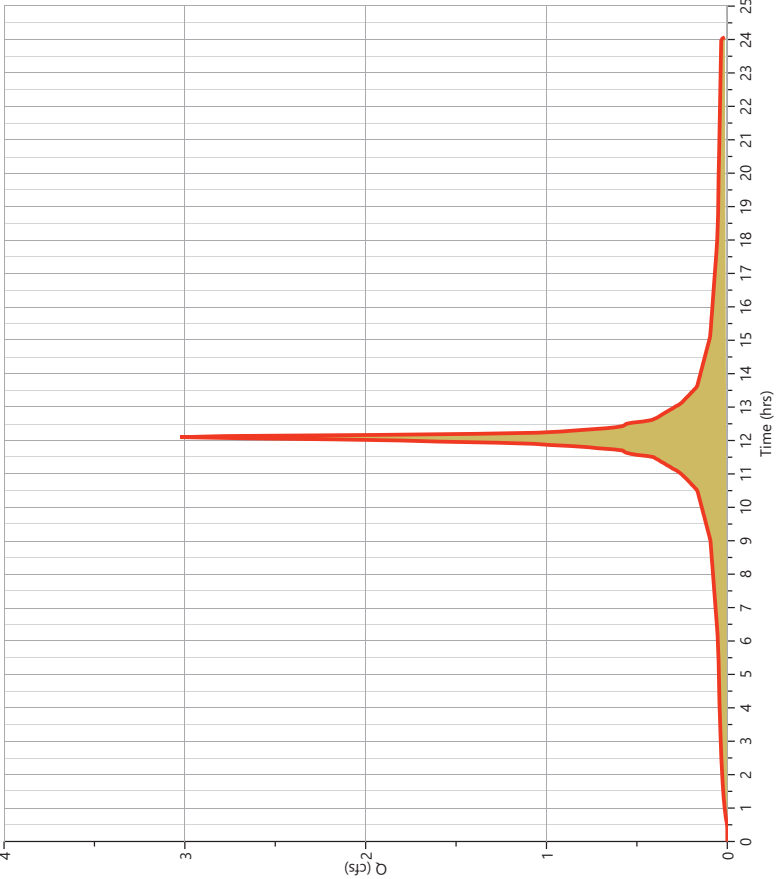
Hydrograph Report

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 3.025 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 10,755 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 3.03 cfs



Hydrograph Report

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.624 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 7,436 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.62 cfs



Hydrograph Report

Project Name:
11-22-2022

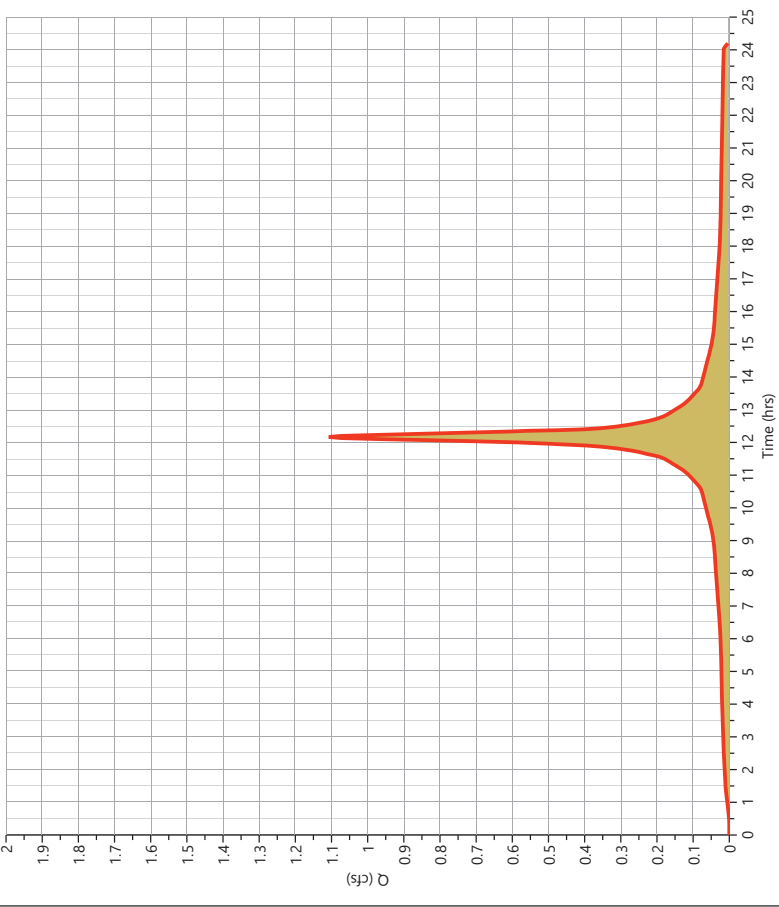
Hydrology Studio v 3.0.0.26

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1,108 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 5,070 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.11 cfs



Hydrograph Report

Project Name:
11-22-2022

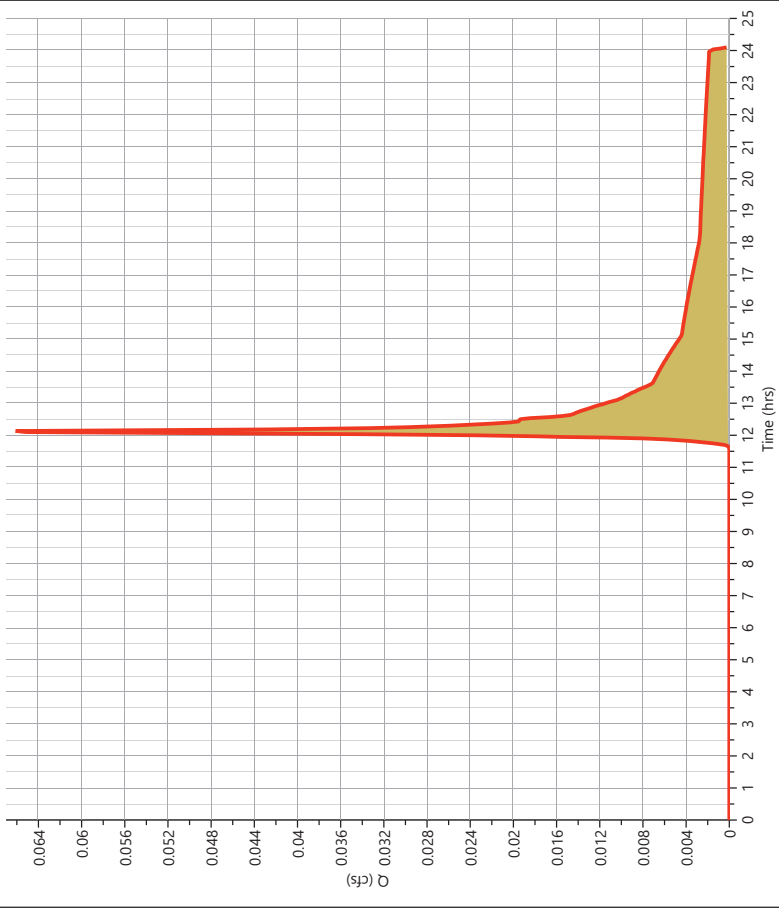
Hydrology Studio v 3.0.0.26

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.066 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 236 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.07 cfs



Hydrograph Report

Project Name:
11-22-2022

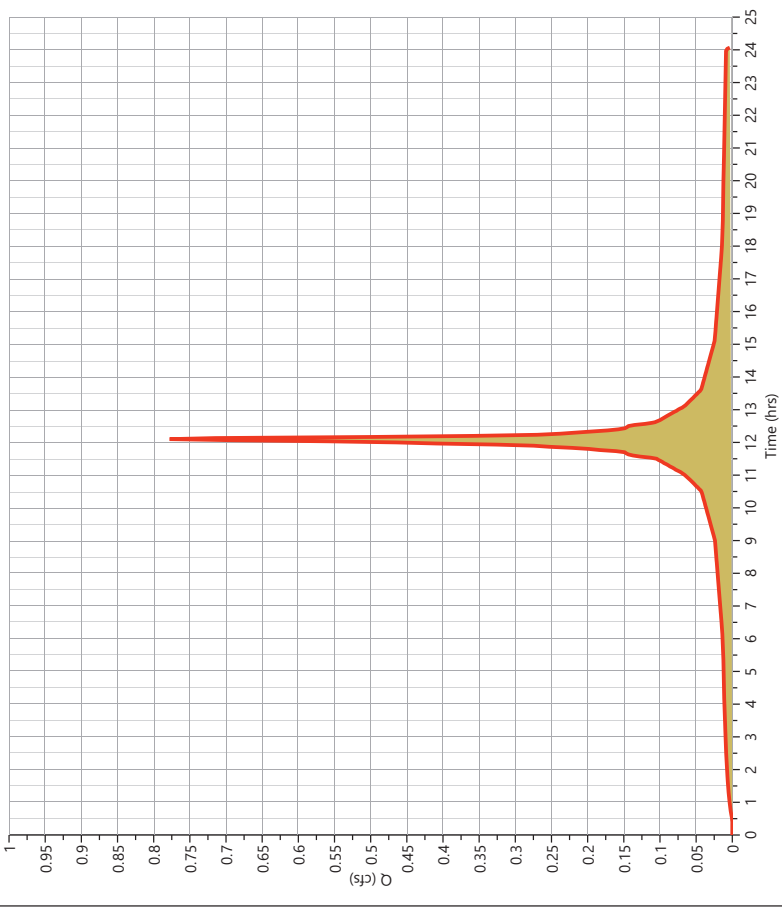
Hydrology Studio v 3.0.0.26

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.778 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 2,766 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.78 cfs



Hydrograph Report

Project Name:
11-22-2022

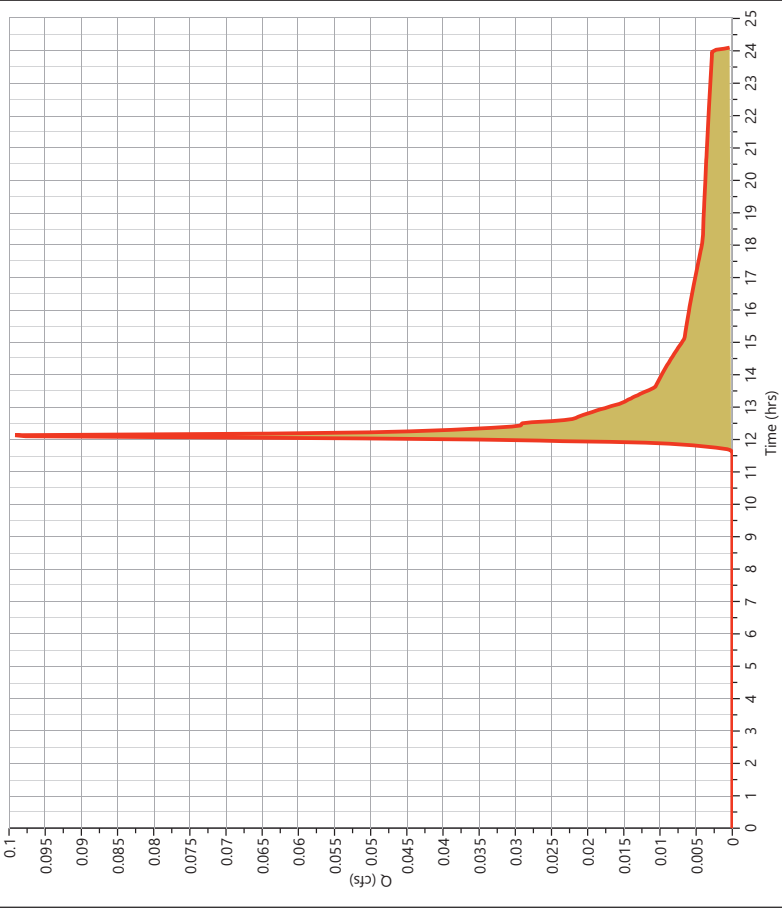
Hydrology Studio v 3.0.0.26

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.099 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 354 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.10 cfs



Hydrograph Report

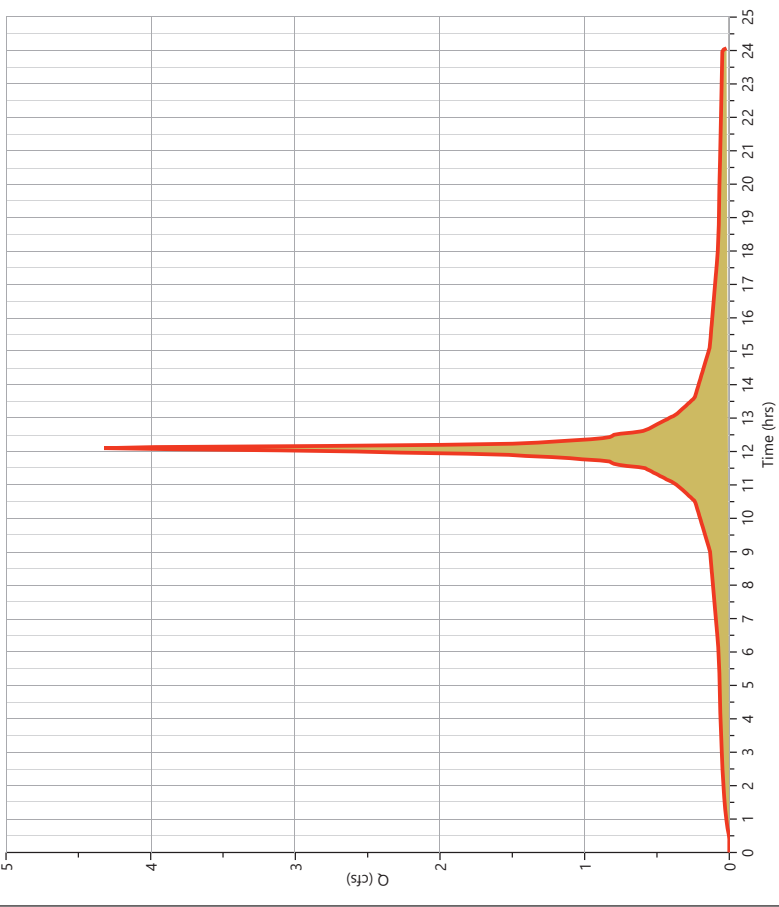
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 4.321 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 15,364 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 4.32 cfs



Hydrograph Report

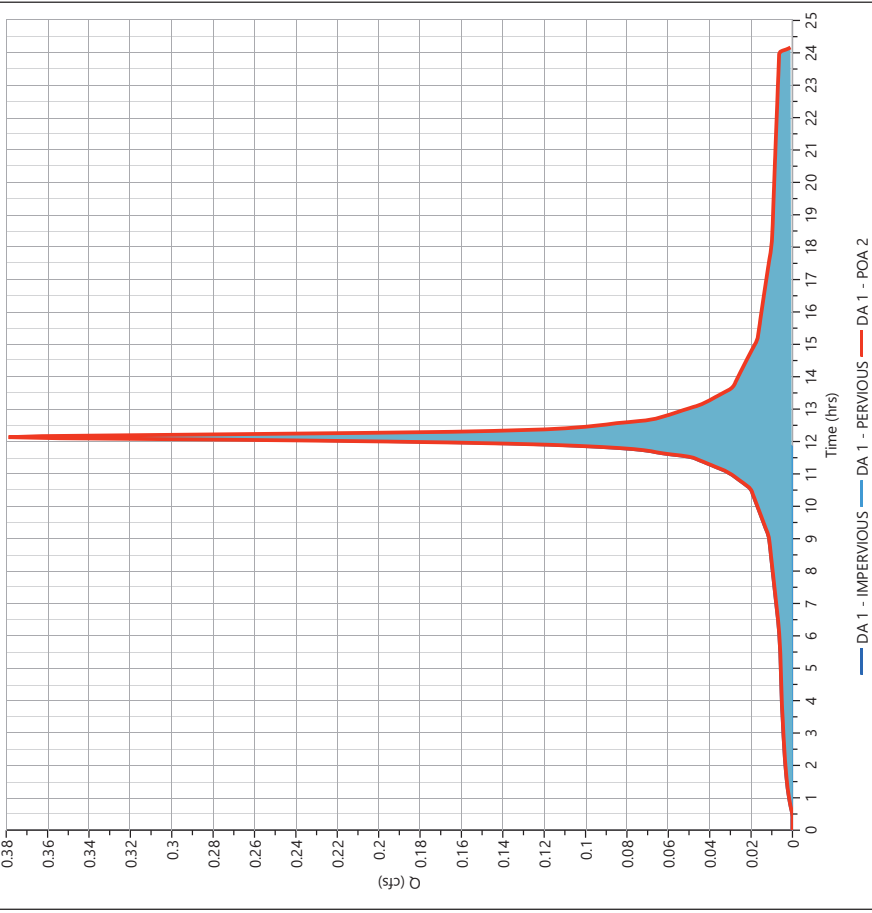
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.379 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,584 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.38 cfs



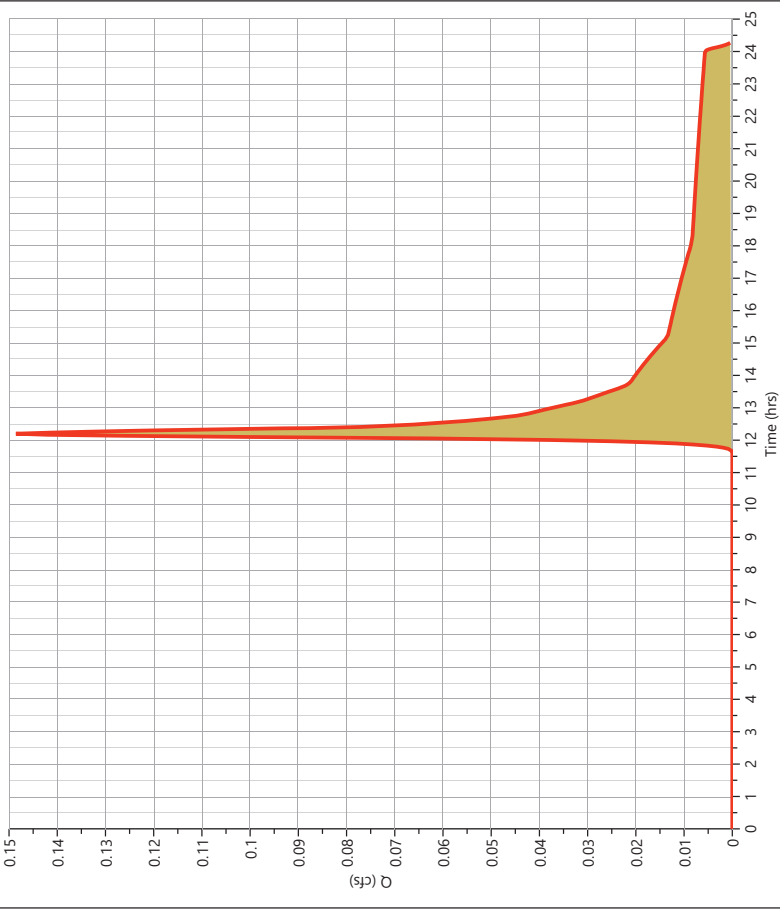
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.149 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 713 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.15 cfs



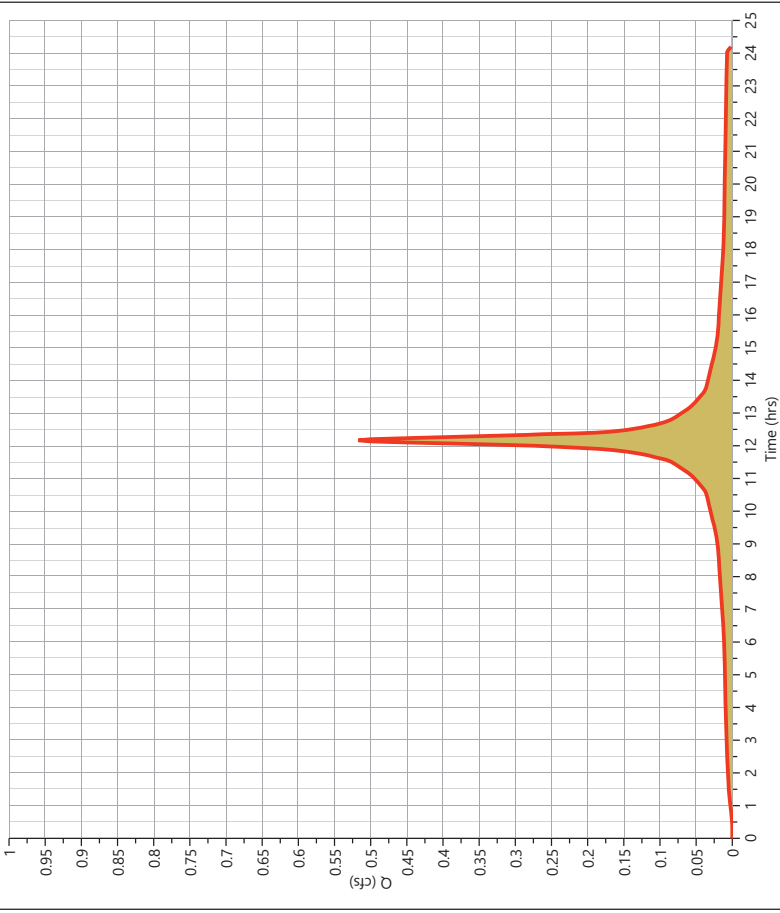
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.517 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,366 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.52 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

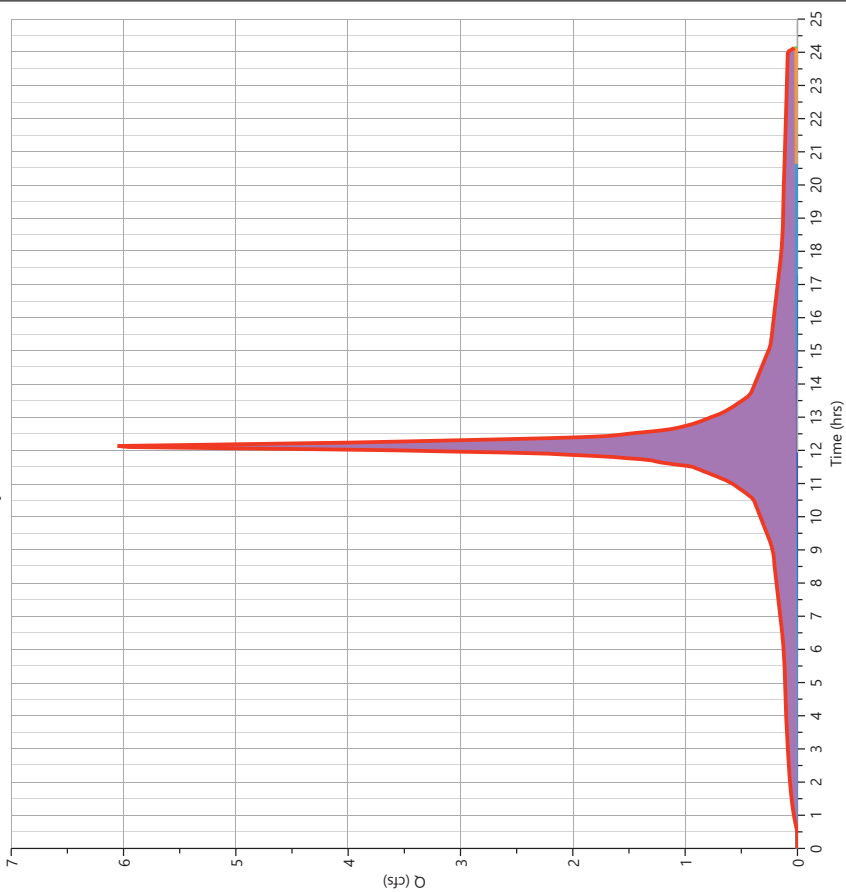
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 6.052 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 26,341 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 6.05 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

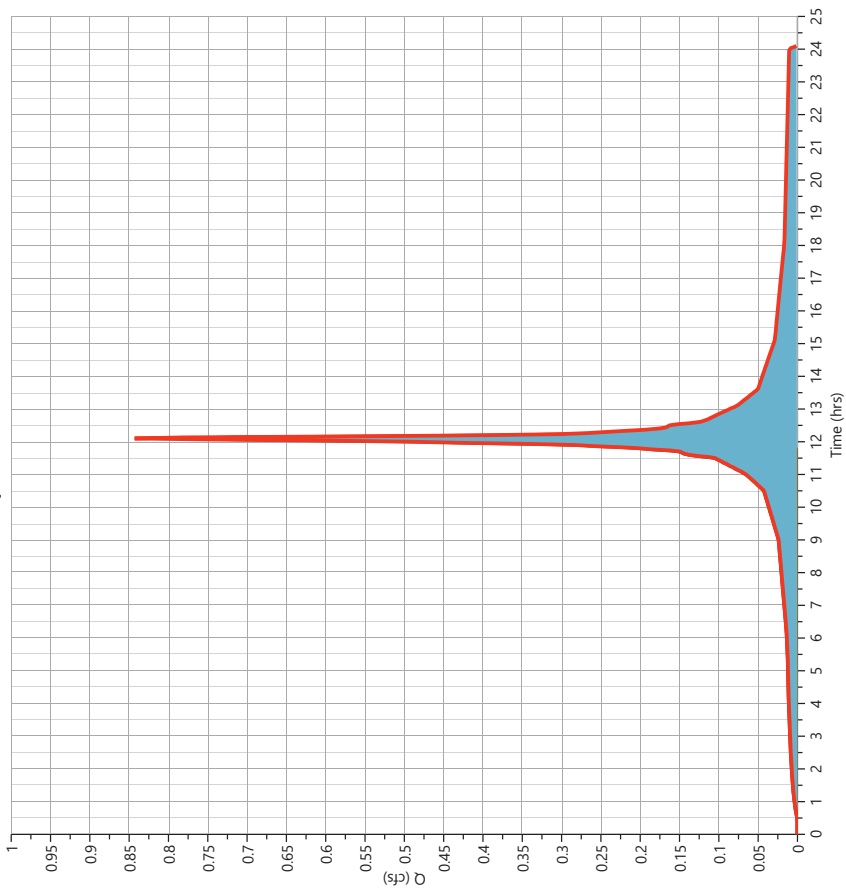
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.843 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 3,001 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.84 cfs



Hydrograph Report

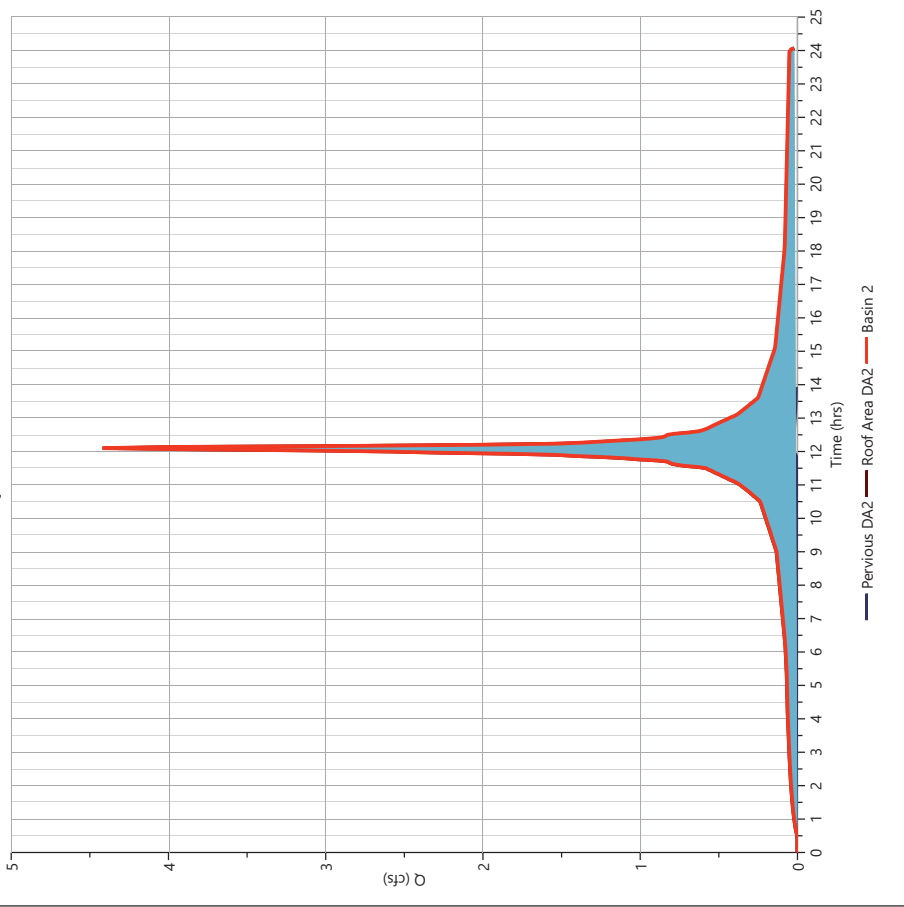
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 4,419 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 15,718 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 4.42 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

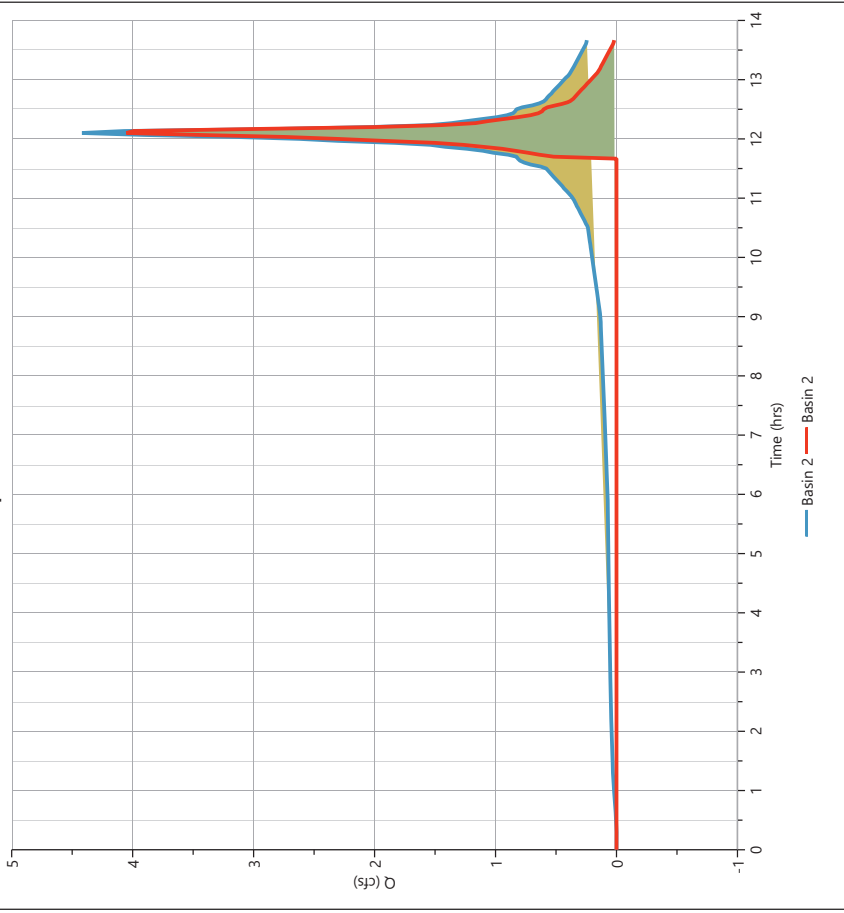
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 4,052 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 5,530 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 15.06 ft
Pond Name	= Basin 2	Max. Storage	= 1,860 cuft

Pond Routing by Storage Indication Method

Qp = 4.05 cfs



Pond Report

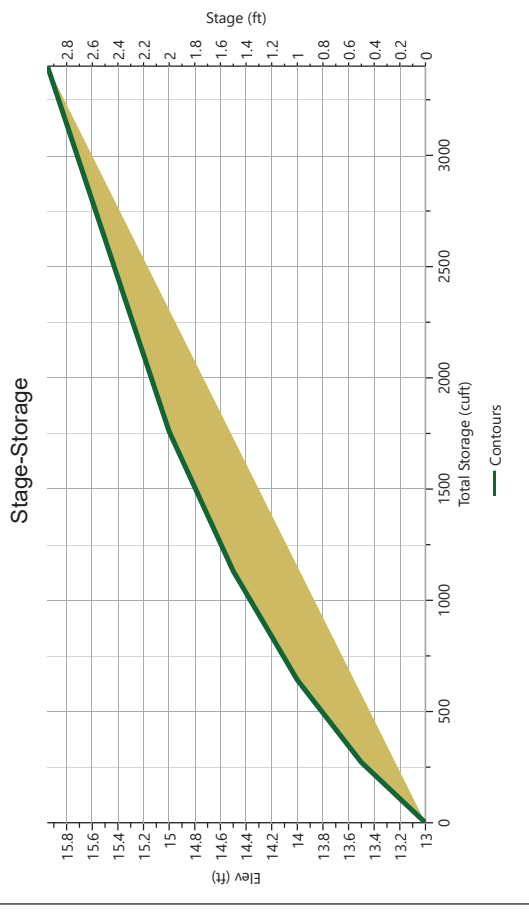
Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Storage

[illegible]

Pond Report

Project Name:

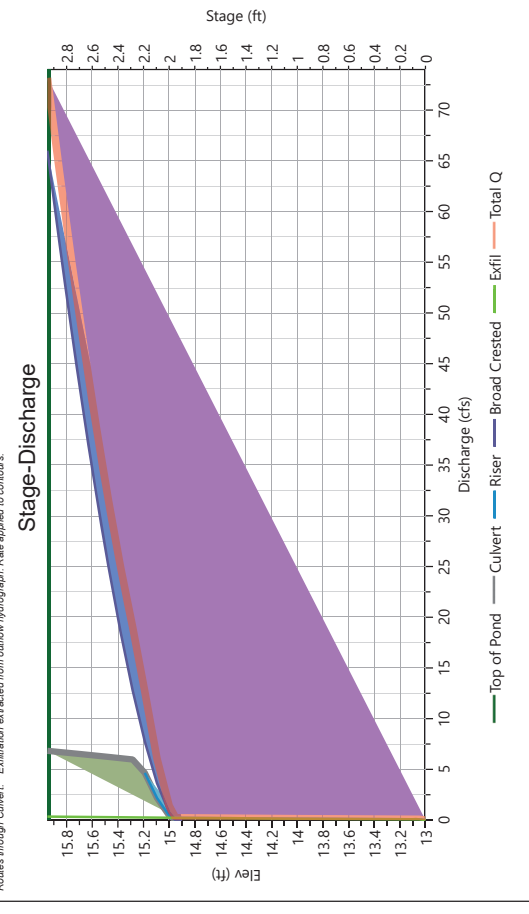
Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2*	3	
Rise, in	12	6	4		Orifice Dia, in
Span, in	12	6	4		No. Orifices
No. Barrels	1	2	2		Invert Elevation, ft
Invert Elevation, ft	12.19	13.67	14.15		Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60		Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
Shape / Type	Box	1	2*	3	
Crest Elevation, ft	14.95	Broad Crested	Rectangular		Exfiltration, in/hr
Crest Length, ft	12	14.95	14.82		
Angle, deg		20	5		
Weir Coefficient, Cw	3.3	3.3	3.3		



Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000
0.50	13.50	271	0.000 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.107	0.107		0.107
1.0	14.00	642	0.000 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.140	0.140		0.140
1.50	14.50	1,132	0.000 oc	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.186	0.186		0.186
2.00	15.00	1,759	0.443 oc	0.000	0.000	0.000	0.443	0.738	0.000	0.000	0.232	0.232		1.413
2.95	15.95	3,401	6.826 oc	0.000	0.000	0.000	0.000	66.00	0.000	0.000	0.344	0.344		73.17

Suffix: rcy, ic = inlet control, oc = outlet control, s = submerged weir

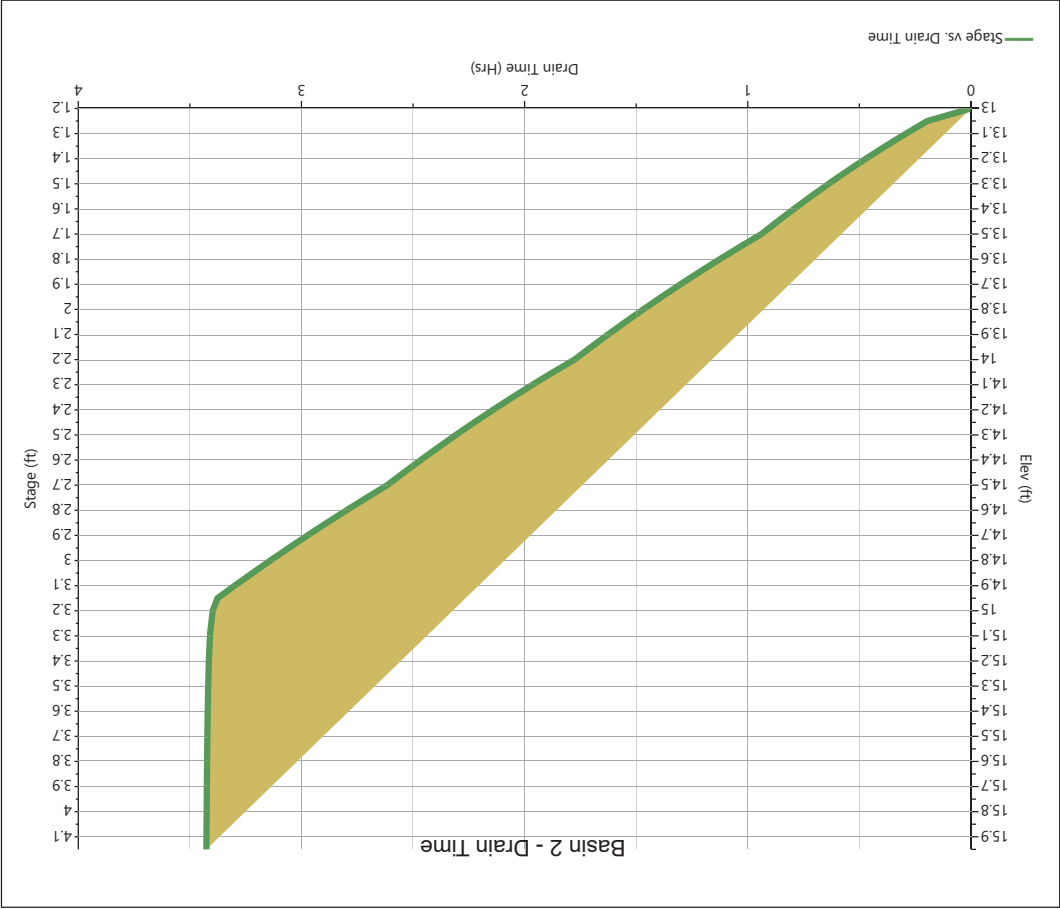
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Pond Drawdown



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

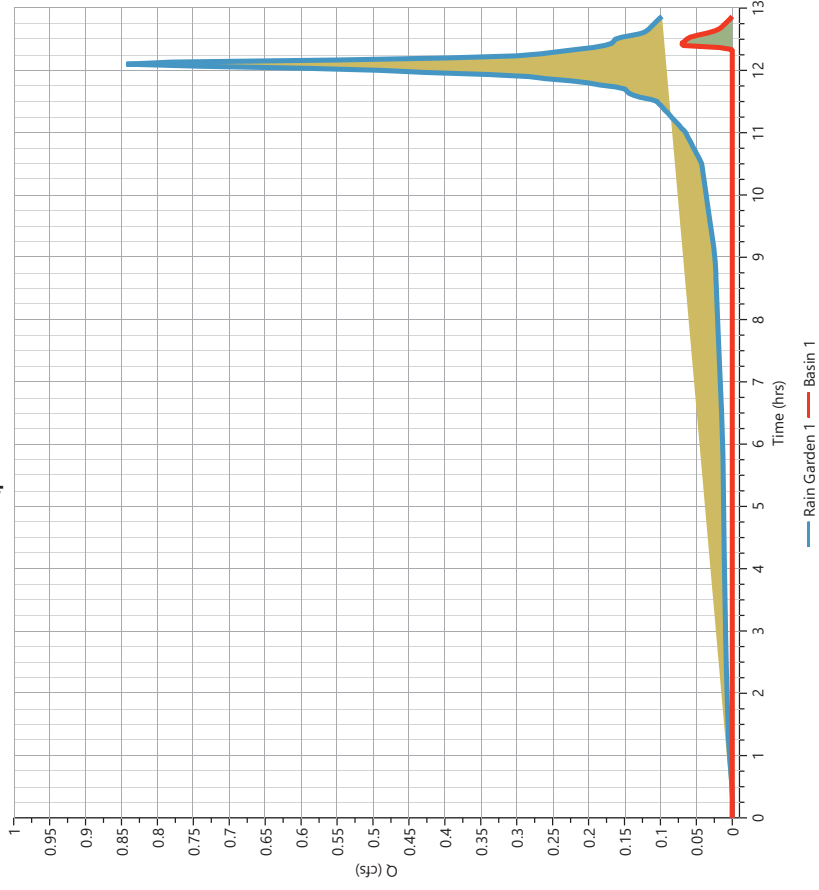
Post Basin 1

Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.071 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.43 hrs
Time Interval	= 2 min	Hydrograph Volume	= 61.5 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.95 ft
Pond Name	= BASIN 1	Max. Storage	= 1,111 cuft

