

# **STORMWATER MANAGEMENT REPORT**

PROPOSED COMMERCIAL DEVELOPMENT BLOCK 701, LOT I TOWNSHIP OF NEPTUNE MONMOUTH COUNTY, NEW JERSEY

**PREPARED FOR:** 

M & M NEPTUNE, LLC

**PREPARED BY:** 

STONEFIELD ENGINEERING & DESIGN, LLC DECEMBER 29, 2020 REVISED MAY 10, 2021 PRI-200142

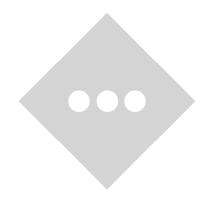
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### **I.0 PROJECT DESCRIPTION**

M&M at Neptune, LLC is proposing to redevelop Block 701 Lot I to accommodate with a supermarket, retail store, fast food restaurant and a convenience store/gas station. The improvements include a total of 36,042 SF of building cover. Additional improvements include off-street parking, lighting, landscaping, and stormwater management facilities.

The project site is bound to the north by Asbury Avenue (County Route 16), to the south by NJ State Highway Route 35, a connection road to the west, and the Hollow Brook along the eastern property line and is assumed to have a 50 ft riparian zone. The site contains two intermediate resource value wetland areas located in the easterly portion of the lot. Generally, the vicinity of the project is developed with residential to the north and east, and commercial to the south and west. The project site itself currently contains a vacant warehouse and parking lot and woods occupying the eastern portion of the site where the wetlands and riparian zone are located.

The project site is 13.577 acres, the extent of land disturbance is 6.529 acres (including areas within the public right-of-way), and 1.139 acres of new impervious surfaces will be created by the project.

This Stormwater Management Report has been prepared to analyze the potential stormwater runoff impacts of the proposed project and discuss the measures proposed to conform to the stormwater management requirements set forth by the Township of Neptune, Freehold Soil Conservation District, Monmouth County Planning Board and the New Jersey Department of Environmental Protection (NJDEP).

# 2.0 EXISTING CONDITIONS

The project site fronts on two roadways, to the north Asbury Avenue (County Route 16), to the south by NJ State Highway Route 35, and a connection road to the west. The project site historically has contained the existing vacant structure and parking lot. The existing development on site will be removed entirely as part of the proposed redevelopment. Aerial Maps depicting the site from 1979 as well as the existing site conditions can be found in **APPENDIX A**.

#### 2.1 EXISTING DRAINAGE AREAS

Under existing conditions, the site drains to one point of interest located within Hollow Brook where the Brook crosses Asbury Avenue.

For purposes of this report the onsite flow to the point of interest is broken up into two areas. The first area contains all constructed site improvements. The second area will be reforested in accordance with NJDEP specifications.

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
POI I (Ex. Areas IA and IB)	Drainage area to Hollow Brook culvert crossing Asbury Avenue (Construction)	289,872 SF	155,960 SF	20.1 Minutes
POI 2	Drainage area to Hollow Brook culvert crossing Asbury Avenue (Reforestation)	53,278 SF	0	6 Minutes*

#### TABLE I: EXISTING DRAINAGE AREAS

\*The minimum time of concentration was utilized as the time of concentration will not change in proposed conditions.

Detailed information regarding each drainage area can be found on the Existing Drainage Area Map in **Appendix E** of this Report.

#### 2.2 **PROJECT SOILS**

Per the National Resource Conservation Service (NRCS) data, the soil underlying the project site consists of:

Soil Unit Code	Soil Description	Approximate Project Coverage	Drainage Class	Hydrologic Soil Group
EkaAr	Elkton loam, 0 to 2% slopes	١3.6%	poorly drained	D
EvuB	Evesboro-Urban land complex, 0% to 5% slopes	31.5%	Poorly drained	D

#### **TABLE 2: NRCS PROJECT SOILS**

The Report of Infiltration Evaluation for the site can be found in Appendix D. This report was conducted by Maser Consulting on May 20, 2019. It was the conclusion of this report that based on the criteria set forth by the NJDEP BMP manual the soils on site are HSG D. Therefore, this was the HSG classification utilized for the stormwater analysis.

# 3.0 PROPOSED CONDITIONS

The proposed development will consist of the four commercial buildings (36,042 SF total) These building include a supermarket, retail store, fast food restaurant and a convenience store/gas station. Additional improvements include an off-street parking lot (242 parking spaces), landscaping, lighting and stormwater

management facilities. The total proposed impervious coverage on site is 34% (204,694 SF). Access to the site will be provided via two full movement access drives on Asbury Avenue and Route 35.

#### 3.1 PROPOSED DRAINAGE AREAS

Under proposed conditions site drains to one point of interest located within Hollow Brook where the Brook crosses Asbury Avenue.

Under proposed conditions, the existing drainage patterns and point of interest will be maintained. The drainage area which contains all constructed improvements will include stormwater bmps's such as porous pavement, manufactured treatment devices, rain gardens and underground basins.

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
POI I (Area I)	Drainage area to Hollow Brook culvert crossing Asbury Avenue	289,872 SF	204,694 SF	6.0 Minutes*
POI 2	Drainage area to Hollow Brook culvert crossing Asbury Avenue (Reforestation)	53,278 SF	0	6 Minutes**

#### **TABLE 3: PROPOSED DRAINAGE AREAS**

\*The minimum time of concentration was utilized for all drainage areas due to the high level of impervious coverage / land disturbance and proximity to existing and proposed stormwater pipe conveyance system.

\*\* The minimum time of concentration was utilized as the time of concentration will not change in proposed conditions.

All proposed drainage areas were delineated based on the proposed grading design overlain on field survey data. Hydrologic calculations and parameters for each drainage area can be found in **APPENDIX C**; specific drainage area delineations and land cover can be found in **APPENDIX E**.

# 4.0 ANALYSIS METHODOLOGY & DESIGN PARAMETERS

#### 4.1 HYDROLOGIC & HYDRAULIC ANALYSES

The analysis program "HydroCAD" Version 10.0 by HydroCAD Software Solutions was utilized to calculate and plot the runoff hydrographs. The program incorporates the time of concentration, C values, rainfall data, and project drainage areas to calculate the runoff characteristics. The existing and proposed drainage areas have been analyzed utilizing Intensity-Duration-Frequency data was obtained from NOAA for the project area; specifics of the rainfall distribution can be found in **Appendix C**. Additional key variables utilized in the analysis include:

Variable	Input	Variable	Input
Runoff Calculation Method	SCS TR-20	NRCS Rainfall Frequency Data Set	Middlesex
Pervious/Impervious CN Calculations	Separate	Storm Intervals (Year Events)	2, 10, 25, 100
Stage-Storage Relationship	Dynamic	Storm Duration	24 Hours
Minimum time of concentration	6 minutes	Storm Curve	NOAA D

#### **TABLE 4: HYDROCAD DESIGN VARIABLES**

Additional information regarding the hydrologic calculations can be found in **APPENIDX C**.

#### HYDRAULIC METHODOLOGY

The analysis program "HydraFlow Storm Sewers" Version 2018 by Autodesk was utilized to generate hydraulic grade lines through the proposed conveyance system model based on various pipe / junction losses and the runoff tributary to each inlet or discharge structure. Additional key variables utilized in the analysis include:

**TABLE 5: Hydraflow Design Variables** 

Variable	Input	Variable	Input
Runoff Calculation Method	Rational	Pipe Conveyance Method	Std. Step
C-value for impervious surfaces	0.98	Initial Hydraulic Grade Line	Normalized
C-value for pervious surfaces	0.25/0.65	Inlet Drainage Area Delineation	Surveyed
Minimum time of concentration	6 minutes	Inlet Geometry & Capacity	NJDOT Std.

Additional information regarding the hydrologic calculations can be found in **APPENDIX C**.

#### 4.2 **New Jersey Stormwater Design Parameters**

The extent of redevelopment proposes to disturb more than one acre of land and add more than one-quarter acre of new impervious surfaces; as such, it is considered a Major Development as defined in the Township Ordinances and NJAC 7:8-1.2. A Major Development is subject to stormwater runoff quantity, quality, and groundwater recharge requirements. See below for a summary of each design parameter and compliance requirements:

Design Parameter	Design Target for Compliance
Stormwater Runoff Quantity	Design stormwater management measures so that the post-construction peak runoff rates for the two, 10 and 100-year storm events are 50, 75 and 80 percent, respectively, of the pre-construction peak runoff rates. The percentages apply only to the post-construction stormwater runoff that is attributable to the portion of the site on which the proposed development or project is to be constructed
Stormwater Runoff Quality	Stormwater management measures shall be designed to reduce the post- construction load of total suspended solids (TSS) in stormwater runoff generated from the water quality design storm by 80 percent of the anticipated load from impervious coverage.
Groundwater Recharge	The project is <b>exempt</b> from groundwater recharge requirements as the project site is located within State Planning Area PA-1 (Metropolitan).

# **TABLE 6: PROJECT STORMWATER DESIGN INTENT SUMMARY TABLE**

#### 4.3 SUBSURFACE STORMWATER INVESTIGATION

A subsurface stormwater investigation was conducted by Maser Consulting on May 20<sup>th</sup> 2020. A total of [7] tests were performed in compliance with the soil testing standards outlined within Appendix E of the NJDEP Best Management Practices (BMP) Manual. All proposed stormwater facilities meet or exceed the minimum separation depth from seasonal high groundwater. The full investigation report and testing results can be found in **Appendix D** of this Report.

# 5.0 PROJECT ANALYSIS RESULTS

### 5.1 STORMWATER QUANTITY CONTROL

Underground detention basins are proposed beneath the off-site parking area and driveway. These basins will attenuate peak stormwater runoff rates for the drainage area which contains the constructed improvements. In addition to the detention basins rain gardens and dry wells are proposed in order to infiltrate enough stormwater runoff volume as to reduce the volume of runoff from the site.

The reforested area will see a natural reduction in runoff volume and peak flow by the planting of trees to create a wooded area where one does not exist today. This area will meet the stormwater regulations by producing a proposed hydrograph that at no point in time exceeded the existing hydrograph.

The tables below summarize the various drainage areas in relation to flow rates and runoff volume during regulatory storm events:

Drainage Area	2-Year Flow Rate / Volume	10-Year Flow Rate / Volume	100-Year Flow Rate / Volume
Drainage Area I	11.31 CFS / 60,714 CF	18.60 CFS / 103,761 CF	33.49 CFS / 193,747 CF
Drainage Area 2	2.13 CFS / 8,252 CF	4.03 CFS / 15,790 CF	7.92 CFS / 31,995 CF

TABLE 7: SUMMARY OF EXISTING DRAINAGE AREA FLOW RATES AND VOLUMES

Drainage Area	2-Year Flow Rate / Volume	10-Year Flow Rate / Volume	100-Year Flow Rate / Volume
Drainage Area I	4.70 CFS / 50,611CF	7.60 CFS / 90,303	23.95 CFS / 174,722 CF
Drainage Area 2	1.62 CFS / 6,351 CF	3.41 CFS / 13,222 CF	7.27 CFS / 28,682 CF

The table below outlines the peak flow reductions required and achieved by way of the underground basins, dry wells and rain gardens:

Rainfall Event	Existing Flow Rate			Proposed Flow Rate	Proposed % Reduction
2-Year Storm	11.31 CFS	50%	5.66 CFS	4.70 CFS	58.4%
10-Year Storm	18.60 CFS	25%	13.95 CFS	7.60 CFS	59.1%
100-Year Storm	33.49 CFS	20%	26.79 CFS	23.95 CFS	71.5%

The proposed underground basins, dry wells, and rain gardens provide sufficient flow rate attenuation and volume infiltration to ensure that no adverse impacts are anticipated downstream of the project site. Detailed hydrologic calculations for each drainage area can be found in **APPENDIX C**.

#### 5.2 **GROUNDWATER RECHARGE**

As indicated in the Township Ordinances and NJAC 7:8-5.4, the project site is exempt from groundwater recharge requirements as the site is located within the Metropolitan Planning Area (PA-I) per the State Plan Policy Map and thus qualifies as an Urban Redevelopment Area (which is exempt from groundwater recharge requirements for all developed areas). The soils on-site area classified as HSG D per "The Report of Infiltration

Evaluation" found in Appendix D of this report. Groundwater recharge is not required for the portion of the site that is within the area of prior tree removal due to no infiltration in existing conditions.

#### 5.3 STORMWATER QUALITY CONTROL

As a Major Development, all proposed impervious vehicular travel surfaces are subject stormwater runoff quality requirements. More specifically, existing impervious areas proposed to be redeveloped shall be required to remove 50% of total suspended solids and all new impervious vehicular travel surfaces shall be required to remove 80% of total suspended solids. Non-vehicular travel surfaces (building roofs, plaza/amenity areas, sidewalks, etc.) are not subject to runoff quality regulations.

Manufactured treatment devices (MTD's), specifically the Stormwater Management StormFilter (Stormfilter) by Contech Engineered Solutions LLC, will be installed downstream of the underground basin. The drainage area to each MTD is designed to be less than 2.5 acres. The basins will be lined with an impervious liner as to prevent any infiltration of untreated runoff. Per the MTD Lab Certification issued by the NJDEP on December 14, 2016 the StormFilter has been certified to provide a TSS removal rate of 80%.

In addition to the MTD's rain gardens and porous pavement areas are proposed on site. These BMP's are also approved by the NJDEP for 80% TSS removal. These BMP's will drain to the detention basin and will ultimately be treated by the MTD's for a treatment train TSS removal efficiency of 96%. The total TSS removal rate for the site is 85.9%.

Stormwater BMP Facility	NJDEP Certified Removal Efficiency	Motor Vehicle Area Treated	Treatment Train Removal Efficiency
Rain Garden	80%	0.27 Acres	96%
Porous Pavement	80%	I.79 Acres	96%
Manufactured Treatment Device	80%	3.84 Acres	80%
Total Site			85.9%

**TABLE 10: STORMWATER BMP TSS REMOVAL EFFICIENCIES**

A copy of the NJDEP certification as well as a design summary for the MTD's can be found in **APPENDIX D**.

#### 5.4 STORMWATER CONVEYANCE SYSTEMS

The stormwater conveyance system has been sized for the 25-year storm and is able to safely convey runoff to stormwater management facilities without overflowing. A summary of the pipe network and pipe profiles can be found in Appendix C of the Report.

#### 5.5 Soil Erosion & Sediment Control

A Soil Erosion & Sediment Control Plan has been prepared in accordance with the latest edition of the Standards for Soil Erosion and Sediment Control in New Jersey. Proposed temporary measures during construction include silt fencing, tree protection fences, stabilized construction entrances, inlet filters, and cover for soil stabilization. Permanent post-construction measures include conduit outlet projection and native vegetation. No land disturbance will occur until a permit has been obtained from the Freehold Soil Conservation District.

#### 5.6 STORMWATER OPERATIONS & MAINTENANCE

A Stormwater Operations & Maintenance Manual will be submitted for approval to the Township of Neptune prior to the start construction. Any required easements or covenants associated with the stormwater improvements will be recorded prior to the start of construction.

# 6.0 OFFSITE DRAINAGE AREA AND DEAL LAKE IMPACT ANALYSIS

The project site will discharge into the Hollow Brook section of Deal Lake. The total drainage area to Deal Lake is approximately 4,130 acres (see Appendix E for Deal Lake Drainage Area Exhibit). The land cover within the drainage area is mostly residential and commercial developments. The project lot, 13.60 acres, makes up a very small part of this drainage area. The limit of disturbance for this project is approximately 6.53 acres and accounts for 0.16 percent of the total drainage area to Deal Lake.

Currently the lot contains an abandoned building and parking lot which provide no environmental benefit to Deal Lake. The proposed project will involve reforestation to the onsite wetlands buffer. This reforestation will help restore the vegetated buffer between Hollow Brook and the developed area of the lot. An underground detention basin is also proposed which will reduce peak flows discharged from the project lot. The basin discharge will be treated by a manufactured treatment device certified by the NJDEP to treat stormwater runoff for eighty percent total suspended solids removal.

The manufactured treatment device will provide some phosphorus removal from the settling of solids and the organic and inorganic phosphorus associated with solids. The fertilizer specified for use on site is LESCO 16-0-8

NPK. The fertilizer is organic, contains no phosphorus and no animal or human waste. There will be no pesticides used on the proposed vegetation. Proposed reforestation will be greater than 85 feet from the top of Hollow Brook, while landscaping for the developed portion of the site will be greater than 150 feet.

The proposed project will not result in any increase in fecal coliform load to Deal Lake. The proposed vegetation will not result in any habitable area for geese. Geese prefer open grassy areas near water. The project will reforest existing open space with woods, the wooded area adjacent to the water way will not be disturbed. The developed portion of the lot will contain driveways, parking lots, and buildings with vegetation in the green spaces. The proposed use is not conducive to bird habitat due to the noise associated with cars and people. The proposed uses of grocery store, retail store, fast food restaurant and convenience store will not result in any pet waste. All trash will be confined to on site trash enclosure as well as trash containers spaced evenly throughout the site. The property will be maintained to prevent litter, this will help ensure no increase in wildlife on site or pollution entering Hollow Brook.

The project will improve stormwater runoff to Deal Lake by reducing stormwater peak flow rates to Hollow Brook and will provide a manufactured treatment device to improve stormwater runoff quality. Vegetation on site will be improved by way of reforestation of open space and proposed landscaping. Fertilizer used on site will not contain any phosphorus or waste. No suitable goose habitat will be created by the project and all trash will be confined to containers. As such the proposed project will be a benefit to Hollow Brook and Deal Lake.

### 7.0 CONCLUSIONS

The increase in runoff flow rate and volume generated by the proposed redevelopment will be satisfactorily mitigated by the introduction of an underground detention basin and on-site stormwater conveyance system. Runoff water quality will be impacted by the increase in vehicular travel surfaces and a manufactured treatment device will provide treatment to remove total suspended solids to a satisfactory regulatory level.

The proposed project complies with all applicable stormwater management regulations and standards. As such, the project is not anticipated to have any adverse drainage impacts on neighboring properties, downstream watercourses, or adjoining conveyance systems.