Pond Routing by Storage Indication Method

Qp = 0.07 cfs



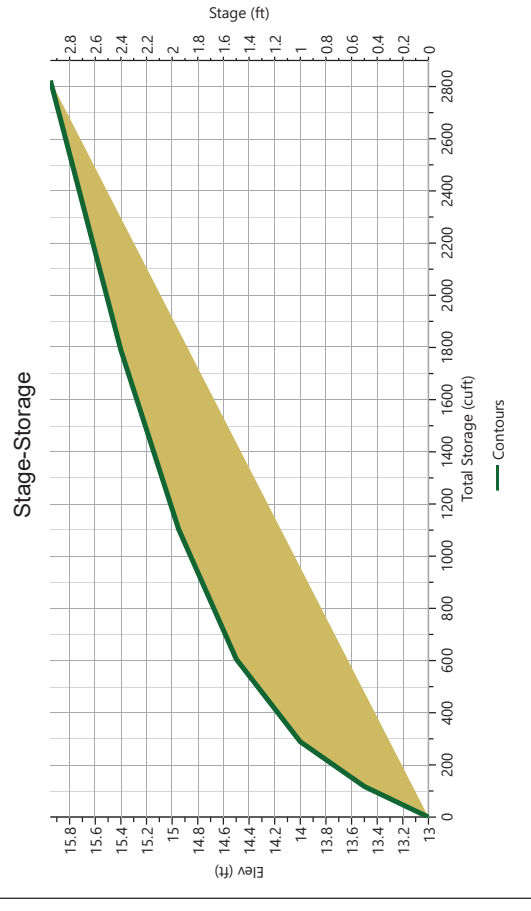
Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

BASIN 1

Stage-Storage

[illegible]

Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

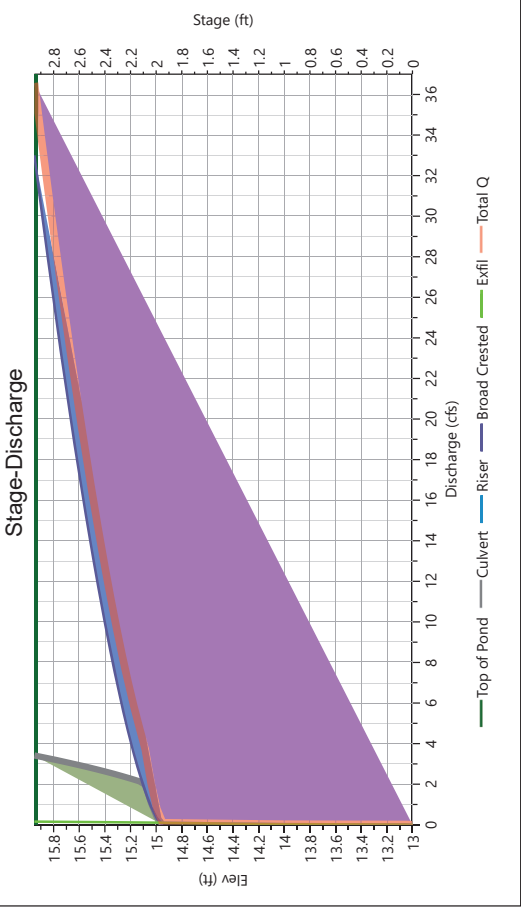
11-22-2022

BASIN 1

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2	3	
Rise, in	12	3			Orifice Dia, in
Span, in	12	3			No. Orifices
No. Barrels	1	1			Invert Elevation, ft
Invert Elevation, ft	13.90	14.77			Height, ft
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co
Length, ft	100				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
		1	2	3	
Shape / Type	Box	Broad Crested			Exfiltration, in/hr
Crest Elevation, ft	14.95	14.95			3.25**
Crest Length, ft	12	10			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to contours.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

BASIN 1

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.00	0.000	0.000	0.000			0.000	0.000				0.000		0.000
0.50	13.50	117	0.000	0.000			0.000	0.000				0.022		0.022
1.00	14.00	289	0.000	0.000			0.000	0.000				0.030		0.030
1.50	14.50	606	0.000	0.000			0.000	0.000				0.066		0.066
1.95	14.95	1,104	0.000	0.000			0.000	0.000				0.101		0.101
2.40	15.40	1,790	2.629 oc	0.000			0.000	9.962				0.128		12.72
2.95	15.95	2,823	3.415 oc	0.000			0.000	33.00				0.155		36.57

Suffix key: /c = inlet control, oc = outlet control, s = submerged weir

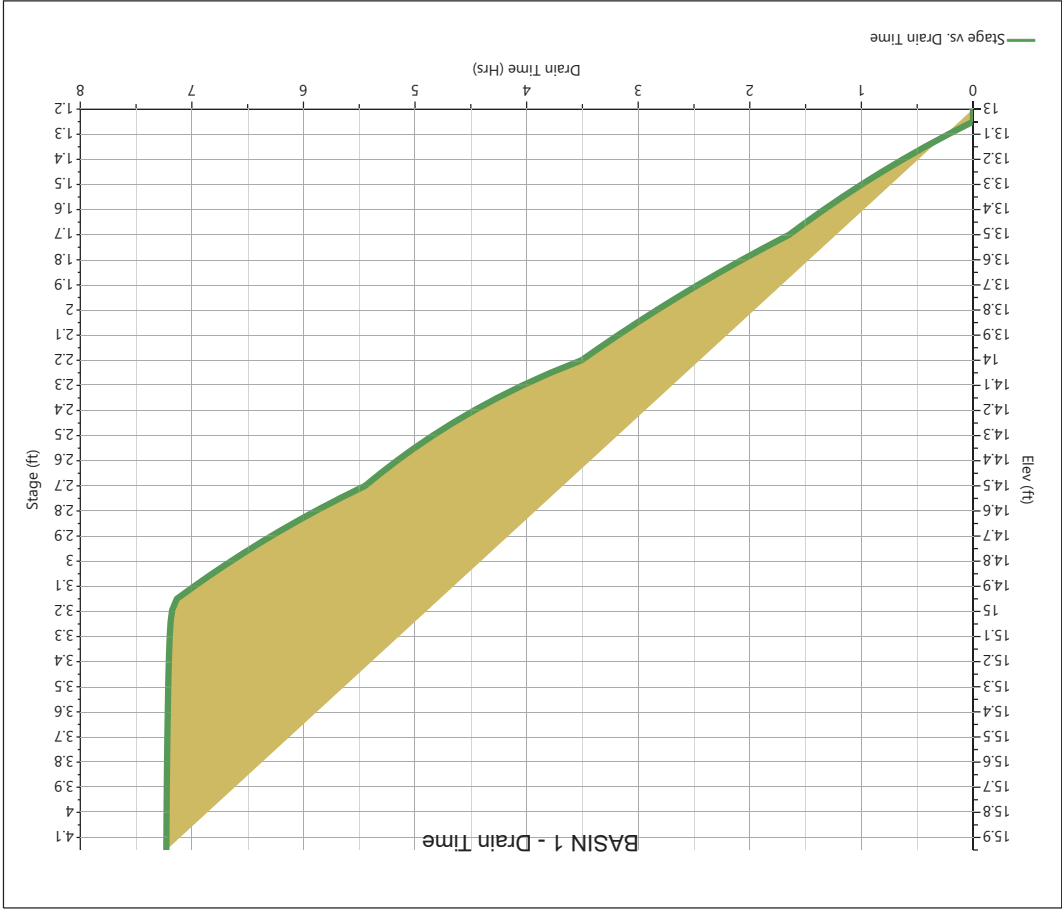
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

BASIN 1

Pond Drawdown



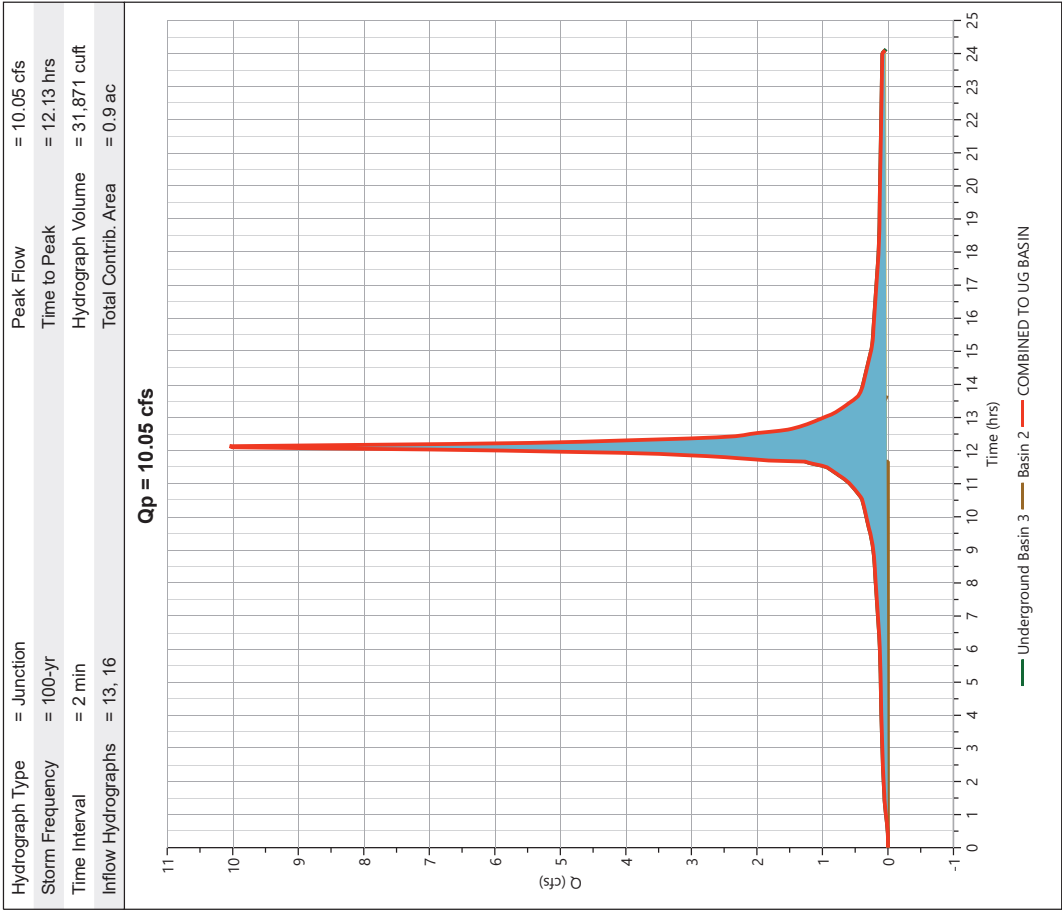
Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18



Hydrograph Report

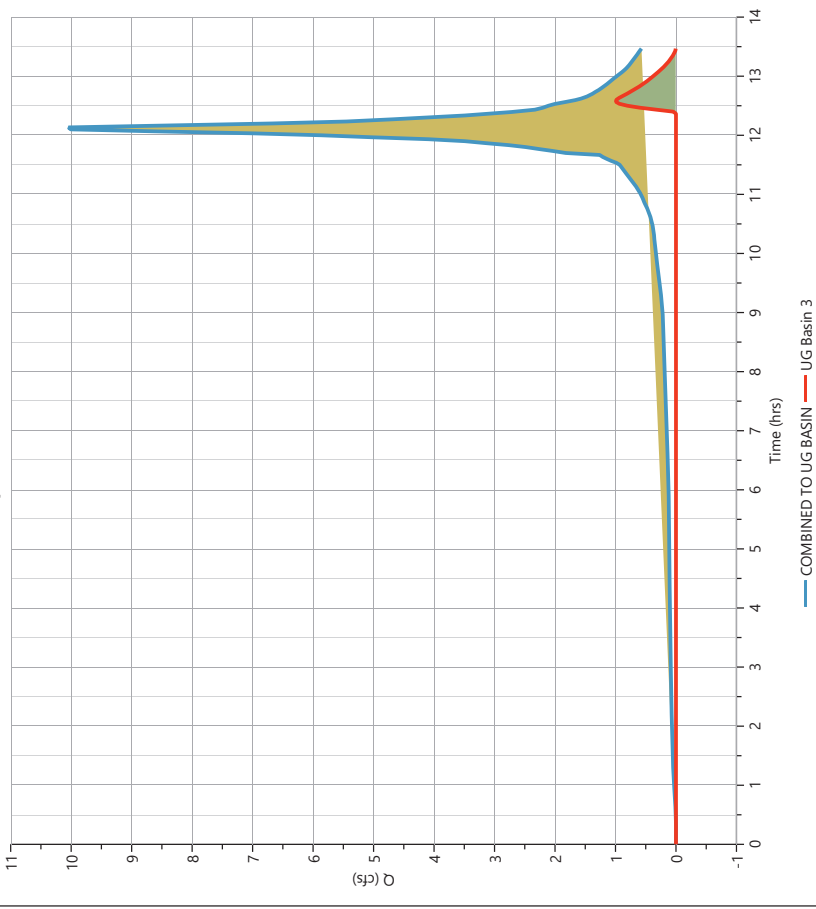
Post UG Basin 3

Hyd. No. 19

Hydrograph Type	= Pond Route	
Storm Frequency	= 100-yr	Peak Flow = 0.993 cfs
Time Interval	= 2 min	Time to Peak = 12.57 hrs
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Hydrograph Volume = 1,713 cuft
Pond Name	= UG BASIN	Max. Elevation = 14.28 ft
		Max. Storage = 11,596 cuft

Pond Routing by Storage Indication Method

Qp = 0.99 cfs

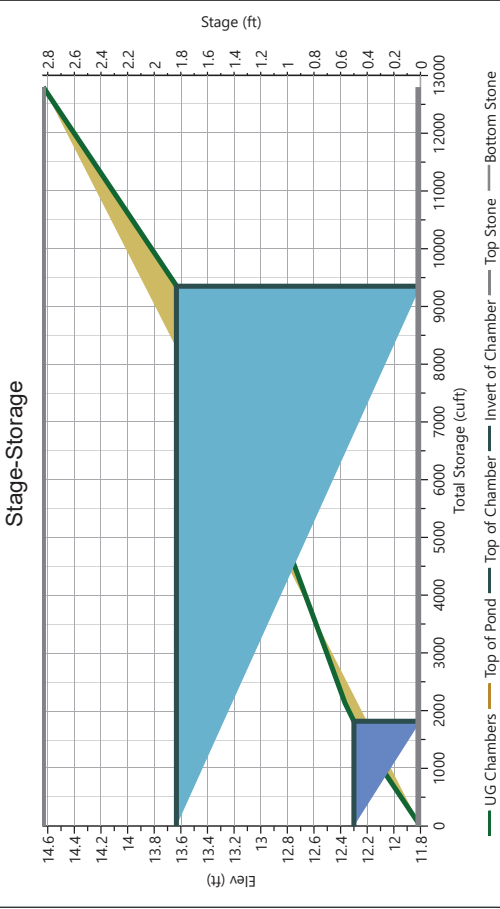


Pond Report

UG BASIN

Stage-Storage

StormTech® SC-310™ Chamber			Stage / Storage Table			
Description	Input	Stage (in)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
Chamber Height, in	16	0.0	11.80	8,601	0.000	0.000
Chamber Shape	Arch	1.7	11.94	8,601	487	487
Chamber Width, in	34	3.4	12.08	8,601	487	975
Installed Length, ft	7.12	5.1	12.23	8,601	487	1,462
No. Chambers	346	8.5	12.51	8,601	668	2,130
Bare Chamber Stor, cuft	5,086	10.2	12.65	8,601	889	3,026
No. Rows	20	11.9	12.79	8,601	877	4,793
Space Between Rows, in	6	13.6	12.93	8,601	860	5,653
Stone Above, in	12	15.3	13.08	8,601	836	6,489
Stone Below, in	6	17.0	13.22	8,601	805	7,294
Stone Sides, in	12	18.7	13.36	8,601	763	8,057
Stone Ends, in	12	20.4	13.50	8,601	705	8,761
Encasement Voids, %	40.00	22.1	13.64	8,601	626	9,388
Encasement Bottom Elevation, ft	11.80	23.8	13.78	8,601	487	9,875
		25.5	13.93	8,601	487	10,363
		27.2	14.07	8,601	487	10,850
		28.9	14.21	8,601	487	11,337
		30.6	14.35	8,601	487	11,825
		32.3	14.49	8,601	487	12,312
		34.0	14.63	8,601	487	12,800



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

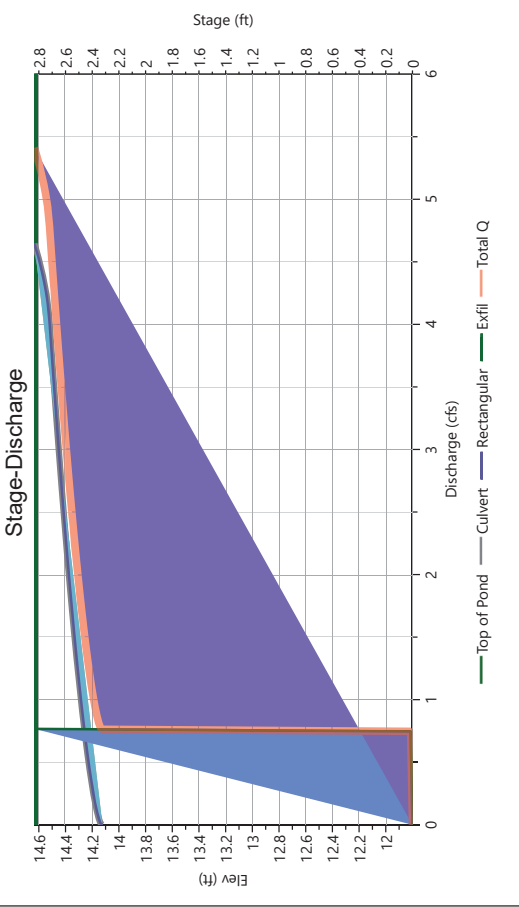
11-22-2022

UG BASIN

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Perforated Riser
		1*	2*	3*	
Rise, in	12	3	4	6	Hole Diameter, in
Span, in	12	3	4	6	No. holes
No. Barrels	1	1	1	1	Invert Elevation, ft
Invert Elevation, ft	12.20	12.75	12.80	13.20	Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60	0.60	Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs		Ancillary	
		1*	2		
Shape / Type		Rectangular			3.75**
Crest Elevation, ft		14.13			
Crest Length, ft		5			
Angle, deg					
Weir Coefficient, Cw		3.3			

*Routes through Culvert. **Exfiltration extracted from outflow hydrograph. Data applied to contours.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

UG BASIN

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	11.80	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.000		0.000
0.14	11.94	487	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.748		0.748
0.28	12.08	975	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.749		0.749
0.43	12.23	1,462	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.750		0.750
0.57	12.37	2,130	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.751		0.751
0.71	12.51	3,026	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.752		0.752
0.85	12.65	3,916	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.753		0.753
0.99	12.79	4,793	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.754		0.754
1.13	12.93	5,653	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.755		0.755
1.28	13.08	6,489	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.756		0.756
1.42	13.22	7,294	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.757		0.757
1.56	13.36	8,057	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.758		0.758
1.70	13.50	8,761	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.759		0.759
1.84	13.64	9,388	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.760		0.760
1.98	13.78	9,875	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.761		0.761
2.13	13.93	10,363	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.762		0.762
2.27	14.07	10,850	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		0.763		0.763
2.41	14.21	11,337	0.362 oc	0.000	0.000	0.000	0.000	0.362	0.000	0.000		0.764		1.125
2.55	14.35	11,825	1,703 oc	0.000	0.000	0.000	0.000	1,703	0.000	0.000		0.765		2,467
2.69	14.49	12,312	3,589 oc	0.000	0.000	0.000	0.000	3,589	0.000	0.000		0.766		4,354
2.83	14.63	12,800	4,645 oc	0.000	0.000	0.000	0.000	4,645 s	0.000	0.000		0.767		5,412

Suffix: key: /c = inlet control, oc = outlet control, s = submerged weir

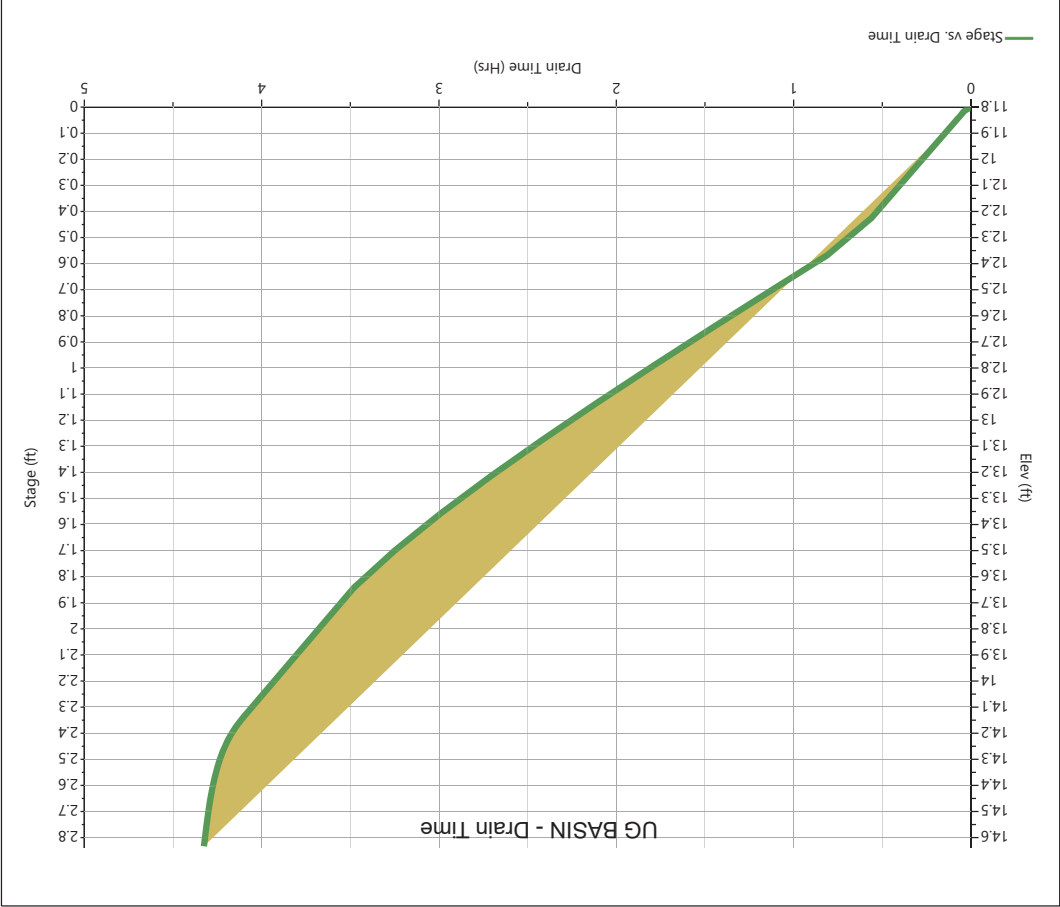
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

UG BASIN

Pond Drawdown



Hydrograph Report

Project Name:
11-22-2022

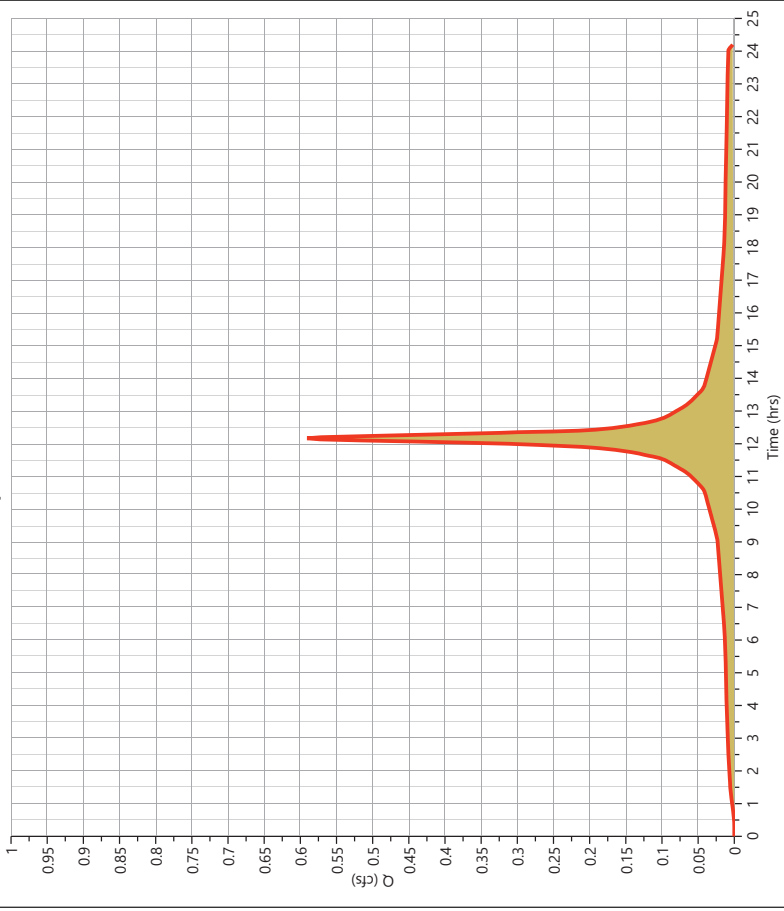
Hydrology Studio v 3.0.0.26

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.591 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,704 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.59 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

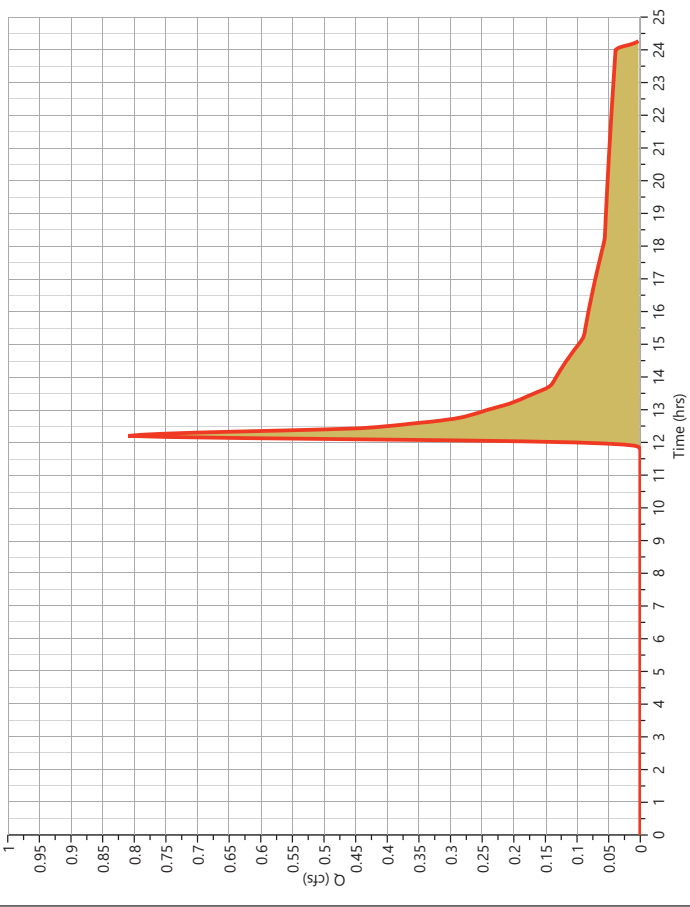
11-22-2022

Pre DA 2 - PERVIOUS

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.810 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 4.475 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.58	39	OPEN SPACE	
0.28	30	WOODS	
0.86	36	Weighted CN Method Employed	

Qp = 0.81 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

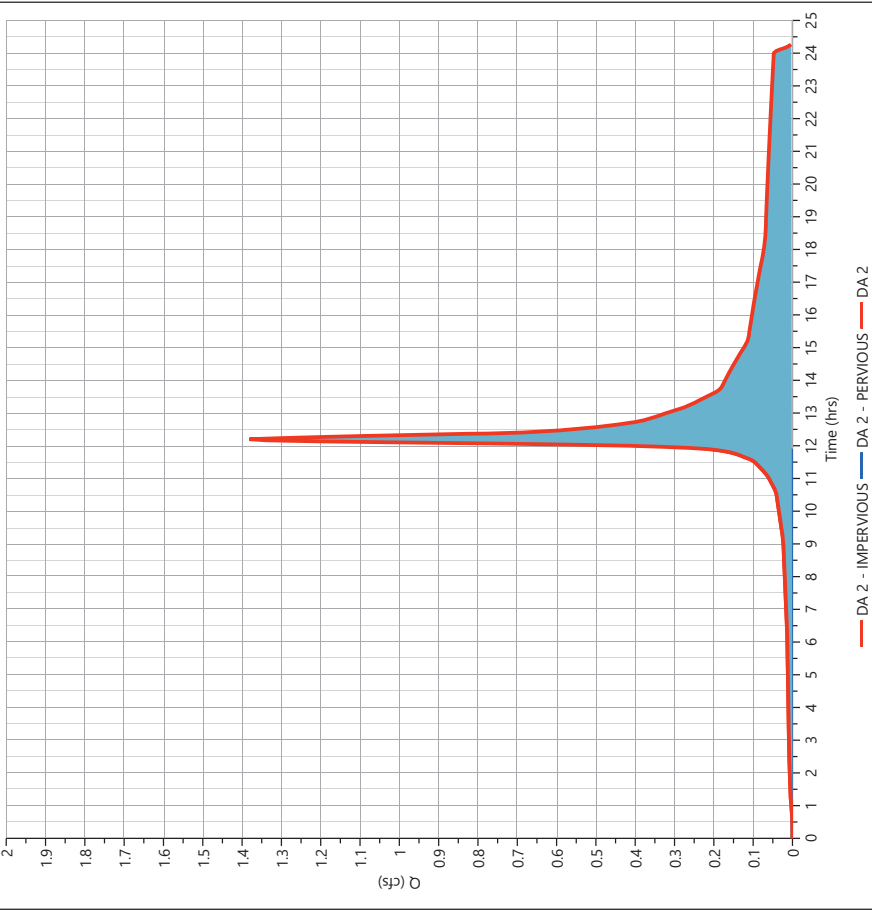
11-22-2022

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 1.381 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 7,179 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 1.38 cfs



Hydrograph Report

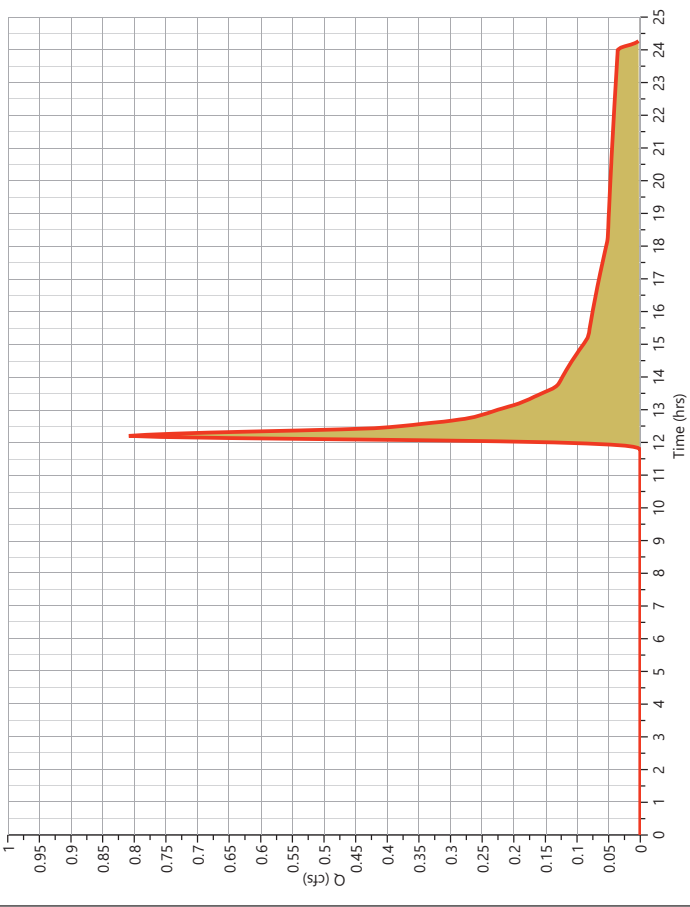
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.809 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 4,218 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	38	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	

Qp = 0.81 cfs



Hydrograph Report

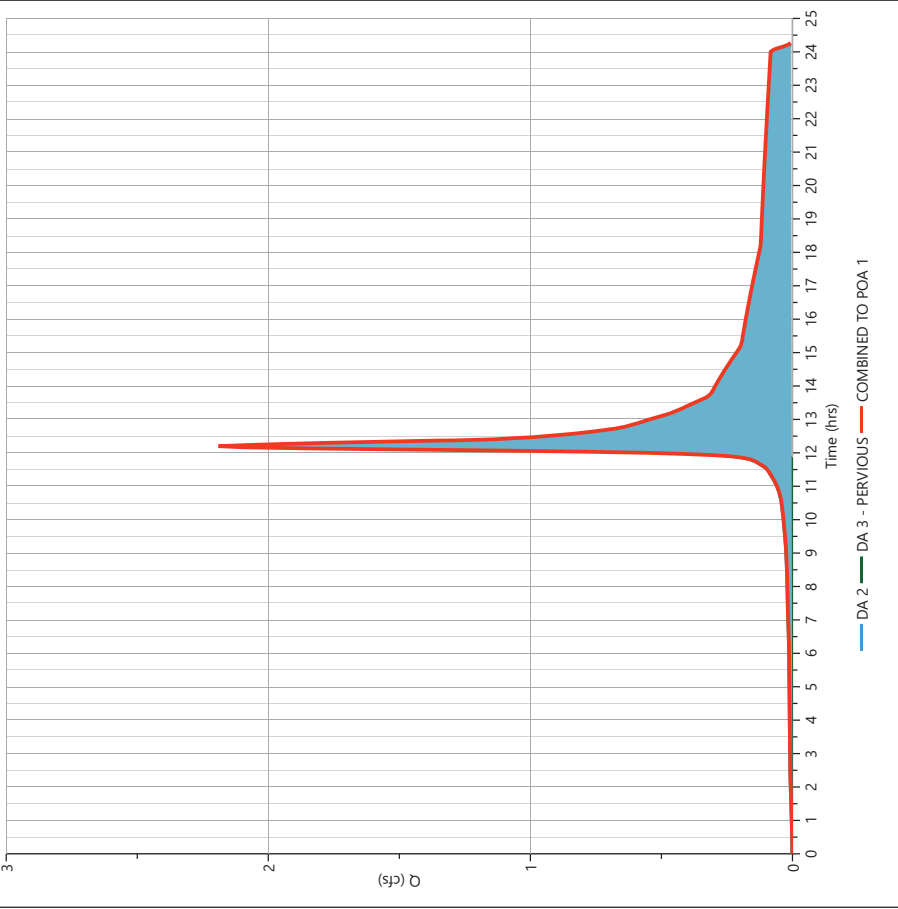
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 2,190 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Hydrograph Volume	= 11,397 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac

Qp = 2.19 cfs



Hydrograph Report

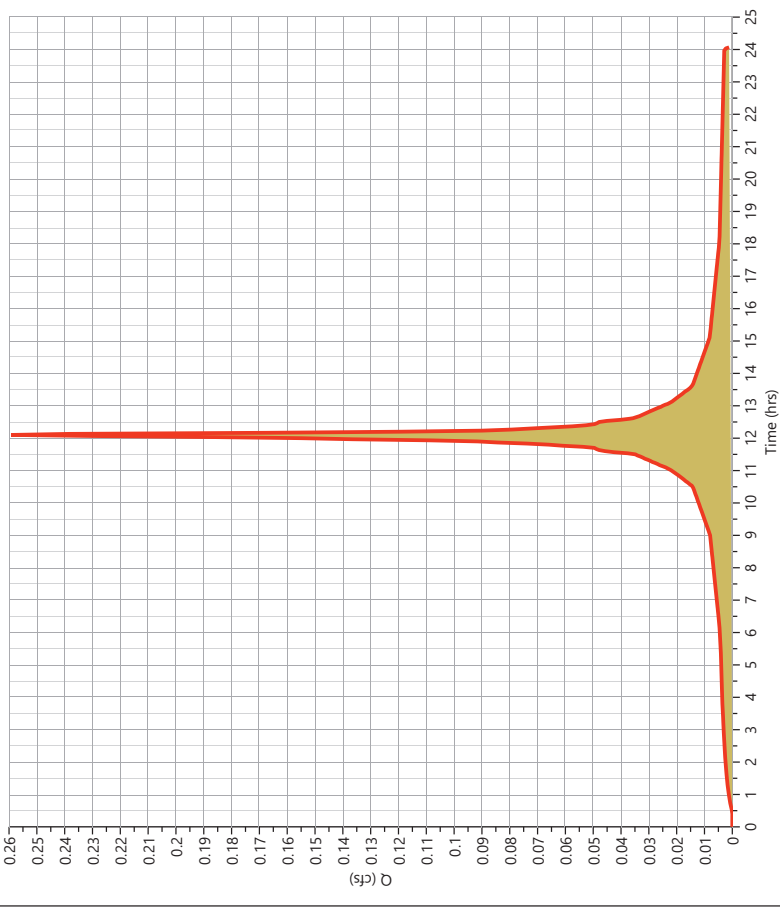
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.259 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 922 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.26 cfs



Hydrograph Report

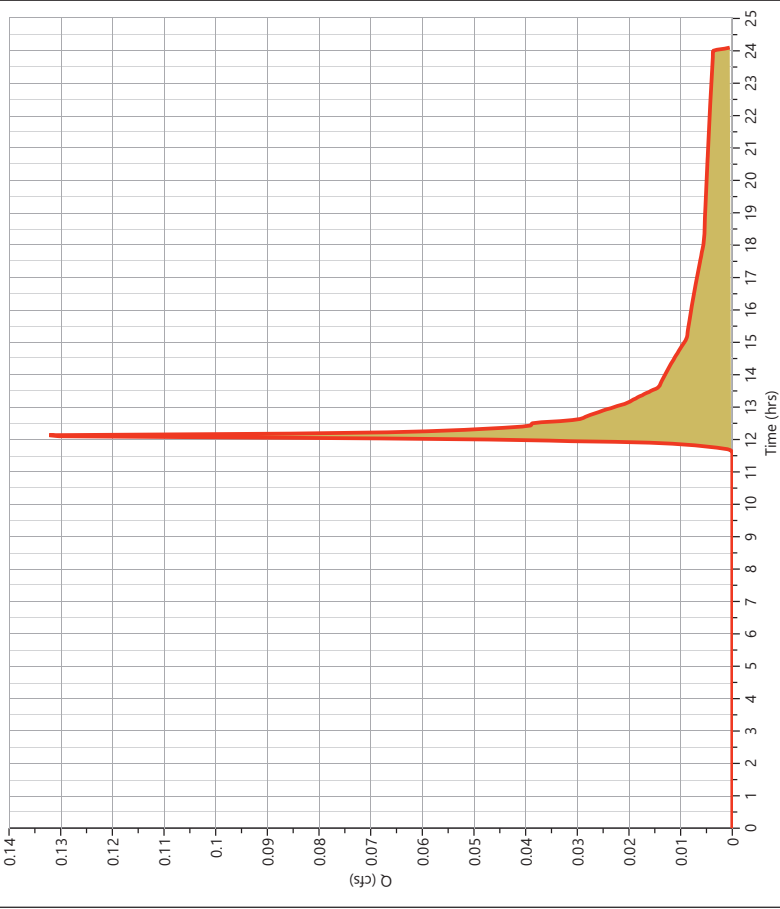
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.132 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 471 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.13 cfs



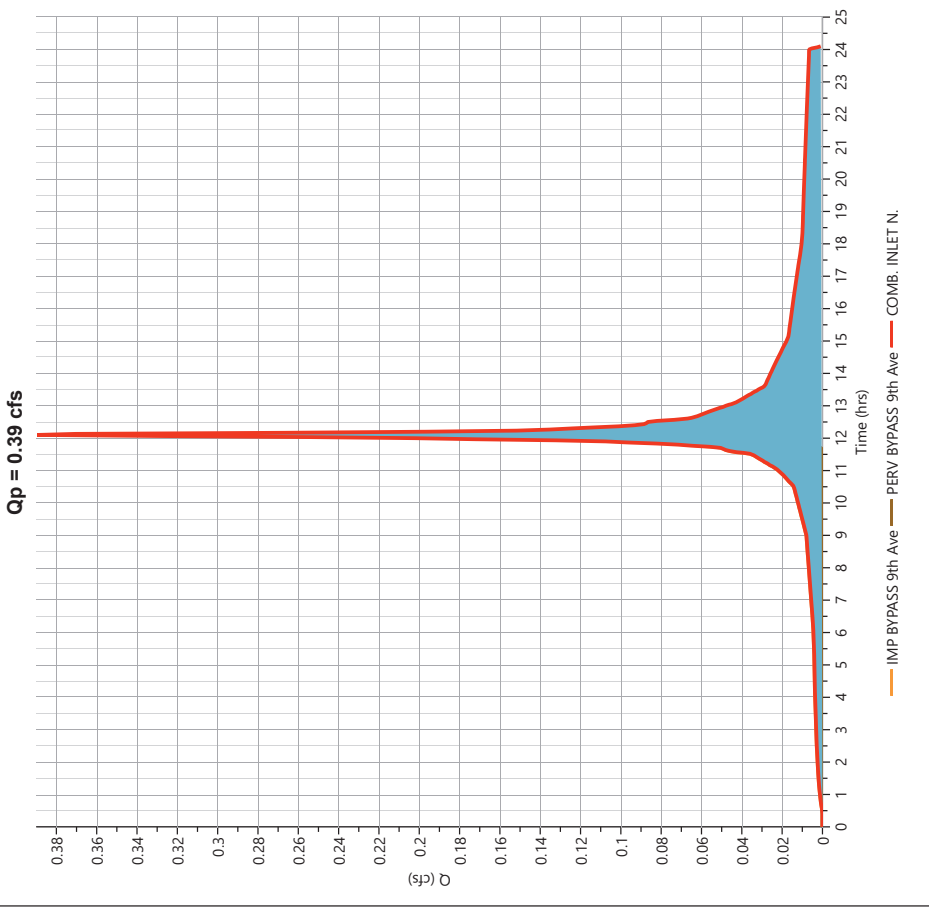
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.390 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,393 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



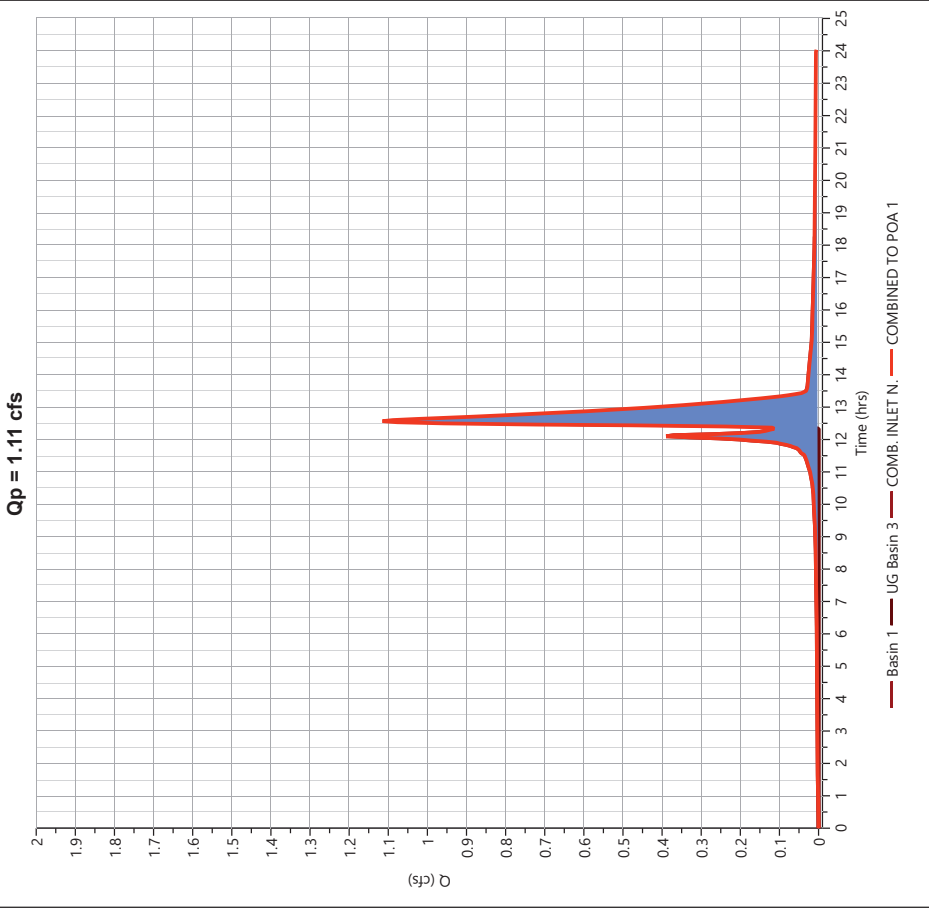
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 1.115 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.57 hrs
Time Interval	= 2 min	Hydrograph Volume	= 3,168 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



Hydrograph Report

Project Name:
11-22-2022

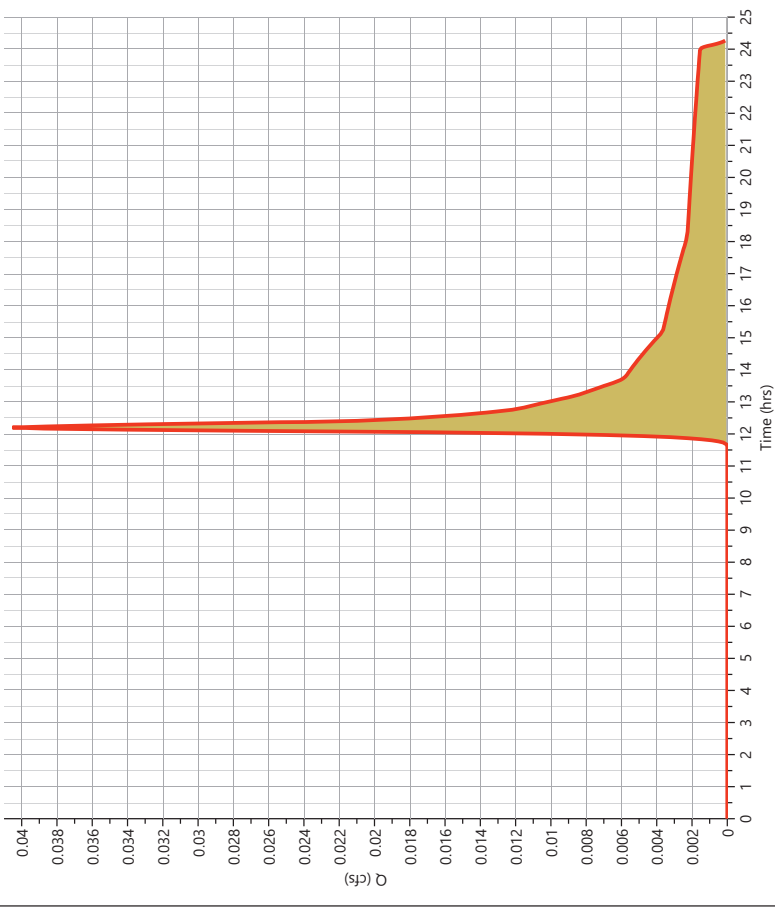
Hydrology Studio v 3.0.0.26

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.041 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 194 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.04 cfs



Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.095 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.20 hrs
Time Interval	= 2 min	Runoff Volume	= 454 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.09 cfs



Hydrograph Report

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.346 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,229 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.35 cfs



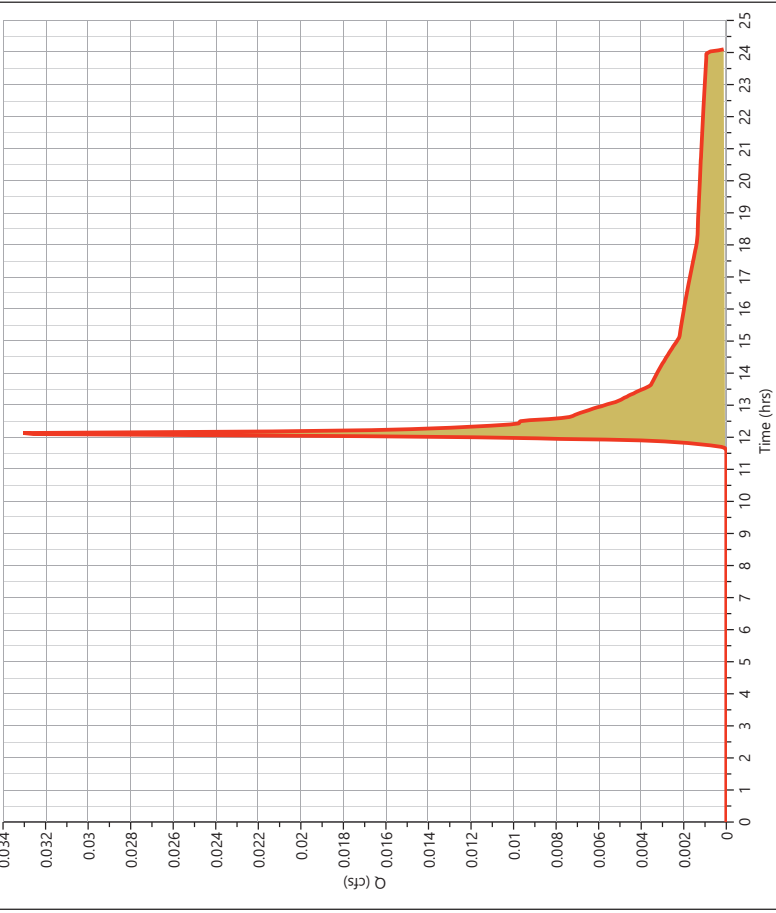
Hydrograph Report

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.033 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 118 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 9.27 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.03 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

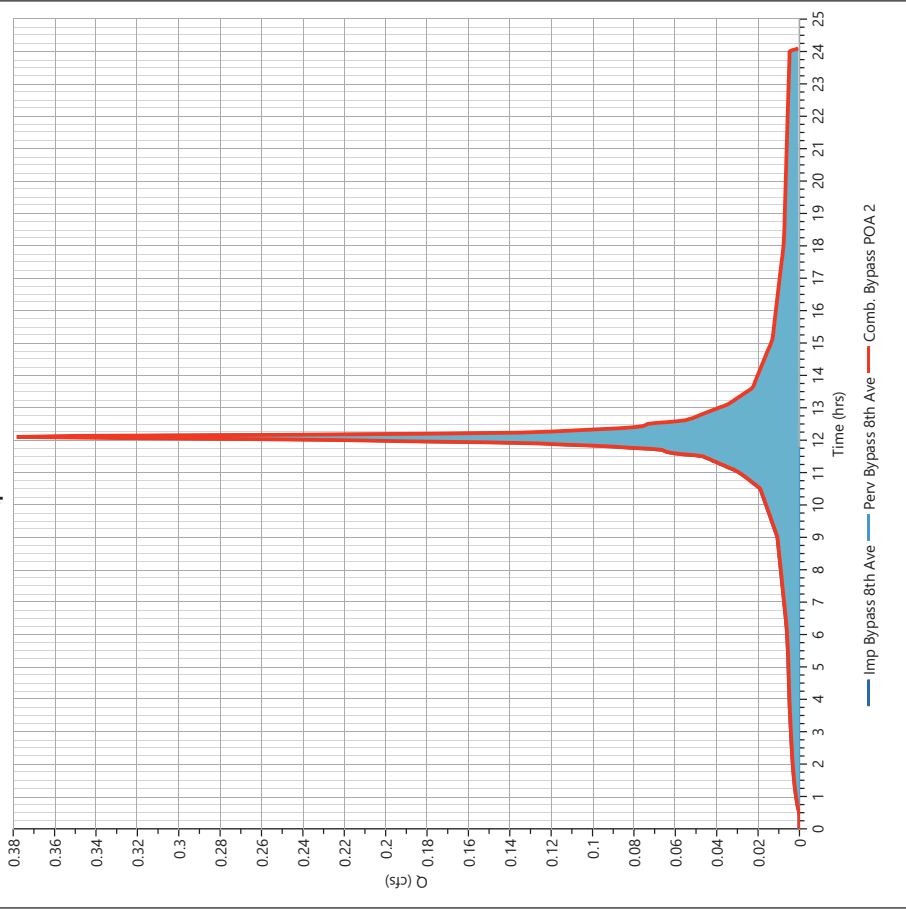
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.378 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,347 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.38 cfs



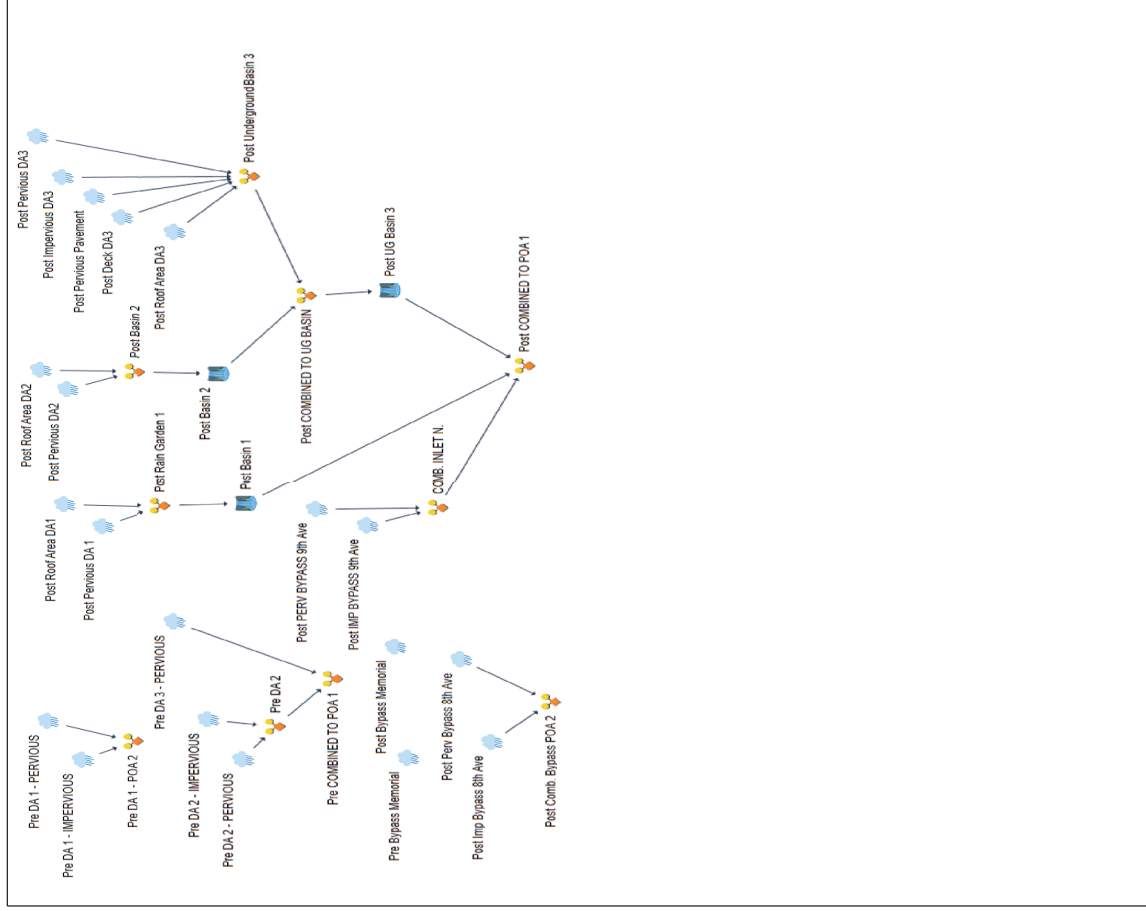
**HYDROGRAPH SUMMARY REPORTS –
STABILITY ANALYSIS**

Basin Model

Hydrology Studio v 3.0.0.26

Project Name:

11-22-2022



Hydrograph 2-yr Summary

Hydrology Studio v 3.0.0.26

Project Name:

11-22-2022

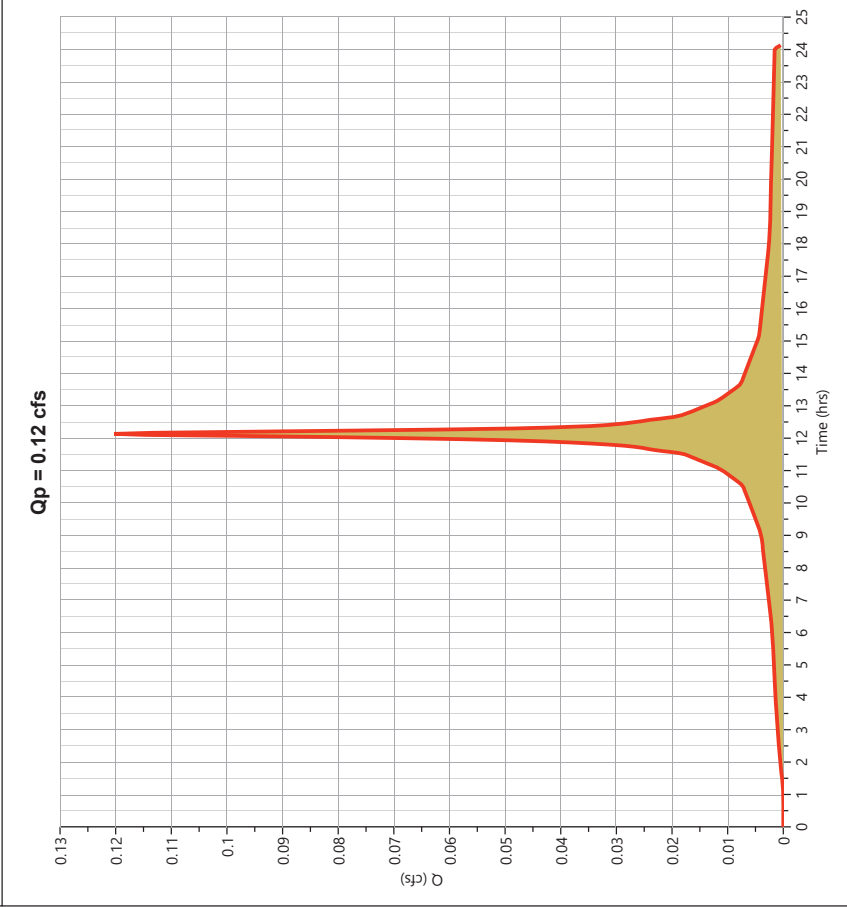
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.120	12.13	474	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.000	24.00	0.095	----	----	----
3	NRCS Runoff	Post Roof Area DA3	1.132	12.10	3.891	----	----	----
4	NRCS Runoff	Post Deck DA3	0.608	12.17	2.690	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.414	12.17	1.834	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.000	24.00	1.18	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.291	12.10	1.000	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.000	24.00	1.76	----	----	----
9	NRCS Runoff	Post Roof Area DA2	1.617	12.10	5.558	----	----	----
10	Junction	Pre DA 1 - POA 2	0.120	12.13	474	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.000	24.00	3.55	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.193	12.17	856	----	----	----
13	Junction	Post Underground Basin 3	2.217	12.13	9.274	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.291	12.10	1.002	6, 7	----	----
15	Junction	Post Basin 2	1.617	12.10	5.560	8, 9	----	----
16	Pond Route	Post Basin 2	1.204	12.17	5.558	15	14.29	512
17	Pond Route	Post Basin 1	0.005	18.83	166	14	14.81	879
18	Junction	Post COMBINED TO UG BASIN 3	3.397	12.13	14.833	13, 16	----	----
19	Pond Route	Post UG Basin 3	0.664	12.67	11.045	18	13.46	8,579
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.221	12.17	978	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.000	0.00	0.000	----	----	----
22	Junction	Pre DA 2	0.221	12.17	978	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.000	24.00	1.47	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.221	12.17	980	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.097	12.10	333	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.000	24.00	2.35	----	----	----
27	Junction	COMB. INLET N	0.097	12.10	336	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	0.677	12.63	11.547	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.000	24.00	0.969	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.000	24.00	2.26	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.129	12.10	445	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.000	24.00	0.588	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.129	12.10	445	31, 32	----	----

Hydrograph Report

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.120 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 474 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



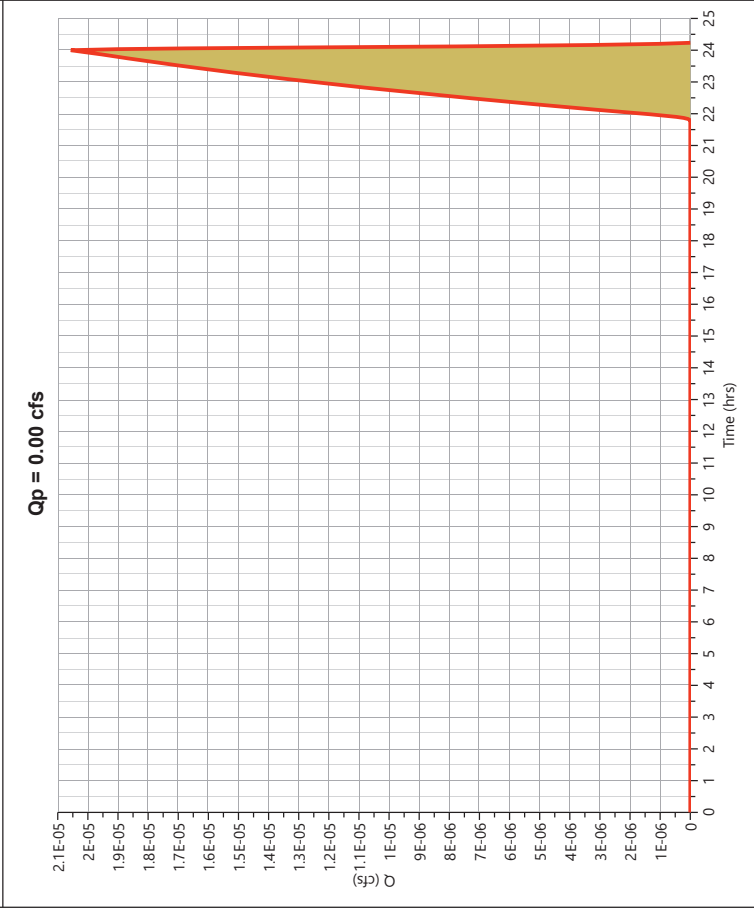
Hydrograph Report

Pre DA 1 - PERVIOUS

Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.095 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet	
AREA (ac)	CN
0.04	39
open space	
0.01	30
wooded	
0.05	37
Weighted CN Method Employed	



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

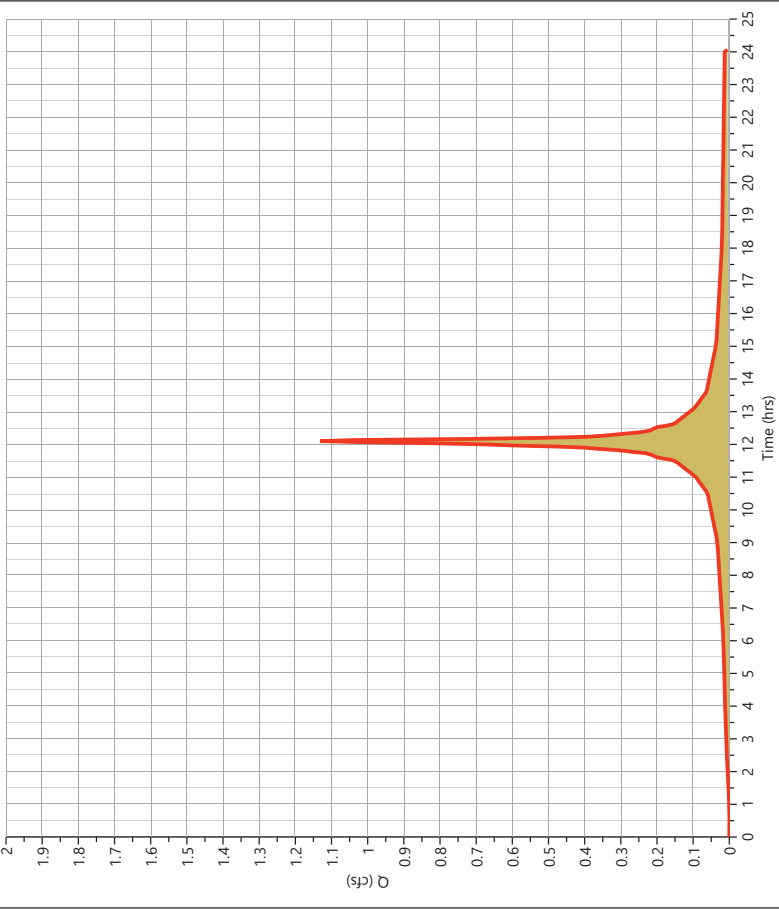
11-22-2022

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.132 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 3.891 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.13 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

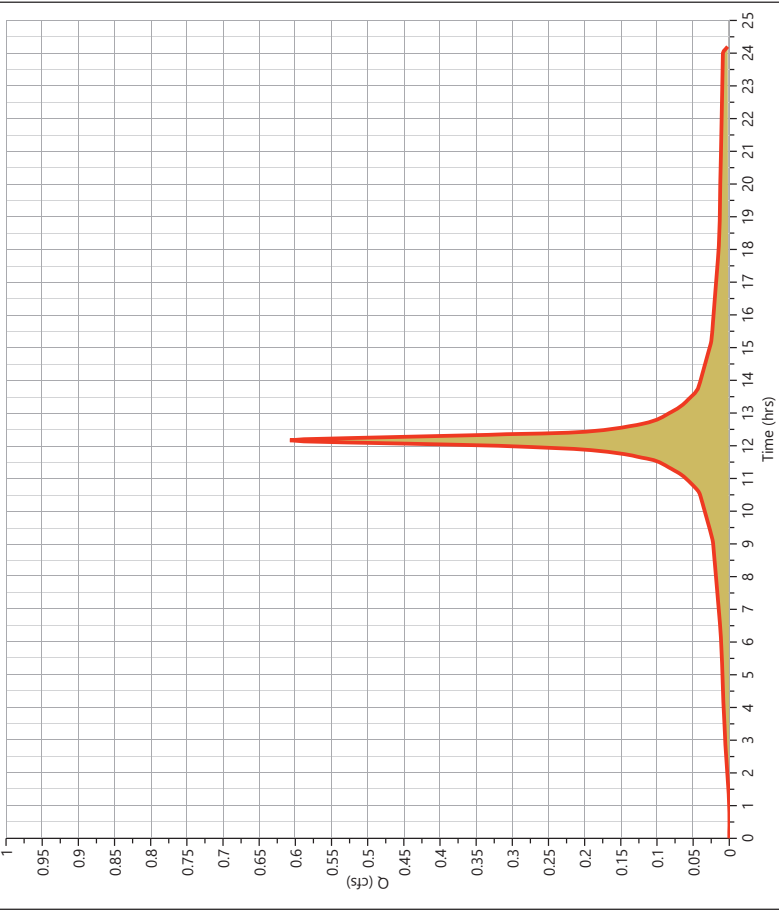
11-22-2022

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.608 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2.690 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.61 cfs



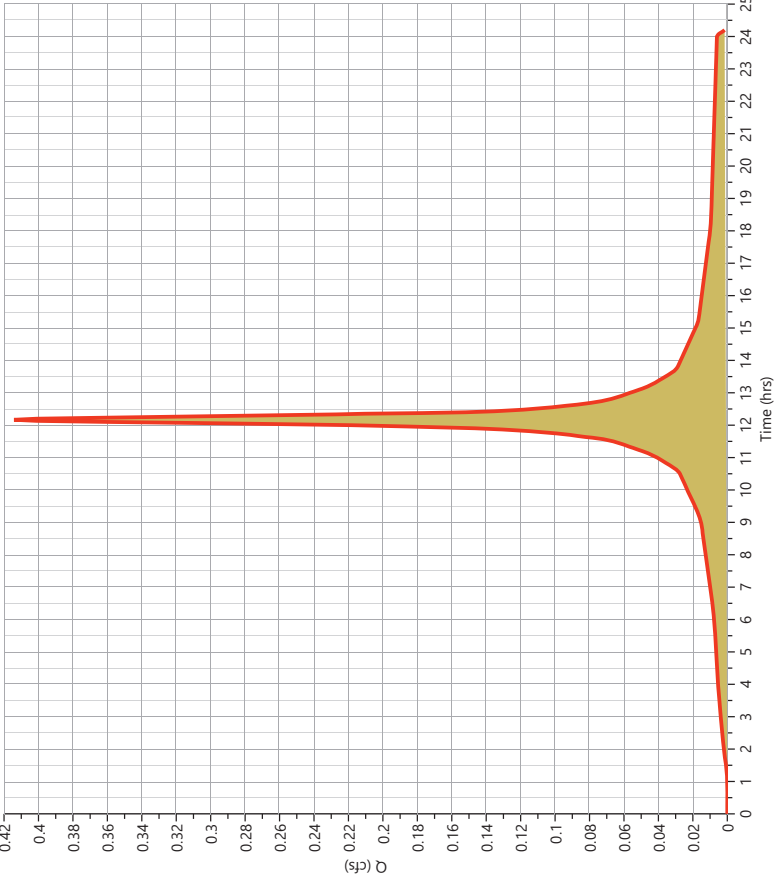
Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.414 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,834 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.41 cfs



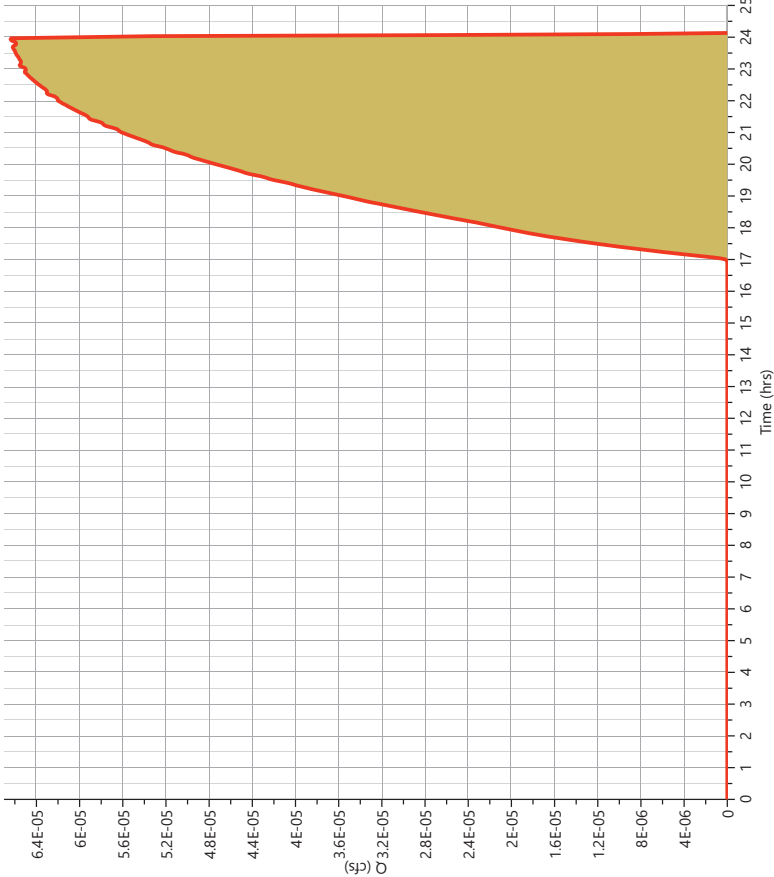
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.18 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



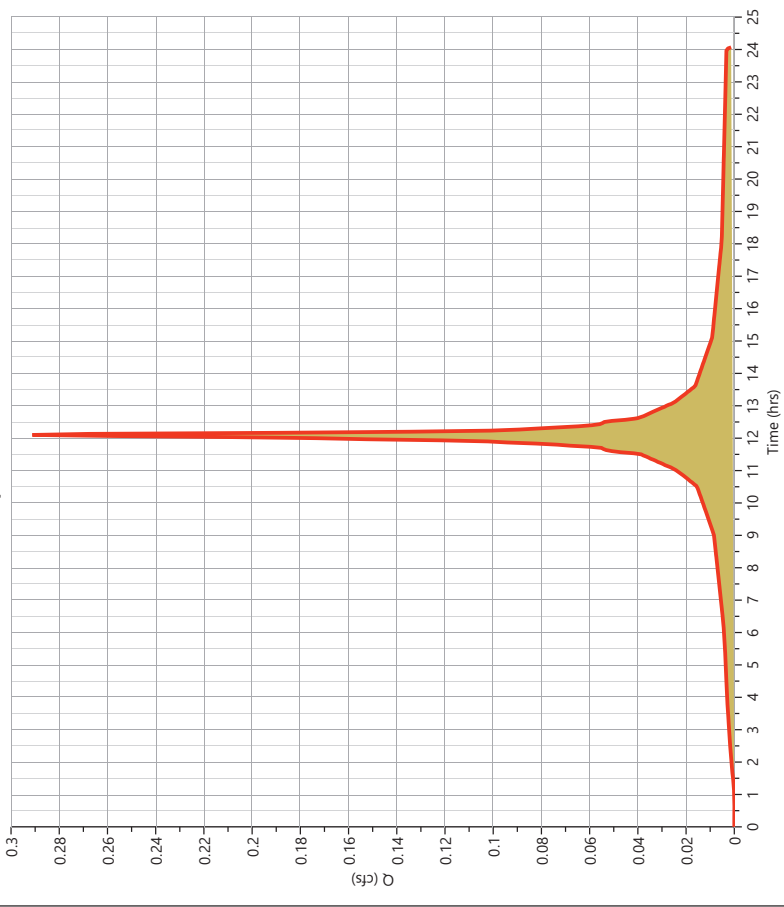
Hydrograph Report

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.291 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,000 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.29 cfs



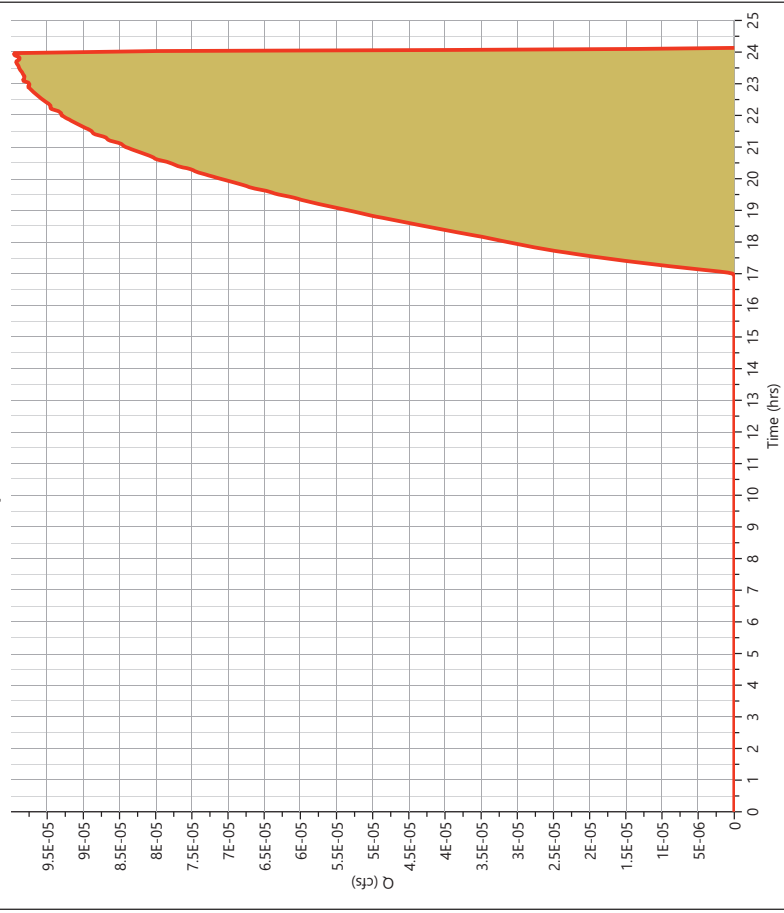
Hydrograph Report

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.76 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

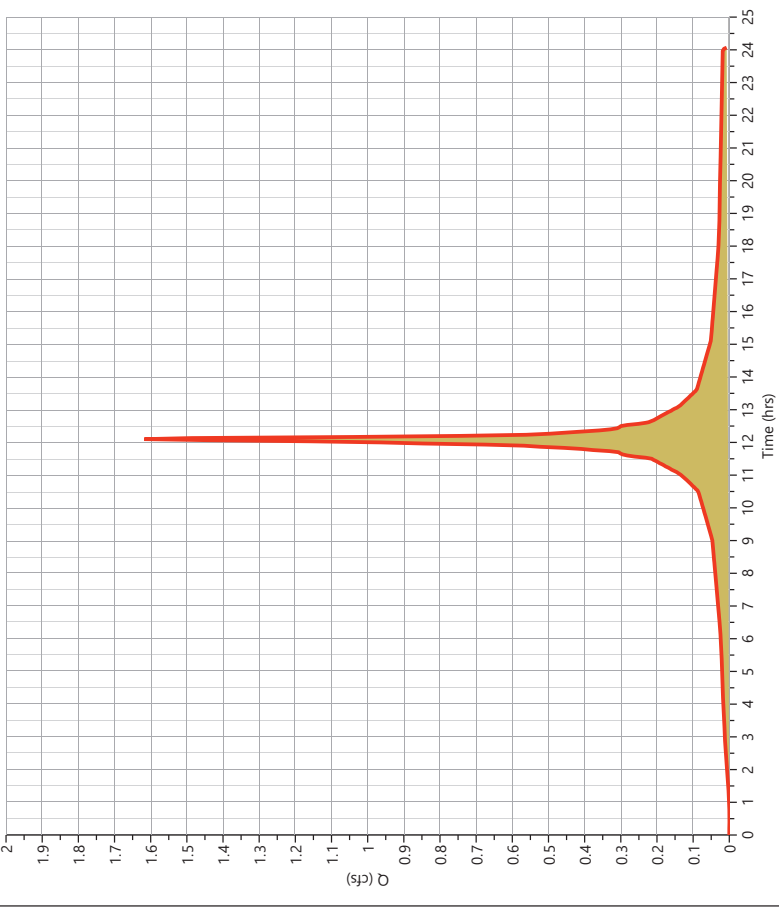
11-22-2022

Post Roof Area DA2

Hyd. No. 9

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.617 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 5,558 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.62 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