# **APPENDIX A PROJECT FIGURES**

**INVENTORY** 

AERIAL MAP

**USGS MAP** 

ΤΑΧ ΜΑΡ

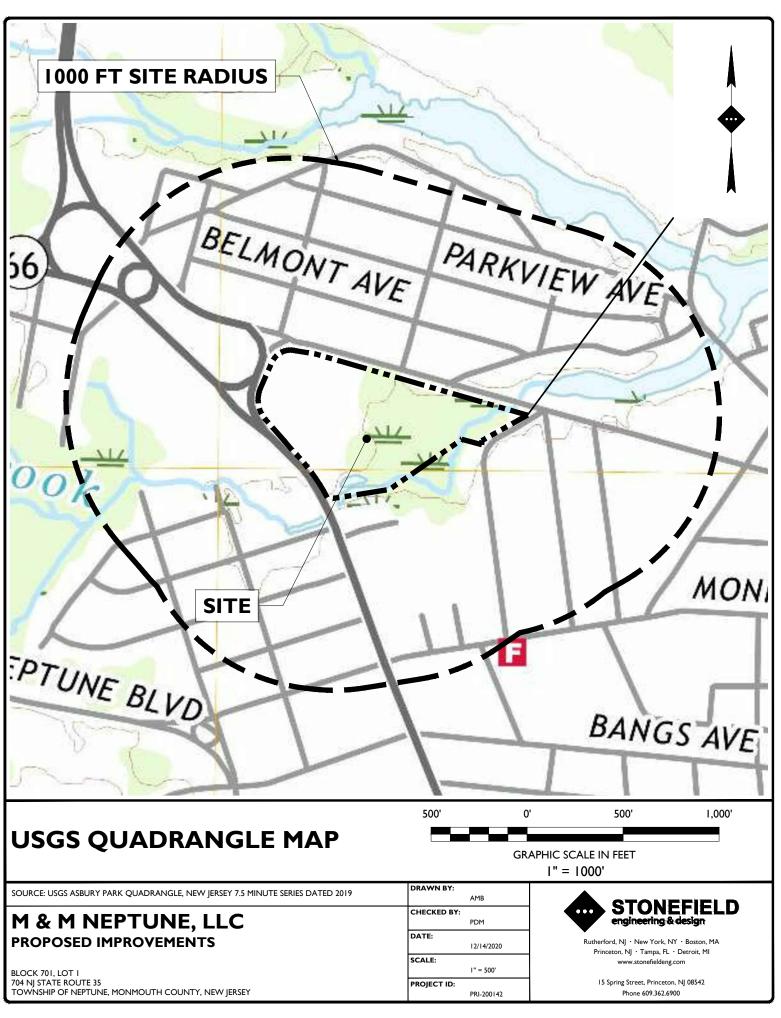
# FEMA FLOOD RATE MAP

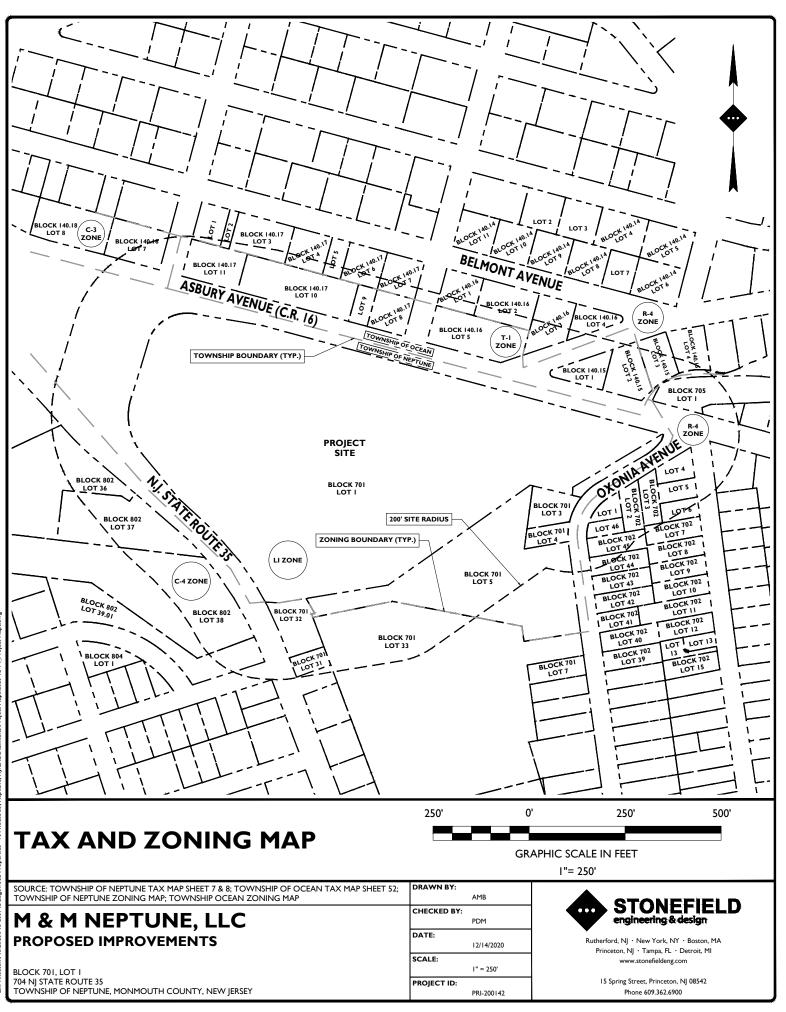
DRAINAGE AREA TO DOWNSTREAM COUNTY STRUCTURE EXHIBIT

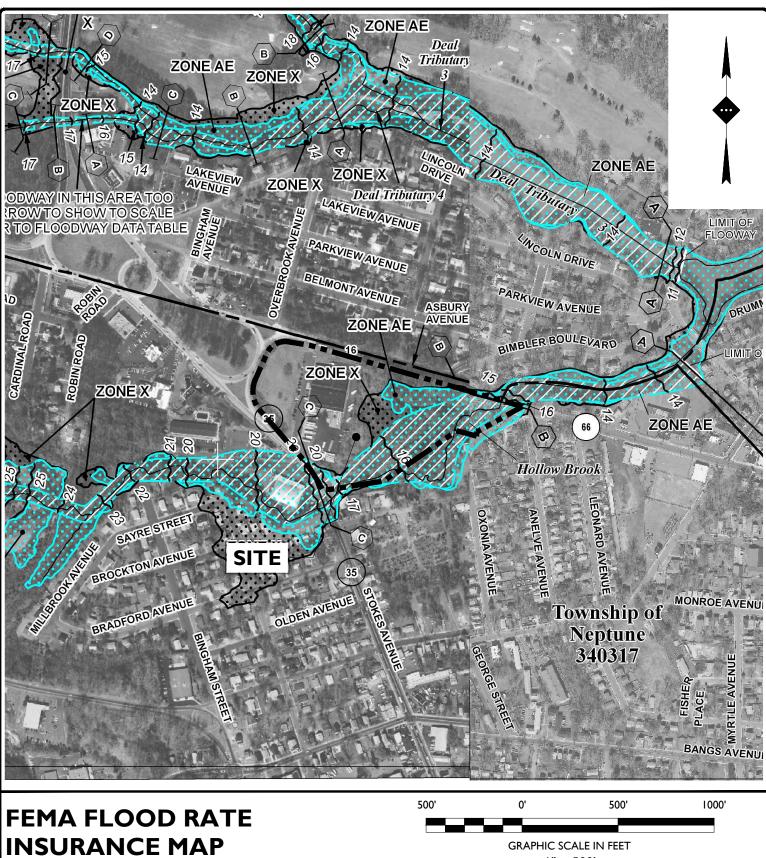




AERIAL MAP	200'	0'		200'	400'
	GRAPHIC SCALE IN FEET I" = 200'				
SOURCE: GOOGLE EARTH PRO 10/19/2020	DRAWN BY:	АМВ		OTONEE	
M & M NEPTUNE, LLC	CHECKED BY:	PDM	Rutherford, NJ · New York, NY · Boston, MA Princeton, NJ · Tampa, FL · Detroit, MI		
PROPOSED IMPROVEMENTS	DATE:	12/14/2020			
BLOCK 701, LOT I	SCALE:	I" = 200'		www.stonefieldeng.com	
704 NJ STATE ROUTE 35 TOWNSHIP OF NEPTUNE, MONMOUTH COUNTY, NEW JERSEY	PROJECT ID:	PRI-200142	15	Spring Street, Princeton, NJ 08 Phone 609.362.6900	3542







SOURCE: FEMA FLOOD INSURANCE RATE MAP (FIRM), MONMOUTH COUNTY MAP NUMBER
34025C0332G & 34025C0331F

M&NEPTUNE, LLC

CHECKED BY:
PDM

### **M & M NEP I UNE, LLC** PROPOSED IMPROVEMENTS

BLOCK 701, LOT I 704 NJ STATE ROUTE 35 TOWNSHIP OF NEPTUNE, MONMOUTH COUNTY, NEW JERSEY DATE:

SCALE:

PROIECT ID:

12/14/2020

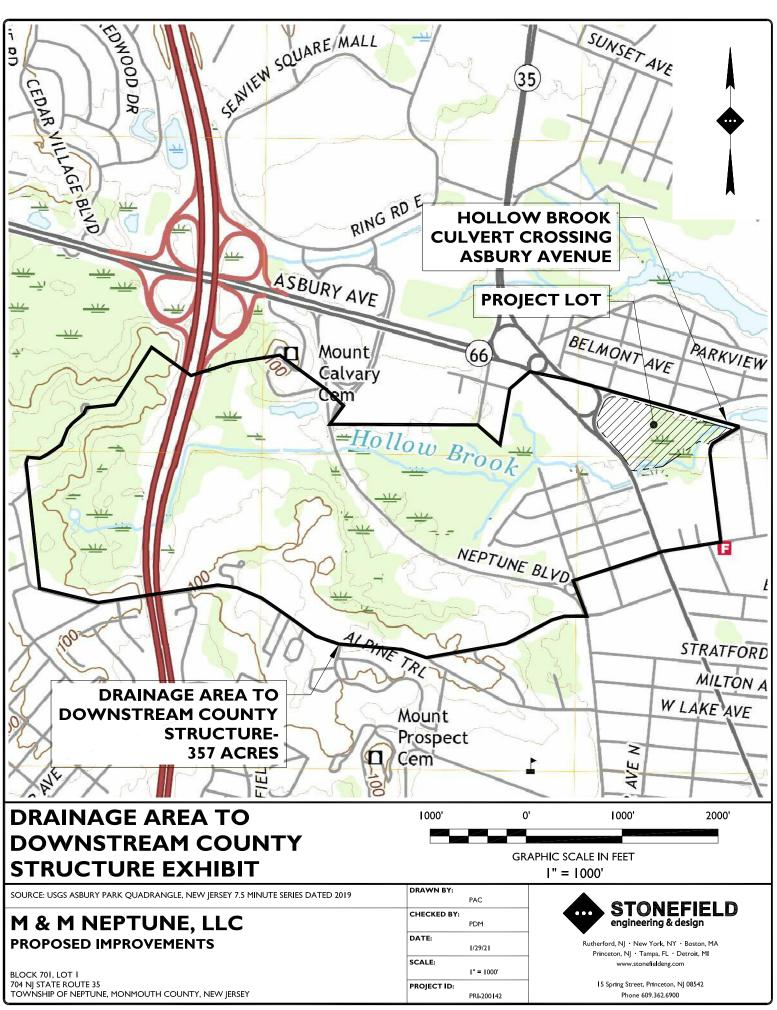
1" = 500

PRI-200142



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# APPENDIX B NRCS COUNTY SOIL SURVEY





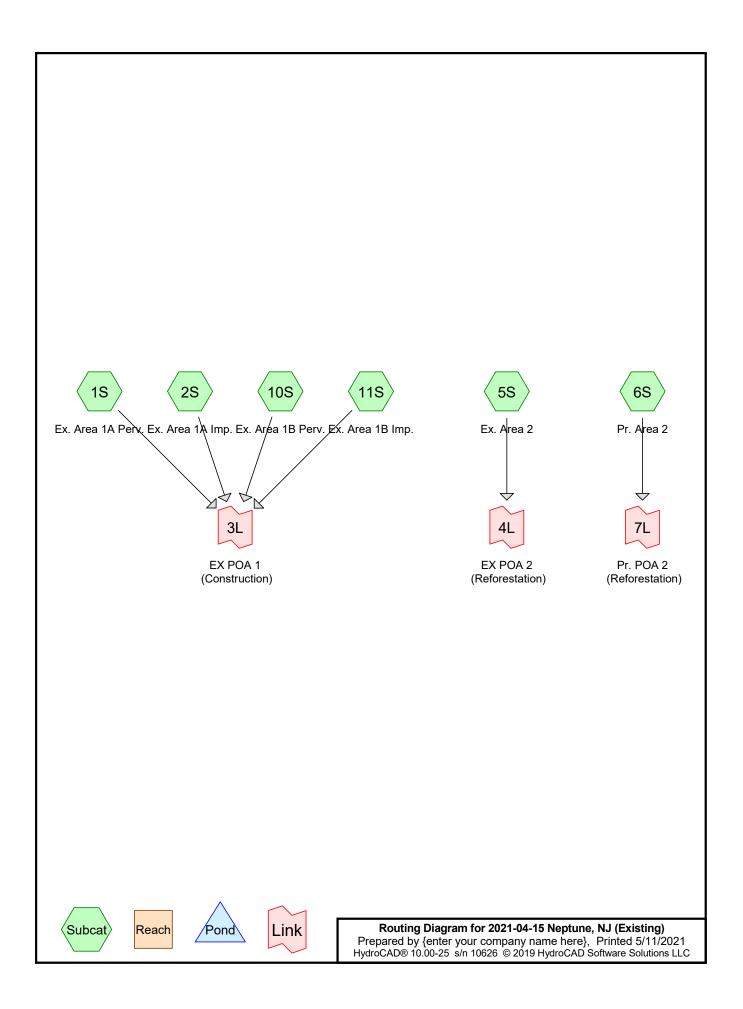
USDA Natural Resources Conservation Service

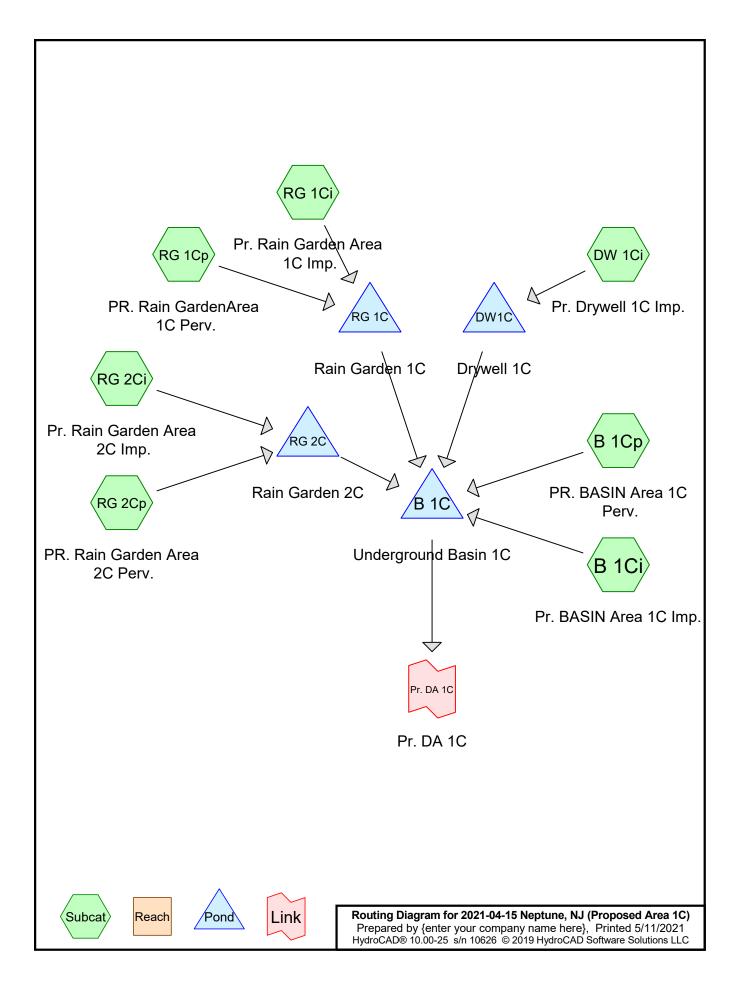
# **APPENDIX C** DESIGN CALCULATIONS & DIAGRAMS

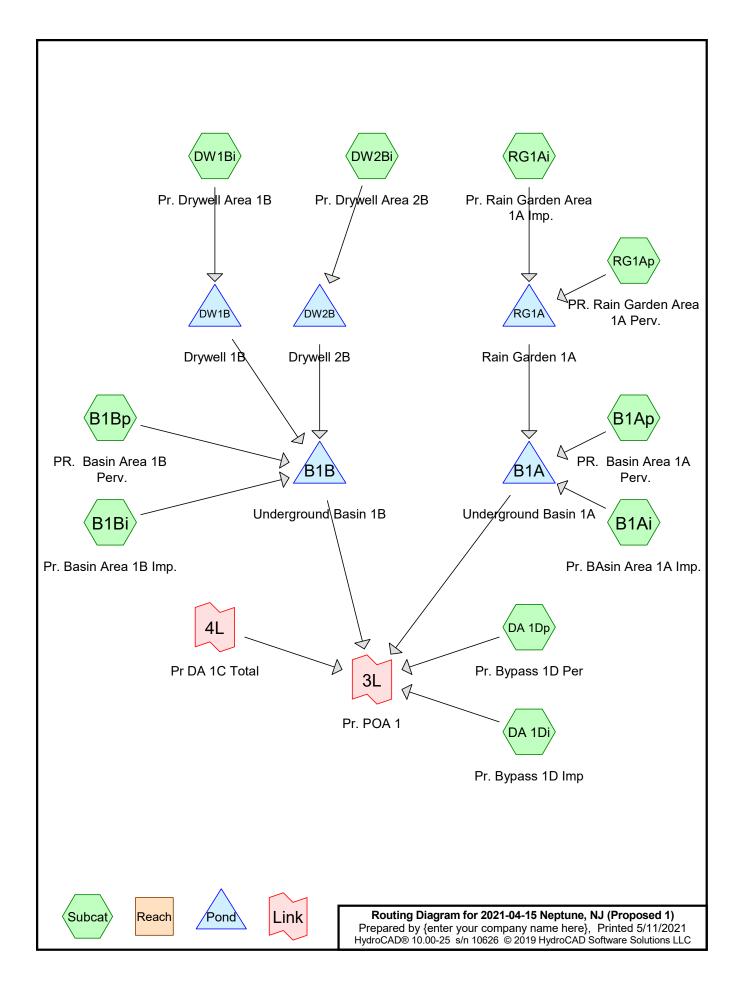


# APPENDIX C-I HydroCAD Routing Diagram









# **APPENDIX C-2** 2-YEAR STORM EVENT HYDROGRAPHS



#### Summary for Subcatchment 1S: Ex. Area 1A Perv.

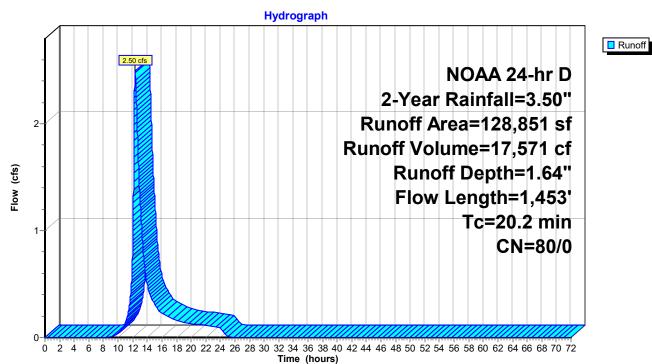
Page 1

2.50 cfs @ 12.34 hrs, Volume= 17,571 cf, Depth= 1.64" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

_	Ai	rea (sf)	CN [	Description		
-	1	15,266	80 >	>75% Gras	s cover, Go	ood, HSG D
_		13,585	77 \	Noods, Go	od, HSG D	
	1	28,851	80 \	Neighted A	verage	
	1	28,851	80 <sup>-</sup>	100.00% Pe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.0	100	0.0600	0.19		Sheet Flow, Sheet Flow
						Grass: Dense n= 0.240 P2= 3.34"
	0.5	75	0.0270	2.65		Shallow Concentrated Flow, Shallow Concentrated
						Unpaved Kv= 16.1 fps
_	10.6	1,278		2.00		Direct Entry, Channel Flow
-	20.2	1,453	Total			

#### Subcatchment 1S: Ex. Area 1A Perv.



### Summary for Subcatchment 2S: Ex. Area 1A Imp.

Runoff = 8.44 cfs @ 12.14 hrs, Volume= 37,031 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

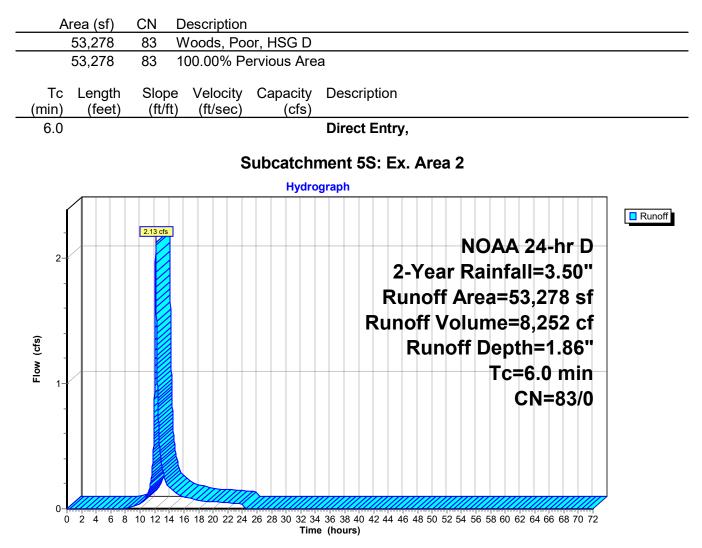
1	rea (sf) 36,039	98 F		ing, HSG D		
1	36,039	98 1	00.00% In	npervious A	Irea	
Tc	Length	Slope	Velocity (ft/sec)	Capacity (cfs)	Description	
<u>min)</u> 6.0	(feet)	(ft/ft)	(11/Sec)	(015)	Direct Entry,	
			Sub	catchmer	nt 2S: Ex. Area 1A Imp.	
					graph	
9	/					Runo
1		8.44 cfs			NOAA 24-hr D	
8-					2-Year Rainfall=3.50"	
7-					Runoff Area=136,039 sf	
6-					Runoff Volume=37,031 cf	
(SD) 5					Runoff Depth=3.27"	
(CID) (CID)					Tc=6.0 min	
4					CN=0/98	
3-						
2-						
1- 1-			<u> </u>			
- - 0-	mm					
Ó	2468	10 12 14 1	6 18 20 22 24		4 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 e (hours)	

#### Summary for Subcatchment 5S: Ex. Area 2

Page 3

2.13 cfs @ 12.14 hrs, Volume= Runoff 8,252 cf, Depth= 1.86" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

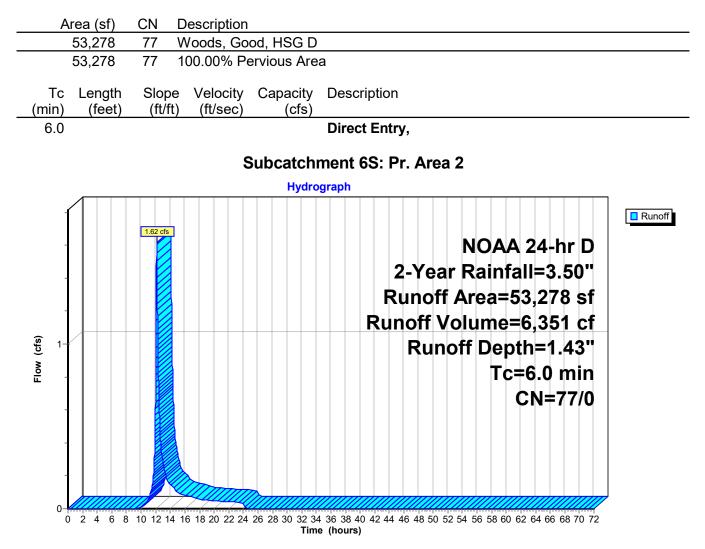


### Summary for Subcatchment 6S: Pr. Area 2

Page 4

Runoff 1.62 cfs @ 12.15 hrs, Volume= 6,351 cf, Depth= 1.43" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"



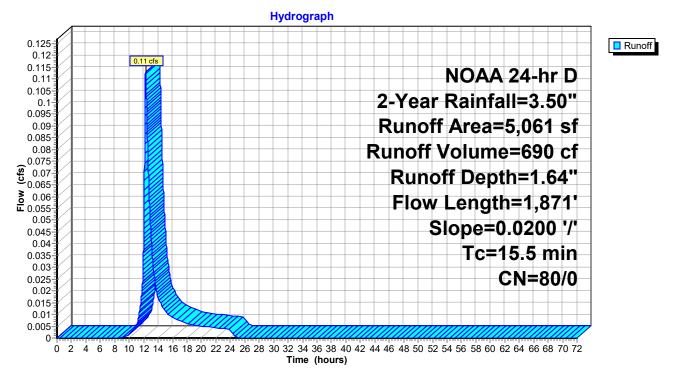
#### Summary for Subcatchment 10S: Ex. Area 1B Perv.