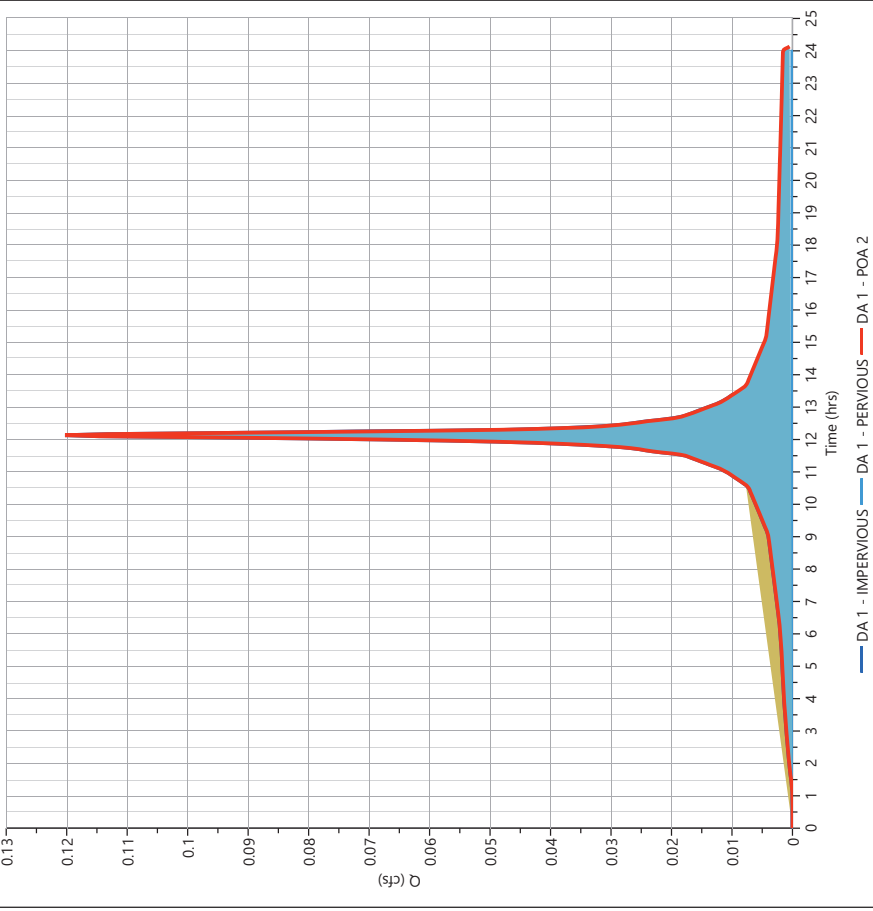
11-22-2022

Pre DA 1 - POA 2

Hyd. No. 10

Hydrograph Type	= Junction	Peak Flow	= 0.120 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 474 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.12 cfs



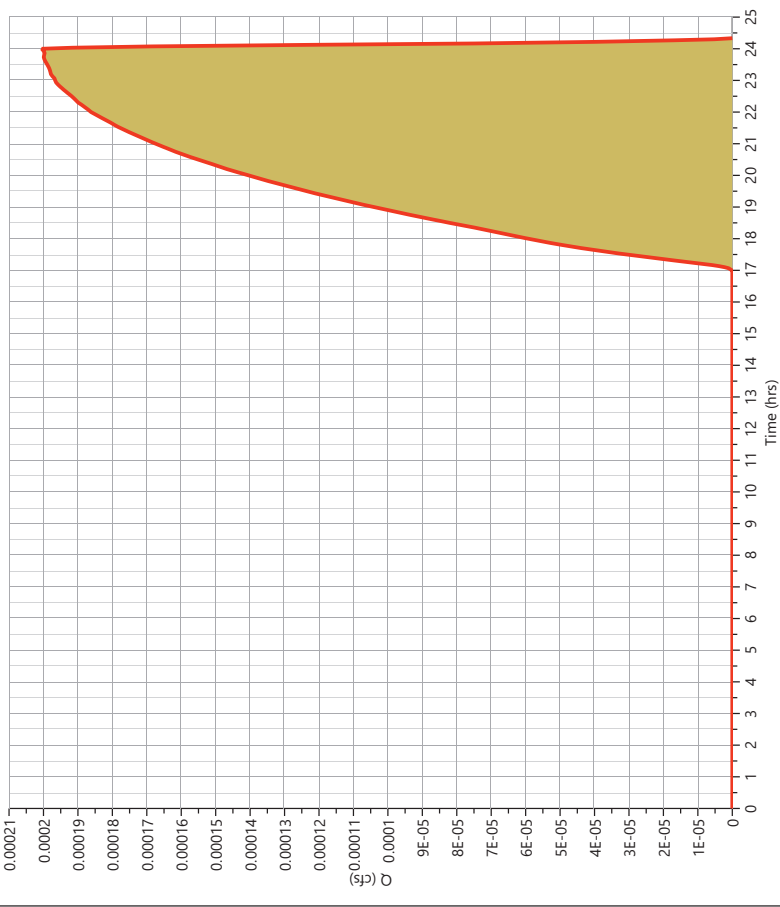
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 3.55 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



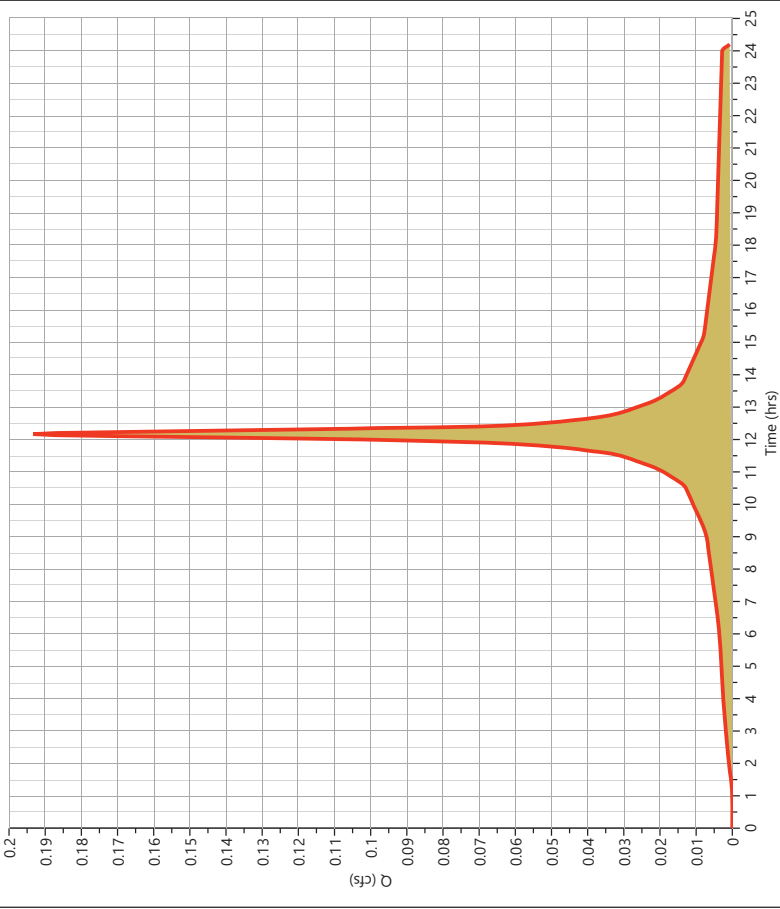
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.193 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 856 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.19 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

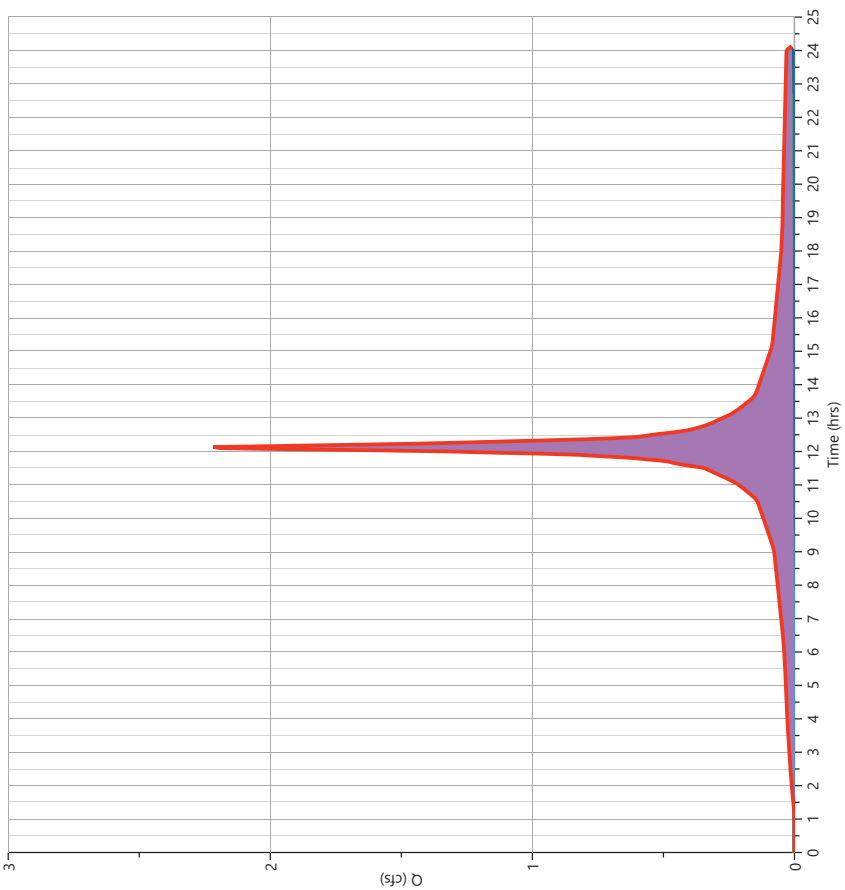
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 2.217 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 9,274 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 2.22 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

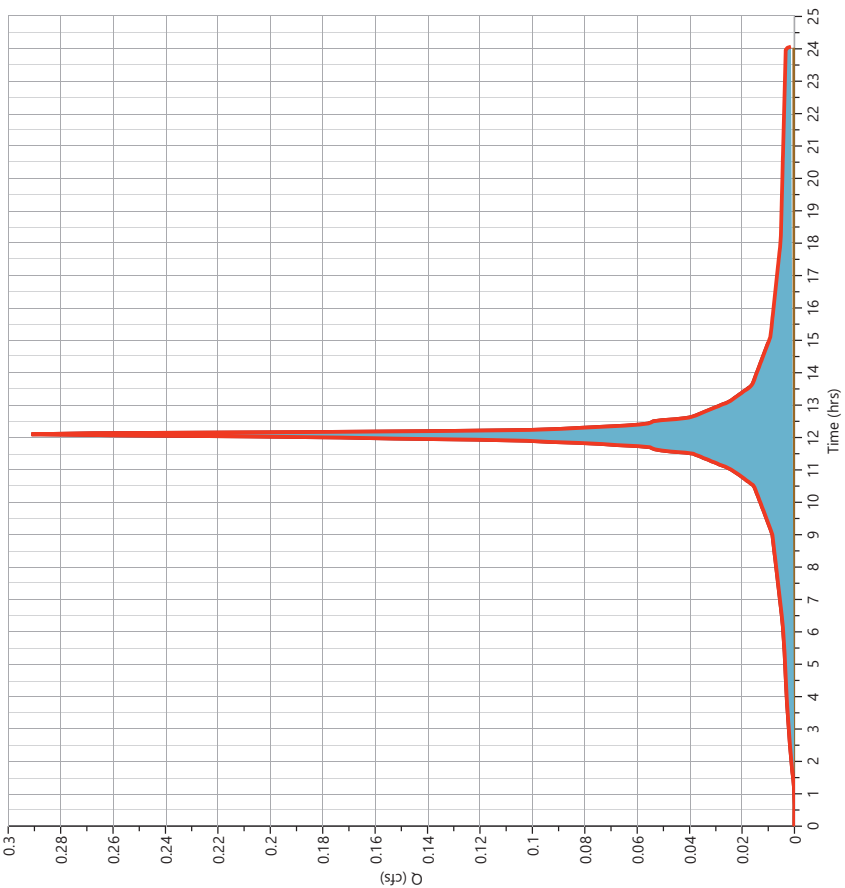
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.291 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,002 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.29 cfs



Hydrograph Report

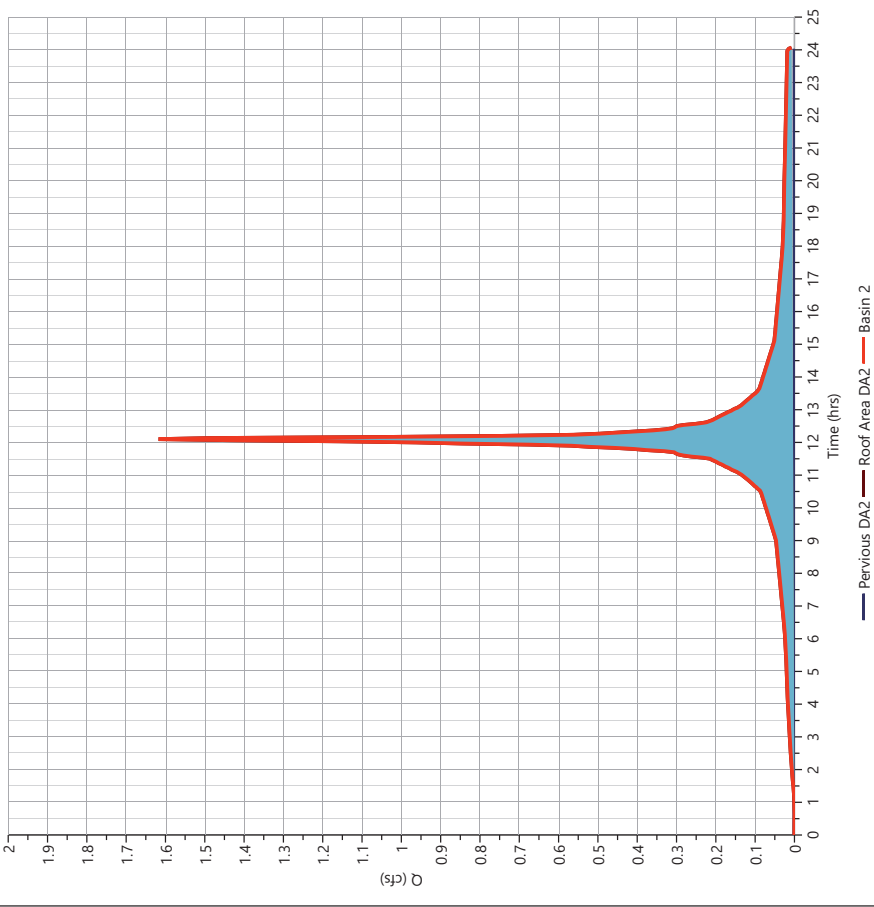
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 1.617 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 5,560 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 1.62 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

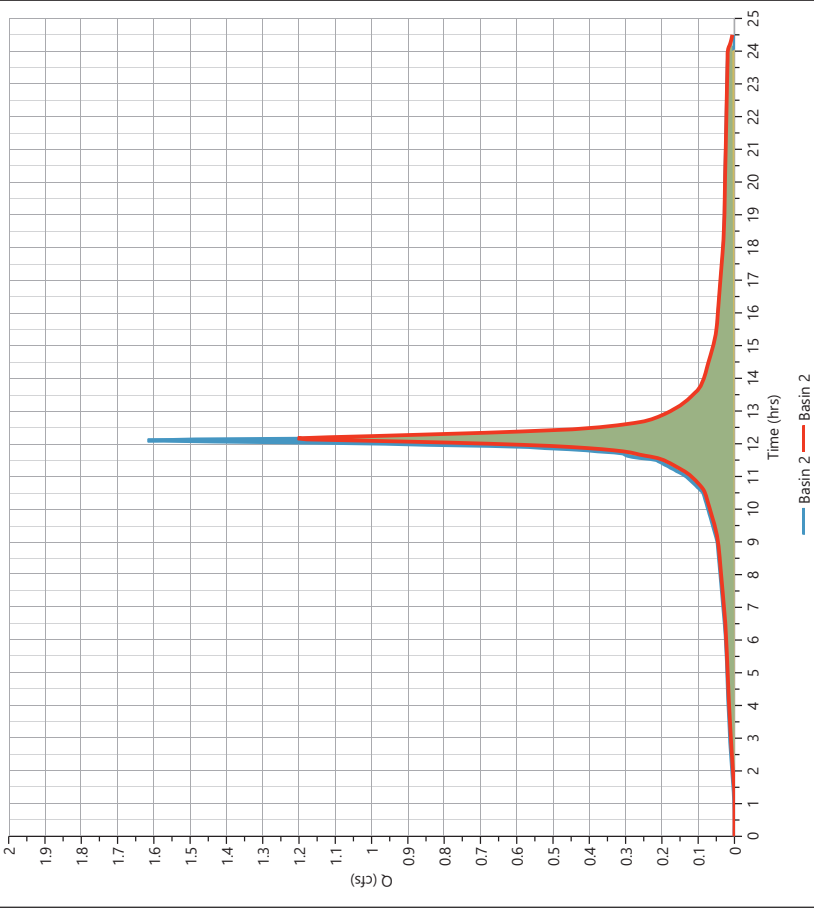
Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 1.204 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 5,558 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.29 ft
Pond Name	= Basin 2	Max. Storage	= 512 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 11 min

Qp = 1.20 cfs



Pond Report

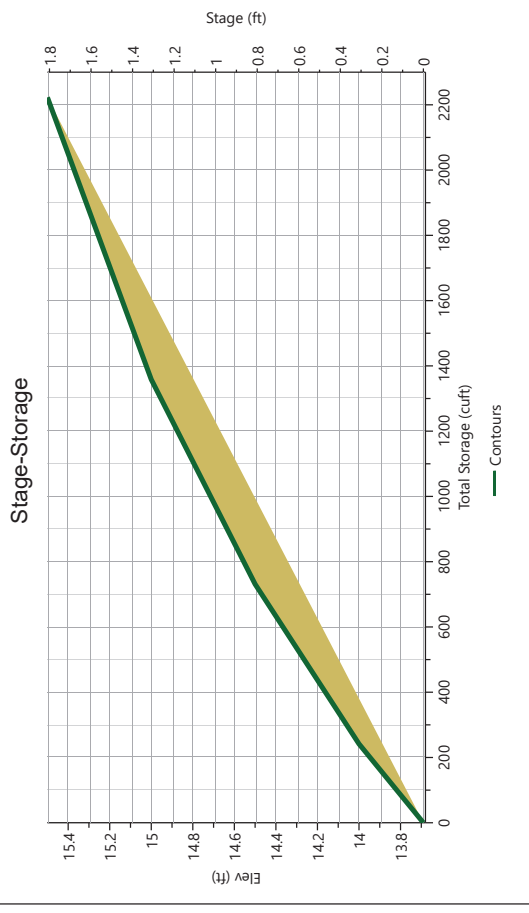
Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Basin 2

Stage-Storage

[illegible]

Pond Report

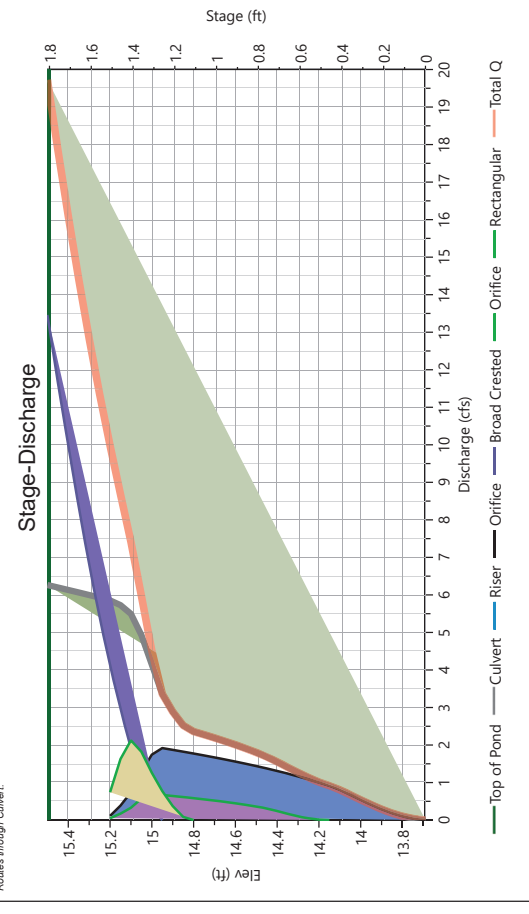
Project Name:

Hydrology Studio v 3.0.0.26

Basin 2

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2*	3	
Rise, in	12	6	4		Orifice Dia, in
Span, in	12	6	4		No. Orifices
No. Barrels	1	2	2		Invert Elevation, ft
Invert Elevation, ft	12.19	13.67	14.15		Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60		Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
		1	2*	3	
Shape / Type	Box	Broad Crested	Rectangular		Exfiltration, in/hr
Crest Elevation, ft	14.95	14.95	14.82		
Crest Length, ft	12	10	5		
Angle, deg					
Weir Coefficient, Cw	3.3	3.3	3.3		



Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.0	13.69	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000			0.000
0.31	14.00	242	0.540 oc	0.540	0.000		0.000	0.000	0.000					0.540
0.81	14.50	731	1.800 oc	1.440	0.360		0.000	0.000	0.000					1.800
1.31	15.00	1,358	4,141 oc	1,744	0.695		0.443	0.369	1.260					4,510
1.81	15.50	2,223	6,264 oc	0.000	0.000		0.000	13.46	0.000					19.72

Suffix: ey_ ic = inlet control, oc = outlet control, s = submerged weir

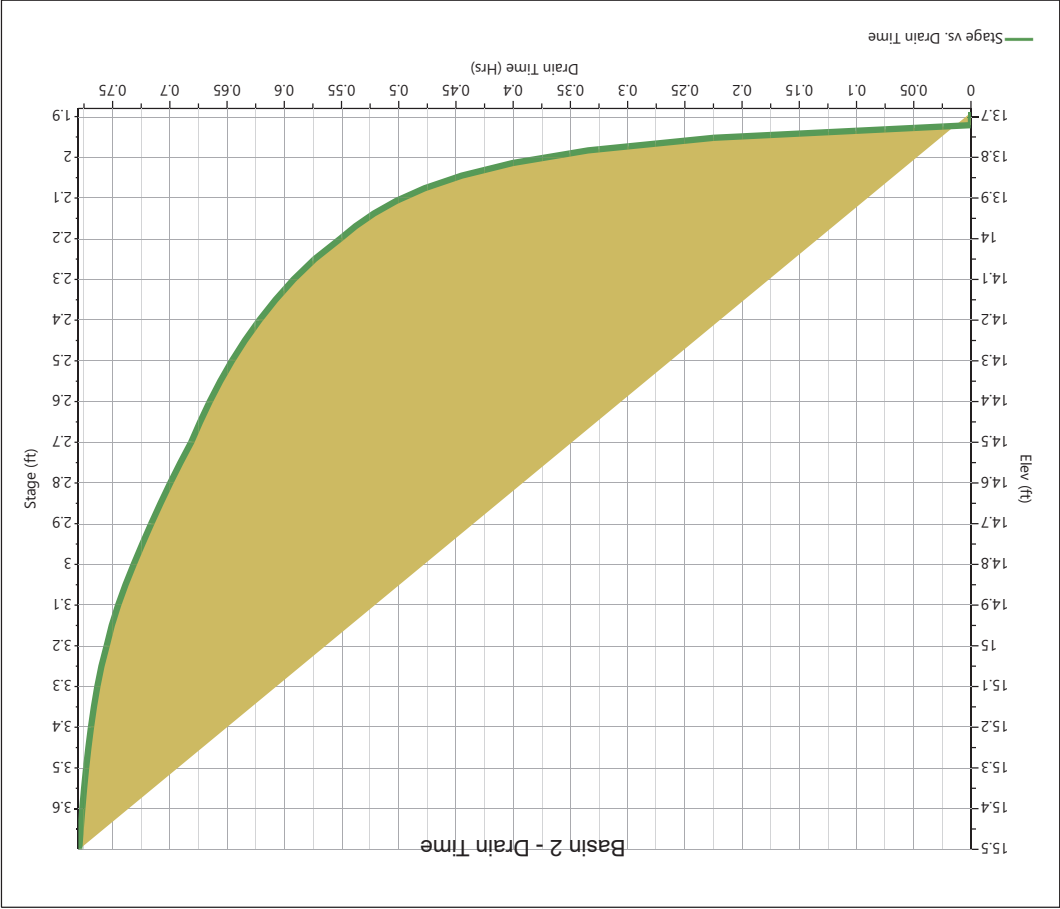
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Basin 2

Pond Drawdown



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Post Basin 1

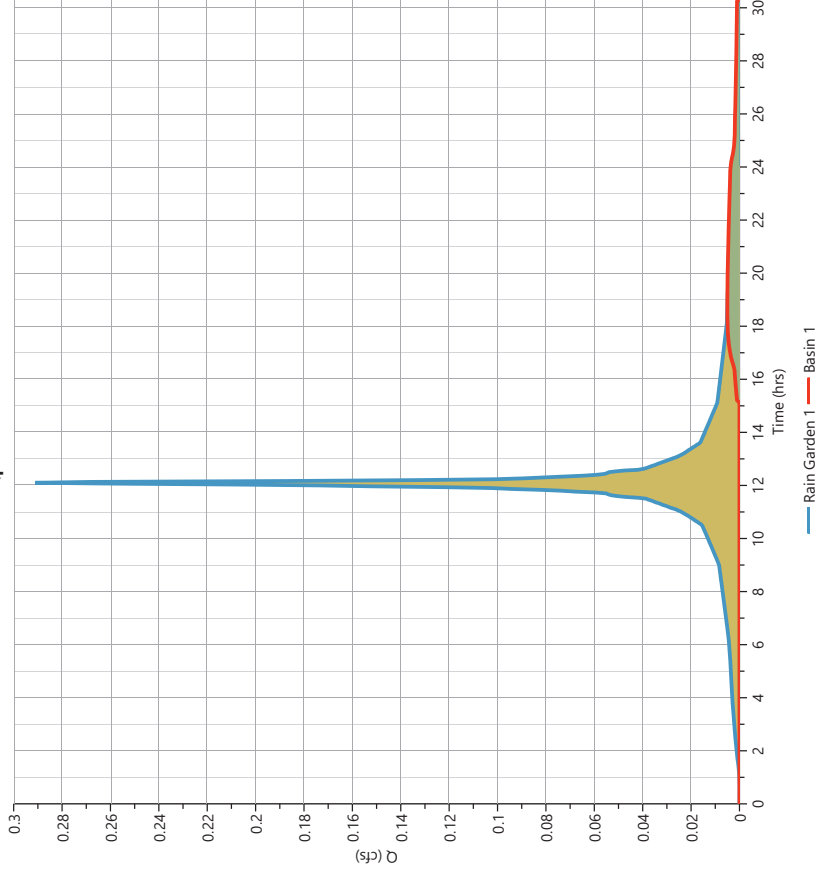
Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.005 cfs
Storm Frequency	= 2-yr	Time to Peak	= 18.83 hrs
Time Interval	= 2 min	Hydrograph Volume	= 166 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.81 ft
Pond Name	= BASIN 1	Max. Storage	= 879 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 8.88 hrs

$Q_p = 0.01 \text{ cfs}$



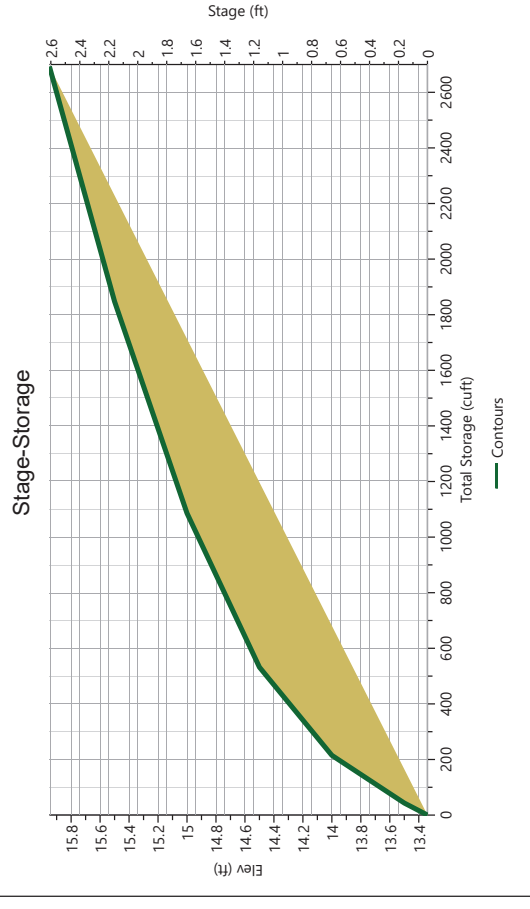
Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

BASIN 1

Stage-Storage

[illegible]

Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

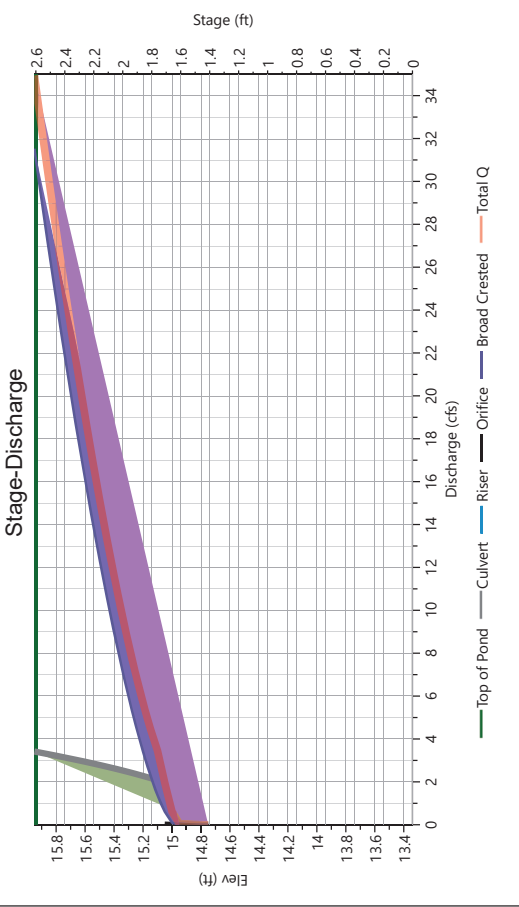
11-22-2022

BASIN 1

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Orifice Plate
		1*	2	3	
Rise, in	12	3			Orifice Dia, in
Span, in	12	3			No. Orifices
No. Barrels	1	1			Invert Elevation, ft
Invert Elevation, ft	13.90	14.77			Height, ft
Orifice Coefficient, Co	0.60	0.60			Orifice Coefficient, Co
Length, ft	100				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs			Ancillary
		1	2	3	
Shape / Type	Box	Broad Crested			Exfiltration, in/hr
Crest Elevation, ft	14.95	14.98			
Crest Length, ft	12	10			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

*Routes through Culvert.



Pond Report

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

BASIN 1

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pr Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	13.34	0.000	0.000	0.000			0.000	0.000						0.000
0.16	13.50	43.4	0.000	0.000			0.000	0.000						0.000
0.66	14.00	215	0.000	0.000			0.000	0.000						0.000
1.16	14.50	532	0.000	0.000			0.000	0.000						0.000
1.66	15.00	1,086	0.520 oc	0.077			0.443	0.093						0.613
2.16	15.50	1,847	2,788 oc	0.000			0.000	12.37						15.16
2.61	15.95	2,693	3,415 oc	0.000			0.000	31.53						34.94

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

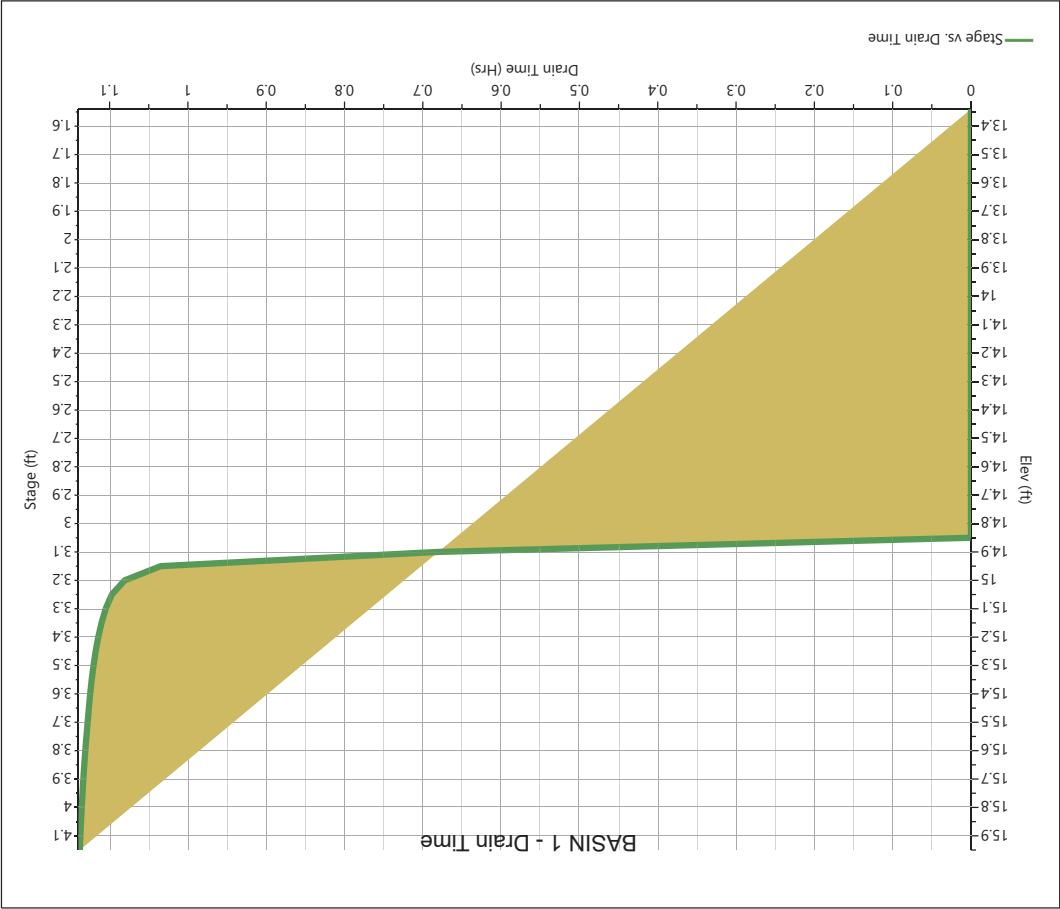
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

BASIN 1

Pond Drawdown



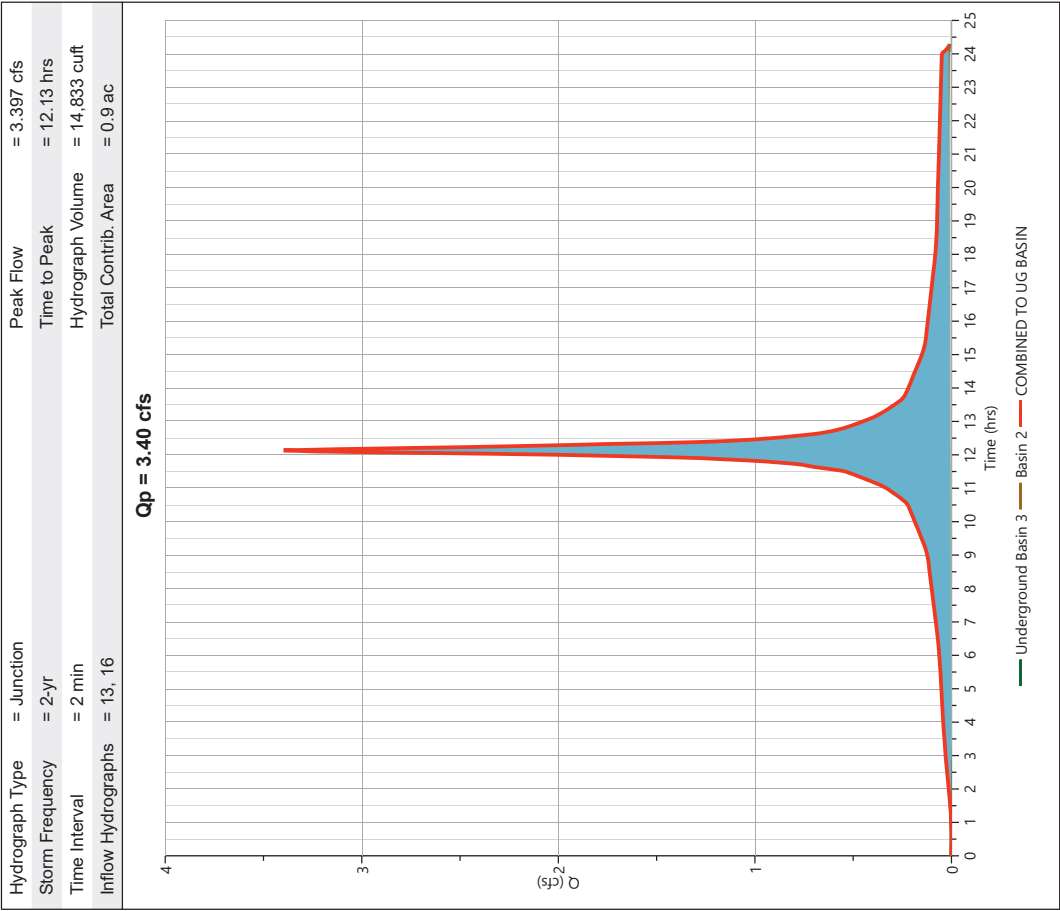
Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18



Hydrograph Type	= Junction	Peak Flow	= 3.397 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 14,833 cuft
Inflow Hydrographs	= 13, 16	Total Contrib. Area	= 0.9 ac

Qp = 3.40 cfs

Hydrograph Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

Post UG Basin 3

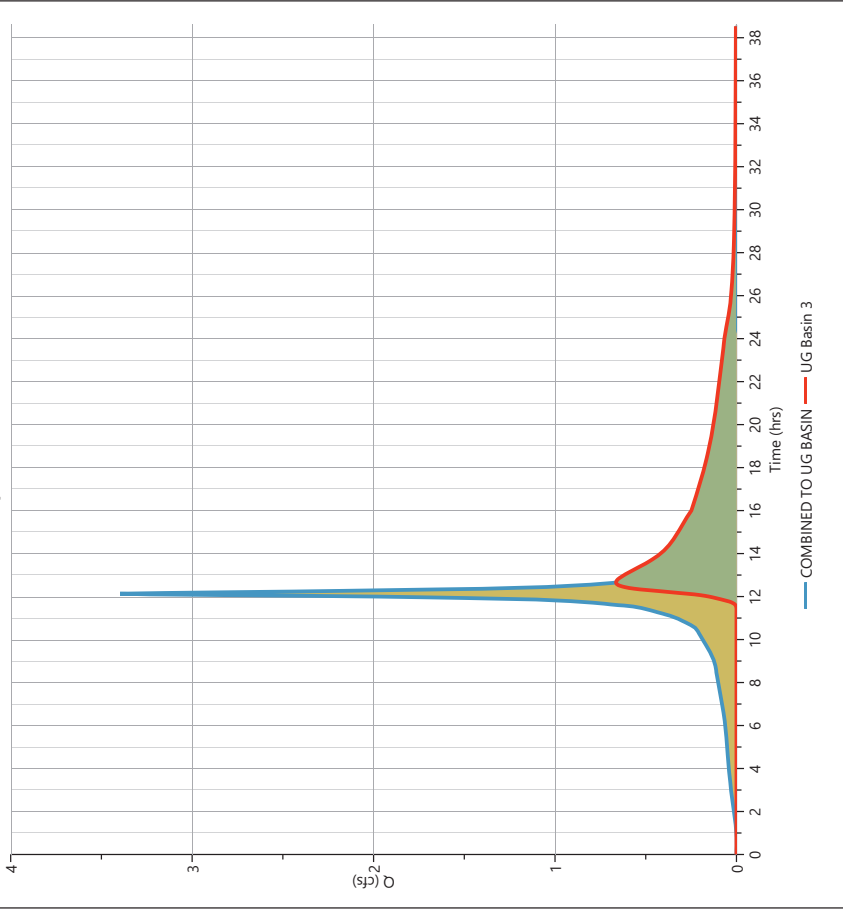
Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 0.664 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.67 hrs
Time Interval	= 2 min	Hydrograph Volume	= 11,045 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 13.46 ft
Pond Name	= UG BASIN	Max. Storage	= 8,579 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 3.82 hrs

Qp = 0.66 cfs



Pond Report

Project Name:
11-22-2022

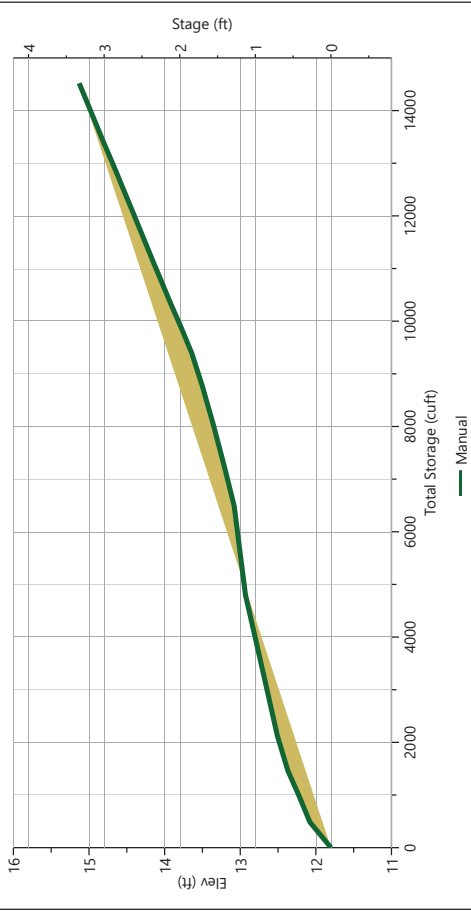
Hydrology Studio v 3.0.0.26

UG BASIN

Stage-Storage

User Defined Storage			Stage / Storage Table			
Description	Input	Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
Bottom Elevation, ft	11.80	0.00	11.80	n/a	0.000	0.000
		0.28	12.08	n/a	487	487
		0.42	12.22	n/a	488	975
		0.57	12.37	n/a	487	1,462
		0.71	12.51	n/a	668	2,130
		0.85	12.65	n/a	896	3,026
		0.99	12.79	n/a	890	3,916
		1.13	12.93	n/a	877	4,793
		1.28	13.08	n/a	1,686	6,489
		1.42	13.22	n/a	805	7,294
		1.56	13.36	n/a	763	8,057
		1.70	13.50	n/a	704	8,761
		1.84	13.64	n/a	627	9,388
		1.98	13.78	n/a	487	9,875
		2.13	13.93	n/a	488	10,363
		2.27	14.07	n/a	487	10,850
		2.41	14.21	n/a	487	11,337
		2.55	14.35	n/a	488	11,825
		2.69	14.49	n/a	487	12,312
		3.00	14.80	n/a	1,061	13,373
		3.33	15.13	n/a	1,147	14,520

Stage-Storage



Pond Report

Project Name:
11-22-2022

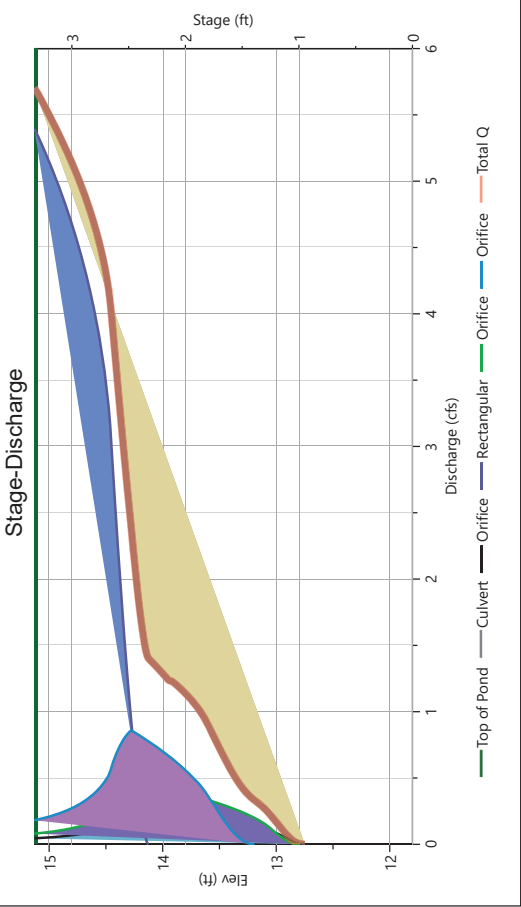
Hydrology Studio v 3.0.0.26

UG BASIN

Stage-Discharge

Culvert / Orifices	Culvert	Orifices			Perforated Riser
		1*	2*	3*	
Rise, in	12	3	4	6	Hole Diameter, in
Span, in	12	3	4	6	No. holes
No. Barrels	1	1	1	1	Invert Elevation, ft
Invert Elevation, ft	12.19	12.75	12.80	13.20	Height, ft
Orifice Coefficient, Co	0.60	0.60	0.60	0.60	Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	.3				
N-Value, n	0.013				
Weirs	Riser*	Weirs		Ancillary	
		1*	2		
Shape / Type		Rectangular		Exfiltration, in/hr	
Crest Elevation, ft		14.13			
Crest Length, ft		5			
Angle, deg					
Weir Coefficient, Cw		3.3			

*Routes through Culvert.



Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

UG BASIN

Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	11.80	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.28	12.08	487	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.42	12.22	975	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.57	12.37	1,462	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.71	12.51	2,130	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.85	12.65	3,026	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000				0.000
0.99	12.79	3,916	0.004 oc	0.004	0.000	0.000	0.000	0.000	0.000	0.000				0.004
1.13	12.93	4,793	0.094 oc	0.055	0.039	0.000	0.000	0.000	0.000	0.000				0.094
1.28	13.08	6,489	0.248 oc	0.107	0.141	0.000	0.000	0.000	0.000	0.000				0.248
1.42	13.22	7,294	0.352 oc	0.139	0.211	0.001	0.000	0.000	0.000	0.000				0.352
1.56	13.36	8,057	0.503 oc	0.165	0.263	0.075	0.000	0.000	0.000	0.000				0.503
1.70	13.50	8,761	0.724 oc	0.187	0.307	0.230	0.000	0.000	0.000	0.000				0.724
1.84	13.64	9,388	0.962 oc	0.204	0.345	0.413	0.000	0.000	0.000	0.000				0.962
1.98	13.78	9,875	1,120 oc	0.208	0.369	0.543	0.000	0.000	0.000	0.000				1,120
2.13	13.93	10,363	1,234 oc	0.208	0.371	0.655	0.000	0.000	0.000	0.000				1,234
2.27	14.07	10,850	1,352 oc	0.219	0.389	0.744	0.000	0.000	0.000	0.000				1,352
2.41	14.21	11,337	1,823 oc	0.225	0.400	0.824	0.373	0.000	0.000	0.000				1,823
2.55	14.35	11,825	3,031 oc	0.196	0.348	0.764	1,703	0.000	0.000	0.000				3,031
2.69	14.49	12,312	4,261 oc	0.125	0.222	0.500	3,414 s	0.000	0.000	0.000				4,261
3.00	14.80	13,373	5,139 oc	0.068	0.121	0.272	4,677 s	0.000	0.000	0.000				5,139
3.33	15.13	14,520	5,703 oc	0.045	0.081	0.182	5,395 s	0.000	0.000	0.000				5,703

Suffix: key: /c = inlet control, oc = orifice control, s = submerged weir

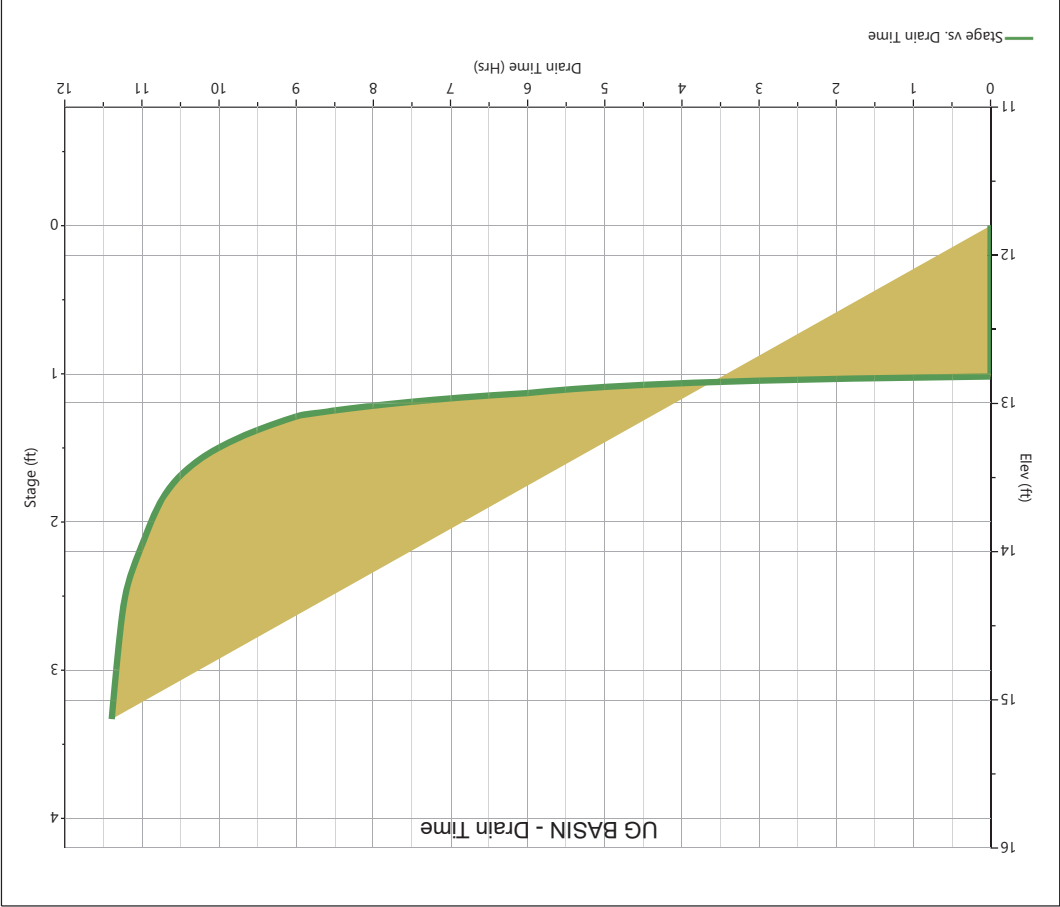
Pond Report

Project Name:
11-22-2022

Hydrology Studio v 3.0.0.26

UG BASIN

Pond Drawdown



Hydrograph Report

Project Name:
11-22-2022

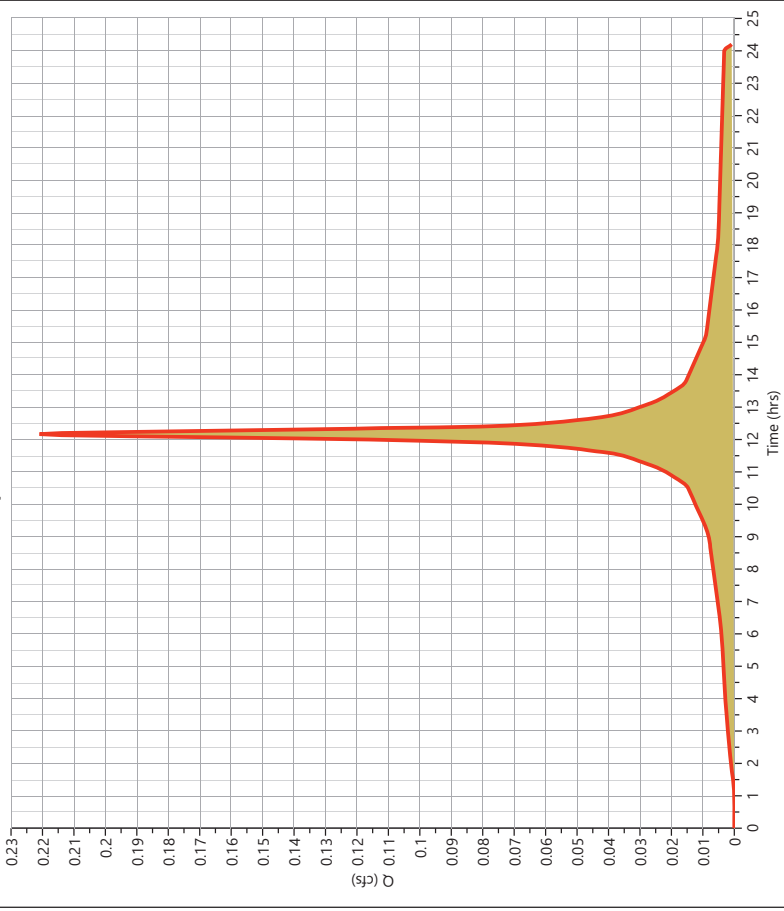
Hydrology Studio v 3.0.0.26

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 978 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.22 cfs



Hydrograph Report

Project Name:

11-22-2022

Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.000 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.58	39	OPEN SPACE
0.28	30	WOODS
0.86	36	Weighted CN Method Employed

Qp = 0.00 cfs

Hydrograph Report

Project Name:

11-22-2022

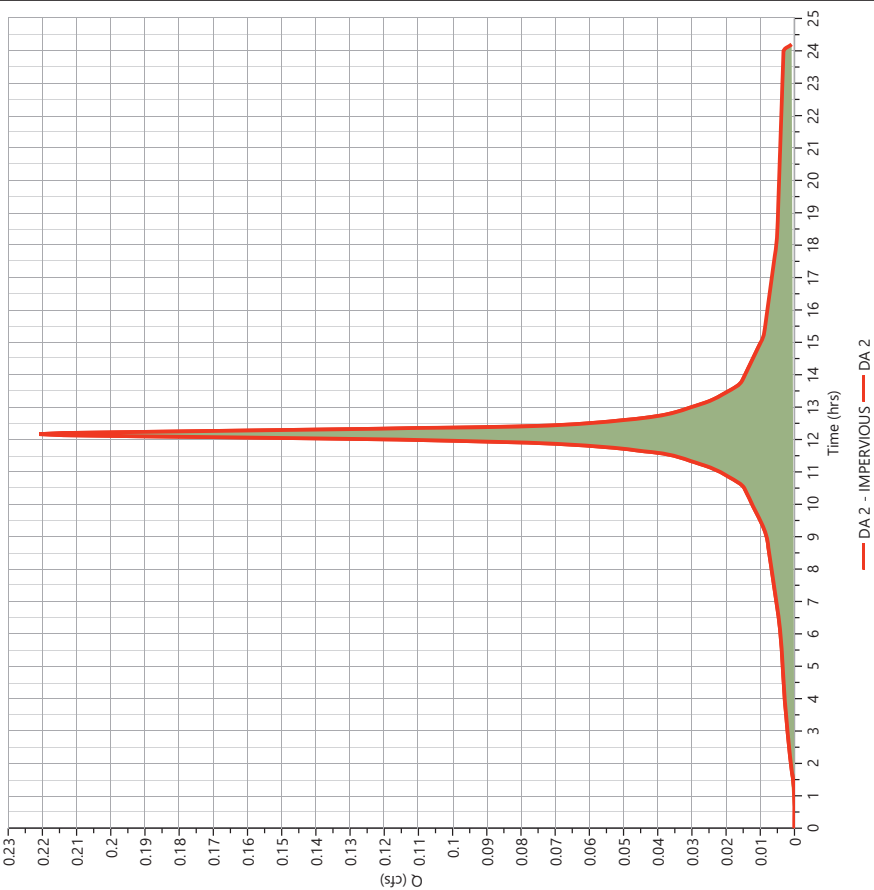
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 978 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.22 cfs



Hydrograph Report

Project Name:
11-22-2022

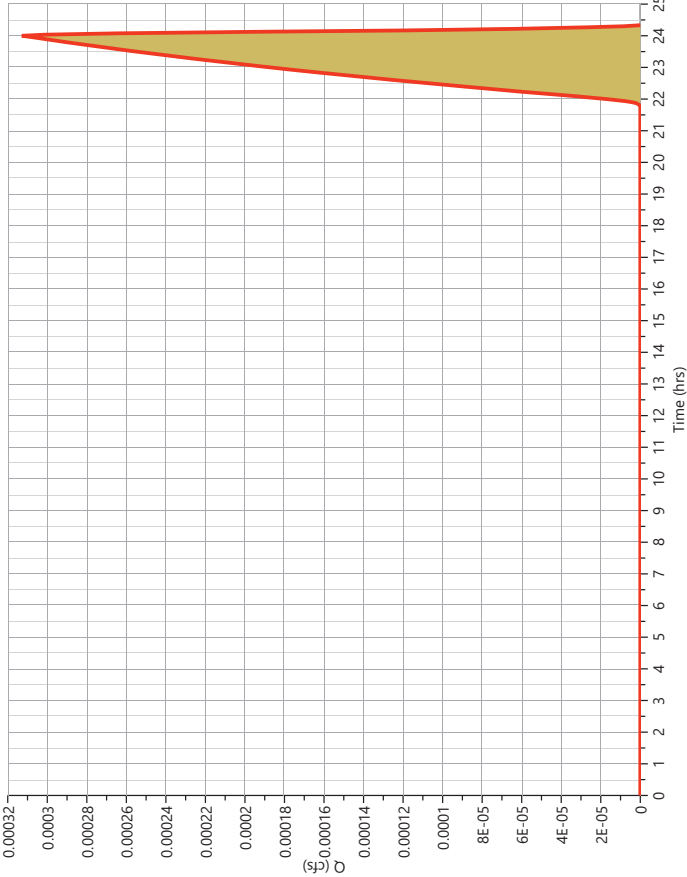
Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 1.47 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	39	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	

Qp = 0.00 cfs



Hydrograph Report

Project Name:
11-22-2022

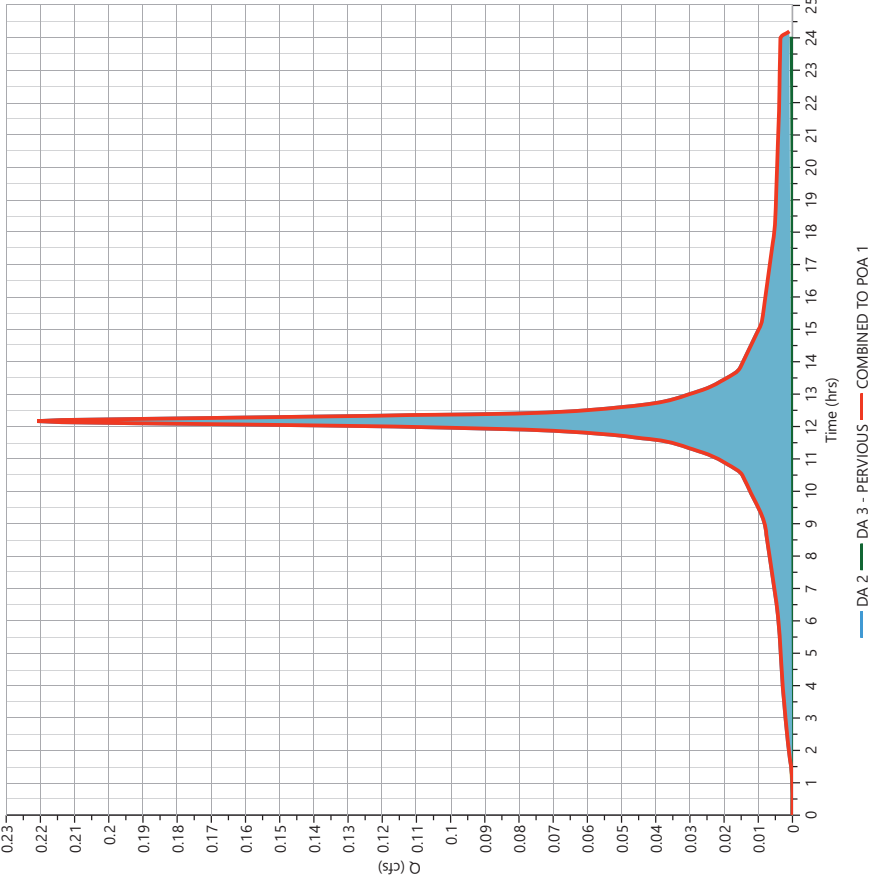
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.221 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 980 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac

Qp = 0.22 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

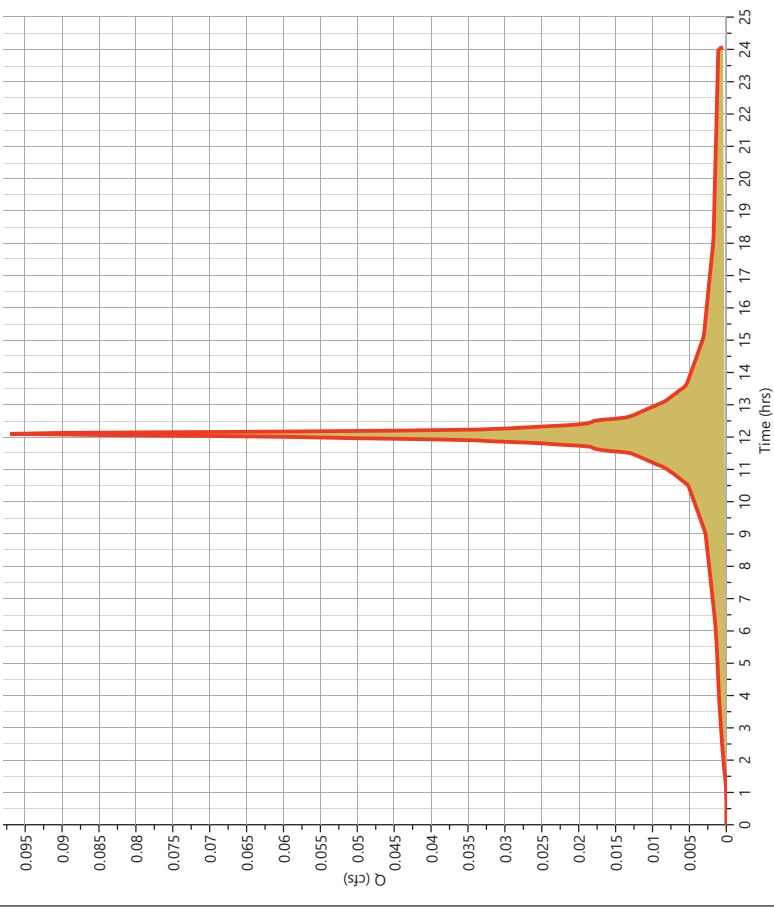
11-22-2022

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.097 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 333 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.10 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

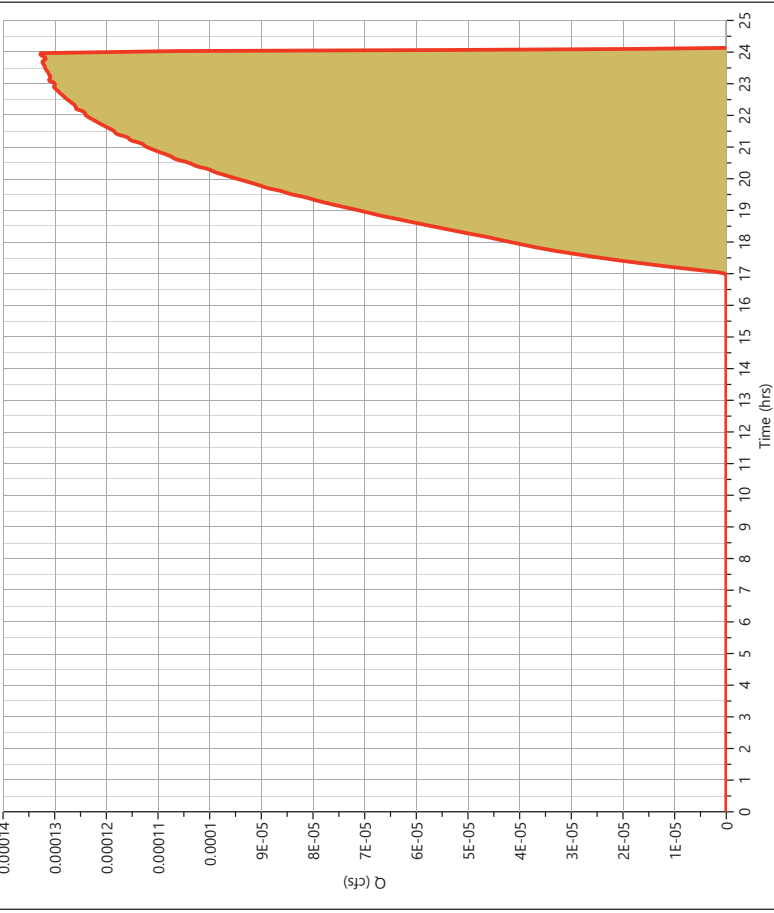
11-22-2022

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 2.35 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



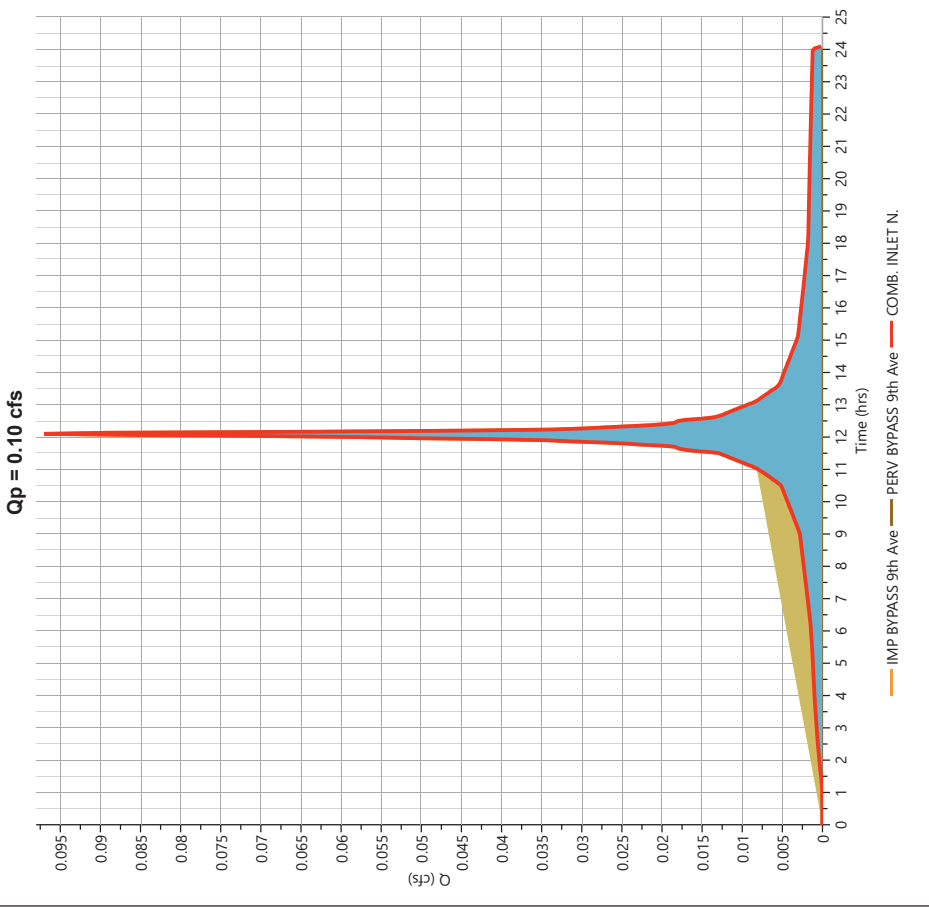
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.097 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 336 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac



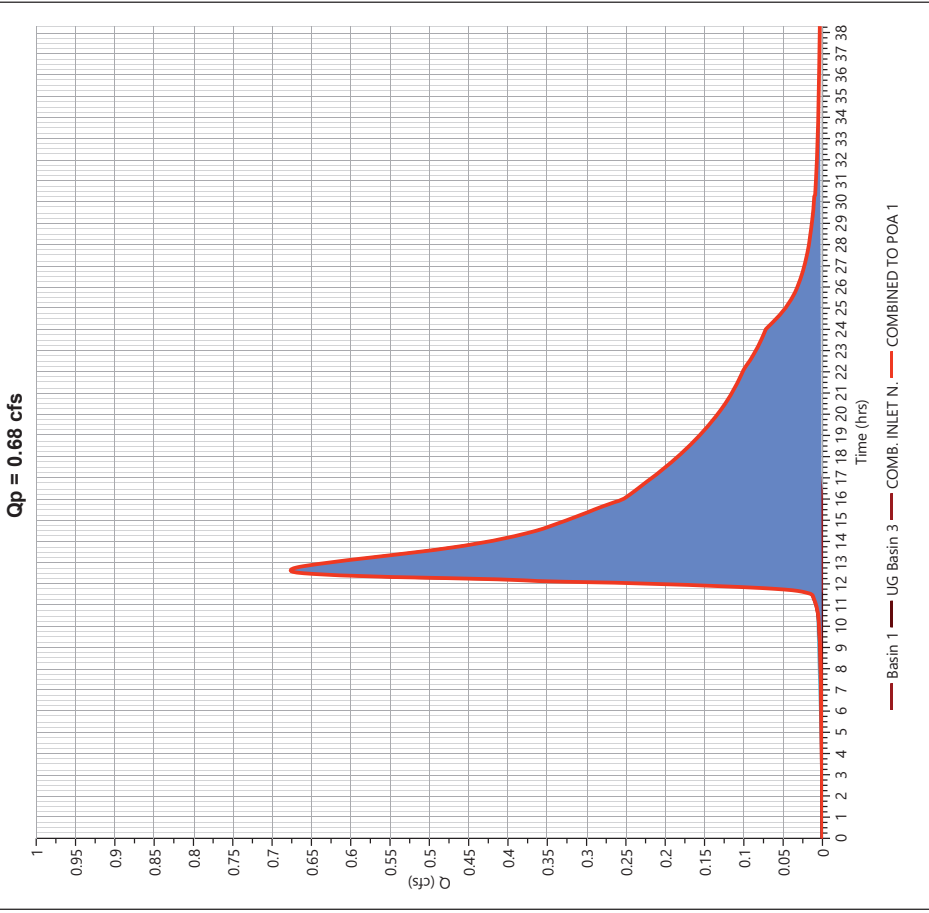
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 0.677 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.63 hrs
Time Interval	= 2 min	Hydrograph Volume	= 11,547 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac



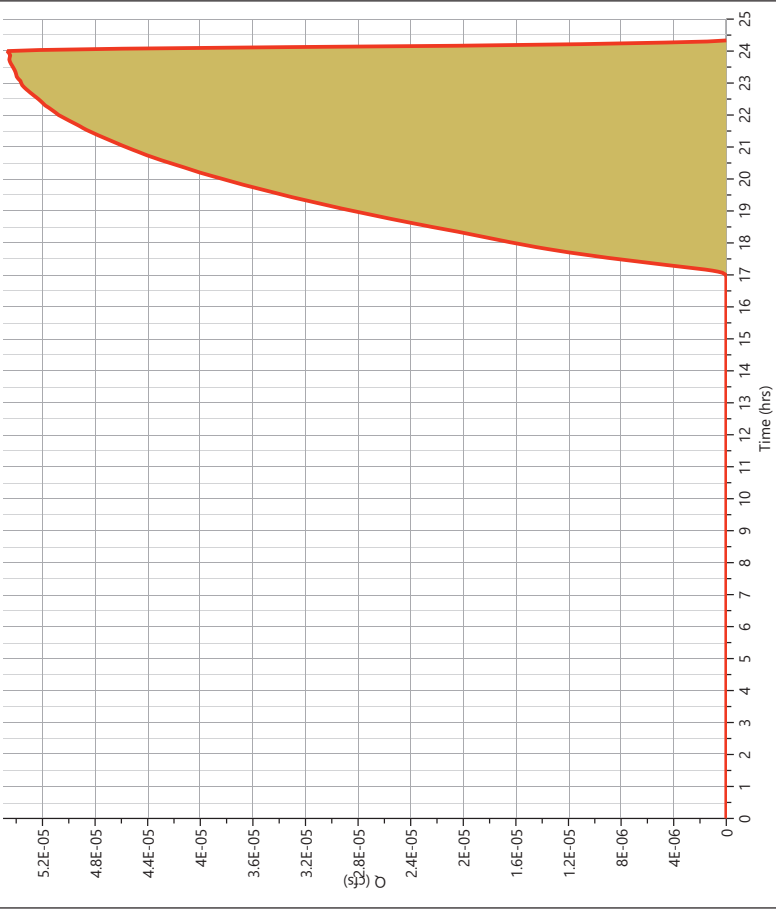
Hydrograph Report

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.969 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



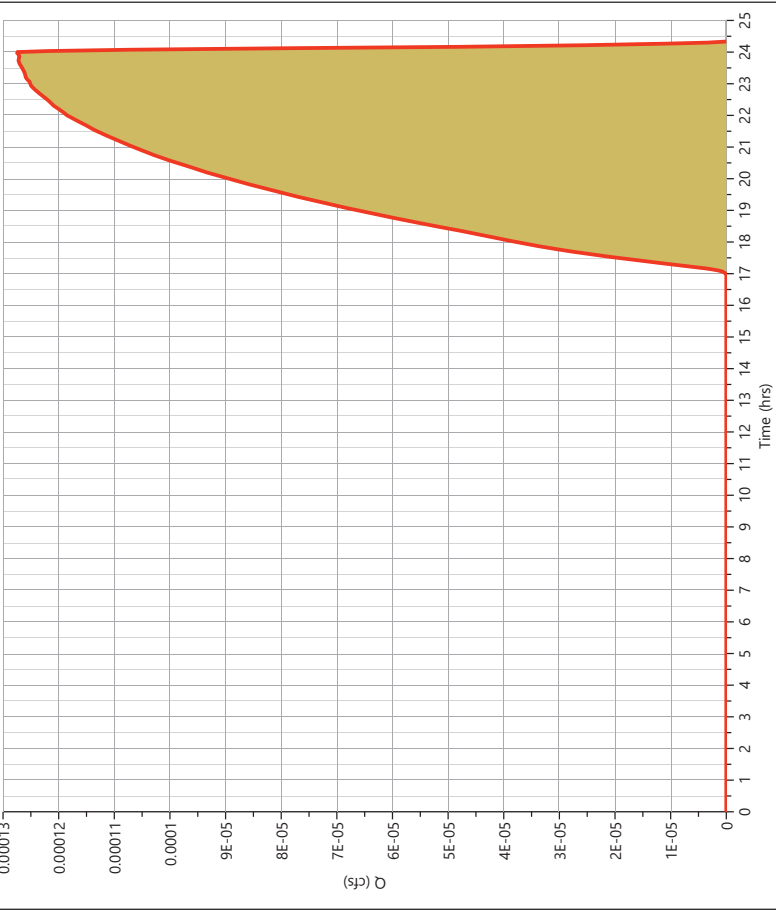
Hydrograph Report

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 2.26 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



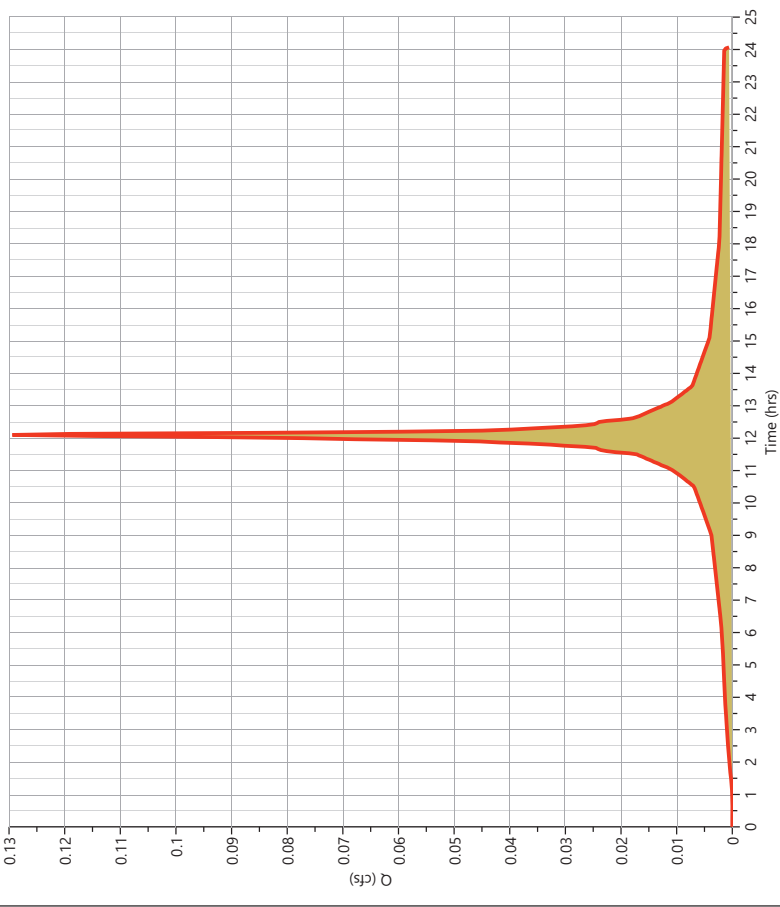
Hydrograph Report

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.129 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 445 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.13 cfs



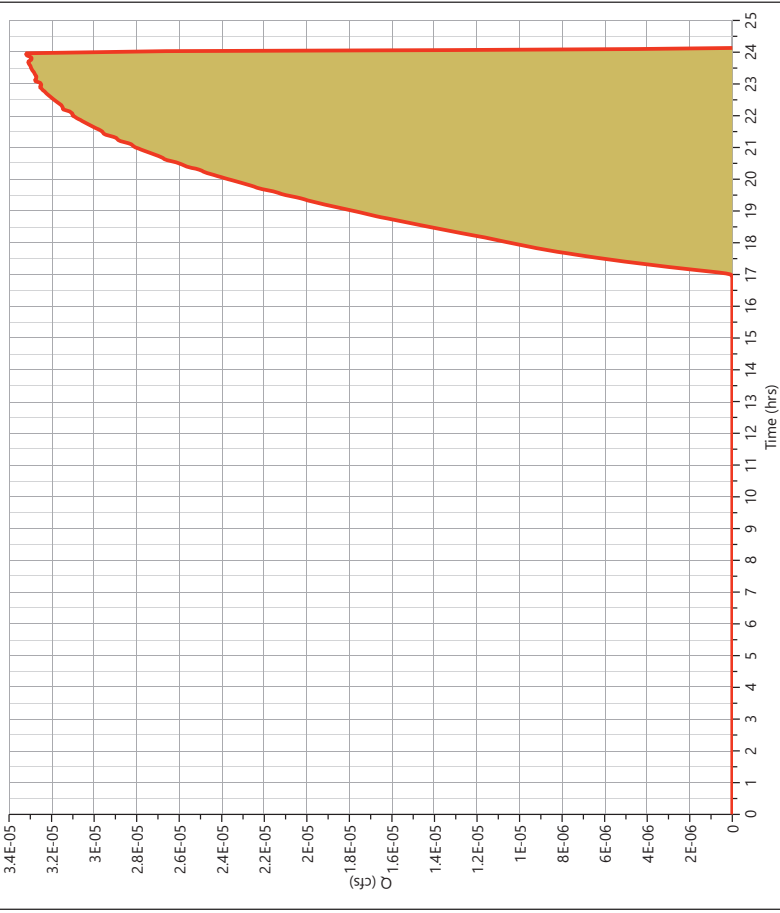
Hydrograph Report

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.000 cfs
Storm Frequency	= 2-yr	Time to Peak	= 24.00 hrs
Time Interval	= 2 min	Runoff Volume	= 0.588 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 3.50 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

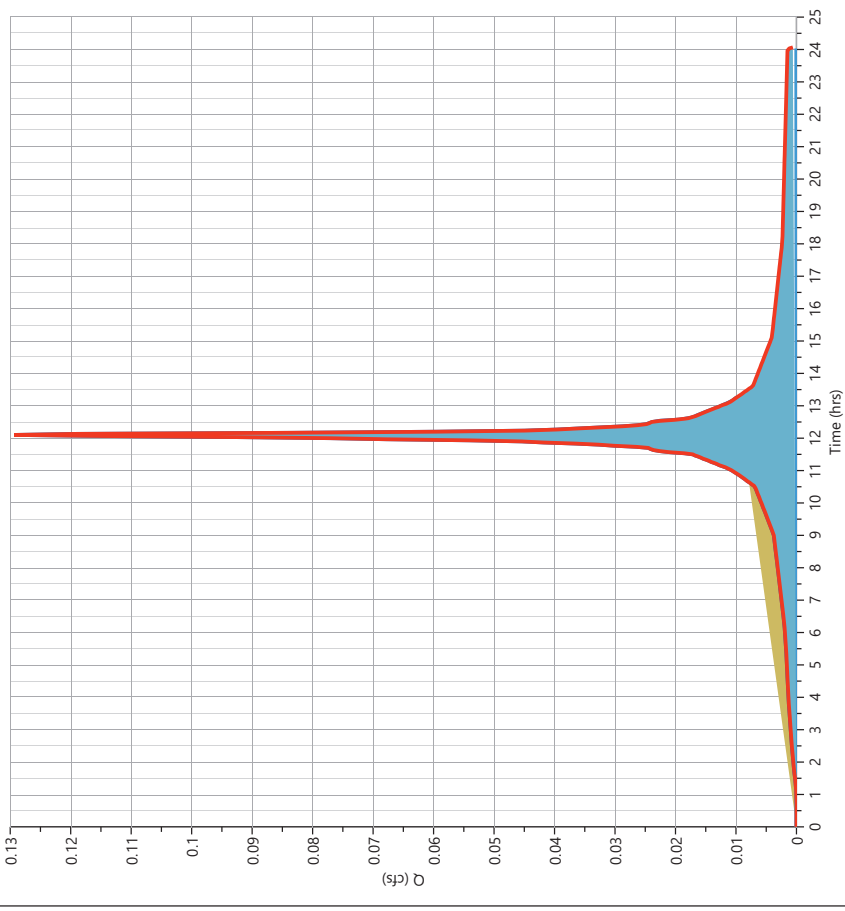
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.129 cfs
Storm Frequency	= 2-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 445 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.13 cfs



Imp Bypass 8th Ave Perv Bypass 8th Ave Comb. Bypass POA 2

Hydrograph 10-yr Summary

Project Name:

Hydrology Studio v 3.0.0.26

11-22-2022

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	NRCS Runoff	Pre DA 1 - IMPERVIOUS	0.187	12.13	753	----	----	----
2	NRCS Runoff	Pre DA 1 - PERVIOUS	0.002	12.97	38.7	----	----	----
3	NRCS Runoff	Post Roof Area DA3	1.763	12.10	6,173	----	----	----
4	NRCS Runoff	Post Deck DA3	0.947	12.17	4,268	----	----	----
5	NRCS Runoff	Post Pervious Pavement	0.646	12.17	2,910	----	----	----
6	NRCS Runoff	Post Pervious DA 1	0.002	12.50	39.9	----	----	----
7	NRCS Runoff	Post Roof Area DA1	0.453	12.10	1,587	----	----	----
8	NRCS Runoff	Post Pervious DA2	0.004	12.50	59.8	----	----	----
9	NRCS Runoff	Post Roof Area DA2	2.519	12.10	8,818	----	----	----
10	Junction	Pre DA 1 - POA 2	0.187	12.13	791	1, 2	----	----
11	NRCS Runoff	Post Pervious DA3	0.007	12.60	121	----	----	----
12	NRCS Runoff	Post Impervious DA3	0.301	12.17	1,358	----	----	----
13	Junction	Post Underground Basin 3	3.455	12.13	14,830	3, 4, 5, 11, 12	----	----
14	Junction	Post Rain Garden 1	0.453	12.10	1,627	6, 7	----	----
15	Junction	Post Basin 2	2.519	12.10	8,878	8, 9	----	----
16	Pond Route	Post Basin 2	1.873	12.17	8,877	15	14.55	774
17	Pond Route	Post Basin 1	0.053	12.83	792	14	14.95	1,025
18	Junction	Post COMBINED TO UG BASIN	5.307	12.13	23,707	13, 16	----	----
19	Pond Route	Post UG Basin 3	2.237	12.37	19,919	18	14.26	11,513
20	NRCS Runoff	Pre DA 2 - IMPERVIOUS	0.344	12.17	1,552	----	----	----
21	NRCS Runoff	Pre DA 2 - PERVIOUS	0.021	13.30	570	----	----	----
22	Junction	Pre DA 2	0.344	12.17	2,122	20, 21	----	----
23	NRCS Runoff	Pre DA 3 - PERVIOUS	0.026	13.00	598	----	----	----
24	Junction	Pre COMBINED TO POA 1	0.344	12.17	2,720	22, 23	----	----
25	NRCS Runoff	Post IMP BYPASS 9th Ave	0.151	12.10	529	----	----	----
26	NRCS Runoff	Post PERV BYPASS 9th Ave	0.005	12.50	79.7	----	----	----
27	Junction	COMB. INLET N	0.151	12.10	609	25, 26	----	----
28	Junction	Post COMBINED TO POA 1	2.310	12.37	21,319	17, 19, 27	----	----
29	NRCS Runoff	Pre Bypass Memorial	0.002	12.60	32.9	----	----	----
30	NRCS Runoff	Post Bypass Memorial	0.004	12.60	76.7	----	----	----
31	NRCS Runoff	Post Imp Bypass 8th Ave	0.202	12.10	705	----	----	----
32	NRCS Runoff	Post Perv Bypass 8th Ave	0.001	12.50	19.9	----	----	----
33	Junction	Post Comb. Bypass POA 2	0.202	12.10	725	31, 32	----	----

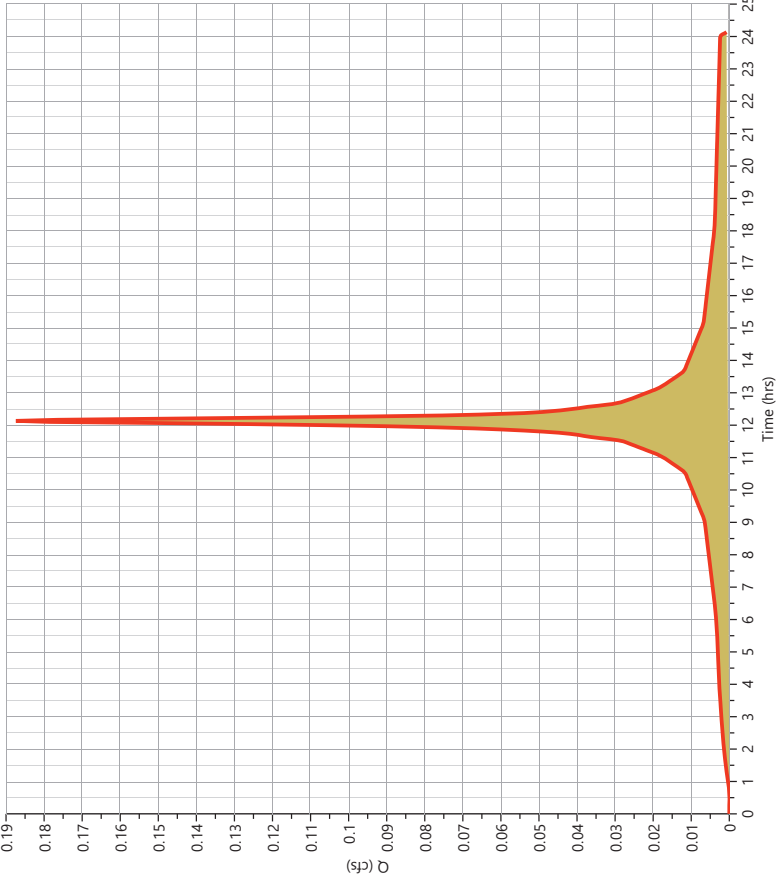
Hydrograph Report

Pre DA 1 - IMPERVIOUS

Hyd. No. 1

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.187 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Runoff Volume	= 753 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.19 cfs



Hydrograph Report

Pre DA 1 - PERVIOUS

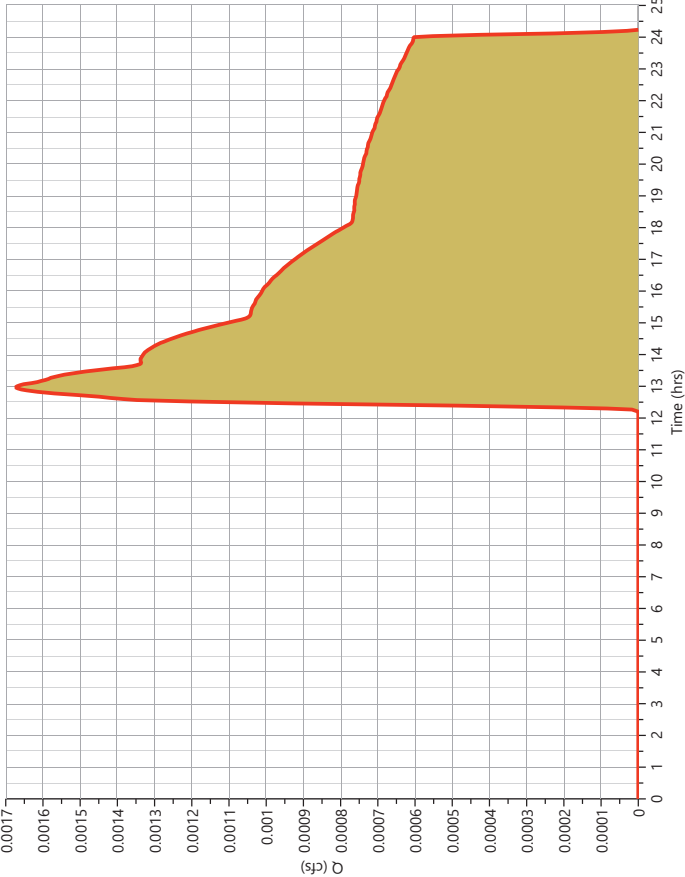
Hyd. No. 2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.97 hrs
Time Interval	= 2 min	Runoff Volume	= 38.7 cuft
Drainage Area	= 0.05 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 9.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA (ac)	CN	DESCRIPTION
0.04	39	open space
0.01	30	wooded
0.05	37	Weighted CN Method Employed

Qp = 0.00 cfs



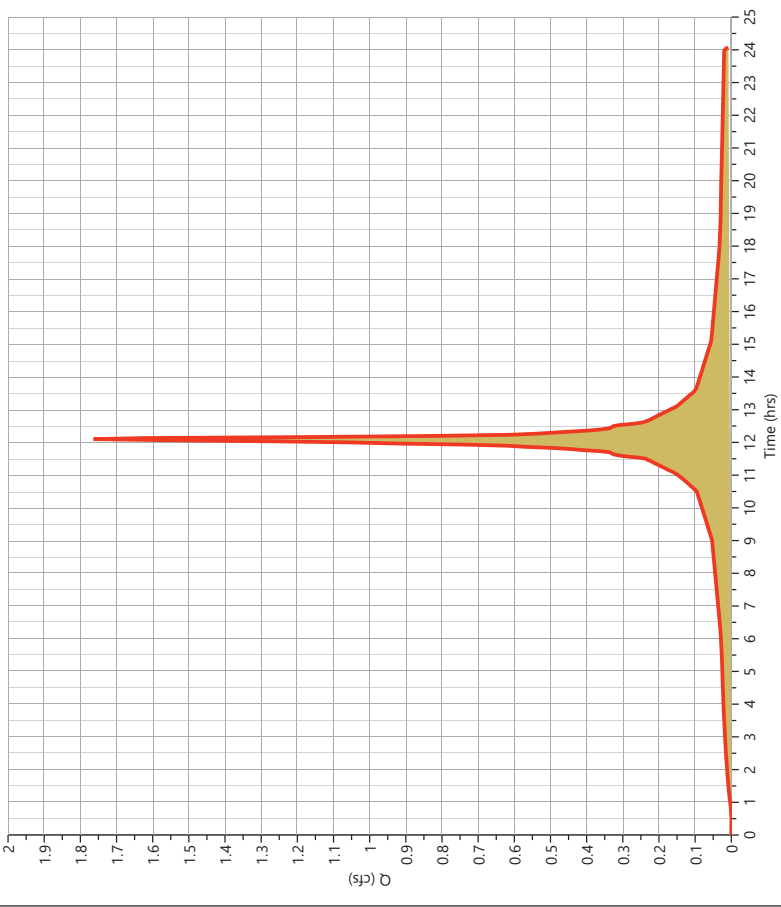
Hydrograph Report

Post Roof Area DA3

Hyd. No. 3

Hydrograph Type	= NRCS Runoff	Peak Flow	= 1.763 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 6,173 cuft
Drainage Area	= 0.35 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 1.76 cfs



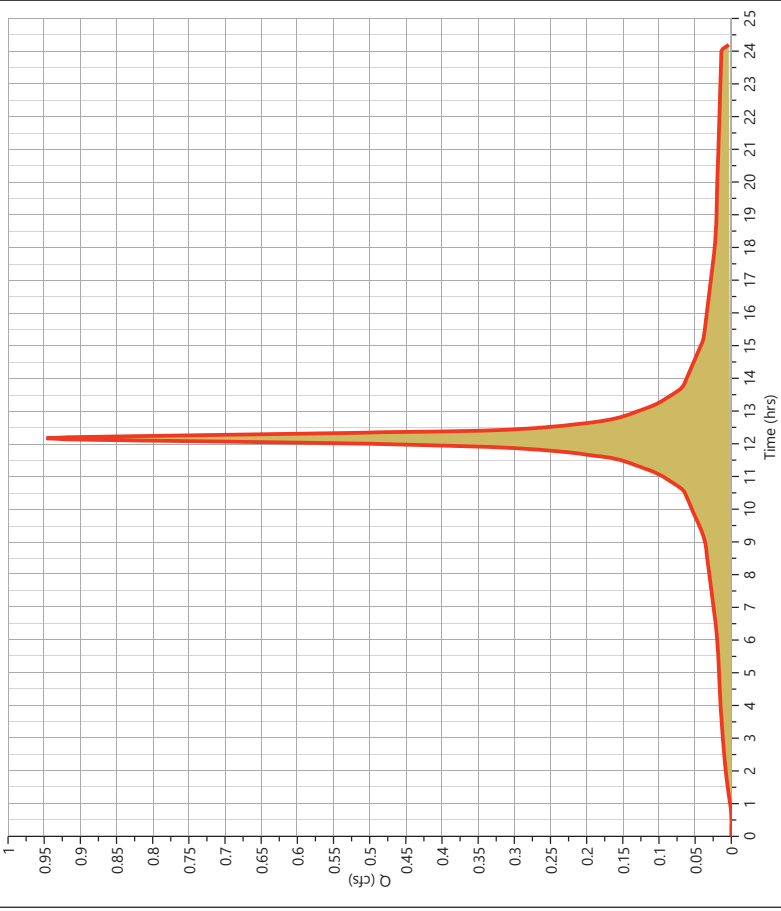
Hydrograph Report

Post Deck DA3

Hyd. No. 4

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.947 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 4,268 cuft
Drainage Area	= 0.22 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.95 cfs



Hydrograph Report

Post Pervious Pavement

Hyd. No. 5

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.646 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 2,910 cuft
Drainage Area	= 0.15 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.65 cfs



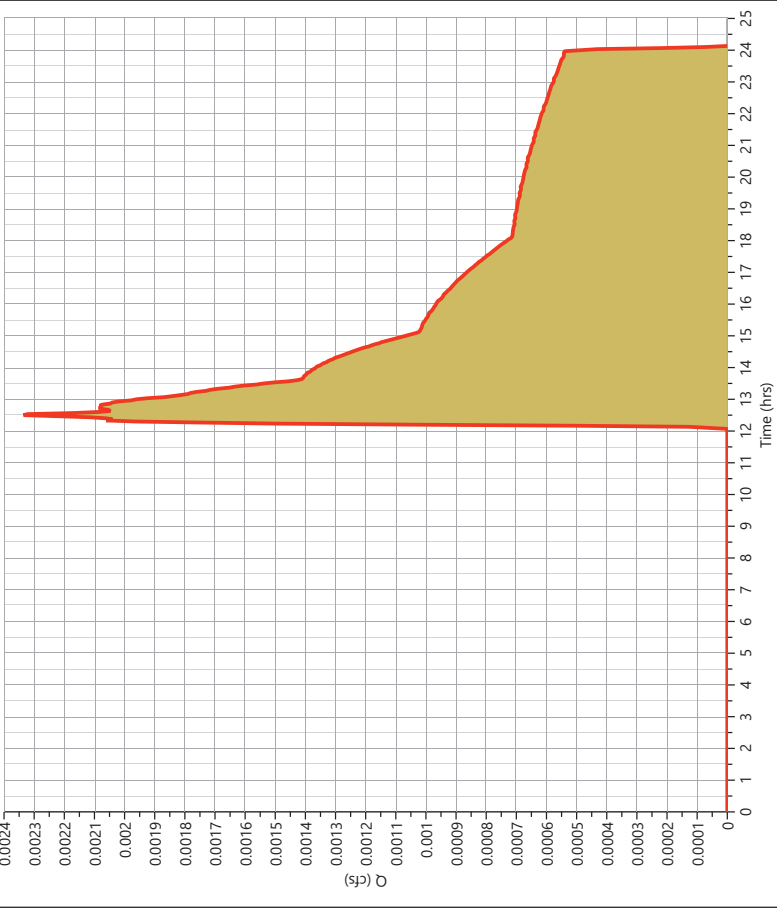
Hydrograph Report

Post Pervious DA 1

Hyd. No. 6

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 39.9 cuft
Drainage Area	= 0.04 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



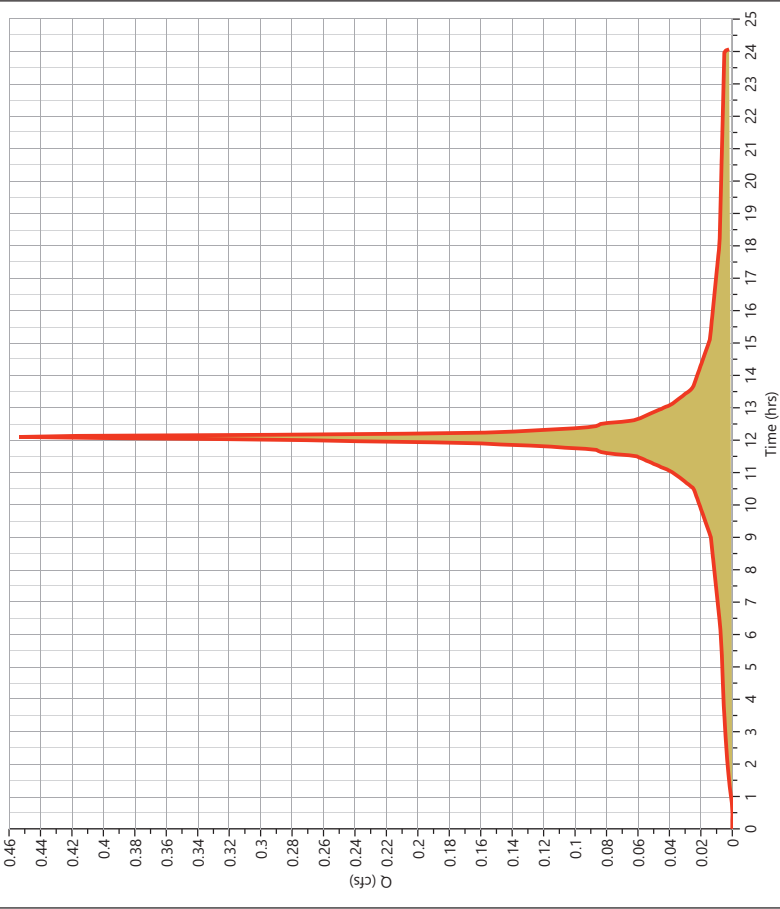
Hydrograph Report

Post Roof Area DA1

Hyd. No. 7

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.453 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 1,587 cuft
Drainage Area	= 0.09 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.45 cfs



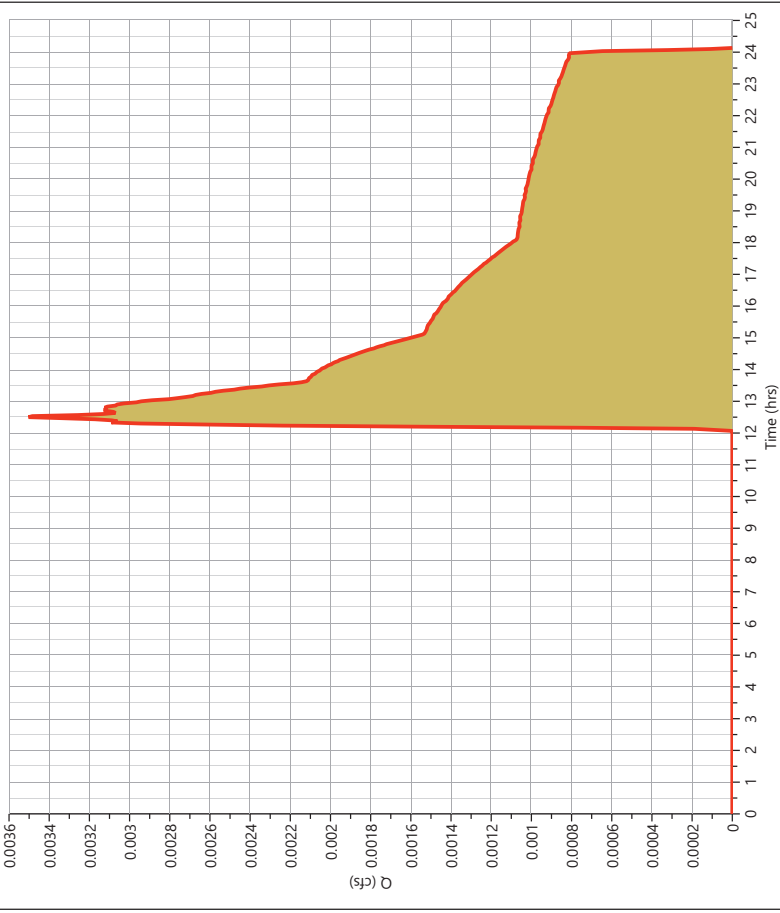
Hydrograph Report

Post Pervious DA2

Hyd. No. 8

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.004 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 59.8 cuft
Drainage Area	= 0.06 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



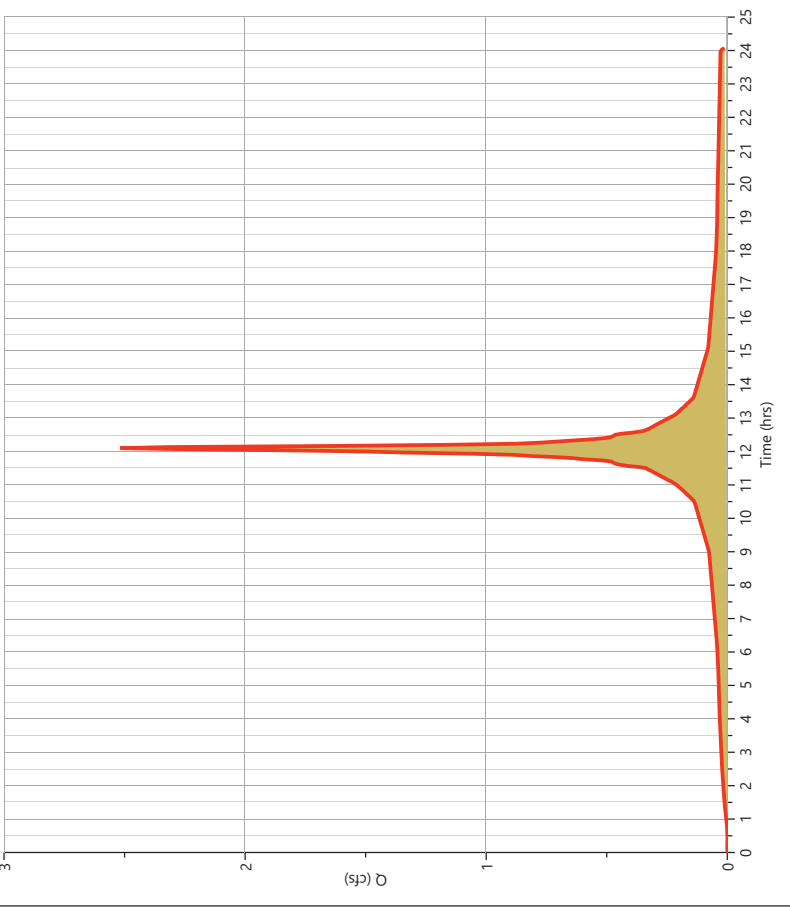
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Roof Area DA2

Hydrograph Type	= NRCS Runoff	Peak Flow	= 2.519 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 8,818 cuft
Drainage Area	= 0.5 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 2.52 cfs



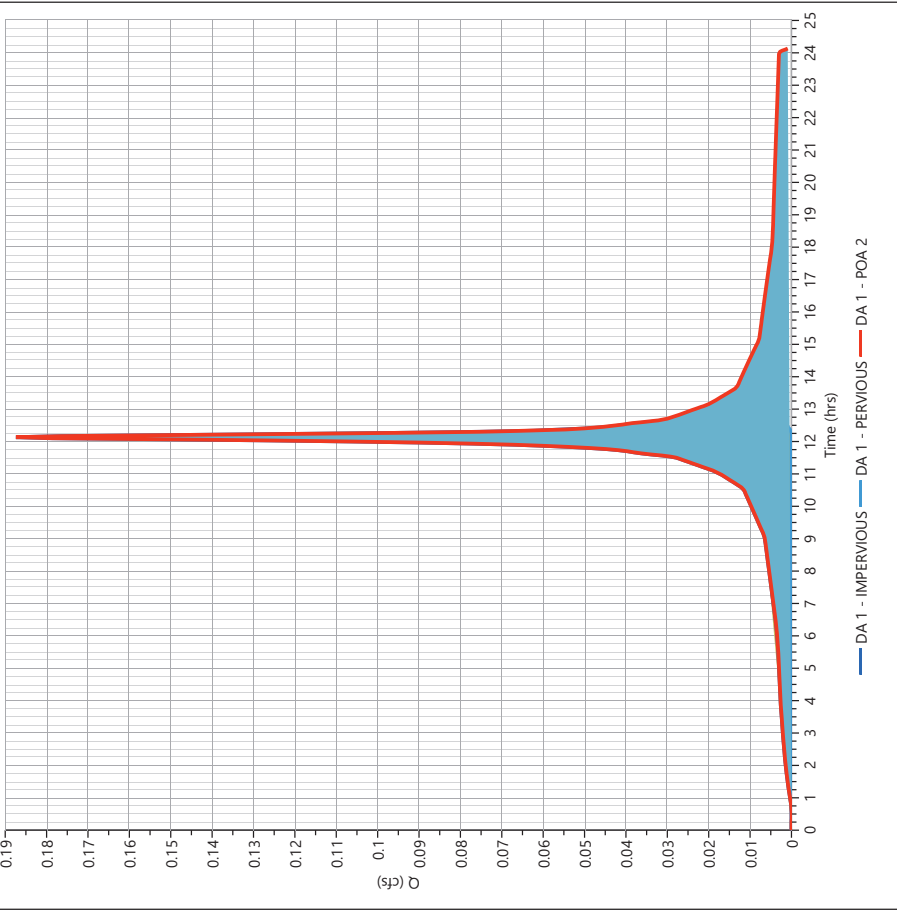
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 1 - POA 2

Hydrograph Type	= Junction	Peak Flow	= 0.187 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 791 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 0.09 ac

Qp = 0.19 cfs



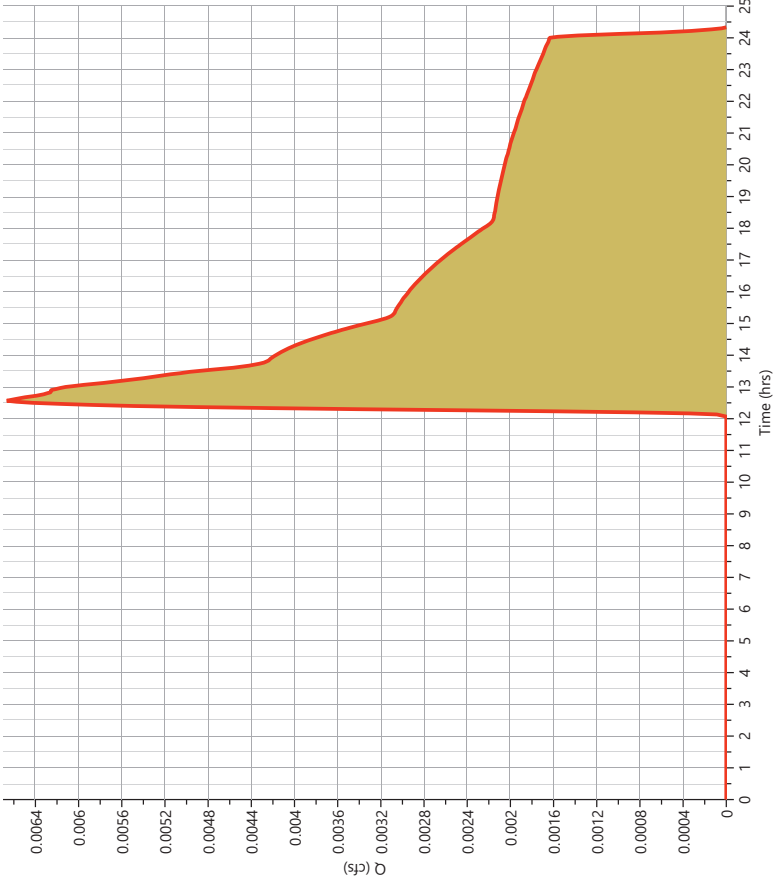
Hydrograph Report

Post Pervious DA3

Hyd. No. 11

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.007 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 121 cuft
Drainage Area	= 0.11 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.01 cfs



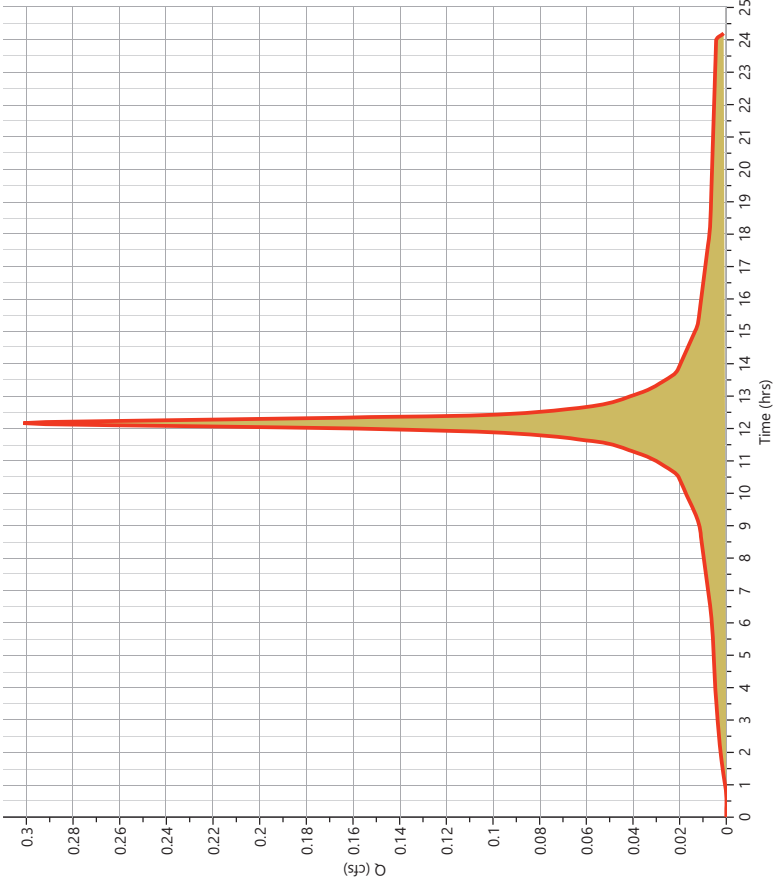
Hydrograph Report

Post Impervious DA3

Hyd. No. 12

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.301 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,358 cuft
Drainage Area	= 0.07 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 12.8 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.30 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

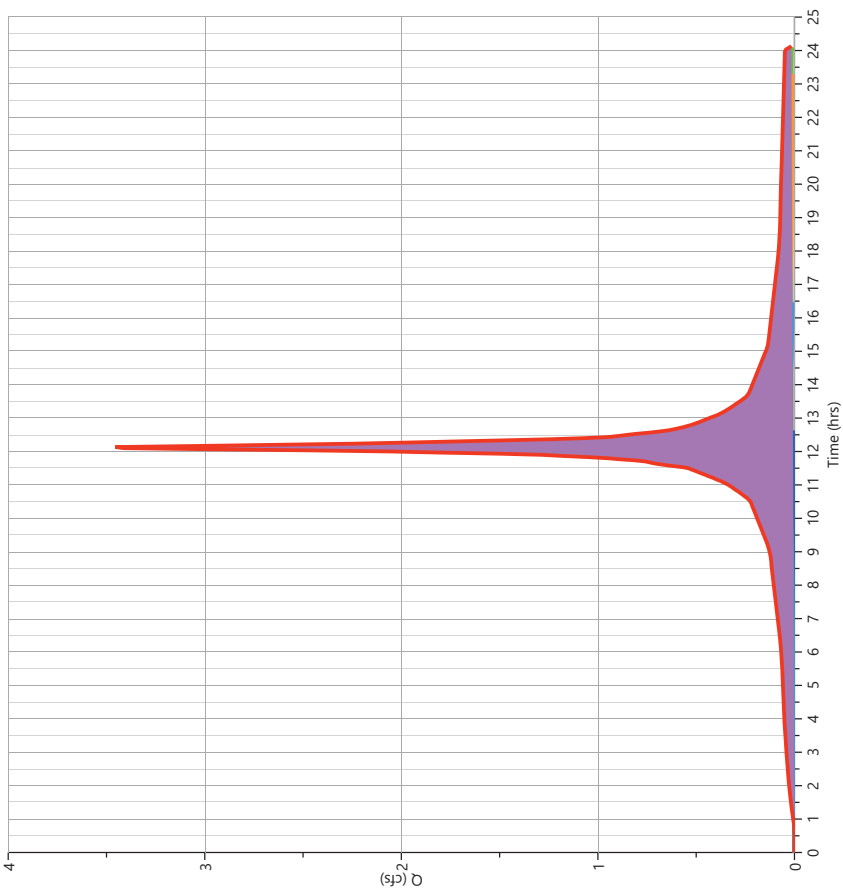
11-22-2022

Post Underground Basin 3

Hyd. No. 13

Hydrograph Type	= Junction	Peak Flow	= 3.455 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 14,830 cuft
Inflow Hydrographs	= 3, 4, 5, 11, 12	Total Contrib. Area	= 0.9 ac

Qp = 3.45 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

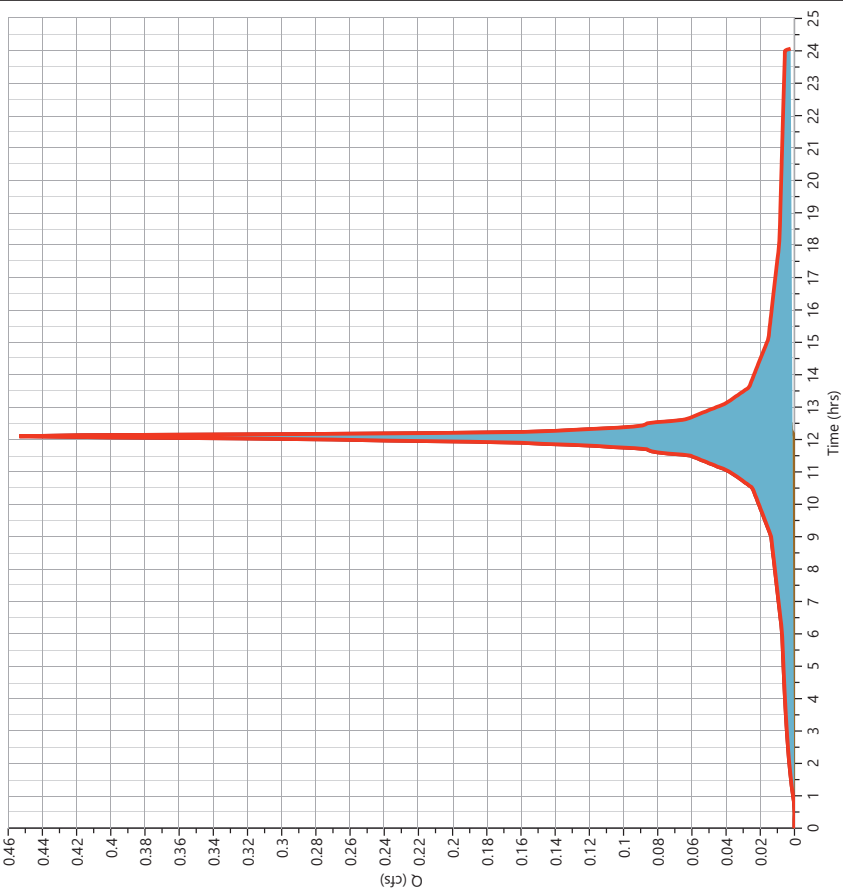
11-22-2022

Post Rain Garden 1

Hyd. No. 14

Hydrograph Type	= Junction	Peak Flow	= 0.453 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 1,627 cuft
Inflow Hydrographs	= 6, 7	Total Contrib. Area	= 0.13 ac

Qp = 0.45 cfs



Hydrograph Report

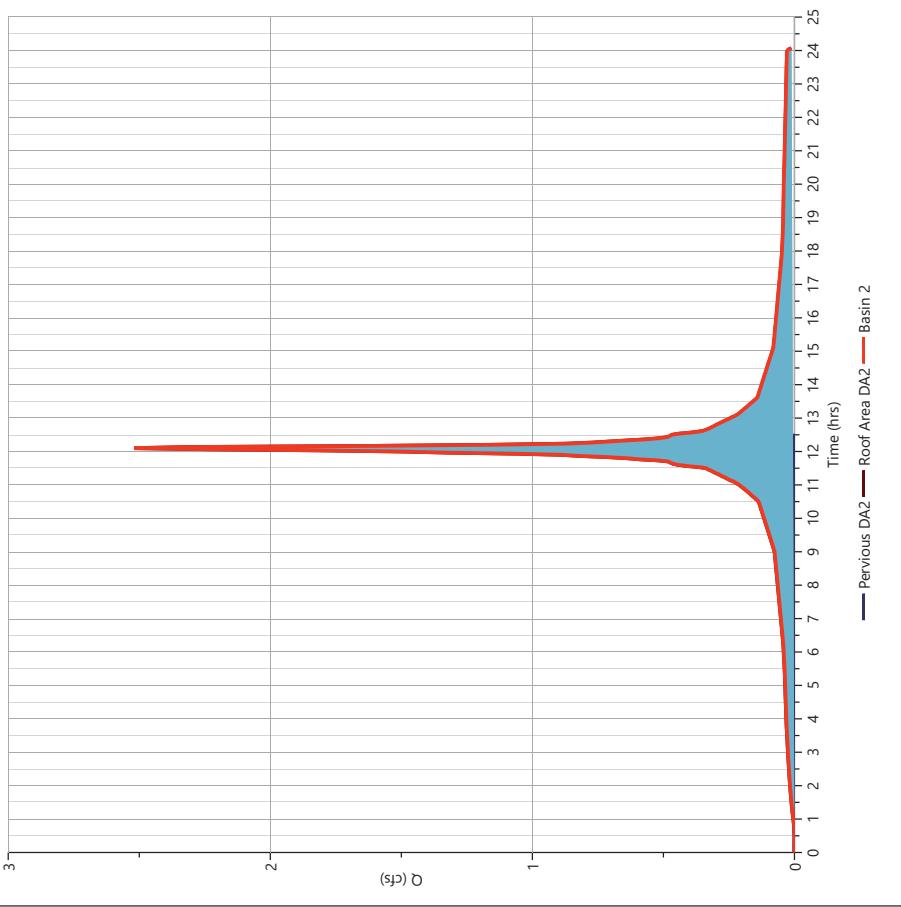
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Basin 2