Runoff = 0.11 cfs @ 12.26 hrs, Volume= 690 cf, Depth= 1.64"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

 A	rea (sf)	CN E	Description						
	5,061	80 >	>75% Grass cover, Good, HSG D						
	5,061	80 1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
1.2	100	0.0200	1.41		Sheet Flow,				
1.1	184	0.0200	2.87		Smooth surfaces n= 0.011 P2= 3.34" <b>Shallow Concentrated Flow,</b> Paved Kv= 20.3 fps				
 13.2	1,587		2.00		Direct Entry,				
 15.5	1,871	Total							

#### Subcatchment 10S: Ex. Area 1B Perv.



### Summary for Subcatchment 11S: Ex. Area 1B Imp.

Runoff = 1.24 cfs @ 12.14 hrs, Volume= 5,423 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

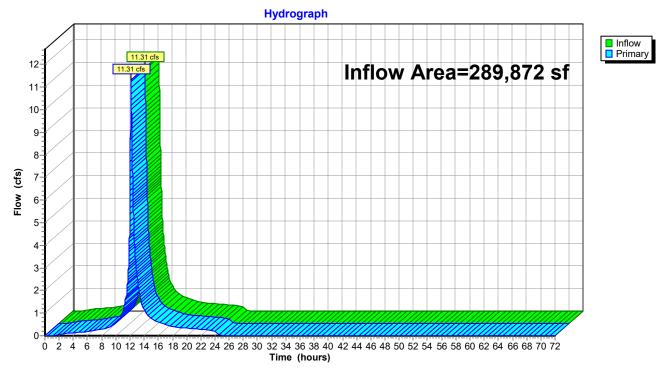
<u>19,921</u> 19,921	
	·
Tc Lengt min) (fee	
6.0	Direct Entry,
	Subcatchment 11S: Ex. Area 1B Imp.
	Hydrograph
	1.24 cfs NOAA 24-hr D
	2-Year Rainfall=3.50"
1-	Runoff Area=19,921 sf
s ,	Runoff Volume=5,423 cf
	Runoff Depth=3.27"
	Tc=6.0 min CN=0/98
-	

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

# Summary for Link 3L: EX POA 1 (Construction)

Inflow Are	a =	289,872 sf, 53.80% Impervious, Inflow Depth = 2.51" for 2-Year event	
Inflow	=	11.31 cfs @ 12.15 hrs, Volume= 60,714 cf	
Primary	=	11.31 cfs @ 12.15 hrs, Volume= 60,714 cf, Atten= 0%, Lag= 0.0 mi	in

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

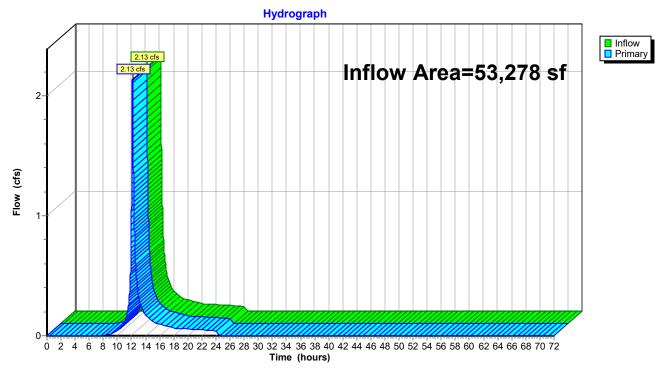


# Link 3L: EX POA 1 (Construction)

# Summary for Link 4L: EX POA 2 (Reforestation)

Inflow Area	=	53,278 sf,	0.00% Impervious,	Inflow Depth = 1.86'	for 2-Year event
Inflow	=	2.13 cfs @ 1	12.14 hrs, Volume=	8,252 cf	
Primary	=	2.13 cfs @ 1	12.14 hrs, Volume=	8,252 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

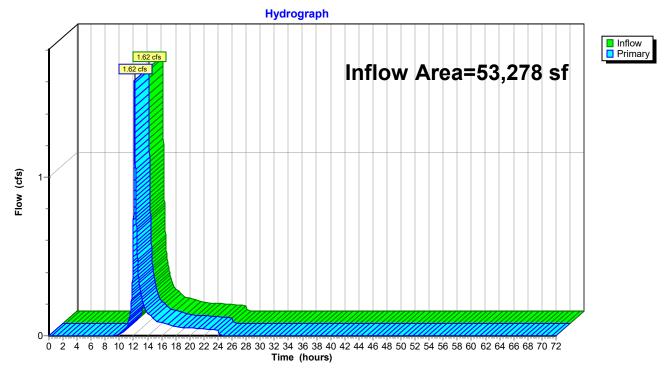


# Link 4L: EX POA 2 (Reforestation)

# Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Area	=	53,278 sf,	0.00% Impervious,	Inflow Depth = 1.43"	for 2-Year event
Inflow	=	1.62 cfs @ 1	12.15 hrs, Volume=	6,351 cf	
Primary	=	1.62 cfs @ 1	12.15 hrs, Volume=	6,351 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



# Link 7L: Pr. POA 2 (Reforestation)

### Summary for Subcatchment B1Ai: Pr. BAsin Area 1A Imp.

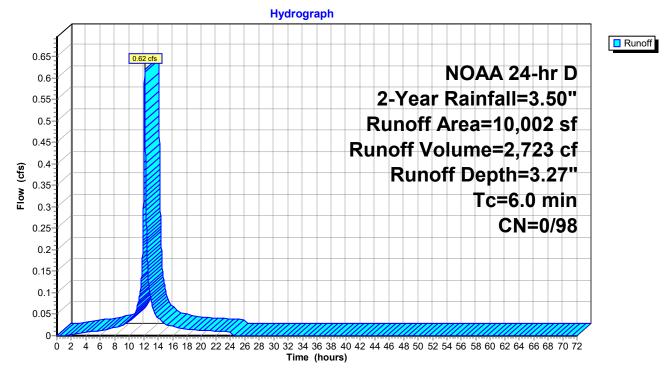
Page 1

Runoff 0.62 cfs @ 12.14 hrs, Volume= 2,723 cf, Depth= 3.27" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Area (sf)	CN	Description		
10,002	98	Paved park	ing, HSG D	
10,002	98	98 100.00% Impervious Area		
Tc Length (min) (feet)	Slop (ft/f		Capacity (cfs)	Description
6.0				Direct Entry,

### Subcatchment B1Ai: Pr. BAsin Area 1A Imp.

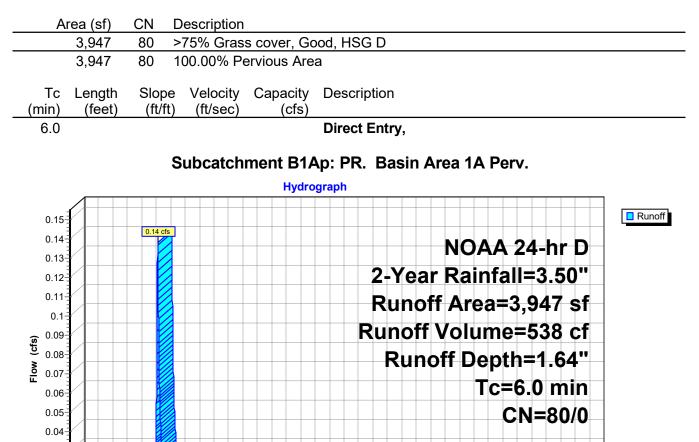


### Summary for Subcatchment B1Ap: PR. Basin Area 1A Perv.

538 cf, Depth= 1.64" Runoff 0.14 cfs @ 12.14 hrs, Volume= =

0.03 0.02 0.01 0

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"



0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

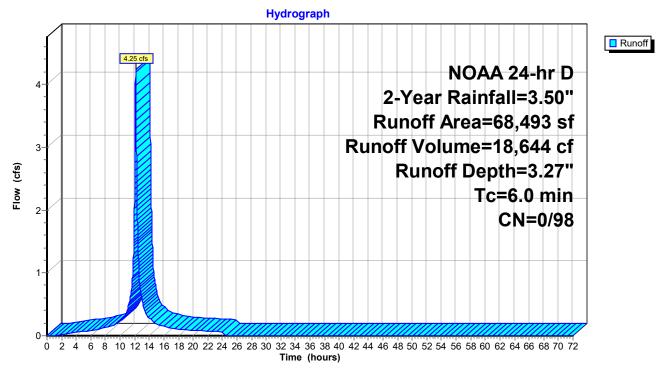
### Summary for Subcatchment B1Bi: Pr. Basin Area 1B Imp.

Runoff = 4.25 cfs @ 12.14 hrs, Volume= 18,644 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

A	rea (sf)	CN	Description						
	68,493	98	98 Paved parking, HSG D						
	68,493	98	100.00% In	npervious A	Area				
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

### Subcatchment B1Bi: Pr. Basin Area 1B Imp.



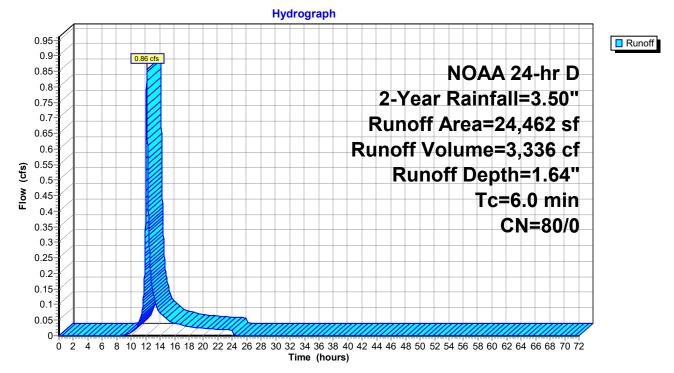
### Summary for Subcatchment B1Bp: PR. Basin Area 1B Perv.

Runoff = 0.86 cfs @ 12.14 hrs, Volume= 3,336 cf, Depth= 1.64"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

A	rea (sf)	CN I	Description						
	24,462	80 ;	80 >75% Grass cover, Good, HSG D						
	24,462	80	100.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

### Subcatchment B1Bp: PR. Basin Area 1B Perv.



0.025 0.02 0.015 0.01 0.005 0CN=0/98

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### Summary for Subcatchment DA 1Di: Pr. Bypass 1D Imp

Runoff 0.07 cfs @ 12.14 hrs, Volume= 308 cf, Depth= 3.27" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

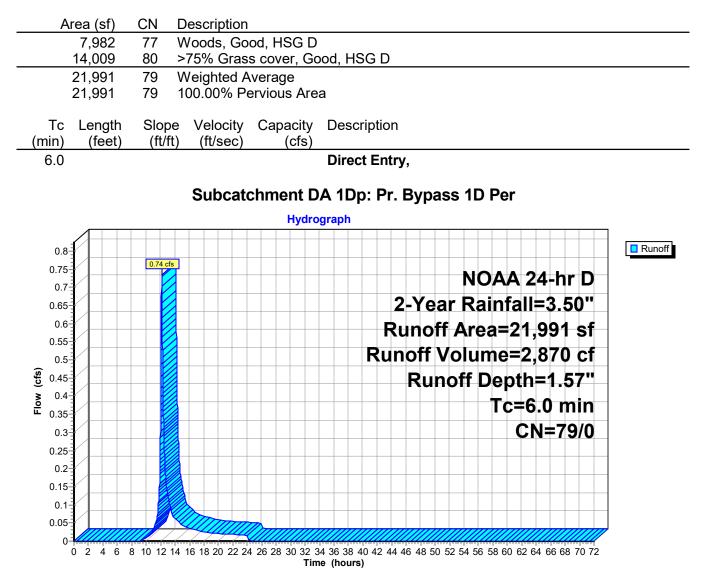
Area (s	f) CN Descriptior	1		
1,13	1 98 Paved parl	king, HSG D		
1,13	1 98 100.00% Ir	npervious Are	ea	
Tc Lenç (min) (fe		Capacity ( (cfs)	Description	
6.0		I	Direct Entry,	
_	Subcat	chment DA <sub>Hydrogr</sub>	A 1Di: Pr. Bypass 1D Imp	1
0.075	0.07 cfs			Runoff
0.07			NOAA 24-hr D	
0.065			2-Year Rainfall=3.50"	
0.055			Runoff Area=1,131 sf	
0.05 0.045			Runoff Volume=308 cf	
້ວ <u>0.04</u>			Runoff Depth=3.27"	
은 0.035 0.03			Tc=6.0 min	

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

### Summary for Subcatchment DA 1Dp: Pr. Bypass 1D Per

Runoff = 0.74 cfs @ 12.14 hrs, Volume= 2,870 cf, Depth= 1.57"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"



### Summary for Subcatchment DW1Bi: Pr. Drywell Area 1B

Runoff = 0.50 cfs @ 12.14 hrs, Volume= 2,195 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

	8,065			ing, HSG D										
	8,065	98 1	00.00% In	npervious A	rea									
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descriptio	n								
6.0	()	(1411)	(	(0.0)	Direct En	t <b>ry</b> ,								
			Subcate	chment D	W1Ri· Dr	Dry	ر الم	۸ro	a 1	R				
			Subcall	Hydro		Diyv			aı	D				
0.55-														Runot
0.5		0.50 cfs						NC	CAC	<b>A</b> :	24	-h	r D	
0.45						2-Ye	ear							
0.4						Rur	noff	A	rea	<b>1=8</b>	3,0	65	sf	
0.35					Ru	noff	Vo	lu	me	=2	2,1	95	cf	
(SU) 0.3 MOI 0.25						Rι	ino	ff [	De	ptł	า=:	3.2	27"	
<b>8</b> 0.25									Т	c=(	6.0	) n	nin	
0.2										С	N=	=0/	/98	
0.15														
0.1														
0.05														

Time (hours)

0.01

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### Summary for Subcatchment DW2Bi: Pr. Drywell Area 2B

Runoff = 0.21 cfs @ 12.14 hrs, Volume= 903 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Ar	ea (sf)	CN I	Description			
	3,316	98	Paved park	ing, HSG D		
	3,316	98	100.00% In	npervious A	rea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	
			Subcate	chment D	W2Bi: Pr. Drywell Area 2B	
				Hydro	•	
0.23-						]
0.23						Runoff
0.21		0.21 cfs				-
0.2 0.19					NOAA 24-hr D	-
0.19					2-Year Rainfall=3.50"	-
0.17						
0.16 0.15					Runoff Area=3,316 sf	-
0.14					Runoff Volume=903 cf	
(ع) 0.13 0.12						-
0.12 0.11 0.11 U					Runoff Depth=3.27"	-
Ĕ 0.1 0.09	()				Tc=6.0 min	
0.09						-
0.07					CN=0/98	
0.06						-
0.05 0.04						1
0.04	$/ \rightarrow \rightarrow$					
0.02	Y ,↓↓					

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

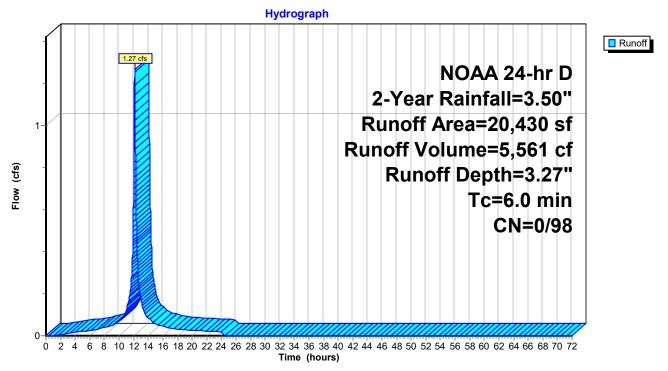
### Summary for Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.

Runoff = 1.27 cfs @ 12.14 hrs, Volume= 5,561 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Ar	ea (sf)	CN	Description						
	20,430	98	98 Paved parking, HSG D						
	20,430	98	100.00% In	npervious A	Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
6.0					Direct Entry,				

### Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.



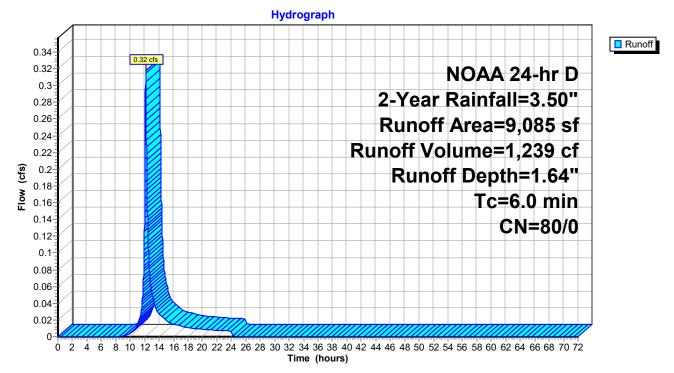
### Summary for Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.

Runoff = 0.32 cfs @ 12.14 hrs, Volume= 1,239 cf, Depth= 1.64"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

A	rea (sf)	CN	Description						
	9,085	80	80 >75% Grass cover, Good, HSG D						
	9,085	80	100.00% Pe	ervious Are	ea				
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

### Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.



### Summary for Pond B1A: Underground Basin 1A

Inflow Area =	43,464 sf, 70.02% Impervious,	Inflow Depth = 0.90" for 2-Year event
Inflow =	0.76 cfs @ 12.14 hrs, Volume=	3,261 cf
Outflow =	0.43 cfs @ 12.31 hrs, Volume=	3,260 cf, Atten= 44%, Lag= 10.3 min
Primary =	0.43 cfs @ 12.31 hrs, Volume=	3,260 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 19.73' @ 12.31 hrs Surf.Area= 1,439 sf Storage= 449 cf

Plug-Flow detention time= 27.4 min calculated for 3,260 cf (100% of inflow) Center-of-Mass det. time= 27.5 min ( 802.8 - 775.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	1,318 cf	11.03'W x 130.42'L x 3.52'H Field A
			5,072 cf Overall - 1,778 cf Embedded = 3,295 cf x 40.0% Voids
#2A	19.60'	1,703 cf	Contech ChamberMaxx 2016 x 36 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		3,021 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	5.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	21.00'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=0.43 cfs @ 12.31 hrs HW=19.73' TW=0.00' (Dynamic Tailwater)

**-1=Culvert** (Passes 0.43 cfs of 1.66 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 0.43 cfs @ 3.14 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Pond B1A: Underground Basin 1A - Chamber Wizard Field A

# Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

18 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 128.42' Row Length +12.0" End Stone x 2 = 130.42' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

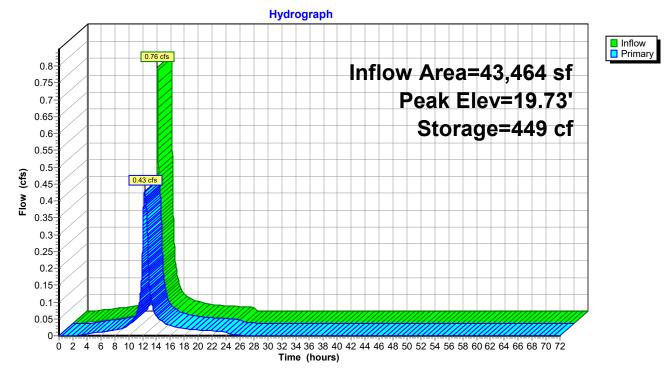
36 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 1,703.2 cf Chamber Storage 36 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 1,777.6 cf Displacement

5,072.2 cf Field - 1,777.6 cf Chambers = 3,294.6 cf Stone x 40.0% Voids = 1,317.8 cf Stone Storage

Chamber Storage + Stone Storage = 3,021.0 cf = 0.069 af Overall Storage Efficiency = 59.6% Overall System Size = 130.42' x 11.03' x 3.52'

36 Chambers 187.9 cy Field 122.0 cy Stone

 $\triangle \triangle$ 



# Pond B1A: Underground Basin 1A

### Summary for Pond B1B: Underground Basin 1B

Inflow Area =	104,336 sf, 76.55% Impervious,	Inflow Depth = 2.59" for 2-Year event
Inflow =	5.11 cfs @ 12.14 hrs, Volume=	22,511 cf
Outflow =	2.05 cfs @ 12.44 hrs, Volume=	22,496 cf, Atten= 60%, Lag= 17.9 min
Primary =	2.05 cfs @ 12.44 hrs, Volume=	22,496 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 20.12' @ 12.44 hrs Surf.Area= 7,780 sf Storage= 5,110 cf

Plug-Flow detention time= 53.3 min calculated for 22,496 cf (100% of inflow) Center-of-Mass det. time= 52.8 min ( 827.2 - 774.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	6,625 cf	53.78'W x 144.65'L x 3.52'H Field A
			27,424 cf Overall - 10,861 cf Embedded = 16,563 cf x 40.0% Voids
#2A	19.60'	10,406 cf	Contech ChamberMaxx 2016 x 220 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		17,031 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
			L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	10.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=2.05 cfs @ 12.44 hrs HW=20.12' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 2.05 cfs of 3.72 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 2.05 cfs @ 3.75 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Pond B1B: Underground Basin 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

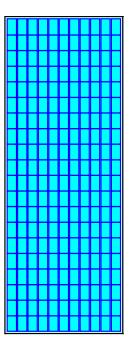
20 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 142.65' Row Length +12.0" End Stone x 2 = 144.65' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

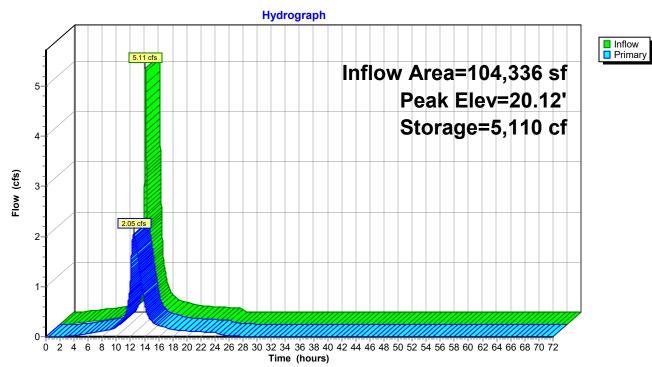
220 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 10,405.7 cf Chamber Storage 220 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 10,860.7 cf Displacement

27,423.7 cf Field - 10,860.7 cf Chambers = 16,563.0 cf Stone x 40.0% Voids = 6,625.2 cf Stone Storage

Chamber Storage + Stone Storage = 17,030.9 cf = 0.391 af Overall Storage Efficiency = 62.1% Overall System Size = 144.65' x 53.78' x 3.52'

220 Chambers 1,015.7 cy Field 613.4 cy Stone





### Pond B1B: Underground Basin 1B

2021-04-15 Neptune, NJ (Proposed 1)

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### Summary for Pond DW1B: Drywell 1B

Inflow Area =	8,065 sf,100.00% Impervious,	Inflow Depth = 3.27" for 2-Year event
Inflow =	0.50 cfs @ 12.14 hrs, Volume=	2,195 cf
Outflow =	0.25 cfs @ 12.35 hrs, Volume=	2,195 cf, Atten= 51%, Lag= 12.5 min
Discarded =	0.01 cfs @ 12.35 hrs, Volume=	1,665 cf
Primary =	0.23 cfs @ 12.35 hrs, Volume=	530 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 24.12' @ 12.35 hrs Surf.Area= 497 sf Storage= 862 cf

Plug-Flow detention time= 404.1 min calculated for 2,195 cf (100% of inflow) Center-of-Mass det. time= 404.1 min (1,164.1 - 760.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	21.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	22.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	23.90'	15.0" Round Culvert
	-		L= 67.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 23.90' / 21.00' S= 0.0433 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	21.50'	
			Conductivity to Groundwater Elevation = 12.75' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.35 hrs HW=24.12' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.23 cfs @ 12.35 hrs HW=24.12' TW=20.11' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.23 cfs @ 1.59 fps)

### Pond DW1B: Drywell 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

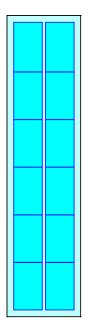
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

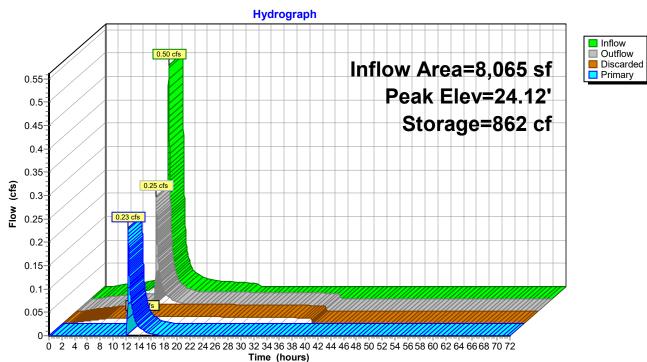
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1B: Drywell 1B

2021-04-15 Neptune, NJ (Proposed 1)

NOAA 24-hr D 2-Year Rainfall=3.50" Printed 5/11/2021 Page 20

### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

### Summary for Pond DW2B: Drywell 2B

Inflow Area =	3,316 sf,100.00% Impervious,	Inflow Depth = 3.27" for 2-Year event
Inflow =	0.21 cfs @ 12.14 hrs, Volume=	903 cf
Outflow =	0.01 cfs @ 14.05 hrs, Volume=	903 cf, Atten= 94%, Lag= 114.5 min
Discarded =	0.01 cfs @ 14.05 hrs, Volume=	902 cf
Primary =	0.00 cfs @ 14.05 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.76' @ 14.05 hrs Surf.Area= 497 sf Storage= 399 cf

Plug-Flow detention time= 258.5 min calculated for 903 cf (100% of inflow) Center-of-Mass det. time= 258.5 min (1,018.5 - 760.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	21.75'	15.0" Round Culvert
			L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 21.75' / 21.00' S= 0.0163 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 14.05 hrs HW=21.76' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 14.05 hrs HW=21.76' TW=19.53' (Dynamic Tailwater) ←1=Culvert (Barrel Controls 0.00 cfs @ 0.35 fps)

### Pond DW2B: Drywell 2B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

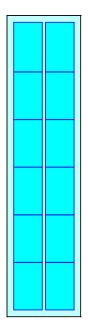
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

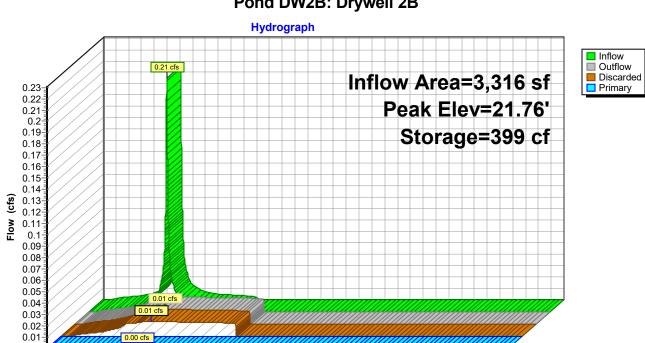
Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone





0-



0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

### Pond DW2B: Drywell 2B

### Summary for Pond RG1A: Rain Garden 1A

Inflow Area =	29,515 sf, 69.22% Impervious,	Inflow Depth = 2.76" for 2-Year event
Inflow =	1.59 cfs @ 12.14 hrs, Volume=	6,800 cf
Outflow =	0.06 cfs @ 15.51 hrs, Volume=	6,800 cf, Atten= 96%, Lag= 202.2 min
Discarded =	0.06 cfs @ 15.51 hrs, Volume=	6,800 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

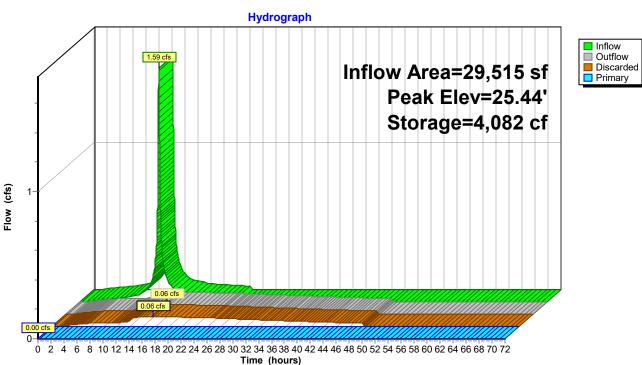
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 25.44' @ 15.51 hrs Surf.Area= 2,483 sf Storage= 4,082 cf

Plug-Flow detention time= 709.9 min calculated for 6,800 cf (100% of inflow) Center-of-Mass det. time= 709.9 min (1,486.8 - 776.9)

Volume	Invert	Avail.Sto	rage	Storage Description		
#1	23.00'	5,5	86 cf	<b>Custom Stage Data</b>	a (Irregular) Listed	below (Recalc)
Elevatio		ırf.Area F	Perim.	Inc.Store	Cum.Store	Wet.Area
fee			(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)
				1	1 1	
23.0	-		175.0	0	0	910
24.0	00	1,532	208.0	1,208	1,208	1,934
25.0	00	2,184	227.0	1,848	3,056	2,627
26.0	00	2,892	246.0	2,530	5,586	3,380
Device	Routing	Invert	Outle	et Devices		
#1	Primary	22.00'	15.0"	' Round Culvert		
			L= 27	7.0' RCP, sq.cut en	d proiectina. Ke= (	0.500
				/ Outlet Invert= 22.00		
				013, Flow Area= 1.2		
#2	Device 1	25.50'		<b>x 48.0" Horiz. Orifi</b>		0
#2	Device I	20.00				10
	<b>D</b> · · · ·	~~~~~		ed to weir flow at low		
#3	Discarded	23.00'		) in/hr Exfiltration or		
			Cond	luctivity to Groundwa	ater Elevation = 10.	80' Phase-In= 0.01'

**Discarded OutFlow** Max=0.06 cfs @ 15.51 hrs HW=25.44' (Free Discharge) **3=Exfiltration** (Controls 0.06 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=23.00' TW=19.10' (Dynamic Tailwater) **1=Culvert** (Passes 0.00 cfs of 3.58 cfs potential flow) **2=Orifice/Grate** (Controls 0.00 cfs)

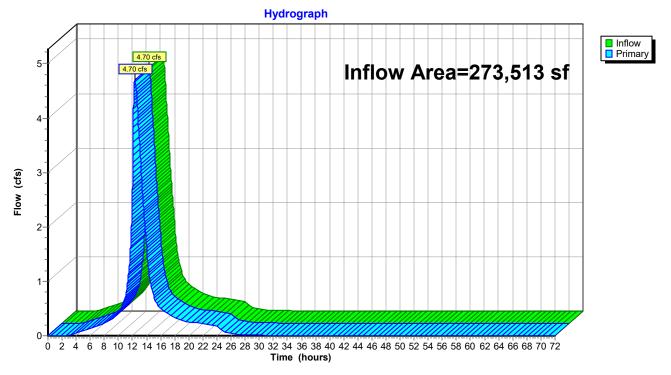


## Pond RG1A: Rain Garden 1A

### Summary for Link 3L: Pr. POA 1

Inflow Are	a =	273,513 sf, 73.09% Impervious, Inflow Depth = 2.22" for 2-Year even	ent
Inflow	=	4.70 cfs @ 12.35 hrs, Volume= 50,611 cf	
Primary	=	4.70 cfs @ 12.35 hrs, Volume= 50,611 cf, Atten= 0%, Lag= 0.0	) min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



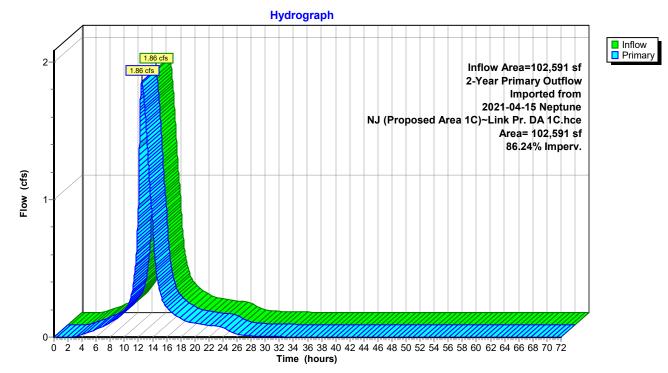
### Link 3L: Pr. POA 1

### Summary for Link 4L: Pr DA 1C Total

Inflow Are	a =	102,591 sf, 86.24% Impervious, Inflow Depth = 2.54" for 2-Year ev	vent
Inflow	=	1.86 cfs @ 12.47 hrs, Volume= 21,678 cf	
Primary	=	1.86 cfs @ 12.47 hrs, Volume= 21,678 cf, Atten= 0%, Lag= 0	).0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

2-Year Primary Outflow Imported from 2021-04-15 Neptune, NJ (Proposed Area 1C)~Link Pr. DA 1C.hce



### Link 4L: Pr DA 1C Total

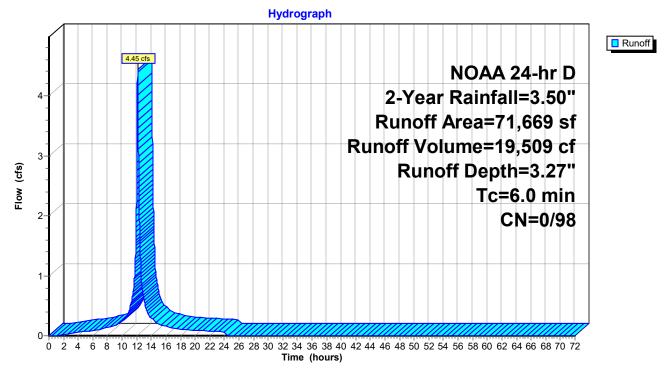
### Summary for Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.

Runoff = 4.45 cfs @ 12.14 hrs, Volume= 19,509 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Area (sf)	CN Description	
71,669	98 Paved parking, HSG D	
71,669	98 100.00% Impervious Area	
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)	
6.0	Direct Entry,	

### Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.



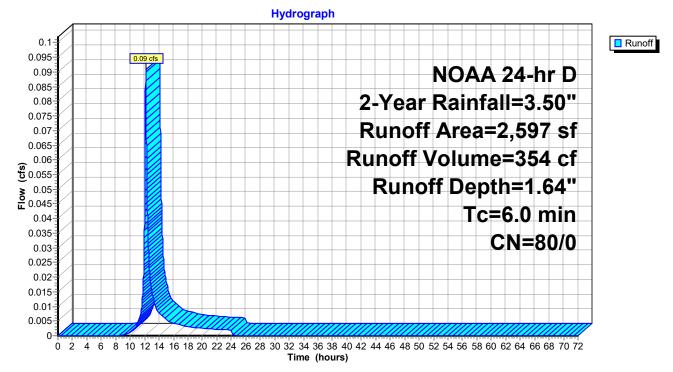
### Summary for Subcatchment B 1Cp: PR. BASIN Area 1C Perv.

Runoff = 0.09 cfs @ 12.14 hrs, Volume= 354 cf, Depth= 1.64"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Area (sf)	CN E	N Description			
2,597	80 >	80 >75% Grass cover, Good, HSG D			
2,597	80 1	00.00% Pe	ervious Are	ea	
Tc Length (min) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0				Direct Entry,	

### Subcatchment B 1Cp: PR. BASIN Area 1C Perv.



### Summary for Subcatchment DW 1Ci: Pr. Drywell 1C Imp.

Runoff = 0.28 cfs @ 12.14 hrs, Volume= 1,229 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

4,515	98 Paved p	arking, HSG D		
4,515	98 100.00%	6 Impervious Are	a	
Tc Length min) (feet)	Slope Veloc (ft/ft) (ft/se		Description	
6.0			Direct Entry,	
	Subc	atchment DW	1Ci: Pr. Drywell 1C Imp.	
	Cube	Hydrogi	•	
0.3	0.28 cfs			Runot
0.28			NOAA 24-hr D	
0.26			2-Year Rainfall=3.50"	*****
0.22			Runoff Area=4,515 sf	
0.2			Runoff Volume=1,229 cf	
<b>6</b> 0.18			Runoff Depth=3.27"	
S 0.18 0.16 0.16			Tc=6.0 min	
0.12			CN=0/98	#######
0.1			CIN-0/98	
0.08				
0.06				_
0.04				

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

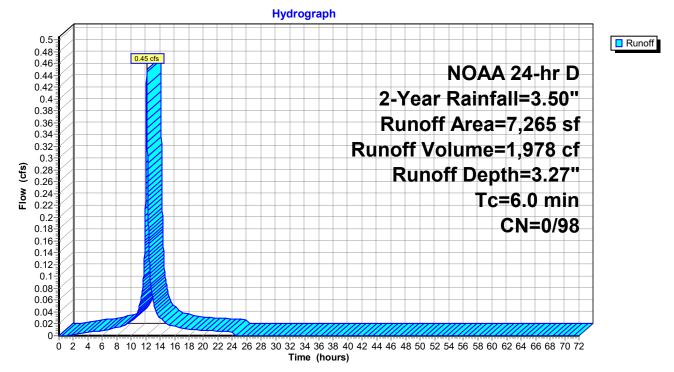
#### Summary for Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.

Runoff = 0.45 cfs @ 12.14 hrs, Volume= 1,978 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

A	rea (sf)	CN	CN Description			
	7,265	98	8 Paved parking, HSG D			
	7,265	98	100.00% Im	npervious A	Area	
Tc (min)	Length (feet)	Slop (ft/ff		Capacity (cfs)	1	
6.0					Direct Entry,	

### Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.



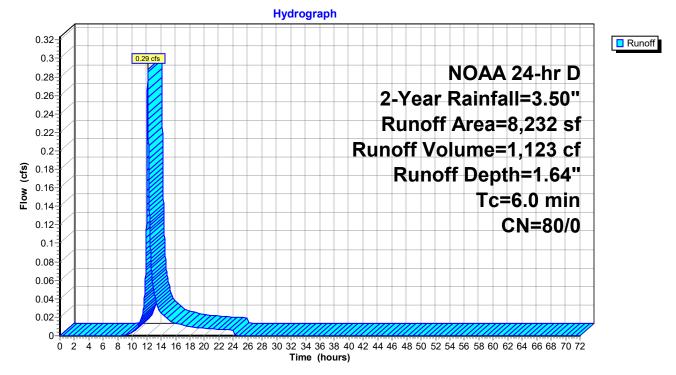
### Summary for Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.

0.29 cfs @ 12.14 hrs, Volume= 1,123 cf, Depth= 1.64" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Area (sf)	CN Description			
8,232	80 >75% Grass cover, Good, HSG D			
8,232	80 100.00% Pervious Area			
Tc Length (min) (feet)				
6.0	Direct Entry,			

### Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.



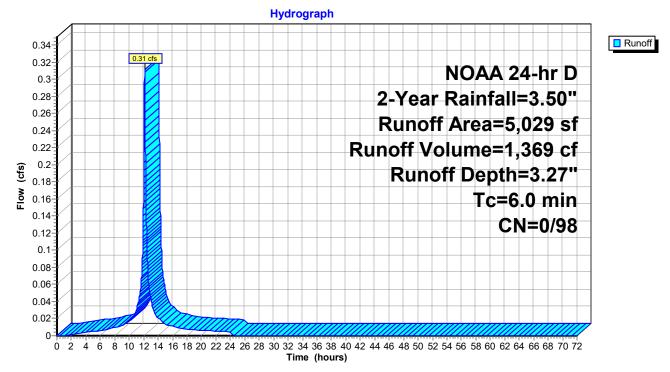
#### Summary for Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.

Runoff = 0.31 cfs @ 12.14 hrs, Volume= 1,369 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

5,029 98 Paved parking, HSG D
5,029 98 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.0 Direct Entry,

### Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.



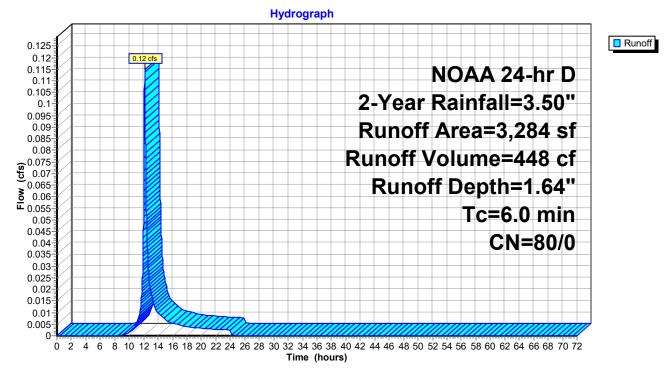
### Summary for Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.

Runoff = 0.12 cfs @ 12.14 hrs, Volume= 448 cf, Depth= 1.64"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"

Area (sf)	CN	Description			
3,284	80	80 >75% Grass cover, Good, HSG D			
3,284	80	100.00% P	ervious Are	ea	
Tc Length (min) (feet)	Slope (ft/ft	,	Capacity (cfs)	Description	
6.0				Direct Entry,	

### Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.



### Summary for Pond B 1C: Underground Basin 1C

Inflow Area =	102,591 sf, 86.24% Impervious,	Inflow Depth = 2.54" for 2-Year event
Inflow =	5.57 cfs @ 12.15 hrs, Volume=	21,704 cf
Outflow =	1.86 cfs @ 12.47 hrs, Volume=	21,678 cf, Atten= 67%, Lag= 19.3 min
Primary =	1.86 cfs @ 12.47 hrs, Volume=	21,678 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 20.02' @ 12.47 hrs Surf.Area= 10,459 sf Storage= 5,947 cf

Plug-Flow detention time= 72.1 min calculated for 21,678 cf (100% of inflow) Center-of-Mass det. time= 71.3 min (833.2 - 761.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	8,886 cf	53.78'W x 194.47'L x 3.52'H Field A
			36,868 cf Overall - 14,653 cf Embedded = 22,215 cf x 40.0% Voids
#2A	19.60'	14,040 cf	Contech ChamberMaxx 2016 x 297 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		22,925 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=1.86 cfs @ 12.47 hrs HW=20.02' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 1.86 cfs of 3.12 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 1.86 cfs @ 3.41 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Pond B 1C: Underground Basin 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

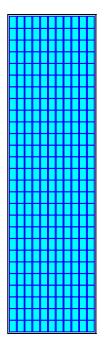
27 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 192.47' Row Length +12.0" End Stone x 2 = 194.47' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

297 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 14,039.6 cf Chamber Storage 297 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 14,653.5 cf Displacement

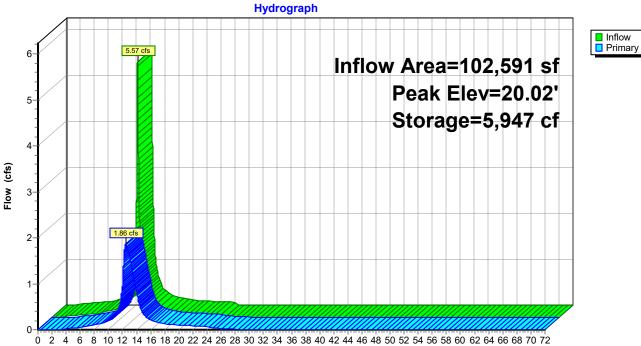
36,868.2 cf Field - 14,653.5 cf Chambers = 22,214.7 cf Stone x 40.0% Voids = 8,885.9 cf Stone Storage

Chamber Storage + Stone Storage = 22,925.5 cf = 0.526 af Overall Storage Efficiency = 62.2% Overall System Size = 194.47' x 53.78' x 3.52'

297 Chambers 1,365.5 cy Field 822.8 cy Stone



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### Pond B 1C: Underground Basin 1C

32 34 36 38 40 42 Time (hours)

### Summary for Pond DW1C: Drywell 1C

Inflow Area =	4,515 sf,100.00% Impervious,	Inflow Depth = 3.27" for 2-Year event
Inflow =	0.28 cfs @ 12.14 hrs, Volume=	1,229 cf
Outflow =	0.01 cfs @ 14.79 hrs, Volume=	1,229 cf, Atten= 95%, Lag= 159.1 min
Discarded =	0.01 cfs @ 14.79 hrs, Volume=	1,229 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.34' @ 14.79 hrs Surf.Area= 497 sf Storage= 613 cf

Plug-Flow detention time= 399.5 min calculated for 1,229 cf (100% of inflow) Center-of-Mass det. time= 399.5 min (1,159.5 - 760.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	22.40'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 22.40' / 21.00' S= 0.0304 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	1.000 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 14.79 hrs HW=22.34' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=20.50' TW=19.10' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.00 cfs)

## Pond DW1C: Drywell 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

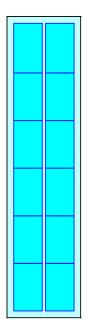
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

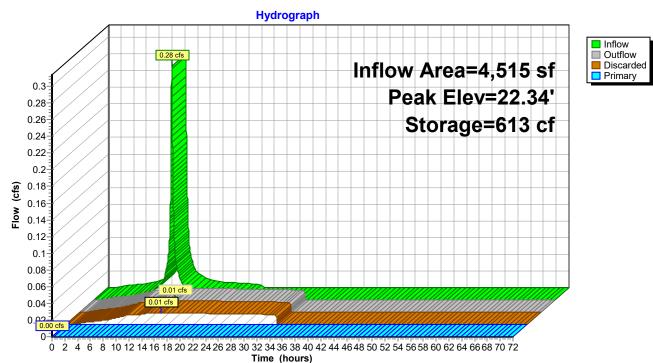
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1C: Drywell 1C

## Summary for Pond RG 1C: Rain Garden 1C

Inflow Area =	15,497 sf, 46.88% Impervious,	Inflow Depth = 2.40" for 2-Year event
Inflow =	0.74 cfs @ 12.14 hrs, Volume=	3,100 cf
Outflow =	0.73 cfs @ 12.16 hrs, Volume=	3,100 cf, Atten= 2%, Lag= 1.0 min
Discarded =	0.03 cfs @ 12.16 hrs, Volume=	1,859 cf
Primary =	0.70 cfs @ 12.16 hrs, Volume=	1,241 cf

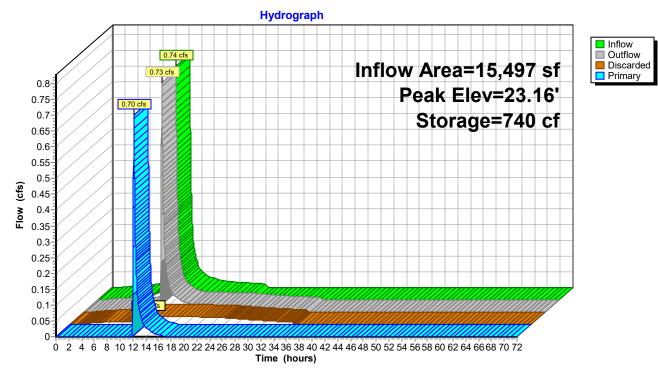
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.16' @ 12.16 hrs Surf.Area= 1,051 sf Storage= 740 cf

Plug-Flow detention time= 208.8 min calculated for 3,100 cf (100% of inflow) Center-of-Mass det. time= 208.8 min (1,002.4 - 793.6)

Volume	Inver	t Avail.S	torage	Storage Description	ו	
#1	22.00	' 1,	964 cf	Custom Stage Data	a (Irregular) Listed	below (Recalc)
Elevatio (fee 22.0 23.0	et) 00 00	urf.Area (sq-ft) 305 922	Perim. (feet) 88.0 137.0	Inc.Store (cubic-feet) 0 586	Cum.Store (cubic-feet) 0 586	Wet.Area (sq-ft) 305 1,190
24.0	00	1,893	201.0	1,379	1,964	2,919
Device	Routing	Inver	t Outle	et Devices		
#1	Primary	20.45	L= 3 Inlet	" <b>Round Culvert</b> 7.0' RCP, sq.cut en / Outlet Invert= 20.4 .013, Flow Area= 1.	5' / 20.25' S= 0.00	
#2	Device 1	23.10		" <b>x 48.0" Horiz. Orifi</b> red to weir flow at low		0
#3	Discarded	22.00	)' 1.00	<b>0 in/hr Exfiltration o</b> ductivity to Groundwa	ver Surface area	54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.03 cfs @ 12.16 hrs HW=23.16' (Free Discharge) **3=Exfiltration** (Controls 0.03 cfs)

**Primary OutFlow** Max=0.70 cfs @ 12.16 hrs HW=23.16' TW=19.82' (Dynamic Tailwater) -1=Culvert (Passes 0.70 cfs of 8.25 cfs potential flow) -2=Orifice/Grate (Weir Controls 0.70 cfs @ 0.78 fps)



## Pond RG 1C: Rain Garden 1C

#### Summary for Pond RG 2C: Rain Garden 2C

Inflow Area =	8,313 sf, 60.50% Impervious,	Inflow Depth = 2.62" for 2-Year event
Inflow =	0.43 cfs @ 12.14 hrs, Volume=	1,817 cf
Outflow =	0.41 cfs @ 12.17 hrs, Volume=	1,817 cf, Atten= 5%, Lag= 1.6 min
Discarded =	0.02 cfs @ 12.17 hrs, Volume=	1,217 cf
Primary =	0.39 cfs @ 12.17 hrs, Volume=	600 cf

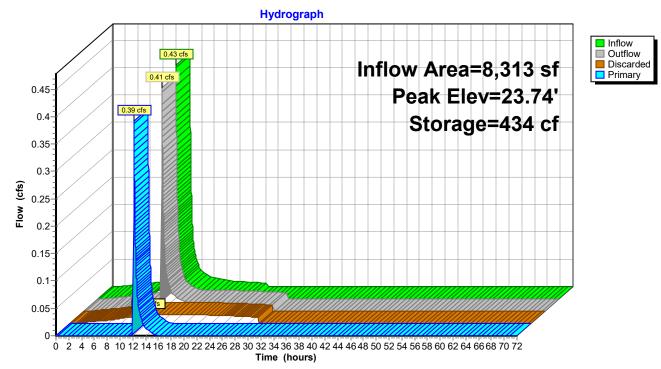
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.74' @ 12.17 hrs Surf.Area= 692 sf Storage= 434 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 172.8 min (955.7 - 782.8)

Volume Invert		t Avail.S	Storage	Storage Description	n					
#1	23.00	1	842 cf	f Custom Stage Data (Irregular) Listed below (Recalc)						
Elevatio (fee 23.0 24.0 24.2	20 20 20	urf.Area (sq-ft) 489 773 1,429	Perim. (feet) 105.0 138.0 180.0	Inc.Store (cubic-feet) 0 626 217	Cum.Store (cubic-feet) 0 626 842	Wet.Area (sq-ft) 489 1,139 2,202				
Device	Routing	Inve	ert Outle	et Devices						
#1	Primary	21.4	L= 6		nd projecting, Ke= (					
				Inlet / Outlet Invert= 21.40' / 21.06' S= 0.0055 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf						
#2 Device 1		23.7		<b>.0" x 48.0" Horiz. Orifice/Grate</b> C= 0.600 nited to weir flow at low heads						
#3	#3 Discarded 23.00'			<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.50'						
			0011			•				

**Discarded OutFlow** Max=0.02 cfs @ 12.17 hrs HW=23.74' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=0.39 cfs @ 12.17 hrs HW=23.74' TW=19.84' (Dynamic Tailwater) **1=Culvert** (Passes 0.39 cfs of 4.40 cfs potential flow) **2=Orifice/Grate** (Weir Controls 0.39 cfs @ 0.64 fps)

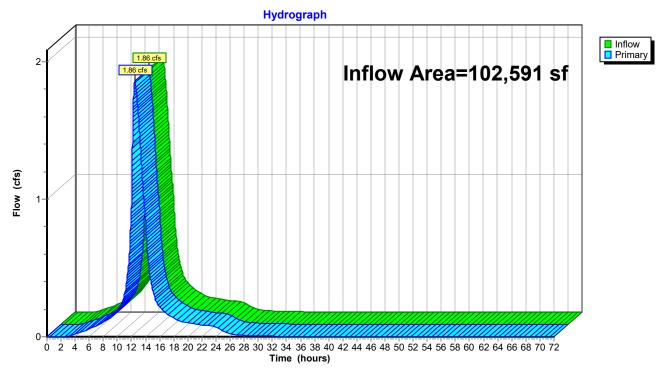


## Pond RG 2C: Rain Garden 2C

## Summary for Link Pr. DA 1C: Pr. DA 1C

Inflow Area	a =	102,591 sf,	86.24% Imp	ervious,	Inflow Depth =	2.54"	for 2-Year event
Inflow	=	1.86 cfs @	12.47 hrs, V	'olume=	21,678 c	f	
Primary	=	1.86 cfs @	12.47 hrs, V	'olume=	21,678 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



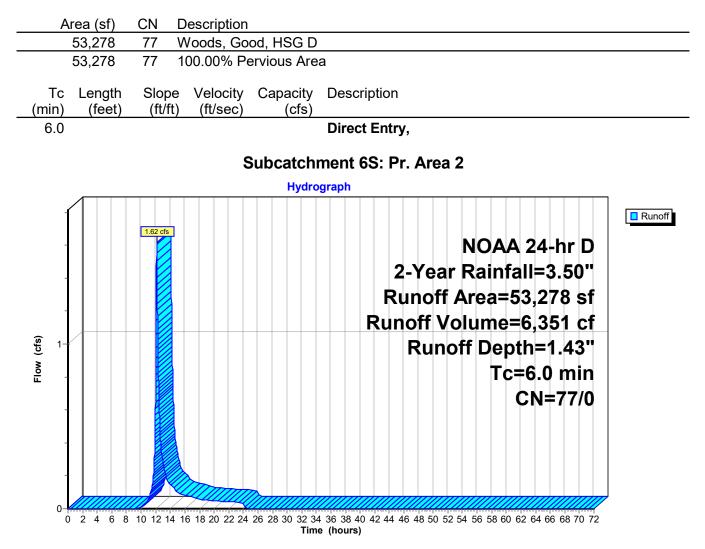
## Link Pr. DA 1C: Pr. DA 1C

## Summary for Subcatchment 6S: Pr. Area 2

Page 1

Runoff 1.62 cfs @ 12.15 hrs, Volume= 6,351 cf, Depth= 1.43" =

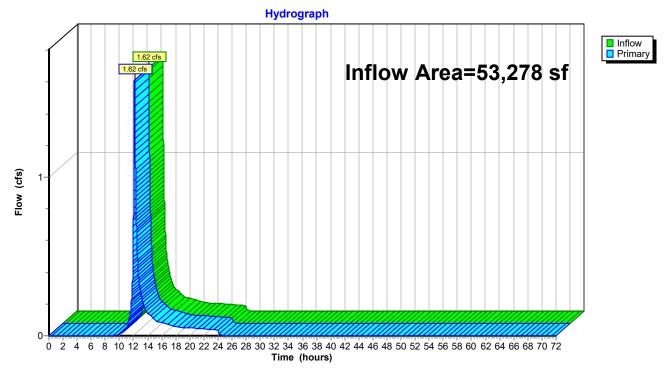
Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.50"



## Summary for Link 7L: Pr. POA 2 (Reforestation)

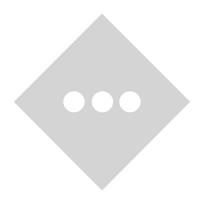
Inflow Area	=	53,278 sf,	0.00% Impervious,	Inflow Depth = 1.43"	for 2-Year event
Inflow	=	1.62 cfs @ 1	12.15 hrs, Volume=	6,351 cf	
Primary	=	1.62 cfs @ 1	12.15 hrs, Volume=	6,351 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Link 7L: Pr. POA 2 (Reforestation)

# **APPENDIX C-3 I0-YEAR STORM EVENT HYDROGRAPHS**



#### Summary for Subcatchment 1S: Ex. Area 1A Perv.

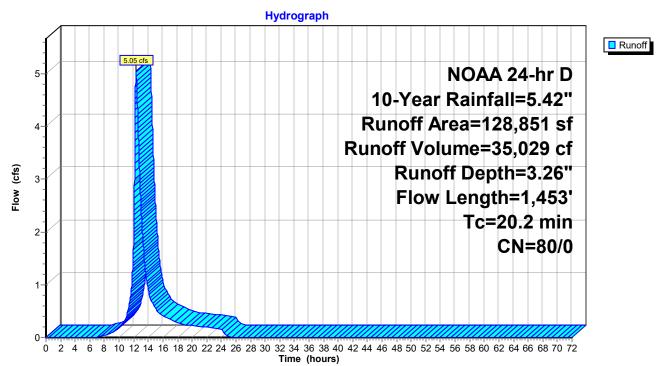
Page 1

5.05 cfs @ 12.34 hrs, Volume= 35,029 cf, Depth= 3.26" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

_	Ai	rea (sf)	CN [	Description		
	1	15,266	80 >	>75% Gras	s cover, Go	ood, HSG D
_		13,585	77 \	Noods, Go	od, HSG D	
128,851 80			80 \	Neighted A	verage	
	,		80 ´	100.00% Pe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.0	100	0.0600	0.19		Sheet Flow, Sheet Flow
						Grass: Dense n= 0.240 P2= 3.34"
	0.5	75	0.0270	2.65		Shallow Concentrated Flow, Shallow Concentrated
						Unpaved Kv= 16.1 fps
_	10.6	1,278		2.00		Direct Entry, Channel Flow
	20.2	1,453	Total			

#### Subcatchment 1S: Ex. Area 1A Perv.



## Summary for Subcatchment 2S: Ex. Area 1A Imp.

Runoff = 13.15 cfs @ 12.14 hrs, Volume= 58,753 cf, Depth= 5.18"

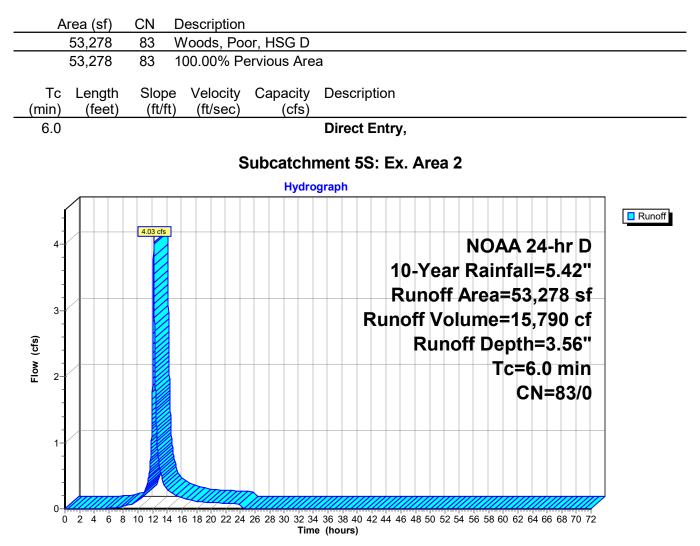
Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

	136,039		aved park														
1	136,039	98 1	00.00% In	npervio	ous A	rea											
Тс	Length	Slope	Velocity	Capa		Des	cripti	on									
min)	(feet)	(ft/ft)	(ft/sec)	(	(cfs)												
6.0						Dire	ect E	ntry,									
			Sub	catch	mer	nt 28	: Ex	. Ar	ea 1	AI	mp.	_					
					Hydro							•					
ſ																	Runof
14		13.15 cfs															
13											N	DA	A	24-	hr	D	
12								10	)-Ye	ear	Ra	in	Ifal	I=5	5.42	2"	
11-								Ru	nof	fΔ	rea	=	136	<b>3 0</b> ?	39	sf	
10														· ·			
9							R	unc						· .			
8									Ru	no	tt L	Je	pti	า=5	5.18	8	
													c=	6.0	m	in	-
6													C	N=	:0/9	98	-
5																	
4																	-
3																	-
2																	-
1			Tim														ļ
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## Summary for Subcatchment 5S: Ex. Area 2

Runoff = 4.03 cfs @ 12.14 hrs, Volume= 15,790 cf, Depth= 3.56"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"



#### Summary for Subcatchment 6S: Pr. Area 2

Page 4

Runoff 3.41 cfs @ 12.14 hrs, Volume= 13,222 cf, Depth= 2.98" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

	53,278 53,278			od, HSG D ervious Are	
	55,276	11 1		eivious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,
			S	Subcatch	ment 6S: Pr. Area 2
	_			Hydro	ograph
1					
1		3.41 cfs			NOAA 24-hr D
3-					10-Year Rainfall=5.42"
-					Runoff Area=53,278 sf
					Runoff Volume=13,222 cf
-10w (cts)					Runoff Depth=2.98"
FION					Tc=6.0 min
-					CN=77/0
1-					
-					
-					

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

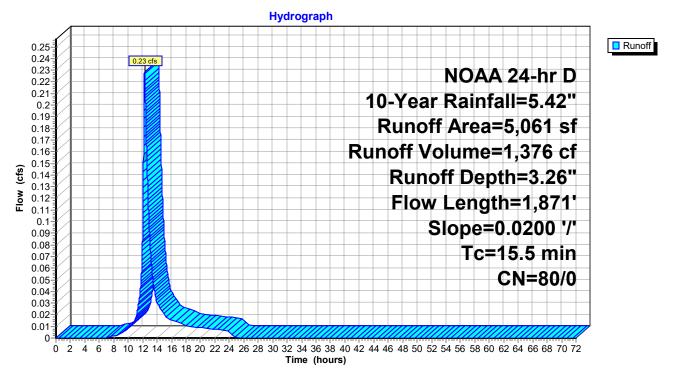
#### Summary for Subcatchment 10S: Ex. Area 1B Perv.

Runoff 0.23 cfs @ 12.26 hrs, Volume= 1,376 cf, Depth= 3.26" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

	A	rea (sf)	CN [	Description		
		5,061	80 >	>75% Gras	s cover, Go	ood, HSG D
		5,061	80 <i>´</i>	100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	1.2	100	0.0200	1.41		Sheet Flow,
	1.1	184	0.0200	2.87		Smooth surfaces n= 0.011 P2= 3.34" <b>Shallow Concentrated Flow,</b> Paved Kv= 20.3 fps
_	13.2	1,587		2.00		Direct Entry,
	15.5	1,871	Total			

#### Subcatchment 10S: Ex. Area 1B Perv.



## Summary for Subcatchment 11S: Ex. Area 1B Imp.

Runoff = 1.93 cfs @ 12.14 hrs, Volume= 8,603 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

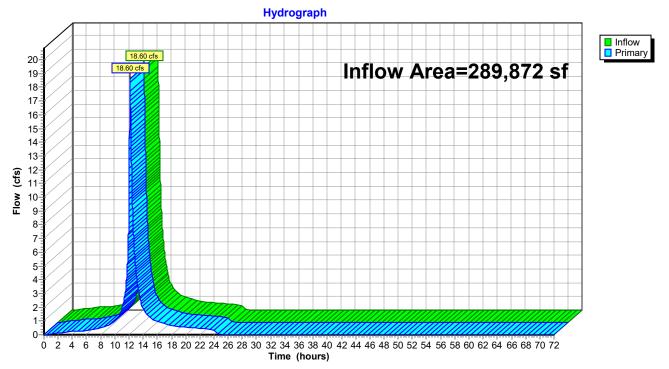
<u> </u>	98Paved parking, HSG D98100.00% Impervious Area	
Tc Length min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)	
6.0	Direct Entry,	
	Subcatchment 11S: Ex. Area 1B Imp.	
	Hydrograph	
2-	NOAA 24-hr D 10-Year Rainfall=5.42" Runoff Area=19,921 sf Runoff Volume=8,603 cf Runoff Depth=5.18"	Runot
	Tc=6.0 min CN=0/98	

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

## Summary for Link 3L: EX POA 1 (Construction)

Inflow Are	a =	289,872 sf, 53.80% Impervious, Inflow Depth = 4.30" for 10-Year event	
Inflow	=	18.60 cfs @ 12.15 hrs, Volume= 103,761 cf	
Primary	=	18.60 cfs @ 12.15 hrs, Volume= 103,761 cf, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

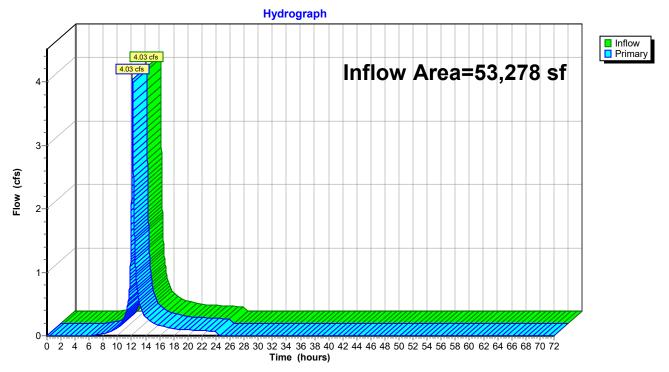


## Link 3L: EX POA 1 (Construction)

## Summary for Link 4L: EX POA 2 (Reforestation)

Inflow Are	a =	53,278 sf,	0.00% Impervious,	Inflow Depth = 3.56"	for 10-Year event
Inflow	=	4.03 cfs @ 1	12.14 hrs, Volume=	15,790 cf	
Primary	=	4.03 cfs @ 1	12.14 hrs, Volume=	15,790 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

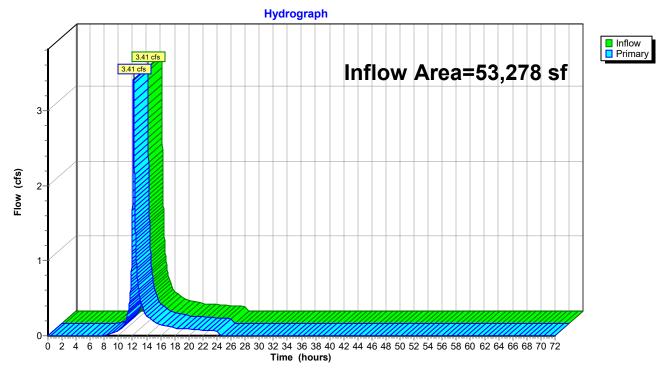


## Link 4L: EX POA 2 (Reforestation)

## Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Area	a =	53,278 sf,	0.00% Impervious,	Inflow Depth = 2.98"	for 10-Year event
Inflow	=	3.41 cfs @ 1	2.14 hrs, Volume=	13,222 cf	
Primary	=	3.41 cfs @ 1	2.14 hrs, Volume=	13,222 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Link 7L: Pr. POA 2 (Reforestation)

#### Summary for Subcatchment 6S: Pr. Area 2

Page 1

Runoff 3.41 cfs @ 12.14 hrs, Volume= 13,222 cf, Depth= 2.98" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

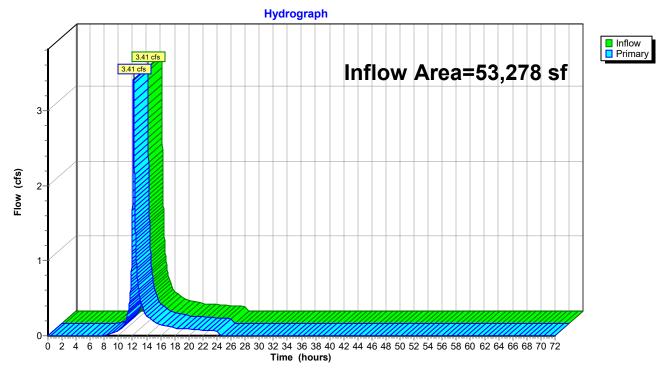
53,278	77 Woods, Good, HSG D
53,278	77 100.00% Pervious Area
Tc Length min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
6.0	Direct Entry,
	Subcatchment 6S: Pr. Area 2
	Hydrograph
	3.41 cfs NOAA 24-hr D
3-	10-Year Rainfall=5.42" Runoff Area=53,278 sf
	Runoff Volume=13,222 cf
2-1 0	Runoff Depth=2.98"
	Tc=6.0 min CN=77/0

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

## Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Are	a =	53,278 sf,	0.00% Impervious,	Inflow Depth = 2.98"	for 10-Year event
Inflow	=	3.41 cfs @ 1	2.14 hrs, Volume=	13,222 cf	
Primary	=	3.41 cfs @ 1	2.14 hrs, Volume=	13,222 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Link 7L: Pr. POA 2 (Reforestation)

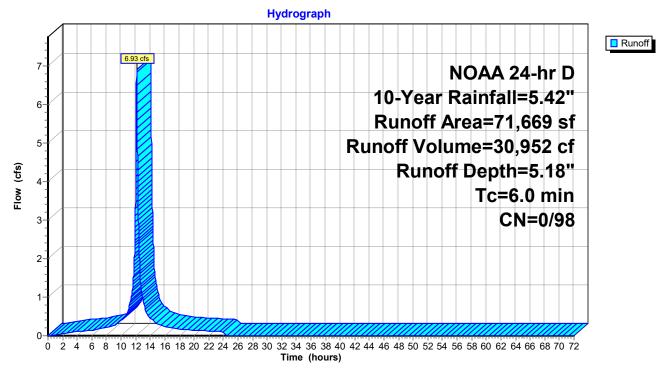
## Summary for Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.

Runoff = 6.93 cfs @ 12.14 hrs, Volume= 30,952 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area	a (sf)	CN [	Description		
71	,669	98 F	Paved park	ing, HSG D	
71	,669	98 ´	100.00% Im	npervious A	Area
Tc L (min)	.ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

## Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.



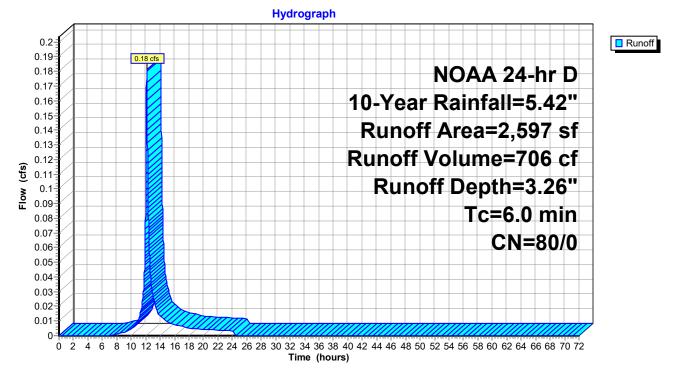
#### Summary for Subcatchment B 1Cp: PR. BASIN Area 1C Perv.

Runoff = 0.18 cfs @ 12.14 hrs, Volume= 706 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN [	Description					
	2,597	80 >	80 >75% Grass cover, Good, HSG D					
	2,597	80 <i>´</i>	100.00% Pe	ervious Are	ea			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)				
6.0					Direct Entry,			

## Subcatchment B 1Cp: PR. BASIN Area 1C Perv.



#### Summary for Subcatchment DW 1Ci: Pr. Drywell 1C Imp.

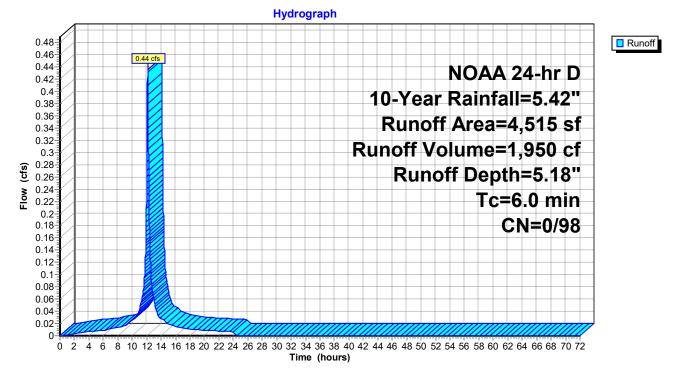
Page 3

1,950 cf, Depth= 5.18" Runoff 0.44 cfs @ 12.14 hrs, Volume= =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description		
	4,515	98	Paved park	ing, HSG D	D
	4,515	98	100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	
6.0					Direct Entry,

## Subcatchment DW 1Ci: Pr. Drywell 1C Imp.



## Summary for Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.

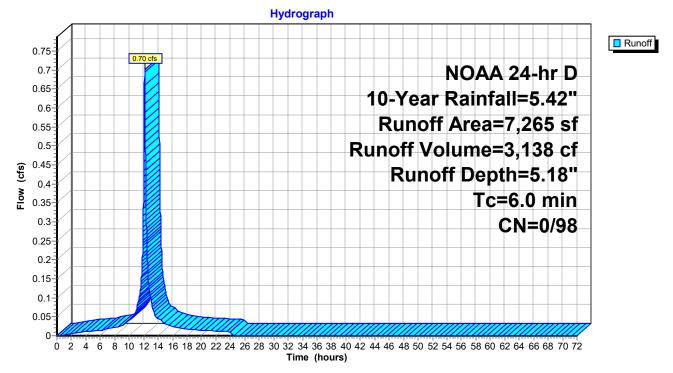
Page 4

0.70 cfs @ 12.14 hrs, Volume= 3,138 cf, Depth= 5.18" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description		
	7,265	98	Paved park	ing, HSG D	
	7,265	98	100.00% Im	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
6.0					Direct Entry,

## Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.



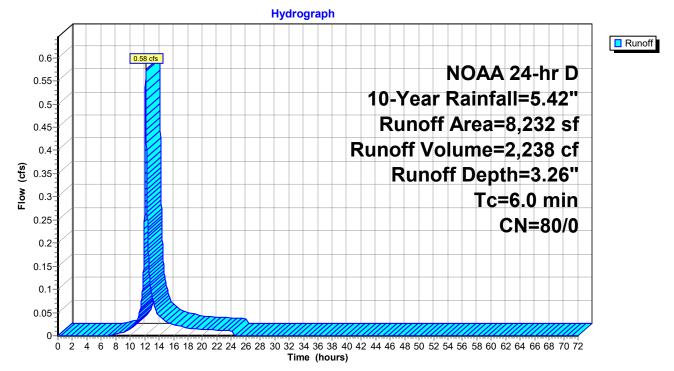
## Summary for Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.

Runoff = 0.58 cfs @ 12.14 hrs, Volume= 2,238 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description					
	8,232	80	80 >75% Grass cover, Good, HSG D					
	8,232	80	100.00% Pe	ervious Are	ea			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

## Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.



## Summary for Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.

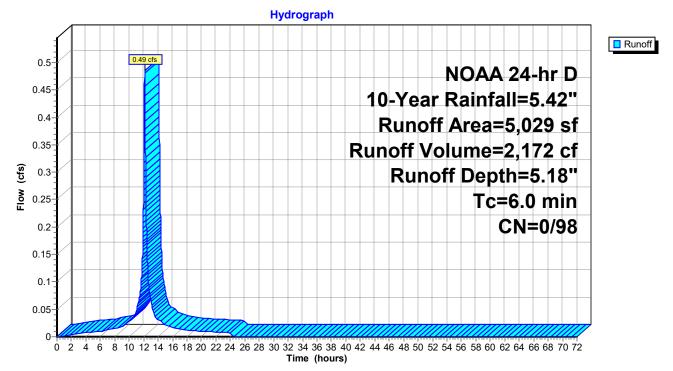
Page 6

0.49 cfs @ 12.14 hrs, Volume= 2,172 cf, Depth= 5.18" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description					
	5,029	98	98 Paved parking, HSG D					
	5,029	98	100.00% Im	npervious A	vrea			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

#### Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.



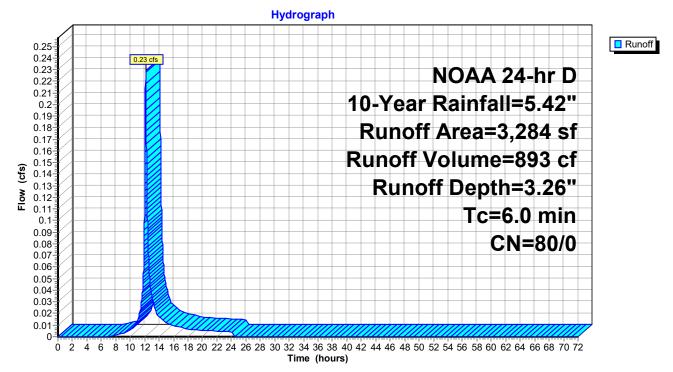
#### Summary for Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.

Runoff = 0.23 cfs @ 12.14 hrs, Volume= 893 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description					
	3,284	80	0 >75% Grass cover, Good, HSG D					
	3,284	80	100.00% Pe	ervious Are	ea			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

#### Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.



## Summary for Pond B 1C: Underground Basin 1C

Inflow Area =	102,591 sf, 86.24% Impervious,	Inflow Depth = 4.32" for 10-Year event
Inflow =	9.02 cfs @ 12.14 hrs, Volume=	36,963 cf
Outflow =	2.75 cfs @ 12.52 hrs, Volume=	36,937 cf, Atten= 70%, Lag= 22.9 min
Primary =	2.75 cfs @ 12.52 hrs, Volume=	36,937 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 20.57' @ 12.52 hrs Surf.Area= 10,459 sf Storage= 10,762 cf

Plug-Flow detention time= 66.5 min calculated for 36,932 cf (100% of inflow) Center-of-Mass det. time= 66.3 min ( 822.3 - 756.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	8,886 cf	53.78'W x 194.47'L x 3.52'H Field A
			36,868 cf Overall - 14,653 cf Embedded = 22,215 cf x 40.0% Voids
#2A	19.60'	14,040 cf	Contech ChamberMaxx 2016 x 297 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		22,925 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=2.75 cfs @ 12.52 hrs HW=20.57' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 2.75 cfs of 6.44 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 2.70 cfs @ 4.94 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 0.05 cfs @ 0.74 fps)

## Pond B 1C: Underground Basin 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

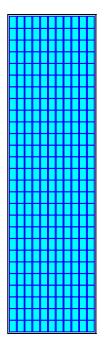
27 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 192.47' Row Length +12.0" End Stone x 2 = 194.47' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

297 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 14,039.6 cf Chamber Storage 297 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 14,653.5 cf Displacement

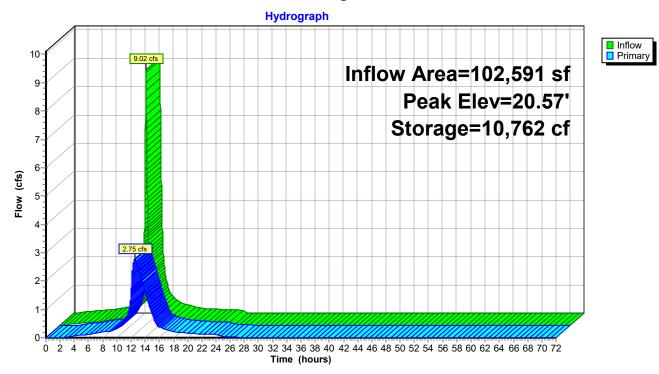
36,868.2 cf Field - 14,653.5 cf Chambers = 22,214.7 cf Stone x 40.0% Voids = 8,885.9 cf Stone Storage

Chamber Storage + Stone Storage = 22,925.5 cf = 0.526 af Overall Storage Efficiency = 62.2% Overall System Size = 194.47' x 53.78' x 3.52'

297 Chambers 1,365.5 cy Field 822.8 cy Stone



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## Pond B 1C: Underground Basin 1C

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## Summary for Pond DW1C: Drywell 1C

Inflow Area =	4,515 sf,100.00% Impervious,	Inflow Depth = 5.18" for 10-Year event
Inflow =	0.44 cfs @ 12.14 hrs, Volume=	1,950 cf
Outflow =	0.23 cfs @ 12.33 hrs, Volume=	1,950 cf, Atten= 48%, Lag= 11.5 min
Discarded =	0.01 cfs @ 12.33 hrs, Volume=	1,455 cf
Primary =	0.21 cfs @ 12.33 hrs, Volume=	495 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.61' @ 12.33 hrs Surf.Area= 497 sf Storage= 706 cf

Plug-Flow detention time= 326.7 min calculated for 1,950 cf (100% of inflow) Center-of-Mass det. time= 326.7 min (1,078.2 - 751.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	22.40'	15.0" Round Culvert
			L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 22.40' / 21.00' S= 0.0304 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.33 hrs HW=22.61' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.21 cfs @ 12.33 hrs HW=22.61' TW=20.49' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.21 cfs @ 1.56 fps)

## Pond DW1C: Drywell 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

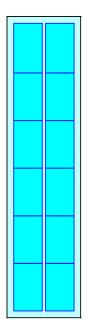
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

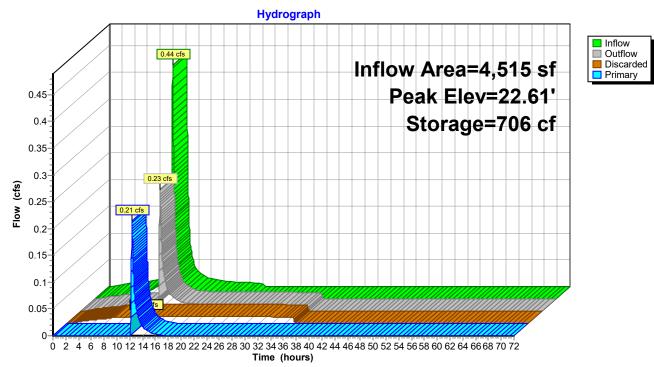
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1C: Drywell 1C

#### Summary for Pond RG 1C: Rain Garden 1C

Inflow Area =	15,497 sf, 46.88% Impervious,	Inflow Depth = 4.16" for 10-Year event
Inflow =	1.28 cfs @ 12.14 hrs, Volume=	5,376 cf
Outflow =	1.26 cfs @ 12.15 hrs, Volume=	5,376 cf, Atten= 1%, Lag= 0.8 min
Discarded =	0.03 cfs @ 12.15 hrs, Volume=	2,174 cf
Primary =	1.24 cfs @ 12.15 hrs, Volume=	3,202 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.18' @ 12.15 hrs Surf.Area= 1,073 sf Storage= 768 cf

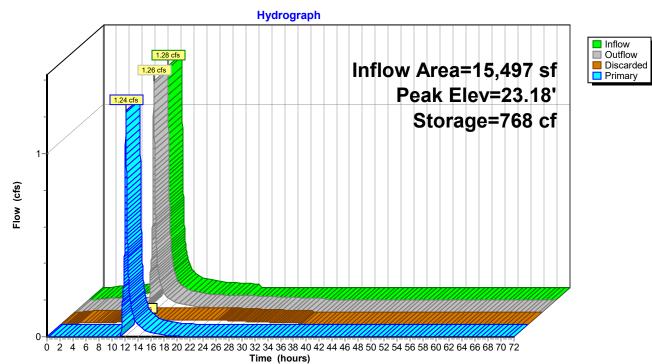
Plug-Flow detention time= 148.8 min calculated for 5,375 cf (100% of inflow) Center-of-Mass det. time= 148.8 min (933.3 - 784.5)

Volume	Invert	t Avail.St	torage	Storage Description	า	
#1	22.00	' 1,	964 cf	Custom Stage Data	a (Irregular) Listed	below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
22.0		305	88.0	0	0	305
23.0	00	922	137.0	586	586	1,190
24.0	00	1,893	201.0	1,379	1,964	2,919
Device #1 #2	Routing Primary Device 1	Inver 20.45 23.10	5 <b>15.0</b> L= 3 Inlet n= 0 <b>48.0</b>	et Devices <b>" Round Culvert</b> 7.0' RCP, sq.cut er / Outlet Invert= 20.4 .013, Flow Area= 1. <b>" x 48.0" Horiz. Orif</b> ited to weir flow at low	.5' / 20.25' S= 0.00 23 sf <b>ice/Grate</b> C= 0.60	054 '/' Cc= 0.900
#3	Discarded	22.00		0 in/hr Exfiltration of ductivity to Groundwa		54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.03 cfs @ 12.15 hrs HW=23.18' (Free Discharge) **3=Exfiltration** (Controls 0.03 cfs)

**Primary OutFlow** Max=1.24 cfs @ 12.15 hrs HW=23.18' TW=20.17' (Dynamic Tailwater)

-1=Culvert (Passes 1.24 cfs of 8.31 cfs potential flow) -2=Orifice/Grate (Weir Controls 1.24 cfs @ 0.94 fps)



## Pond RG 1C: Rain Garden 1C

#### Summary for Pond RG 2C: Rain Garden 2C

Inflow Area =	8,313 sf, 60.50% Impervious,	Inflow Depth = 4.42" for 10-Year event
Inflow =	0.72 cfs @ 12.14 hrs, Volume=	3,065 cf
Outflow =	0.71 cfs @ 12.15 hrs, Volume=	3,065 cf, Atten= 1%, Lag= 0.6 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,456 cf
Primary =	0.69 cfs @ 12.15 hrs, Volume=	1,608 cf

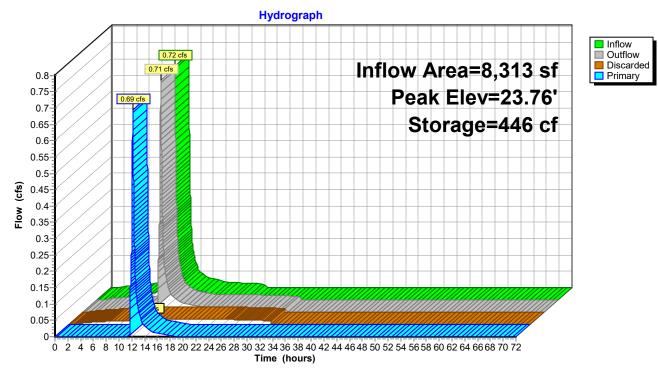
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.76' @ 12.15 hrs Surf.Area= 698 sf Storage= 446 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 132.1 min (906.7 - 774.6)

Volume	Inve	rt Avail.	.Storage	Storage Description	on		
#1	23.00	)'	842 cf	Custom Stage Da	<b>ata (Irregular)</b> Liste	d below (Recalc)	
Elevatio (fee 23.0 24.0 24.2	et) 00 00	Surf.Area (sq-ft) 489 773 1,429	Perim. (feet) 105.0 138.0 180.0	Inc.Store (cubic-feet) 0 626 217	Cum.Store (cubic-feet) 0 626 842	Wet.Area (sq-ft) 489 1,139 2,202	
Device	Routing	Inv	ert Outle	et Devices			
#1	Primary	21.	L= 6 Inlet		end projecting, Ke= .40' / 21.06' S= 0.( 0 79 sf		
#2	Device 1	23.	70' <b>48.0</b> '	" x 48.0" Horiz. Or ted to weir flow at h	ifice/Grate C= 0.0	600	
#3	Discardeo	d 23.	00' <b>1.00</b>	0 in/hr Exfiltration	<b>over Surface area</b> water Elevation = 6		

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=23.76' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=0.69 cfs @ 12.15 hrs HW=23.76' TW=20.16' (Dynamic Tailwater) -1=Culvert (Passes 0.69 cfs of 4.42 cfs potential flow) -2=Orifice/Grate (Weir Controls 0.69 cfs @ 0.77 fps)

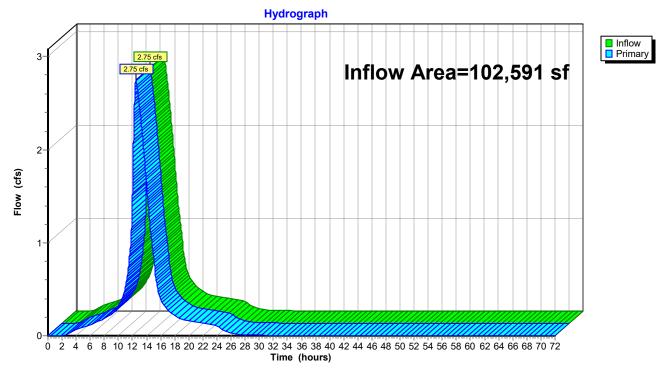


# Pond RG 2C: Rain Garden 2C

## Summary for Link Pr. DA 1C: Pr. DA 1C

Inflow Area =	102,59	1 sf, 86.24% Impervious	, Inflow Depth = 4.32"	for 10-Year event
Inflow =	2.75 cfs	@ 12.52 hrs, Volume=	36,937 cf	
Primary =	2.75 cfs	@ 12.52 hrs, Volume=	36,937 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



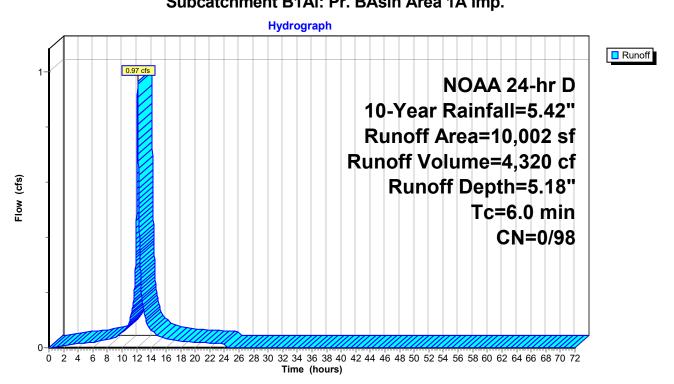
# Link Pr. DA 1C: Pr. DA 1C

### Summary for Subcatchment B1Ai: Pr. BAsin Area 1A Imp.

Runoff = 0.97 cfs @ 12.14 hrs, Volume= 4,320 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf)	CN Description
10,002	98 Paved parking, HSG D
10,002	98 100.00% Impervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
6.0	Direct Entry,
	Subcatchment B1Ai: Pr. BAsin Area 1A Imp.



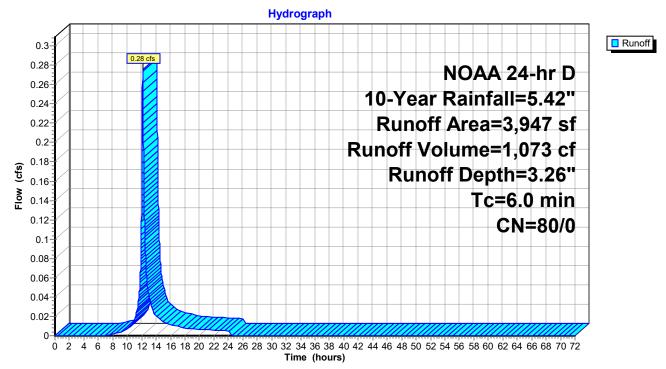
#### Summary for Subcatchment B1Ap: PR. Basin Area 1A Perv.

Runoff = 0.28 cfs @ 12.14 hrs, Volume= 1,073 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description				
	3,947	947 80 >75% Grass cover, Good, HSG D					
	3,947	80	100.00% Pe	ervious Are	ea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		

#### Subcatchment B1Ap: PR. Basin Area 1A Perv.



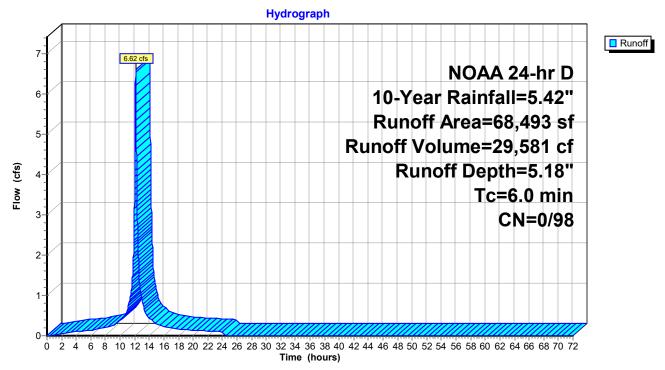
#### Summary for Subcatchment B1Bi: Pr. Basin Area 1B Imp.

Runoff = 6.62 cfs @ 12.14 hrs, Volume= 29,581 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf)	CN Description	
68,493	98 Paved parking, HSG D	
68,493	98 100.00% Impervious Area	
Tc Length (min) (feet) 6.0		
0.0	Direct Littry,	

#### Subcatchment B1Bi: Pr. Basin Area 1B Imp.



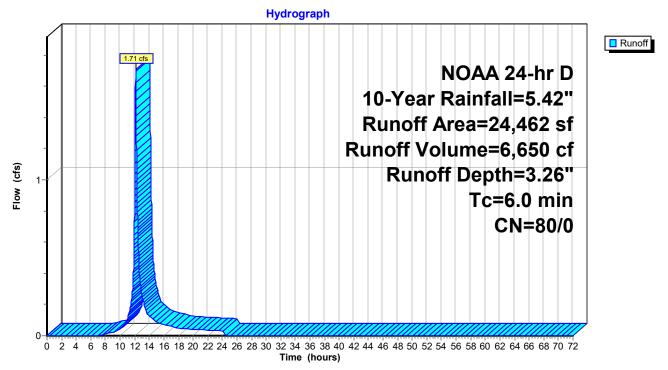
#### Summary for Subcatchment B1Bp: PR. Basin Area 1B Perv.

Runoff = 1.71 cfs @ 12.14 hrs, Volume= 6,650 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf)	CN Description
24,462	80 >75% Grass cover, Good, HSG D
24,462	80 100.00% Pervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
6.0	Direct Entry,

## Subcatchment B1Bp: PR. Basin Area 1B Perv.



0.085 0.08 0.075

<u>ද</u> 0.07 ව<u>්</u> 0.065

8 0.06 0.055

0.05 0.045

0.04 0.035 0.03 0.025 0.02 0.015 0.01 0.005

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## Summary for Subcatchment DA 1Di: Pr. Bypass 1D Imp

Runoff = 0.11 cfs @ 12.14 hrs, Volume= 488 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf)	CN Description	
1,131	98 Paved parking, HSG D	
1,131	98 100.00% Impervious Area	
Tc Length (min) (feet)		
6.0	Direct Entry,	
	Subcatchment DA 1Di: Pr. Bypass 1D Imp Hydrograph	
0.12		off
0.11	NOAA 24-hr D	
0.1 0.095 0.09	10-Year Rainfall=5.42"	

Runoff Area=1,131 sf

Runoff Depth=5.18"

Tc=6.0 min

CN=0/98

Runoff Volume=488 cf

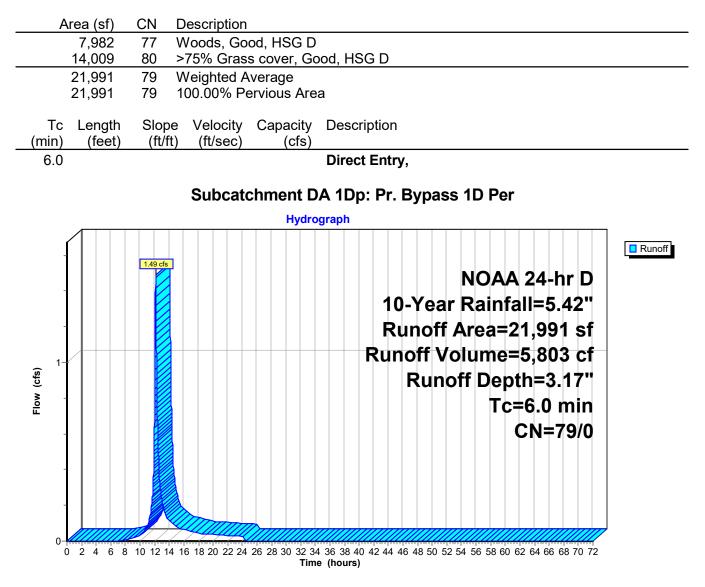
0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

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## Summary for Subcatchment DA 1Dp: Pr. Bypass 1D Per

Runoff = 1.49 cfs @ 12.14 hrs, Volume= 5,803 cf, Depth= 3.17"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"



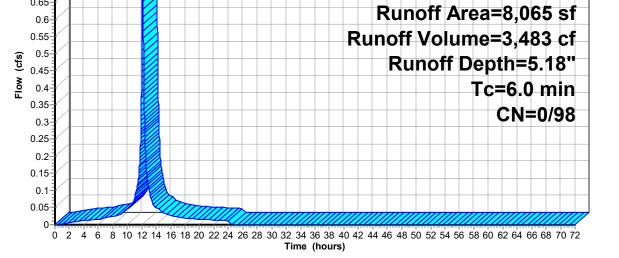
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## Summary for Subcatchment DW1Bi: Pr. Drywell Area 1B

Runoff = 0.78 cfs @ 12.14 hrs, Volume= 3,483 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf)	CN Description		
8,065	98 Paved parki	ng, HSG D	
8,065	98 100.00% Im	pervious Area	
Tc Length (min) (feet)	Slope Velocity (ft/ft) (ft/sec)	Capacity Description (cfs)	
6.0		Direct Entry,	
	Subcatc	hment DW1Bi: Pr. Drywell Area 1B	
	Subcatc	hment DW1Bi: Pr. Drywell Area 1B	<b>—</b>
0.85		•	Runoff
0.85	Subcatc	•	



0.14

0.12-0.1-0.08-0.06-0.04-0.02-0CN=0/98

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## Summary for Subcatchment DW2Bi: Pr. Drywell Area 2B

Runoff = 0.32 cfs @ 12.14 hrs, Volume= 1,432 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (sf) 3,316 3,316 Tc Length	CNDescription98Paved parking, HSG D98100.00% Impervious ASlopeVelocityCapacity	
(min) (feet)	(ft/ft) (ft/sec) (cfs)	
6.0		Direct Entry,
		W2Bi: Pr. Drywell Area 2B
0.34 0.32 0.3 0.28 0.26 0.24 0.22 0.22 0.22 0.18 0.16		NOAA 24-hr D 10-Year Rainfall=5.42" Runoff Area=3,316 sf Runoff Volume=1,432 cf Runoff Depth=5.18"
<b>ô</b> 문 0.16		Tc=6.0 min

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

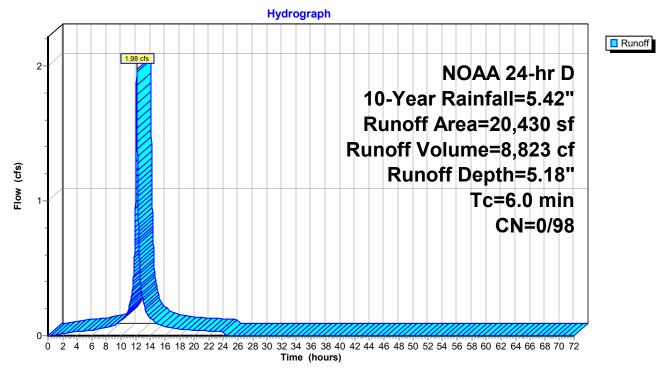
#### Summary for Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.

Runoff = 1.98 cfs @ 12.14 hrs, Volume= 8,823 cf, Depth= 5.18"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

Area (s	f) CN	Description		
20,43	0 98	Paved park	ing, HSG D	
20,43	0 98	100.00% Im	npervious A	Area
Tc Leng (min) (fe	/ /		Capacity (cfs)	Description
6.0				Direct Entry,

#### Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.



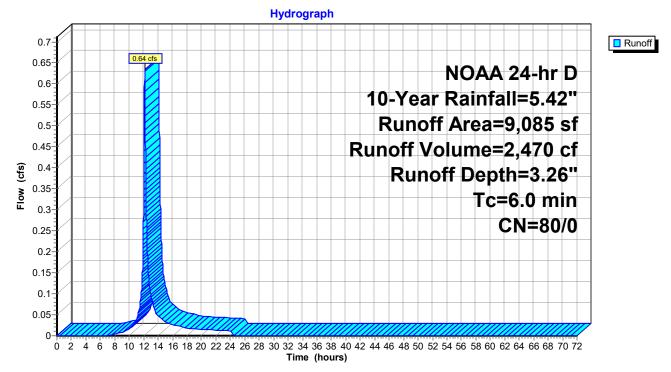
#### Summary for Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.

Runoff = 0.64 cfs @ 12.14 hrs, Volume= 2,470 cf, Depth= 3.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.42"

A	rea (sf)	CN	Description				
	9,085	80	>75% Gras	s cover, Go	ood, HSG D		
	9,085	80	100.00% Pe	ervious Are	а		
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

#### Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.



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#### Summary for Pond B1A: Underground Basin 1A

Inflow Area =	43,464 sf, 70.02% Impervious,	Inflow Depth = 2.45" for 10-Year event
Inflow =	2.48 cfs @ 12.26 hrs, Volume=	8,888 cf
Outflow =	1.00 cfs @ 12.57 hrs, Volume=	8,887 cf, Atten= 60%, Lag= 18.9 min
Primary =	1.00 cfs @ 12.57 hrs, Volume=	8,887 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.12' @ 12.57 hrs Surf.Area= 1,439 sf Storage= 1,980 cf

Plug-Flow detention time= 28.1 min calculated for 8,887 cf (100% of inflow) Center-of-Mass det. time= 28.0 min ( 803.0 - 775.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	1,318 cf	11.03'W x 130.42'L x 3.52'H Field A
			5,072 cf Overall - 1,778 cf Embedded = 3,295 cf x 40.0% Voids
#2A	19.60'	1,703 cf	Contech ChamberMaxx 2016 x 36 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		3,021 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	5.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	21.00'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=1.00 cfs @ 12.57 hrs HW=21.12' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 1.00 cfs of 8.89 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 0.88 cfs @ 6.48 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 0.11 cfs @ 0.96 fps)

## Pond B1A: Underground Basin 1A - Chamber Wizard Field A

# Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

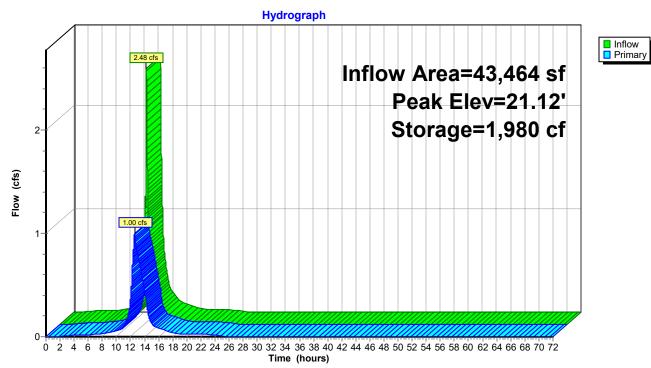
18 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 128.42' Row Length +12.0" End Stone x 2 = 130.42' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

36 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 1,703.2 cf Chamber Storage 36 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 1,777.6 cf Displacement

5,072.2 cf Field - 1,777.6 cf Chambers = 3,294.6 cf Stone x 40.0% Voids = 1,317.8 cf Stone Storage

Chamber Storage + Stone Storage = 3,021.0 cf = 0.069 af Overall Storage Efficiency = 59.6% Overall System Size = 130.42' x 11.03' x 3.52'

36 Chambers 187.9 cy Field 122.0 cy Stone



# Pond B1A: Underground Basin 1A

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#### Summary for Pond B1B: Underground Basin 1B

Inflow Area =	104,336 sf, 76.55% Impervious,	Inflow Depth = 4.39" for 10-Year event
Inflow =	9.08 cfs @ 12.14 hrs, Volume=	38,204 cf
Outflow =	3.40 cfs @ 12.44 hrs, Volume=	38,188 cf, Atten= 63%, Lag= 18.0 min
Primary =	3.40 cfs @ 12.44 hrs, Volume=	38,188 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 20.79' @ 12.44 hrs Surf.Area= 7,780 sf Storage= 9,334 cf

Plug-Flow detention time= 48.8 min calculated for 38,183 cf (100% of inflow) Center-of-Mass det. time= 48.9 min ( 815.6 - 766.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	6,625 cf	53.78'W x 144.65'L x 3.52'H Field A
			27,424 cf Overall - 10,861 cf Embedded = 16,563 cf x 40.0% Voids
#2A	19.60'	10,406 cf	Contech ChamberMaxx 2016 x 220 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		17,031 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
			L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=3.40 cfs @ 12.44 hrs HW=20.79' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 3.40 cfs of 7.69 cfs potential flow)

**—2=Orifice/Grate** (Orifice Controls 2.96 cfs @ 5.43 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 0.44 cfs @ 1.53 fps)

#### Pond B1B: Underground Basin 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

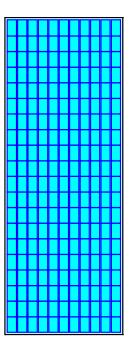
20 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 142.65' Row Length +12.0" End Stone x 2 = 144.65' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

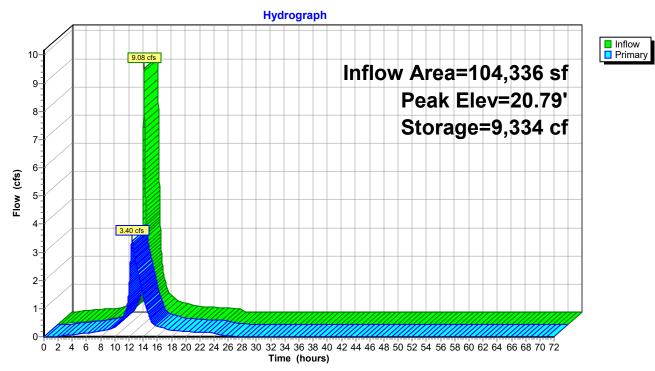
220 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 10,405.7 cf Chamber Storage 220 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 10,860.7 cf Displacement

27,423.7 cf Field - 10,860.7 cf Chambers = 16,563.0 cf Stone x 40.0% Voids = 6,625.2 cf Stone Storage

Chamber Storage + Stone Storage = 17,030.9 cf = 0.391 af Overall Storage Efficiency = 62.1% Overall System Size = 144.65' x 53.78' x 3.52'

220 Chambers 1,015.7 cy Field 613.4 cy Stone





# Pond B1B: Underground Basin 1B

2021-04-15 Neptune, NJ (Proposed 1)

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#### Summary for Pond DW1B: Drywell 1B

Inflow Area =	8,065 sf,100.00% Impervious,	Inflow Depth = 5.18" for 10-Year event
Inflow =	0.78 cfs @ 12.14 hrs, Volume=	3,483 cf
Outflow =	0.77 cfs @ 12.15 hrs, Volume=	3,483 cf, Atten= 1%, Lag= 0.8 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,857 cf
Primary =	0.76 cfs @ 12.15 hrs, Volume=	1,626 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 24.31' @ 12.15 hrs Surf.Area= 497 sf Storage= 892 cf

Plug-Flow detention time= 298.9 min calculated for 3,483 cf (100% of inflow) Center-of-Mass det. time= 299.0 min (1,050.6 - 751.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	21.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	22.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	23.90'	15.0" Round Culvert
	-		L= 67.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 23.90' / 21.00' S= 0.0433 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	21.50'	
			Conductivity to Groundwater Elevation = 12.75' Phase-In= 0.01'

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=24.31' (Free Discharge) **2=Exfiltration** (Controls 0.02 cfs)

Primary OutFlow Max=0.75 cfs @ 12.15 hrs HW=24.31' TW=20.33' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.75 cfs @ 2.17 fps)

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## Pond DW1B: Drywell 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

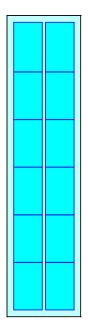
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

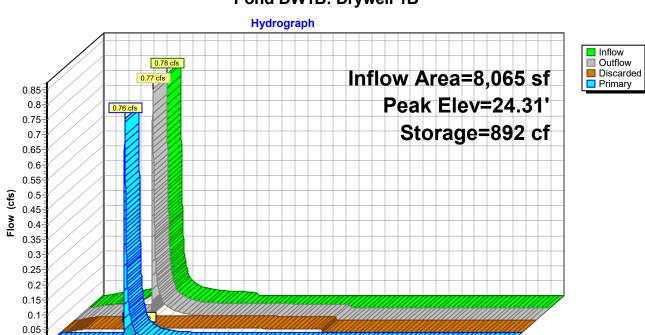
Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone





0-



# Pond DW1B: Drywell 1B

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

2021-04-15 Neptune, NJ (Proposed 1)

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#### Summary for Pond DW2B: Drywell 2B

Inflow Area =	3,316 sf,100.00% Impervious,	Inflow Depth = 5.18" for 10-Year event
Inflow =	0.32 cfs @ 12.14 hrs, Volume=	1,432 cf
Outflow =	0.17 cfs @ 12.33 hrs, Volume=	1,432 cf, Atten= 47%, Lag= 11.4 min
Discarded =	0.01 cfs @ 12.33 hrs, Volume=	1,086 cf
Primary =	0.16 cfs @ 12.33 hrs, Volume=	346 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.93' @ 12.33 hrs Surf.Area= 497 sf Storage= 464 cf

Plug-Flow detention time= 211.7 min calculated for 1,432 cf (100% of inflow) Center-of-Mass det. time= 211.7 min (963.2 - 751.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	21.75'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 21.75' / 21.00' S= 0.0163 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.33 hrs HW=21.93' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

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## Pond DW2B: Drywell 2B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

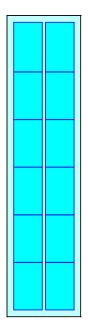
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

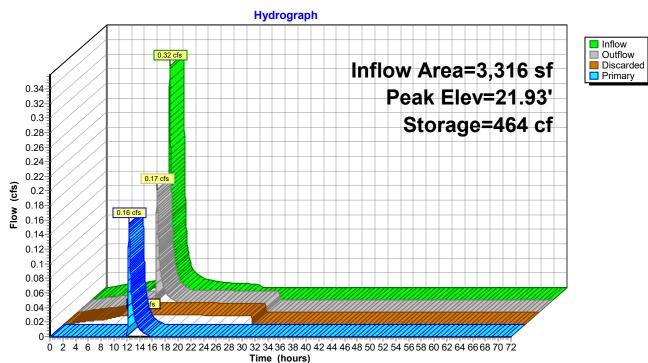
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW2B: Drywell 2B

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#### Summary for Pond RG1A: Rain Garden 1A

Inflow Area =	29,515 sf, 69.22% Impervious,	Inflow Depth = 4.59" for 10-Year event
Inflow =	2.61 cfs @ 12.14 hrs, Volume=	11,293 cf
Outflow =	1.71 cfs @ 12.27 hrs, Volume=	11,293 cf, Atten= 34%, Lag= 7.8 min
Discarded =	0.07 cfs @ 12.27 hrs, Volume=	7,798 cf
Primary =	1.64 cfs @ 12.27 hrs, Volume=	3,495 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 25.60' @ 12.27 hrs Surf.Area= 2,597 sf Storage= 4,487 cf

Plug-Flow detention time= 522.4 min calculated for 11,292 cf (100% of inflow) Center-of-Mass det. time= 522.6 min (1,291.4 - 768.8)

Volume	Invert	Avail.S	Storage	Storage Description					
#1	#1 23.00' 5,586 cf		,586 cf	Custom Stage Data (Irregular) Listed below (Recalc)					
		<i>.</i>	<b>.</b> .						
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>			
23.0	00	910	175.0	0	0	910			
24.0	00	1,532	208.0	1,208	1,208	1,934			
25.0	00	2,184	227.0	1,848	3,056	2,627			
26.0	00	2,892	246.0	2,530	5,586	3,380			
Device	Routing	Inve	rt Outle	et Devices					
#1	Primary 22.00'		0' <b>15.0</b> '	" Round Culvert					
	2		L= 2	L= 27.0' RCP, sq.cut end projecting, Ke= 0.500					
				/ Outlet Invert= 22.0					
			n= 0	.013, Flow Area= 1.	23 sf				
#2	#2 Device 1 25.50'		0' <b>48.0</b> '	48.0" x 48.0" Horiz. Orifice/Grate C= 0.600					
			Limit	Limited to weir flow at low heads					
#3	#3 Discarded 23.00'		0' <b>1.00</b>	1.000 in/hr Exfiltration over Surface area					
			Cond	ductivity to Groundwa	ater Elevation = 10.	.80' Phase-In= 0.01'			
Discard		Max-0.07	cfc @ 1	2 27 hrs HIM-25 60	(Eree Discharge)				

**Discarded OutFlow** Max=0.07 cfs @ 12.27 hrs HW=25.60' (Free Discharge) **3=Exfiltration** (Controls 0.07 cfs)

**Primary OutFlow** Max=1.64 cfs @ 12.27 hrs HW=25.60' TW=20.34' (Dynamic Tailwater) **1=Culvert** (Passes 1.64 cfs of 10.19 cfs potential flow) **2=Orifice/Grate** (Weir Controls 1.64 cfs @ 1.03 fps)

Hydrograph InflowOutflow 2.61 cfs Discarded Inflow Area=29,515 sf Primary Peak Elev=25.60' Storage=4,487 cf 1.71 cfs 2 1.64 cfs Flow (cfs) 1 0 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72

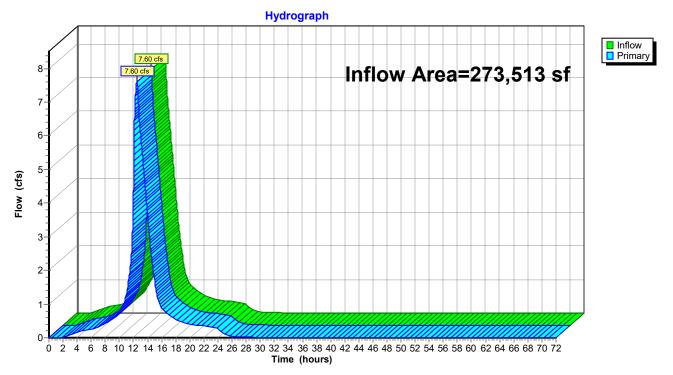
Time (hours)

# Pond RG1A: Rain Garden 1A

# Summary for Link 3L: Pr. POA 1

Inflow Are	a =	273,513 sf, 73.09% Impervious, Inflow Depth = 3.96" for 10-Year event	epth = 3.96" for 10-Year event
Inflow	=	7.60 cfs @ 12.48 hrs, Volume= 90,303 cf	0,303 cf
Primary	=	7.60 cfs @ 12.48 hrs, Volume= 90,303 cf, Atten= 0%, Lag= 0.0 min	0,303 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



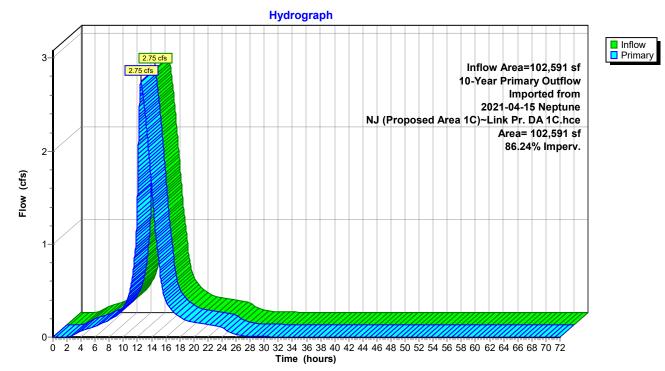
#### Link 3L: Pr. POA 1

## Summary for Link 4L: Pr DA 1C Total

Inflow Area	. =	102,591 sf, 86.24% Impervious, Inflow Depth = 4.32"	for 10-Year event
Inflow	=	2.75 cfs @ 12.52 hrs, Volume= 36,937 cf	
Primary	=	2.75 cfs @ 12.52 hrs, Volume= 36,937 cf, Atten	= 0%, Lag= 0.0 min

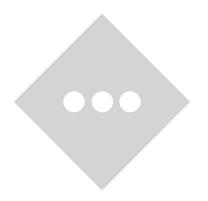
Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

10-Year Primary Outflow Imported from 2021-04-15 Neptune, NJ (Proposed Area 1C)~Link Pr. DA 1C.hce



## Link 4L: Pr DA 1C Total

# **APPENDIX C-3A** 25-Year Storm Event Hydrographs



#### Summary for Subcatchment 1S: Ex. Area 1A Perv.

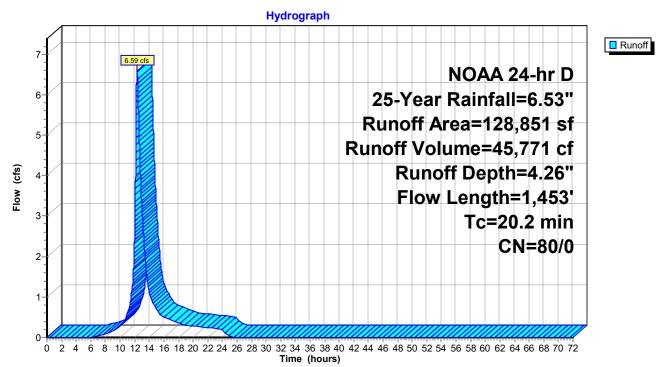
Page 1

6.59 cfs @ 12.31 hrs, Volume= 45,771 cf, Depth= 4.26" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

_	A	rea (sf)	CN I	Description		
	1	15,266	80 >	>75% Gras	s cover, Go	ood, HSG D
_		13,585	77 \	Noods, Go	od, HSG D	
	1	28,851	80 V	Neighted A	verage	
	1	28,851	80 <sup>-</sup>	100.00% Pe	ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.0	100	0.0600	0.19		Sheet Flow, Sheet Flow
						Grass: Dense n= 0.240 P2= 3.34"
	0.5	75	0.0270	2.65		Shallow Concentrated Flow, Shallow Concentrated
						Unpaved Kv= 16.1 fps
_	10.6	1,278		2.00		Direct Entry, Channel Flow
	20.2	1,453	Total			

#### Subcatchment 1S: Ex. Area 1A Perv.



#### Summary for Subcatchment 2S: Ex. Area 1A Imp.

Runoff = 15.87 cfs @ 12.14 hrs, Volume= 71,322 cf, Depth= 6.29"

4-3-2-1-0-

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

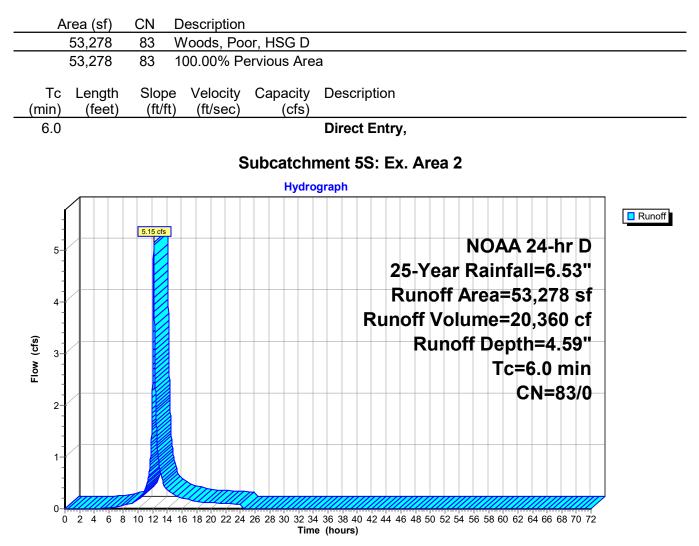
A	rea (sf)	CN D	escription					
1	36,039	98 P	aved park	ing, HSG D				
1	136,039 98 100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	-			
6.0					Direct Entry,			
			Sub	catchmer	nt 2S: Ex. Area 1A Imp.			
				Hydro	ograph			
17								
16		15.87 cfs			NOAA 24-hr D			
15- 14-					25-Year Rainfall=6.53"			
13					Runoff Area=136,039 sf			
12- 11-					Runoff Volume=71,322 cf			
_10 روزي					Runoff Depth=6.29"			
Flow (cfs)					Tc=6.0 min			
7	$\downarrow \downarrow \downarrow$				CN=0/98			
6-1 5-1								

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

#### Summary for Subcatchment 5S: Ex. Area 2

Runoff = 5.15 cfs @ 12.14 hrs, Volume= 20,360 cf, Depth= 4.59"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

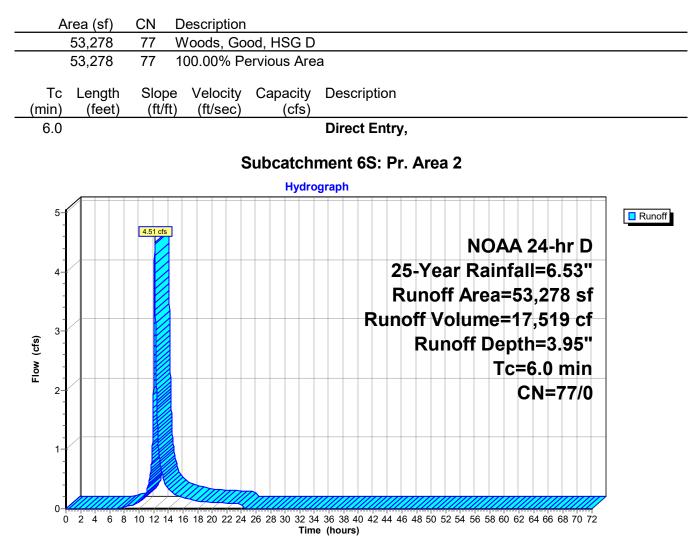


#### Summary for Subcatchment 6S: Pr. Area 2

Page 4

4.51 cfs @ 12.14 hrs, Volume= Runoff 17,519 cf, Depth= 3.95" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"



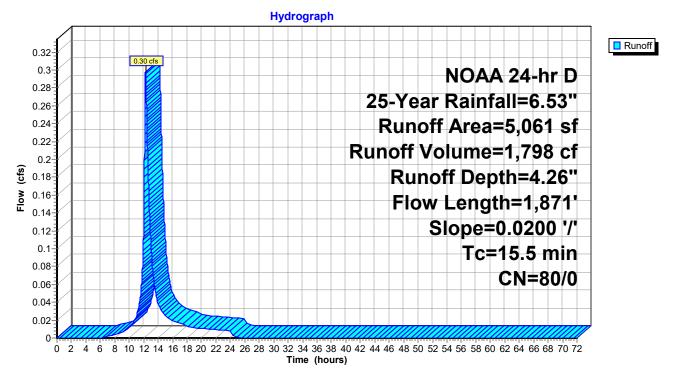
#### Summary for Subcatchment 10S: Ex. Area 1B Perv.

1,798 cf, Depth= 4.26" Runoff 0.30 cfs @ 12.26 hrs, Volume= =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

 A	rea (sf)	CN [	Description					
	5,061	80 >	80 >75% Grass cover, Good, HSG D					
	5,061	80 ´	80 100.00% Pervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.2	100	0.0200	1.41		Sheet Flow,			
1.1	184	0.0200	2.87		Smooth surfaces n= 0.011 P2= 3.34" <b>Shallow Concentrated Flow,</b> Paved Kv= 20.3 fps			
 13.2	1,587		2.00		Direct Entry,			
 15.5	1,871	Total						

#### Subcatchment 10S: Ex. Area 1B Perv.

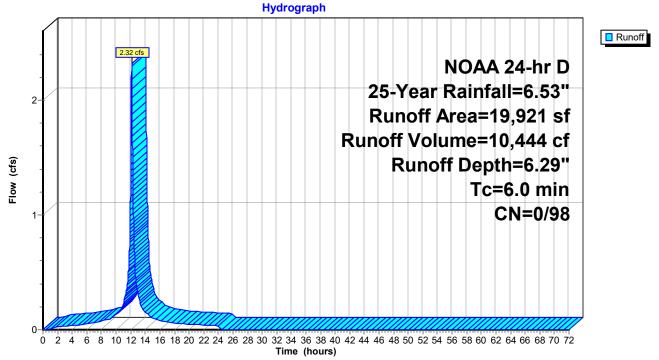


## Summary for Subcatchment 11S: Ex. Area 1B Imp.

Runoff = 2.32 cfs @ 12.14 hrs, Volume= 10,444 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

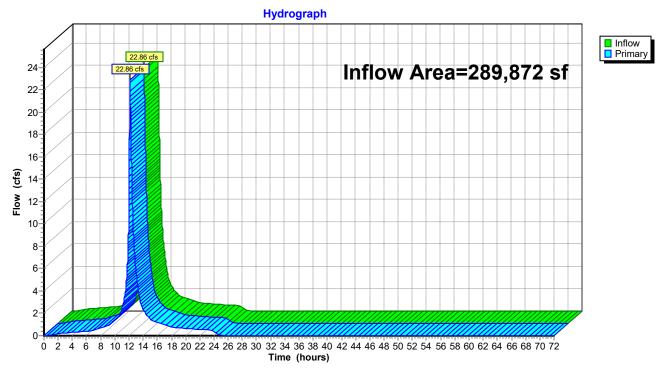
Are	a (sf)	CN Description							
19	9,921	98	98 Paved parking, HSG D						
19	9,921	98	100.00% In	npervious A	Area				
Tc L _(min)	_ength (feet)								
6.0		Direct Entry,							
Subcatchment 11S: Ex. Area 1B Imp.									



## Summary for Link 3L: EX POA 1 (Construction)

Inflow Are	ea =	289,872 sf, 53.80% Impervious, Inflow Depth = 5.35" for 25-Year event	-Year event
Inflow	=	22.86 cfs @ 12.15 hrs, Volume= 129,335 cf	
Primary	=	22.86 cfs @ 12.15 hrs, Volume= 129,335 cf, Atten= 0%, Lag= 0.0 min	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

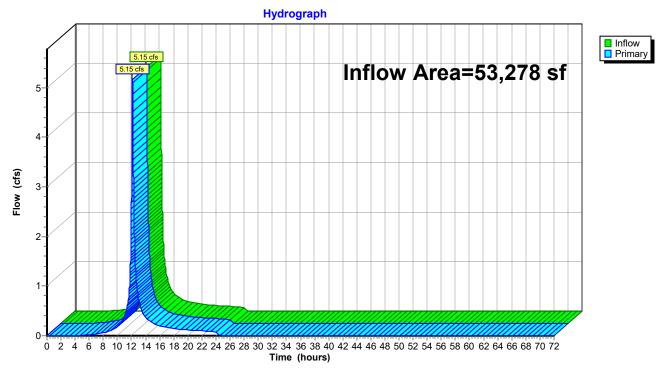


## Link 3L: EX POA 1 (Construction)

#### Summary for Link 4L: EX POA 2 (Reforestation)

Inflow Are	ea =	53,278 sf,	0.00% Impervious,	Inflow Depth = 4.59"	for 25-Year event
Inflow	=	5.15 cfs @ 1	2.14 hrs, Volume=	20,360 cf	
Primary	=	5.15 cfs @ 1	12.14 hrs, Volume=	20,360 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

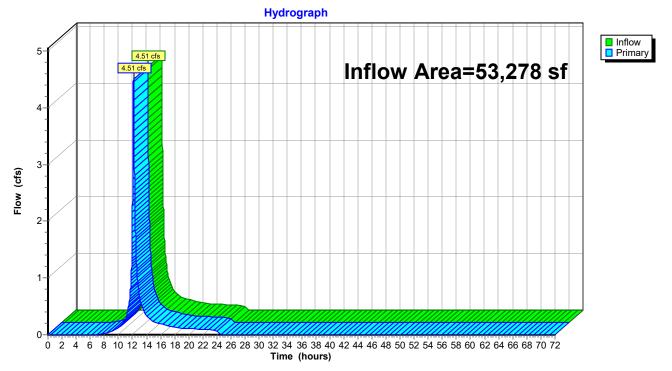


## Link 4L: EX POA 2 (Reforestation)

## Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Are	a =	53,278 sf,	0.00% Impervious,	Inflow Depth =	3.95" for 25-Year event
Inflow	=	4.51 cfs @ 1	12.14 hrs, Volume=	17,519 cf	
Primary	=	4.51 cfs @ 1	12.14 hrs, Volume=	17,519 cf,	, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



# Link 7L: Pr. POA 2 (Reforestation)