Hyd. No. 15

Hydrograph Type	= Junction	Peak Flow	= 2.519 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 8.878 cuft
Inflow Hydrographs	= 8, 9	Total Contrib. Area	= 0.56 ac

Qp = 2.52 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

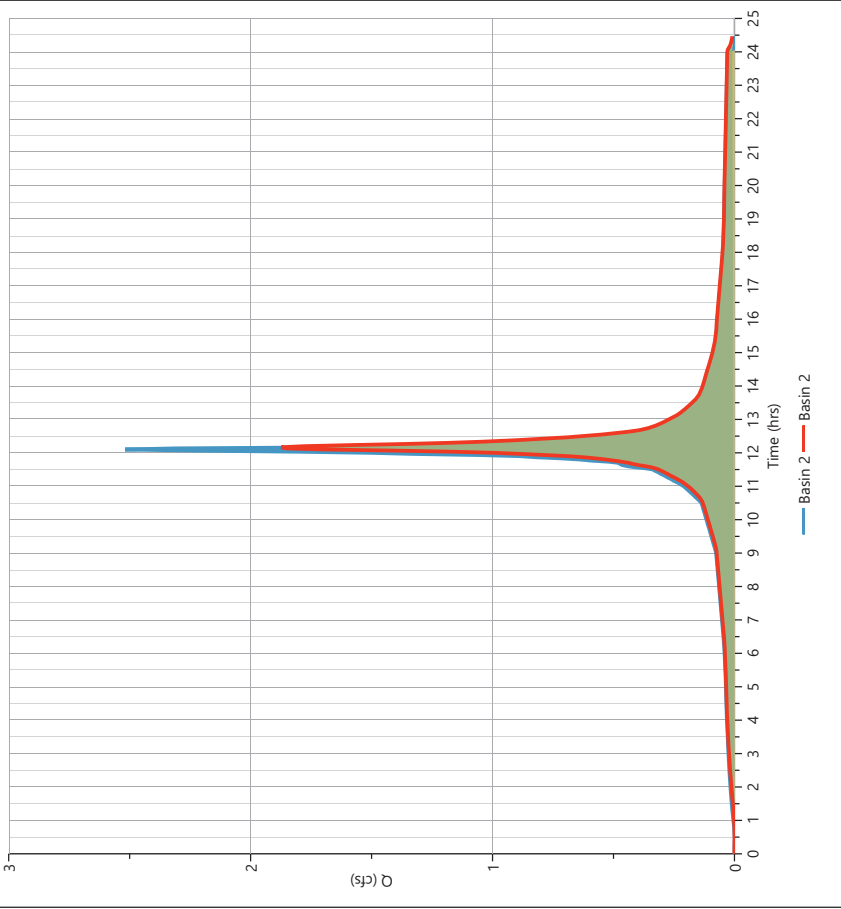
Post Basin 2

Hyd. No. 16

Hydrograph Type	= Pond Route	Peak Flow	= 1.873 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 8.877 cuft
Inflow Hydrograph	= 15 - Basin 2	Max. Elevation	= 14.55 ft
Pond Name	= Basin 2	Max. Storage	= 774 cuft

Pond Routing by Storage Indication Method

Qp = 1.87 cfs



Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

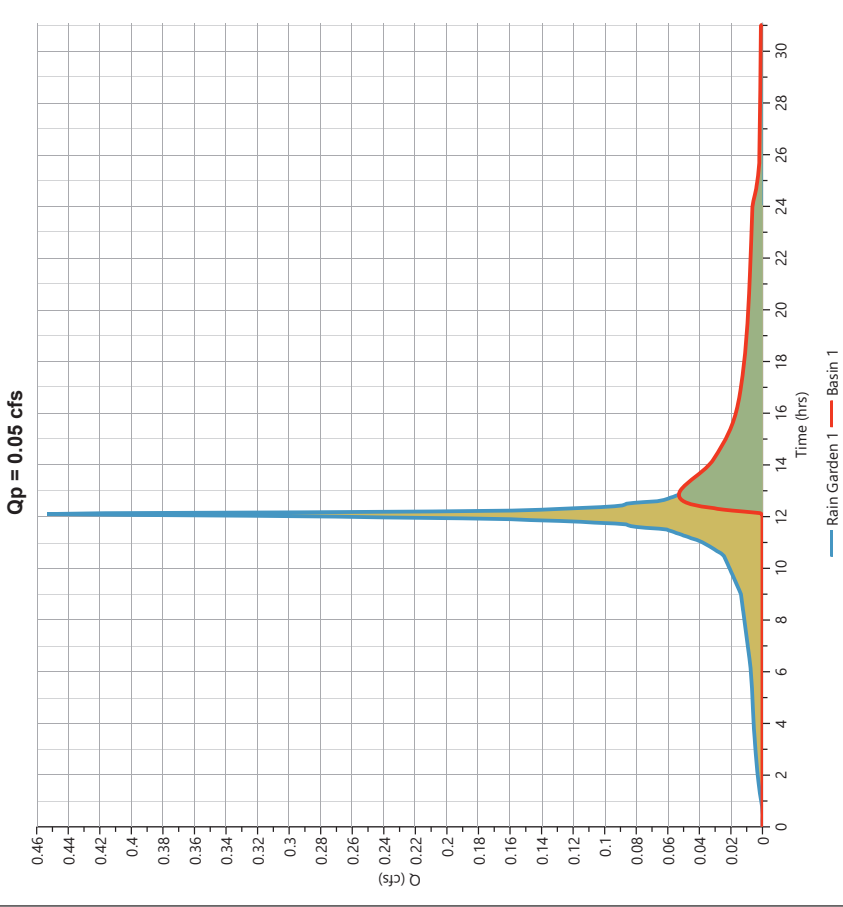
Post Basin 1

Hyd. No. 17

Hydrograph Type	= Pond Route	Peak Flow	= 0.053 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.83 hrs
Time Interval	= 2 min	Hydrograph Volume	= 792 cuft
Inflow Hydrograph	= 14 - Rain Garden 1	Max. Elevation	= 14.95 ft
Pond Name	= BASIN 1	Max. Storage	= 1,025 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 4.06 hrs



Hydrograph Report

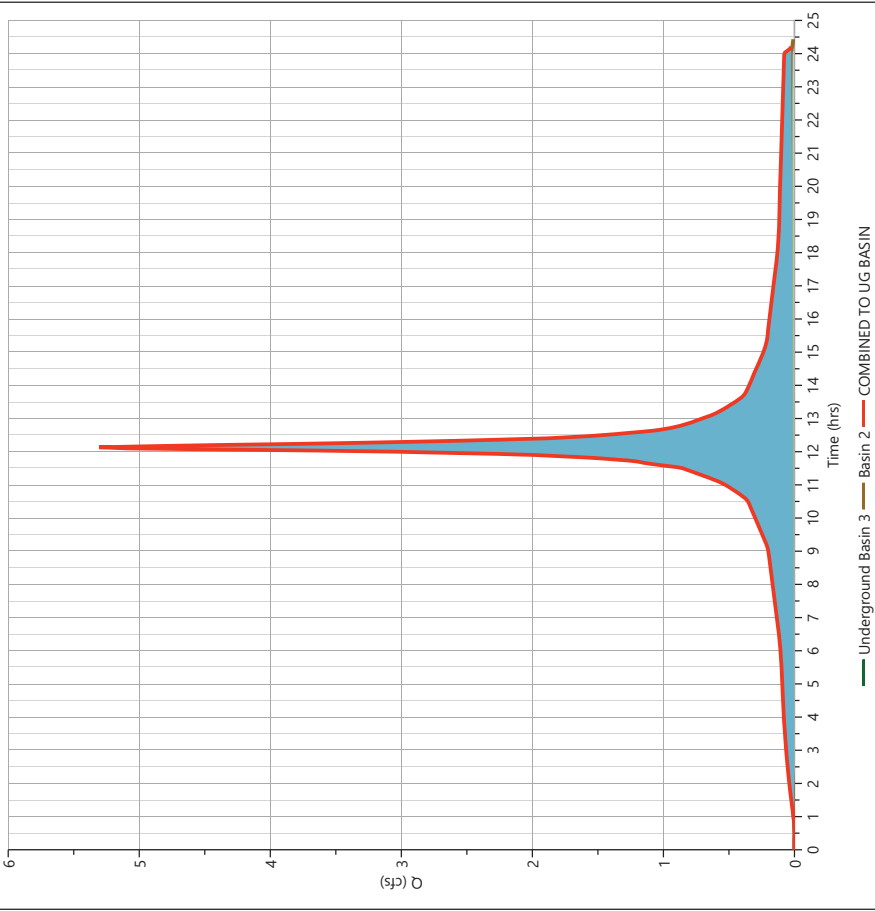
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO UG BASIN

Hyd. No. 18

Hydrograph Type	= Junction	Peak Flow	= 5.307 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.13 hrs
Time Interval	= 2 min	Hydrograph Volume	= 23,707 cuft
Inflow Hydrographs	= 13, 16	Total Contrib. Area	= 0.9 ac

Qp = 5.31 cfs



Hydrograph Report

Post UG Basin 3

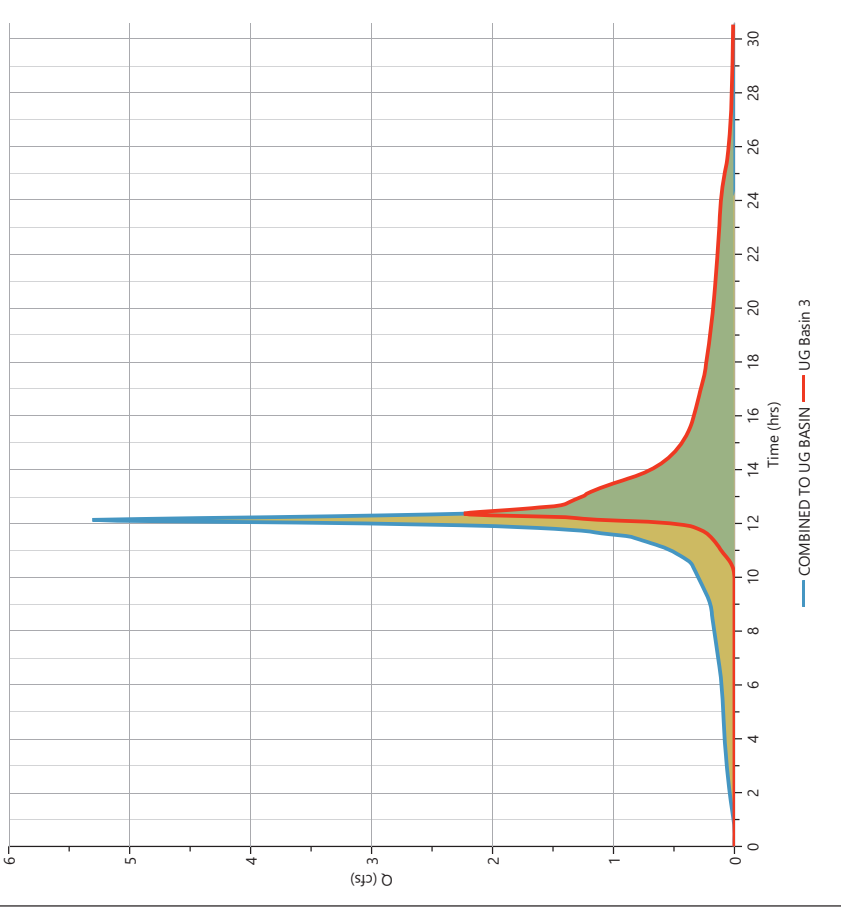
Hyd. No. 19

Hydrograph Type	= Pond Route	Peak Flow	= 2,237 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.37 hrs
Time Interval	= 2 min	Hydrograph Volume	= 19,919 cuft
Inflow Hydrograph	= 18 - COMBINED TO UG BASIN	Max. Elevation	= 14.26 ft
Pond Name	= UG BASIN	Max. Storage	= 11,513 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 2.94 hrs

Qp = 2.24 cfs



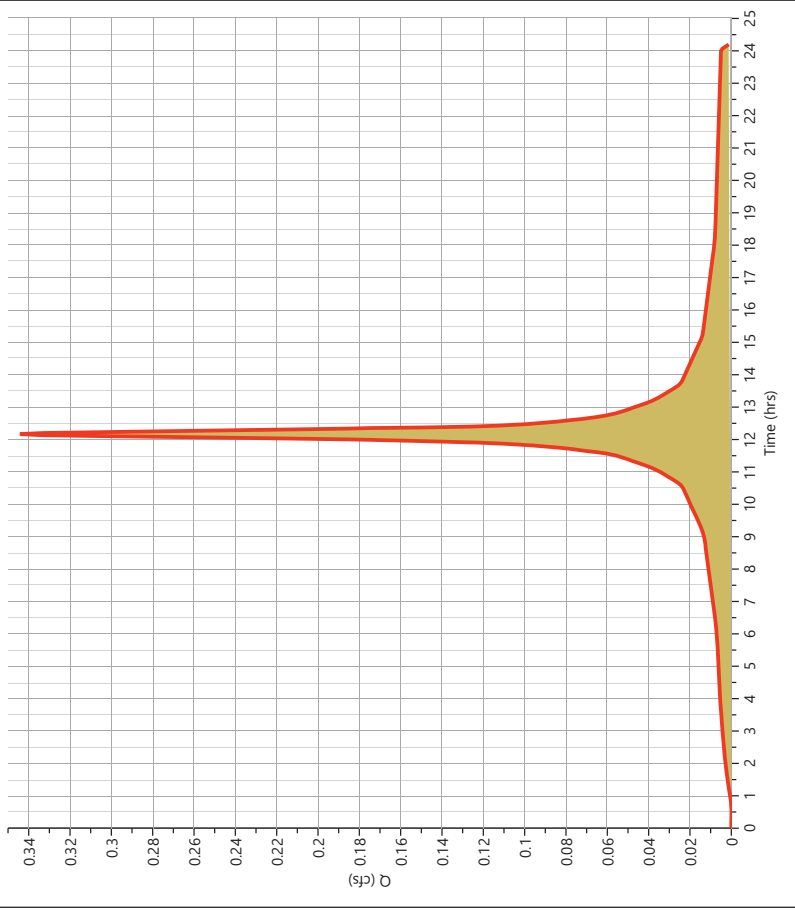
Hydrograph Report

Pre DA 2 - IMPERVIOUS

Hyd. No. 20

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Runoff Volume	= 1,552 cuft
Drainage Area	= 0.08 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.34 cfs



Hydrograph Report

Hydrology Studio v 3.0.0.26

Pre DA 2 - PERVIOUS

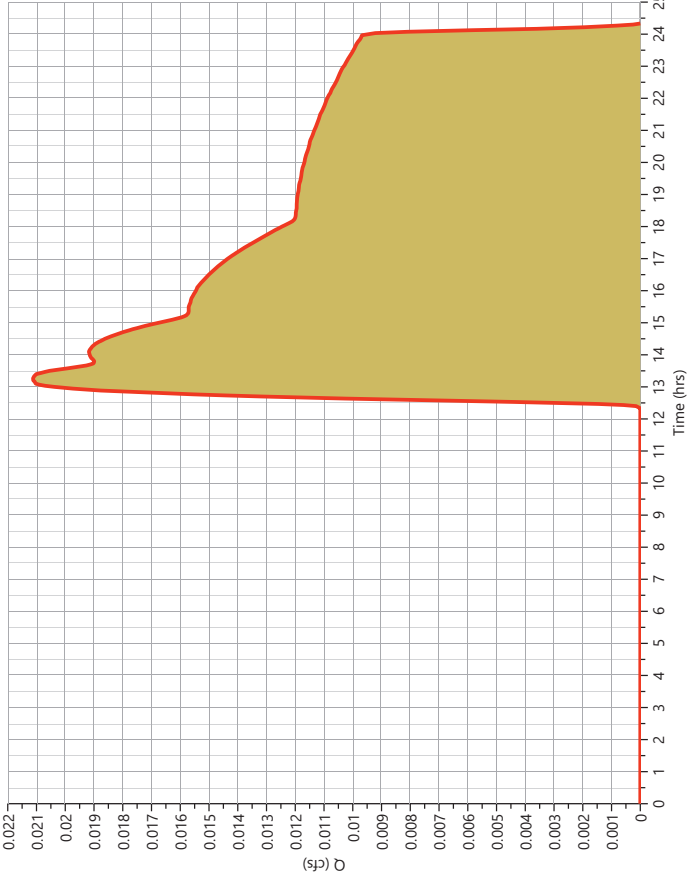
Hyd. No. 21

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.021 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.30 hrs
Time Interval	= 2 min	Runoff Volume	= 570 cuft
Drainage Area	= 0.86 ac	Curve Number	= 36*
Tc Method	= User	Time of Conc. (Tc)	= 11.1 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

* Composite CN Worksheet

AREA(ac)	CN	DESCRIPTION
0.58	39	OPEN SPACE
0.28	30	WOODS
0.86	36	Weighted CN Method Employed

Qp = 0.02 cfs



Hydrograph Report

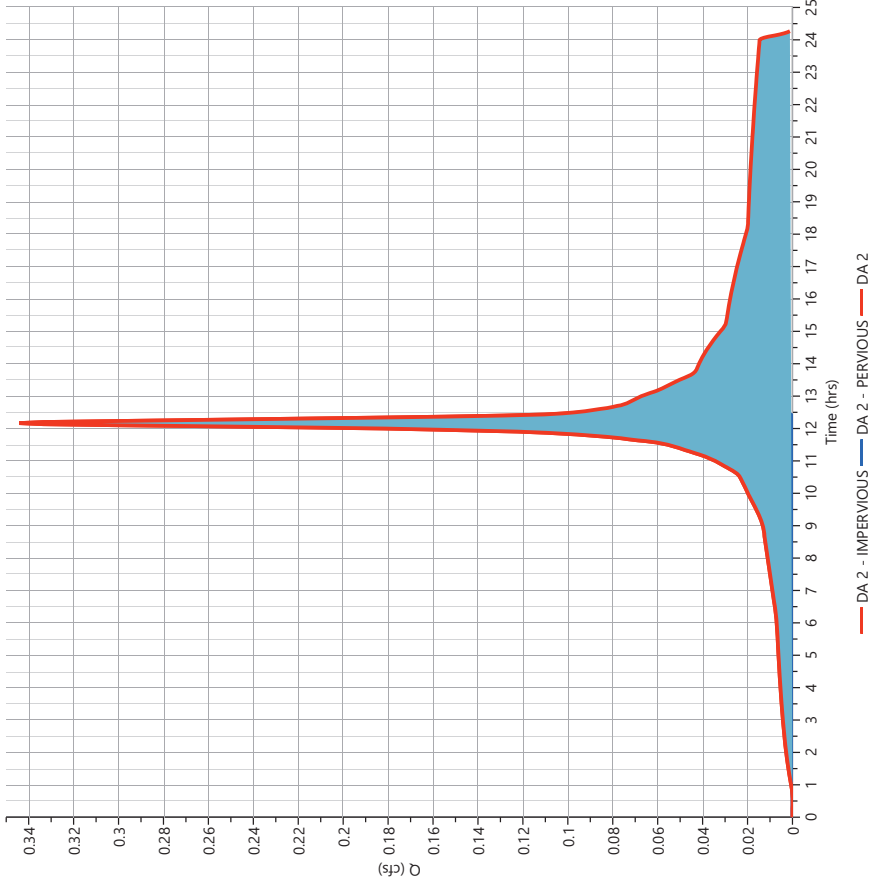
Hydrology Studio v 3.0.0.26

Pre DA 2

Hyd. No. 22

Hydrograph Type	= Junction	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,122 cuft
Inflow Hydrographs	= 20, 21	Total Contrib. Area	= 0.94 ac

Qp = 0.34 cfs



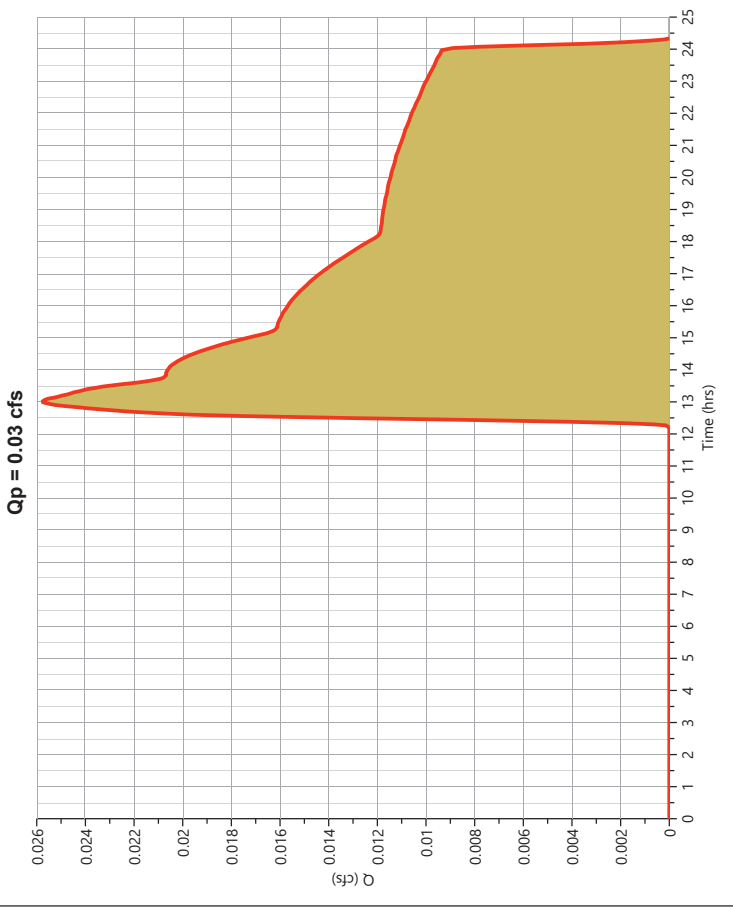
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre DA 3 - PERVIOUS

Hyd. No. 23

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.026 cfs
Storm Frequency	= 10-yr	Time to Peak	= 13.00 hrs
Time Interval	= 2 min	Runoff Volume	= 598 cuft
Drainage Area	= 0.75 ac	Curve Number	= 37*
Tc Method	= User	Time of Conc. (Tc)	= 10.3 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484
* Composite CN Worksheet			
AREA (ac)	CN	DESCRIPTION	
0.62	38	OPEN SPACE	
0.13	30	WOODS	
0.75	37	Weighted CN Method Employed	



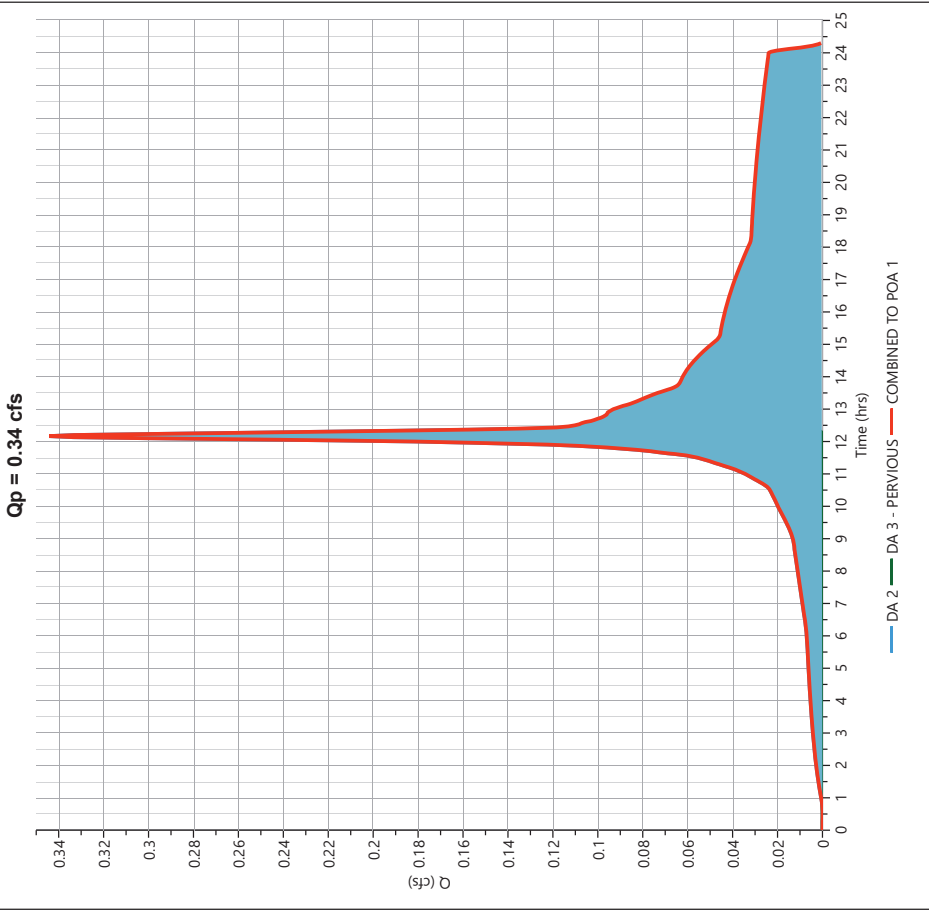
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Pre COMBINED TO POA 1

Hyd. No. 24

Hydrograph Type	= Junction	Peak Flow	= 0.344 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.17 hrs
Time Interval	= 2 min	Hydrograph Volume	= 2,720 cuft
Inflow Hydrographs	= 22, 23	Total Contrib. Area	= 1.69 ac



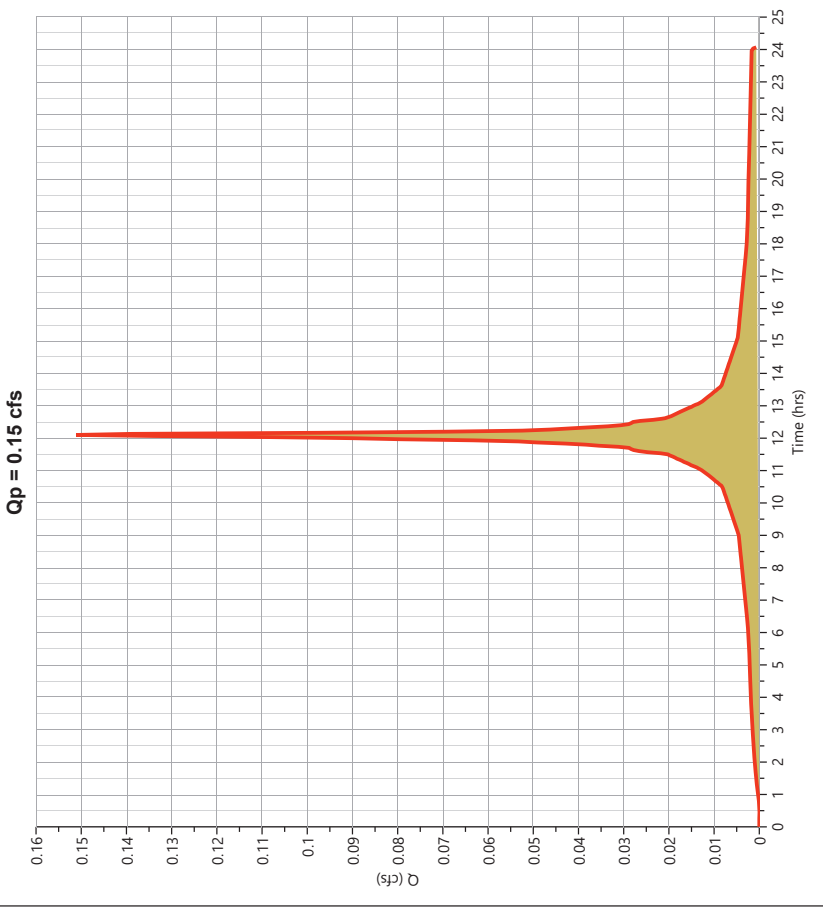
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post IMP BYPASS 9th Ave

Hyd. No. 25

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 529 cuft
Drainage Area	= 0.03 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



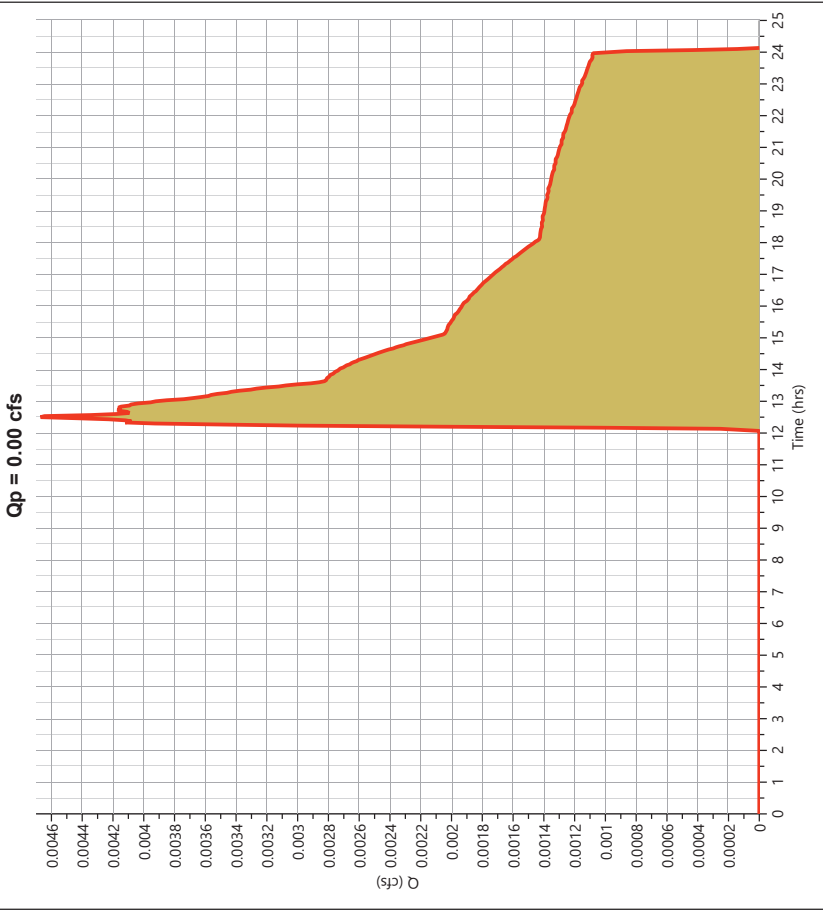
Hydrograph Report

Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post PERV BYPASS 9th Ave

Hyd. No. 26

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.005 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 79.7 cuft
Drainage Area	= 0.08 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

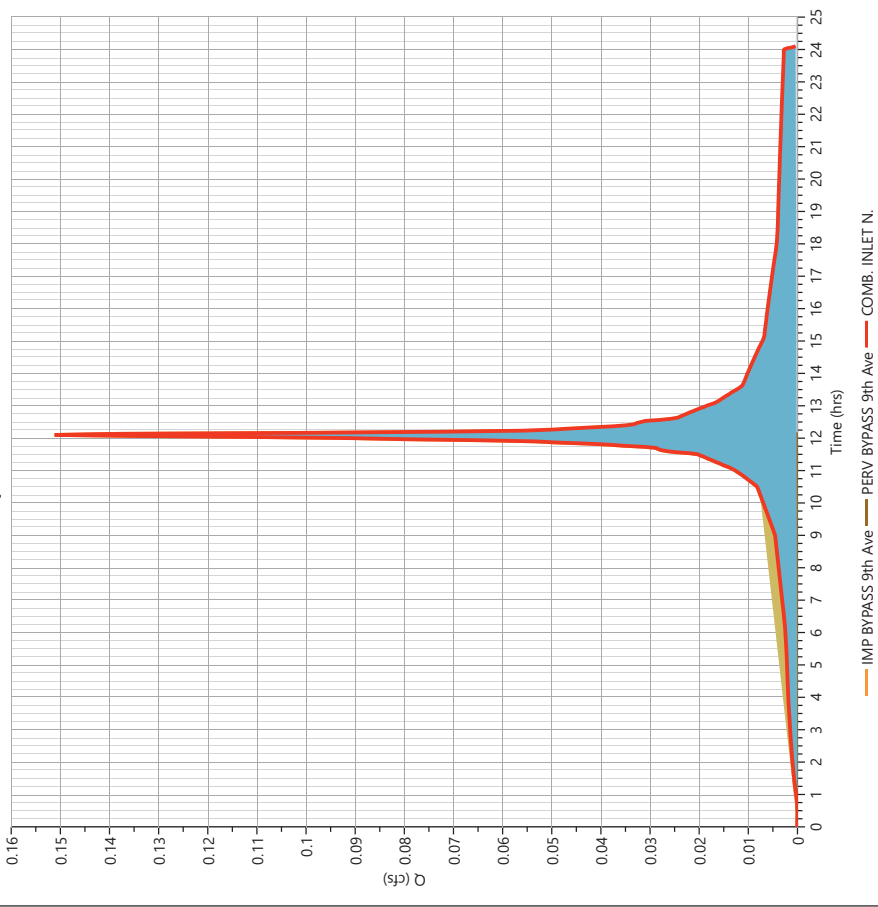
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

COMB. INLET N.

Hyd. No. 27

Hydrograph Type	= Junction	Peak Flow	= 0.151 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 609 cuft
Inflow Hydrographs	= 25, 26	Total Contrib. Area	= 0.11 ac

Qp = 0.15 cfs



Hydrograph Report

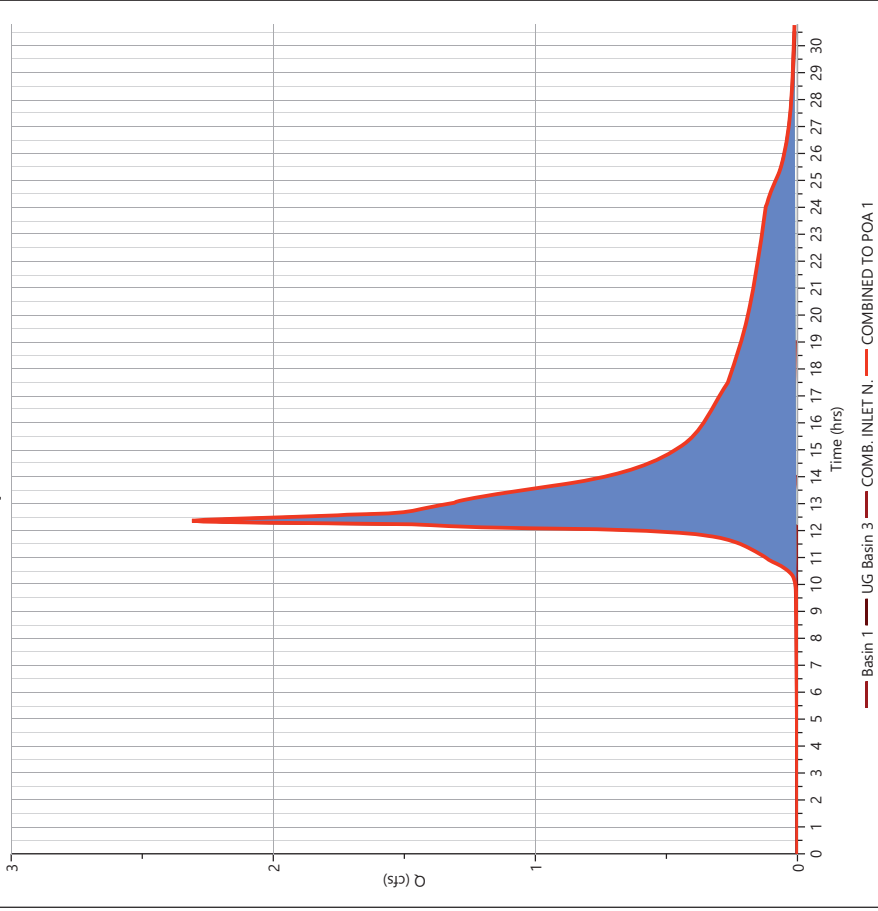
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post COMBINED TO POA 1

Hyd. No. 28

Hydrograph Type	= Junction	Peak Flow	= 2.310 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.37 hrs
Time Interval	= 2 min	Hydrograph Volume	= 21,319 cuft
Inflow Hydrographs	= 17, 19, 27	Total Contrib. Area	= 0.11 ac

Qp = 2.31 cfs

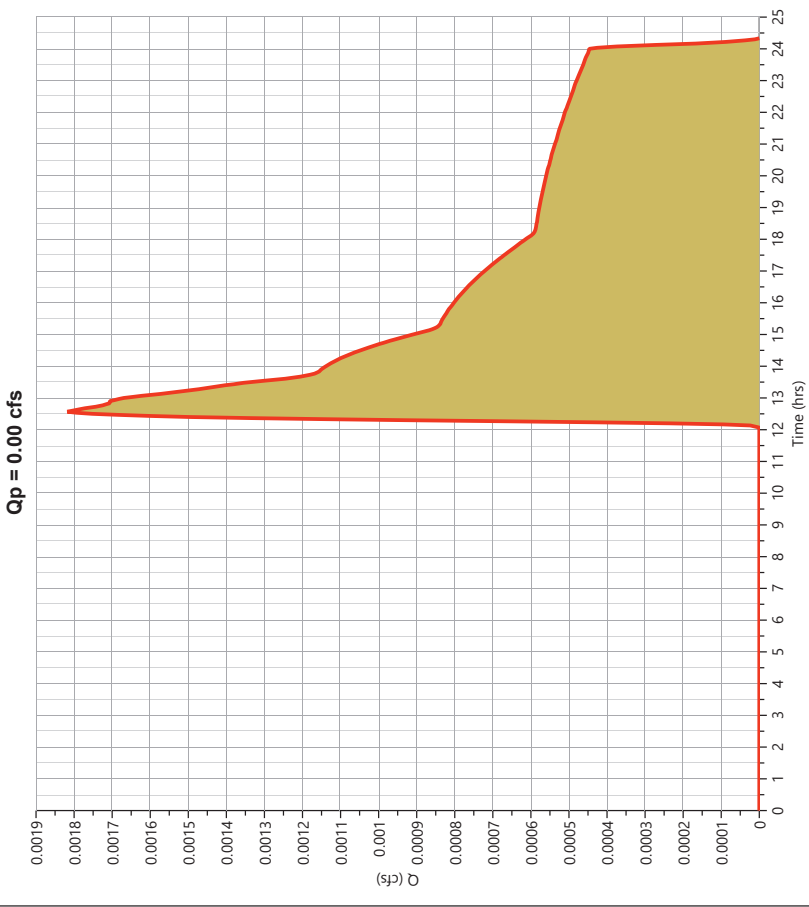


Hydrograph Report

Pre Bypass Memorial

Hyd. No. 29

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.002 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 32.9 cuft
Drainage Area	= 0.03 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

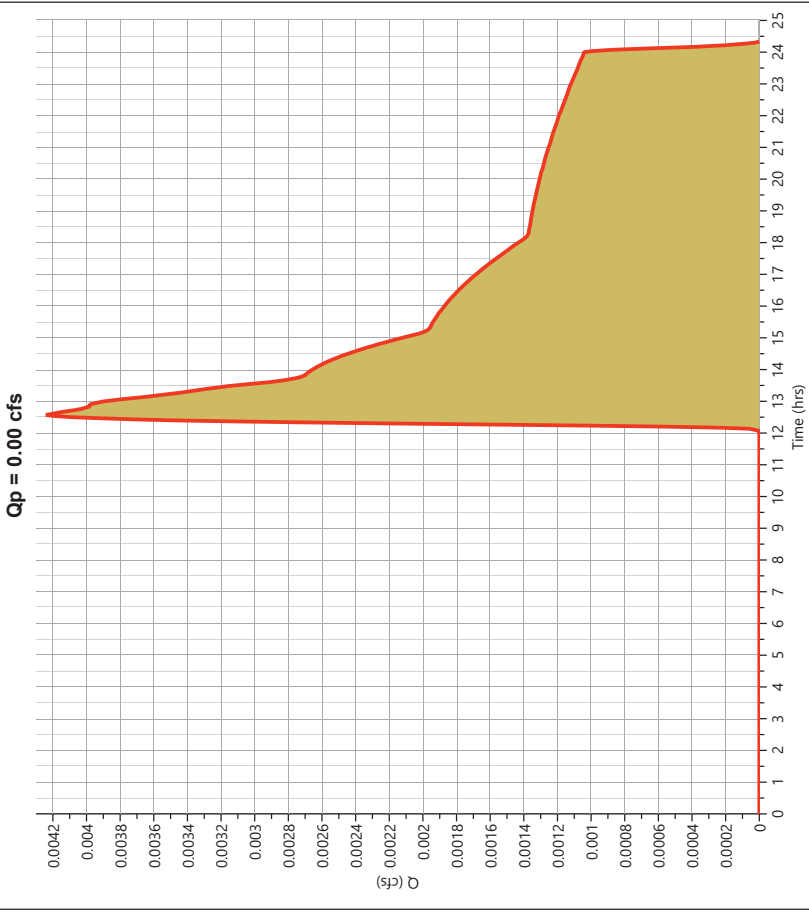


Hydrograph Report

Post Bypass Memorial

Hyd. No. 30

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.004 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.60 hrs
Time Interval	= 2 min	Runoff Volume	= 76.7 cuft
Drainage Area	= 0.07 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 12.9 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484



Hydrograph Report

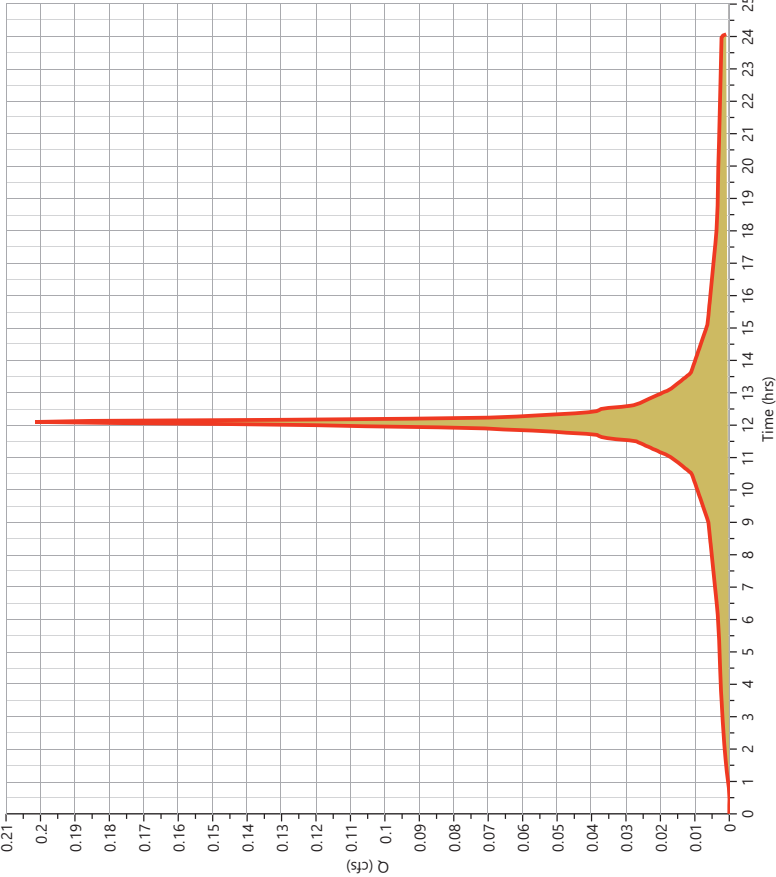
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Imp Bypass 8th Ave

Hyd. No. 31

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.202 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Runoff Volume	= 705 cuft
Drainage Area	= 0.04 ac	Curve Number	= 98
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.20 cfs



Hydrograph Report

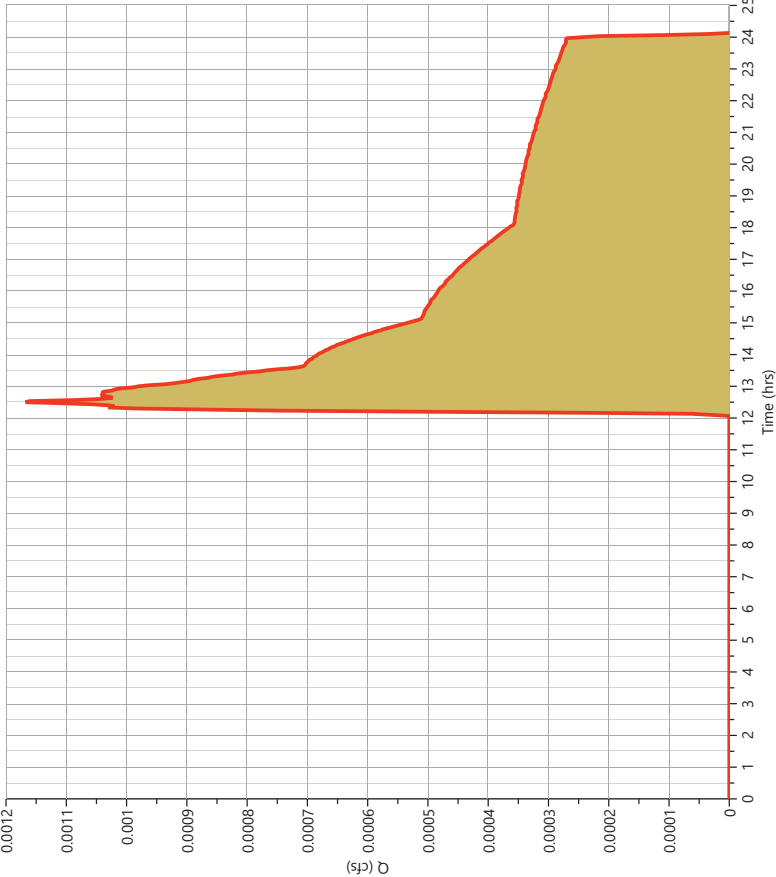
Project Name: 11-22-2022
Hydrology Studio v 3.0.0.26

Post Perv Bypass 8th Ave

Hyd. No. 32

Hydrograph Type	= NRCS Runoff	Peak Flow	= 0.001 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.50 hrs
Time Interval	= 2 min	Runoff Volume	= 19.9 cuft
Drainage Area	= 0.02 ac	Curve Number	= 39
Tc Method	= User	Time of Conc. (Tc)	= 6.0 min
Total Rainfall	= 5.42 in	Design Storm	= NOAA-D
Storm Duration	= 24 hrs	Shape Factor	= 484

Qp = 0.00 cfs



Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.26

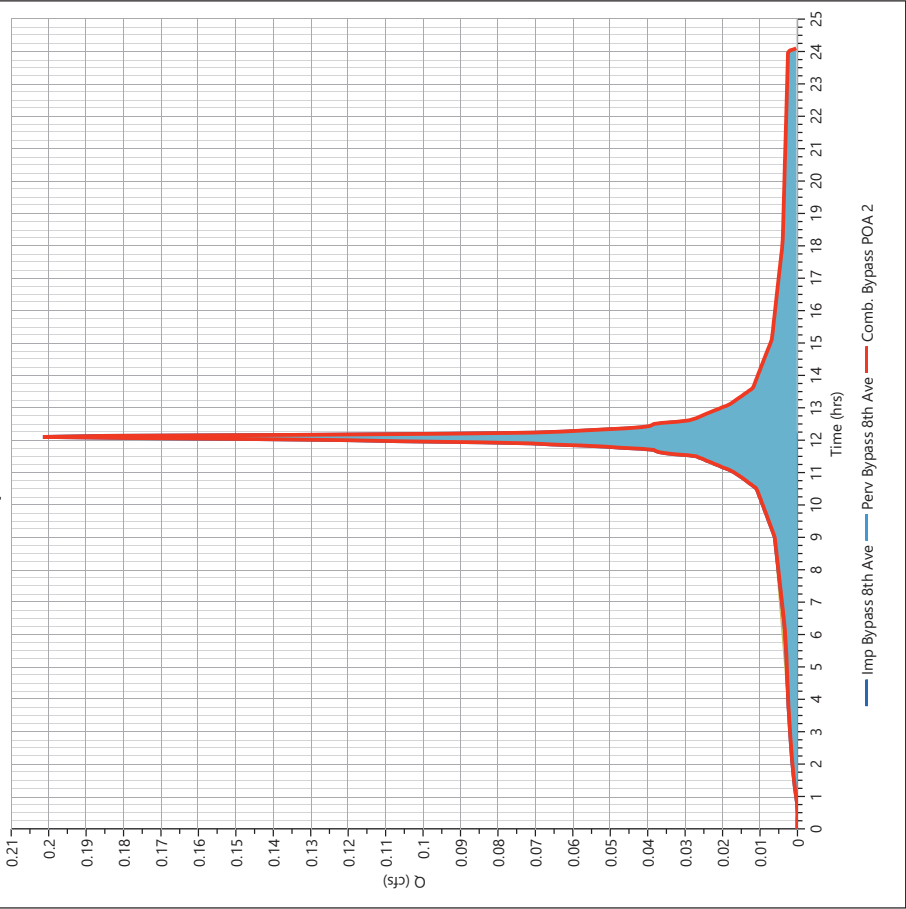
11-22-2022

Post Comb. Bypass POA 2

Hyd. No. 33

Hydrograph Type	= Junction	Peak Flow	= 0.202 cfs
Storm Frequency	= 10-yr	Time to Peak	= 12.10 hrs
Time Interval	= 2 min	Hydrograph Volume	= 725 cuft
Inflow Hydrographs	= 31, 32	Total Contrib. Area	= 0.06 ac

Qp = 0.20 cfs



STORMWATER COLLECTION CALCULATION (PIPE SIZING)



Stormwater Collection System Calculations

Project: SURFSIDE CROSSING

Computed By: KS

Job #: 2241-99-002

Checked By: SRC

Location: MEMORIAL DRIVE & 8TH AVE

Date: 6/14/2022

Design Storm: 25

Revised: 11/22/2022

NOTES:

1) Design method used is Rational Method

2) Refer to Weighted Runoff Coefficient table for calculation of incremental areas and C values

PIPE SECTION		SUBCATCH MENT AREA	INCREMENTAL		CUMULATIVE	TIME OF CONCENTRATION			I	PEAK RUNOFF		PIPING INPUT			PIPING DATA			
FROM	TO	Area (Acres)	"C"	A x C Ac	A x C (acres)	Tc to Inlet (min)	Tc in Pipe (min.)	Final Tc (min)	(In/Hr)	Q to Inlet (CFS)	Q cum. for Pipe (CFS)	Dia. (In)	Length (Ft)	Man. "n"	Slope (ft/ft)	Pipe Capacity (cfs)	Full Pipe Velocity (fps)	Actual Pipe Velocity (fps)
OCS 101	SWMH 102	0.01	0.77	0.01	0.01	10.00	0.05	10.00	6.80	0.07	0.07	12	15.0	0.011	0.0105	4.31	5.49	0.78
OCS 201	SWMH 202	0.39	0.79	0.31	0.31	10.00	0.11	10.00	6.80	2.11	2.11	12	30.0	0.011	0.0075	3.64	4.64	4.93
OCS 301	SWMH 202	0.02	0.70	0.01	0.01	10.00	0.14	10.00	6.80	0.07	0.07	12	77.0	0.011	0.0286	7.12	9.07	0.81
SWMH 202	EX 203	0.00	0.95	0.00	0.32	10.00	0.24	10.14	6.80	0.00	2.18	12	42.0	0.011	0.0030	2.31	2.94	3.25

INFILTRATION BASIN DRAIN TIME



DYNAMIC ENGINEERING

Infiltration Basin Drain time Calculations

Project:	Surfside Crossing	Computed By:	SMM
Job #:	2241-99-002	Checked By:	SRC
Location:	Memorial Drive & 8th Ave, Neptune, NJ	Date:	6/14/2022
		Revised:	11/22/2022
Basin Name:	Basin 1		

Volume of Runoff to be Infiltrated =	905	cubic feet
Surface Area of Infiltration Area =	180	square feet
Effective Depth of Runoff to be Infiltrated = Volume of Runoff to be Infiltrated / Surface Area of Infiltration Area =	1.77	feet = 21.24 inches
Field Tested Recharge Rate =	6.5	inches per hour
Design Recharge Rate * =	3.25	inches per hour

Effective Depth of Runoff to be Infiltrated / Design Recharge Rate = Time to Empty Basin = 6.54 hours**

**Note : Factor of Safety of 2 is applied to the Field Tested Recharge Rate to establish the Design Recharge Rate*

***Note : Time to Empty Basin must be less than 72 hours. Therefore;*

Drain time is less than 72 Hours, therefore, design is acceptable



**DYNAMIC
ENGINEERING**

Infiltration Basin Drain time Calculations

Project:	Surfside Crossing	Computed By:	SMM
Job #:	2241-99-002	Checked By:	SRC
Location:	Memorial Drive & 8th Ave, Neptune, NJ	Date:	6/14/2022
		Revised:	11/22/2022
Basin Name:	Basin 2		

	Volume of Runoff to be Infiltrated =	382	cubic feet
	Surface Area of Infiltration Area =	443	square feet
Effective Depth of Runoff to be Infiltrated = Volume of Runoff to be Infiltrated /	Surface Area of Infiltration Area =	0.67	feet = 8.04 inches
	Field Tested Recharge Rate =	14.4	inches per hour
	Design Recharge Rate * =	7.2	inches per hour

Effective Depth of Runoff to be Infiltrated / Design Recharge Rate = Time to Empty Basin = 1.12 hours**

**Note : Factor of Safety of 2 is applied to the Field Tested Recharge Rate to establish the Design Recharge Rate*

***Note : Time to Empty Basin must be less than 72 hours. Therefore;*

Drain time is less than 72 Hours, therefore, design is acceptable



DYNAMIC ENGINEERING

Infiltration Basin Drain time Calculations

Project:	Surfside Crossing	Computed By:	SMM
Job #:	2241-99-002	Checked By:	SRC
Location:	Memorial Drive & 8th Ave, Neptune, NJ	Date:	6/14/2022
		Revised:	11/22/2022
Basin Name:	Basin 3		

Volume of Runoff to be Infiltrated = 4,534 cubic feet
Surface Area of Infiltration Area = 10,205 square feet
Effective Depth of Runoff to be Infiltrated = Volume of Runoff to be Infiltrated / Surface Area of Infiltration Area = 0.95 feet = 11.40 inches
Field Tested Recharge Rate = 7.5 inches per hour
Design Recharge Rate * = 3.75 inches per hour

Effective Depth of Runoff to be Infiltrated / Design Recharge Rate = Time to Empty Basin = 3.04 hours**

*Note : Factor of Safety of 2 is applied to the Field Tested Recharge Rate to establish the Design Recharge Rate

**Note : Time to Empty Basin must be less than 72 hours. Therefore;

Drain time is less than 72 Hours, therefore, design is acceptable

RUNOFF RATE REDUCTIONS



Runoff Rate Reduction Performance

Project: Surfside Crossing
 Job #: 2241-99-002
 Location: Neptune, NJ

Computed By: SMM
 Checked By: SRC
 Date: 6/10/2022
 Last Revised: 11/22/2022

FLOW TO POA 1

Design Storm	Existing Runoff Rate from Disturbed Area (CFS)	Hydrograph #	Runoff Rate Required Reduction	Maximum Total Allowable Runoff Rate (CFS)	Proposed Runoff Rate (CFS)	Hydrograph #
2 Year	0.22	24	50%	0.11	0.10	28
10 Year	0.34	24	25%	0.26	0.15	28
25 Year	0.59	24	0%	0.59	0.45	28
100 Year	2.19	24	20%	1.75	1.56	28

FLOW TO POA 2

Design Storm	Existing Runoff Rate from Disturbed Area	Hydrograph #	Runoff Rate Required	Maximum Total Allowable Runoff	Proposed Runoff Rate (CFS)	Hydrograph #
2 Year	0.12	10	50%	0.06	0.129*	33
10 Year	0.19	10	25%	0.14	0.202*	33
25 Year	0.24	10	0%	0.24	0.258*	33
100 Year	0.38	10	20%	0.30	0.378*	33

FLOW TO POA 3

Design Storm	Existing Runoff Rate from Disturbed Area	Hydrograph #	Runoff Rate Required	Maximum Total Allowable Runoff	Proposed Runoff Rate (CFS)	Hydrograph #
2 Year	0.00	29	50%	0.00	0.00	30
10 Year	0.00	29	25%	0.00	0.00	30
25 Year	0.01	29	0%	0.01	0.021*	30
100 Year	0.04	29	20%	0.03	0.095*	30

*** Denotes a de minimis increase in CFS

NJGRS SPREADSHEETS

Annual Groundwater Recharge Analysis (based on GSR-32)

Select Township ↓	Average Annual P (in)	Climatic Factor
MONMOUTH CO., NEPTUNE TWP	47.4	1.55

Project Name: Surfside Crossing

Description: 70 Multi-Family Unit Dwelling

Analysis Date: 11/22/22

Pre-Developed Conditions

Land Segment	Area (acres)	TR-55 Land Cover	Soil	Annual Recharge (in)	Annual Recharge (cu.ft)
1	0.43	Woods	Fort Mott	17.1	26,693
2	0.12	Impervious areas	Fort Mott	0.0	-
3	1.26	Open space	Fort Mott	17.5	79,948
4	0				
5	0				
6	0				
7	0				
8	0				
9	0				
10	0				
11	0				
12	0				
13	0				
14	0				
15	0				
Total =	1.8			Total Annual Recharge (in)	Total Annual Recharge (cu-ft)
				16.2	106,641

Post-Developed Conditions

Land Segment	Area (acres)	TR-55 Land Cover	Soil	Annual Recharge (in)	Annual Recharge (cu.ft)
1	1.43	Impervious areas	Fort Mott	0.0	-
2	0.38	Open space	Fort Mott	17.5	24,111
3	0				
4	0				
5	0				
6	0				
7	0				
8	0				
9	0				
10	0				
11	0				
12	0				
13	0				
14	0				
15	0				
Total =	1.8			Total Annual Recharge (in)	Total Annual Recharge (cu.ft)
				3.7	24,111

Annual Recharge Requirements Calculation ↓

% of Pre-Developed Annual Recharge to Preserve =

100%

Post-Development Annual Recharge Deficit=

82,529

(cubic feet)

Recharge Efficiency Parameters Calculations (area averages)

RWC=	1.72	(in)	DRWC=	0.00	(in)
ERWC =	0.39	(in)	EDRWC=	0.00	(in)

Procedure to fill the Pre-Development and Post-Development Conditions Tables

For each land segment, first enter the area, then select TR-55 Land Cover, then select Soil. Start from the top of the table and proceed downward. Don't leave blank rows (with A=0) in between your segment entries. Rows with A=0 will not be displayed or used in calculations. For impervious areas outside of standard lots select "Impervious Areas" as the Land Cover. Soil type for impervious areas are only required if an infiltration facility will be built within these areas.

Project Name	Description	Analysis Date	BMP or LID Type
Surfside Crossing	70 Multi-Family Unit Dwelling	11/22/22	AG Basin 1

Recharge BMP Input Parameters				Root Zone Water capacity Calculated Parameters				Recharge Design Parameters			
Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit
BMP Area	ABMP	180.0	sq.ft	Empty Portion of RWC under Post-D Natural Recharge	ERWC	0.39	in	Inches of Runoff to capture	Qdesign	0.96	in
BMP Effective Depth, this is the design variable	dBMP	21.2	in	ERWC Modified to consider dEXC	EDRWC	0.16	in	Inches of Rainfall to capture	Pdesign	1.17	in
Upper level of the BMP surface (negative if above ground)	dBMPu	2.2	in	Empty Portion of RWC under Infiltr. BMP	RERWC	0.13	in	Recharge Provided Avg. over Imp. Area		30.9	in
Depth of lower surface of BMP, must be >= dBMPu	dEXC	23.4	in					Runoff Captured Avg. over imp. Area		31.4	in
Post-development Land Segment Location of BMP, Input Zero if Location is distributed or undetermined	SegBMP	0	unitless								

BMP Calculated Size Parameters				CALCULATION CHECK MESSAGES	
ABMP/Aimp	Aratio	0.05	unitless	Volume Balance-->	Solve Problem to satisfy Annual Recharge
BMP Volume	VBMP	318	cu.ft	dBMP Check-->	OK
				dEXC Check-->	OK
				BMP Location-->	Location is selected as distributed or undetermined

Parameters from Annual Recharge Worksheet				System Performance Calculated Parameters			
Post-D Deficit Recharge (or desired recharge volume)	Vdef	10,258	cu.ft	Annual BMP Recharge Volume		10,258	cu.ft
Post-D Impervious Area (or target Impervious Area)	Aimp	3,981	sq.ft	Avg BMP Recharge Efficiency		98.5%	Represents % Infiltration Recharged
Root Zone Water Capacity	RWC	1.72	in	%Rainfall became Runoff		78.2%	%
RWC Modified to consider dEXC	DRWC	0.73	in	%Runoff Infiltrated		84.6%	%
Climatic Factor	C-factor	1.55	no units	%Runoff Recharged		5.3%	%
Average Annual P	Pavg	47.4	in	%Rainfall Recharged		4.2%	%
Recharge Requirement over Imp. Area	dr	2.0	in				

OTHER NOTES

Pdesign is accurate only after BMP dimensions are updated to make rech volume= deficit volume. The portion of BMP infiltration prior to filling and the area occupied by BMP are ignored in these calculations. Results are sensitive to dBMP, make sure dBMP selected is small enough for BMP to empty in less than 3 days. For land Segment Location of BMP if you select "impervious areas" RWC will be minimal but not zero as determined by the soil type and a shallow root zone for this Land Cover allowing consideration of lateral flow and other losses.

How to solve for different recharge volumes: By default the spreadsheet assigns the values of total deficit recharge volume "Vdef" and total proposed impervious area "Aimp" from the "Annual Recharge" sheet to "Vdef" and "Aimp" on this page. This allows solution for a single BMP to handle the entire recharge requirement assuming the runoff from entire impervious area is available to the BMP. To solve for a smaller BMP or a LID-IMP to recharge only part of the recharge requirement, set Vdef to your target value and Aimp to impervious area directly connected to your infiltration facility and then solve for ABMP or dBMP. To go back to the default configuration click the "Default Vdef & Aimp" button.

Project Name	Description	Analysis Date	BMP or LID Type
Surfside Crossing	70 Multi-Family Unit Dwelling	11/22/22	AG Basin 2

Recharge BMP Input Parameters				Root Zone Water capacity Calculated Parameters				Recharge Design Parameters			
Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit
BMP Area	ABMP	382.0	sq.ft	Empty Portion of RWC under Post-D Natural Recharge	ERWC	0.39	in	Inches of Runoff to capture	Qdesign	0.15	in
BMP Effective Depth, this is the design variable	dBMP	8.0	in	ERWC Modified to consider dEXC	EDRWC	0.05	in	Inches of Rainfall to capture	Pdesign	0.21	in
Upper level of the BMP surface (negative if above ground)	dBMPu	15.4	in	Empty Portion of RWC under Infiltr. BMP	RERWC	0.04	in	Recharge Provided Avg. over Imp. Area		10.0	in
Depth of lower surface of BMP, must be >= dBMPu	dEXC	23.4	in					Runoff Captured Avg. over imp. Area		10.0	in
Post-development Land Segment Location of BMP, Input Zero if Location is distributed or undetermined	SegBMP	0	unitless								

BMP Calculated Size Parameters				CALCULATION CHECK MESSAGES	
ABMP/Aimp	Aratio	0.02	unitless	Volume Balance-->	Solve Problem to satisfy Annual Recharge
BMP Volume	VBMP	255	cu.ft	dBMP Check-->	OK
				dEXC Check-->	OK
				BMP Location-->	Location is selected as distributed or undetermined

Parameters from Annual Recharge Worksheet				System Performance Calculated Parameters			
Post-D Deficit Recharge (or desired recharge volume)	Vdef	17,961	cu.ft	Annual BMP Recharge Volume		17,961	cu.ft
Post-D Impervious Area (or target Impervious Area)	Aimp	21,618	sq.ft	Avg BMP Recharge Efficiency		99.4%	Represents % Infiltration Recharged
Root Zone Water Capacity	RWC	1.72	in	%Rainfall became Runoff		78.2%	%
RWC Modified to consider dEXC	DRWC	0.22	in	%Runoff Infiltrated		27.0%	%
Climatic Factor	C-factor	1.55	no units	%Runoff Recharged		9.3%	%
Average Annual P	Pavg	47.4	in	%Rainfall Recharged		7.3%	%
Recharge Requirement over Imp. Area	dr	3.5	in				

OTHER NOTES

Pdesign is accurate only after BMP dimensions are updated to make rech volume= deficit volume. The portion of BMP infiltration prior to filling and the area occupied by BMP are ignored in these calculations. Results are sensitive to dBMP, make sure dBMP selected is small enough for BMP to empty in less than 3 days. For land Segment Location of BMP if you select "impervious areas" RWC will be minimal but not zero as determined by the soil type and a shallow root zone for this Land Cover allowing consideration of lateral flow and other losses.

How to solve for different recharge volumes: By default the spreadsheet assigns the values of total deficit recharge volume "Vdef" and total proposed impervious area "Aimp" from the "Annual Recharge" sheet to "Vdef" and "Aimp" on this page. This allows solution for a single BMP to handle the entire recharge requirement assuming the runoff from entire impervious area is available to the BMP. To solve for a smaller BMP or a LID-IMP to recharge only part of the recharge requirement, set Vdef to your target value and Aimp to impervious area directly connected to your infiltration facility and then solve for ABMP or dBMP. To go back to the default configuration click the "Default Vdef & Aimp" button.

Project Name	Description	Analysis Date	BMP or LID Type
Surfside Crossing	70 Multi-Family Unit Dwelling	11/22/22	UG Basin

Recharge BMP Input Parameters				Root Zone Water capacity Calculated Parameters				Recharge Design Parameters			
Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit	Parameter	Symbol	Value	Unit
BMP Area	ABMP	10205.0	sq.ft	Empty Portion of RWC under Post-D Natural Recharge	ERWC	0.39	in	Inches of Runoff to capture	Qdesign	2.94	in
BMP Effective Depth, this is the design variable	dBMP	11.4	in	ERWC Modified to consider dEXC	EDRWC	0.00	in	Inches of Rainfall to capture	Pdesign	3.17	in
Upper level of the BMP surface (negative if above ground)	dBMPu	16.6	in	Empty Portion of RWC under Infiltr. BMP	RERWC	0.00	in	Recharge Provided Avg. over Imp. Area		36.3	in
Depth of lower surface of BMP, must be >= dBMPu	dEXC	48.0	in					Runoff Captured Avg. over imp. Area		36.3	in
Post-development Land Segment Location of BMP, Input Zero if Location is distributed or undetermined	SegBMP	0	unitless								

BMP Calculated Size Parameters				CALCULATION CHECK MESSAGES	
ABMP/Aimp	Aratio	0.19	unitless	Volume Balance-->	Solve Problem to satisfy Annual Recharge
BMP Volume	VBMP	9,695	cu.ft	dBMP Check-->	OK
				dEXC Check-->	OK
				BMP Location-->	Location is selected as distributed or undetermined

Parameters from Annual Recharge Worksheet				System Performance Calculated Parameters			
Post-D Deficit Recharge (or desired recharge volume)	Vdef	82,529	cu.ft	Annual BMP Recharge Volume		162,345	cu.ft
Post-D Impervious Area (or target Impervious Area)	Aimp	53,663	sq.ft	Avg BMP Recharge Efficiency		100.0%	Represents % Infiltration Recharged
Root Zone Water Capacity	RWC	1.72	in	%Rainfall became Runoff		78.2%	%
RWC Modified to consider dEXC	DRWC	0.00	in	%Runoff Infiltrated		97.9%	%
Climatic Factor	C-factor	1.55	no units	%Runoff Recharged		84.4%	%
Average Annual P	Pavg	47.4	in	%Rainfall Recharged		66.0%	%
Recharge Requirement over Imp. Area	dr	15.9	in				

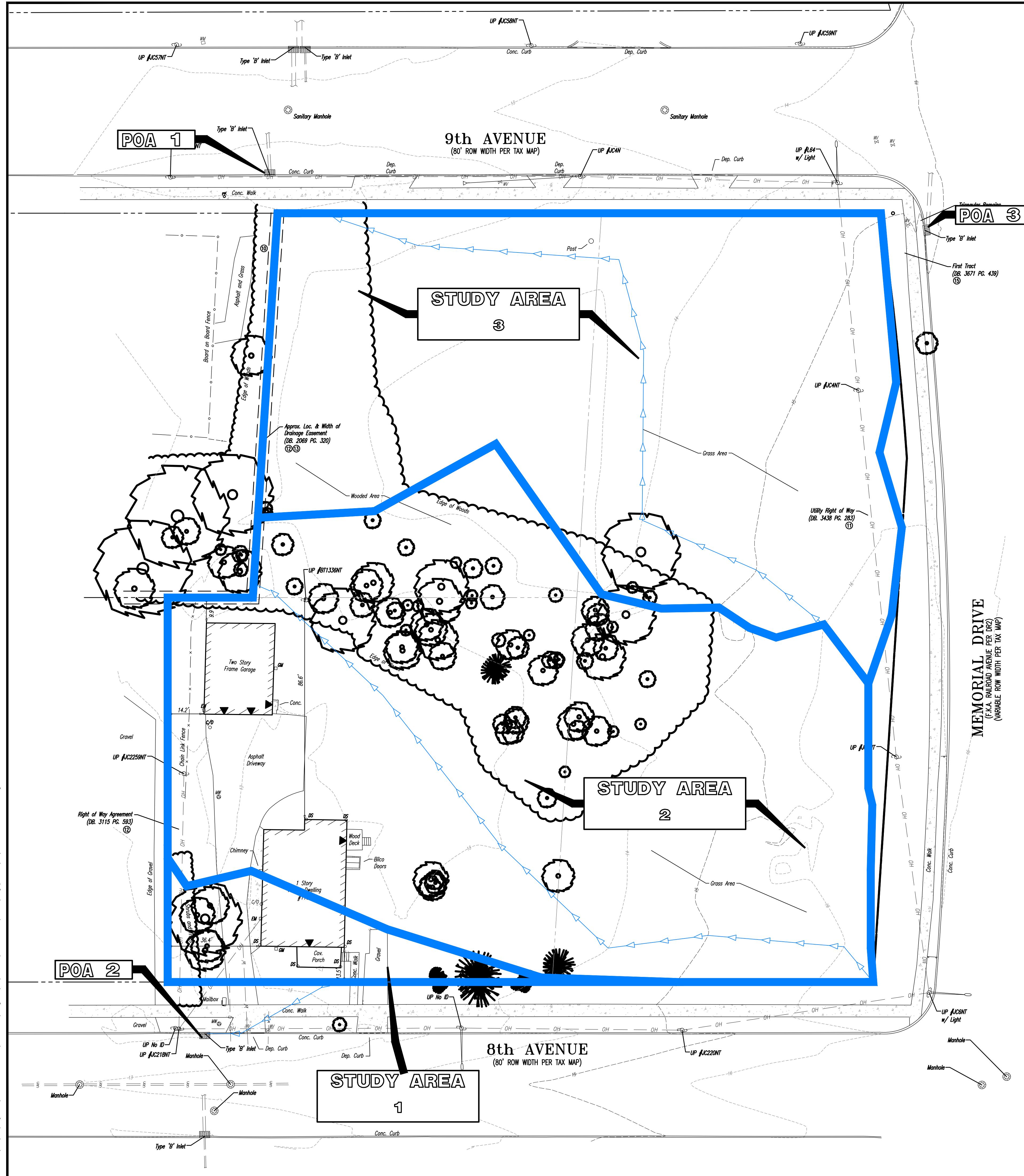
OTHER NOTES

Pdesign is accurate only after BMP dimensions are updated to make rech volume= deficit volume. The portion of BMP infiltration prior to filling and the area occupied by BMP are ignored in these calculations. Results are sensitive to dBMP, make sure dBMP selected is small enough for BMP to empty in less than 3 days. For land Segment Location of BMP if you select "impervious areas" RWC will be minimal but not zero as determined by the soil type and a shallow root zone for this Land Cover allowing consideration of lateral flow and other losses.

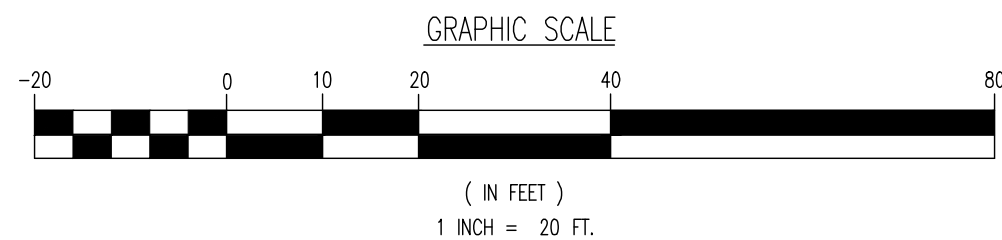
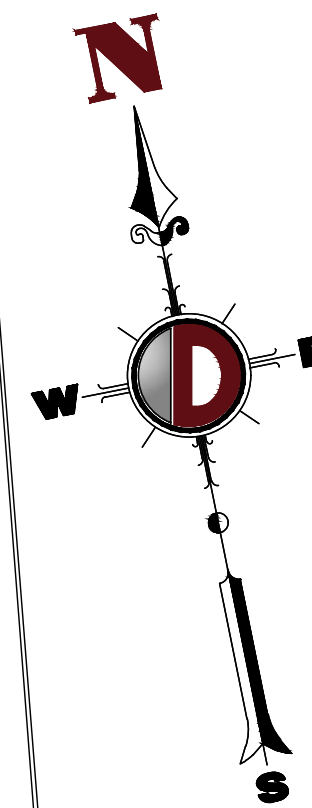
How to solve for different recharge volumes: By default the spreadsheet assigns the values of total deficit recharge volume "Vdef" and total proposed impervious area "Aimp" from the "Annual Recharge" sheet to "Vdef" and "Aimp" on this page. This allows solution for a single BMP to handle the entire recharge requirement assuming the runoff from entire impervious area is available to the BMP. To solve for a smaller BMP or a LID-IMP to recharge only part of the recharge requirement, set Vdef to your target value and Aimp to impervious area directly connected to your infiltration facility and then solve for ABMP or dBMP. To go back to the default configuration click the "Default Vdef & Aimp" button.

DRAINAGE AREA MAPS

Plotted: 11/22/22 - 4:17 PM, By: newtown
File: P:\deep projects\2241 newtown\112222\112222.dwg, Map: 01 EXISTING DRAINAGE AREA MAP



THIS PLAN TO BE UTILIZED FOR STORMWATER/DRAINAGE ANALYSIS PURPOSES ONLY



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REV.	DATE	COMMENTS
1	11/22/22	REV. PER TOWNSHIP COMMENTS

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DESIGNED BY: MR. SMM. CHECKED BY: SRC.

PROJECT: **SURFSIDE CROSSING**
PROPOSED MULTI-FAMILY DWELLINGS
BLOCK 405, LOTS 5, 6, & 7
1102 9TH AVENUE, 105.8 8TH AVENUE, & MEMORIAL DRIVE
NEPTUNE TOWNSHIP, MONMOUTH COUNTY, NEW JERSEY

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STEVEN R. CATTANI
PROFESSIONAL ENGINEER
NEW JERSEY LICENSE No. 40014

TITLE: **EXISTING DRAINAGE AREA MAP**

SCALE: (H) 1"=20'
(V) 1"=20'
PROJECT No: 2241-99-002

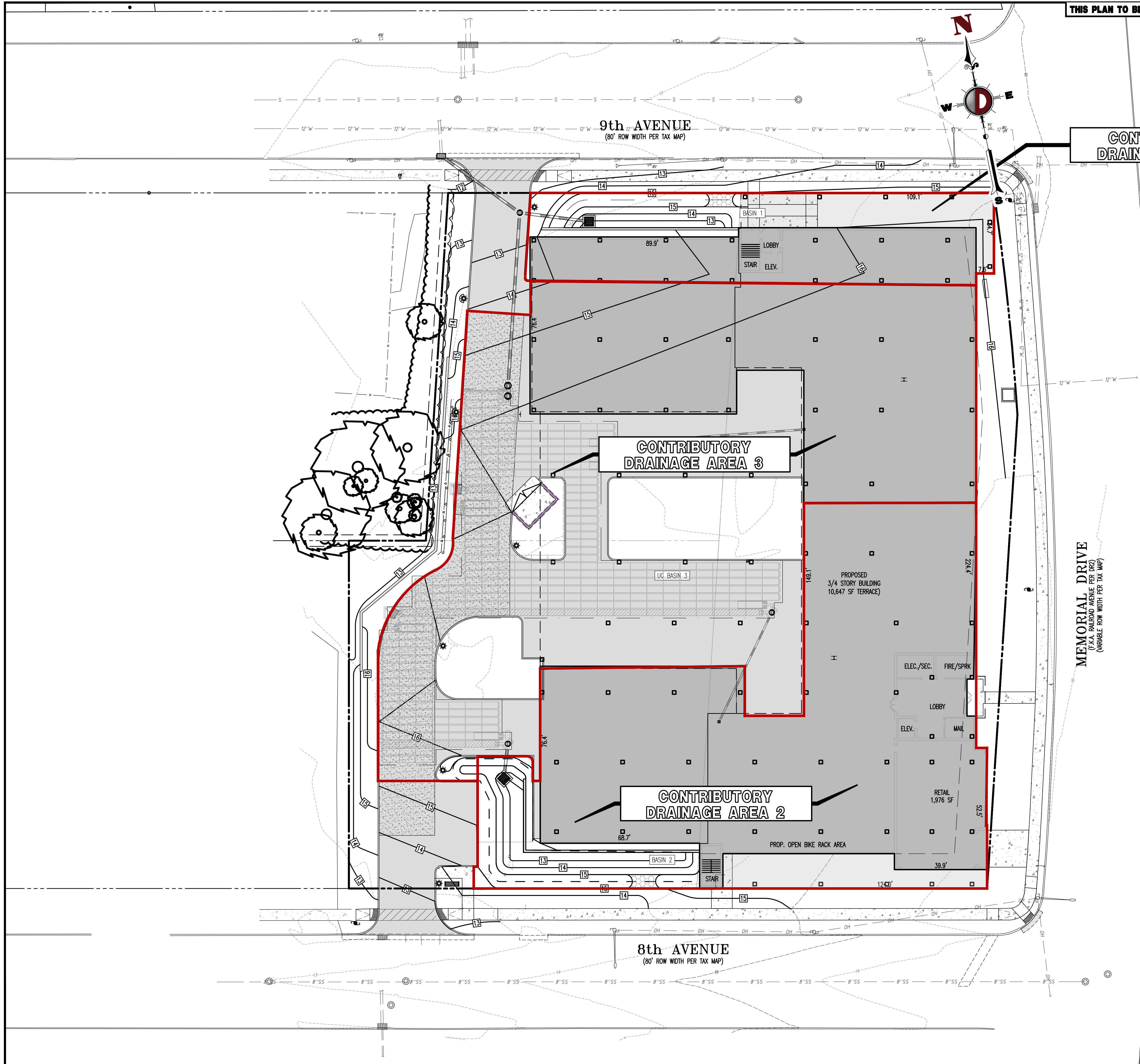
DATE: 06/13/22

SHEET No: **1**

Rev. #: 0

OF 26

Plotted: 11/22/22 - 4:18 PM. By: newtown
File: P:\deepc projects\2241 later developers llc springhill properties\99-002 neptune\dwg\da Maps\2241199002\00.dwg, ----> 03 INLET AREA MAP



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1	11/22/22	REV. PER TOWNSHIP COMMENTS	NUS

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DRAWN BY: MR DESIGNED BY: SMM CHECKED BY: SRC

PROJECT: **SURFSIDE CROSSING**
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CONTRIBUTORY AREA MAP

SCALE: (H) 1"=20'
(V) 1"=20'

PROJECT No: 2241-99-002

SHEET No: **3** OF 26

DATE: 06/13/22

Rev. #: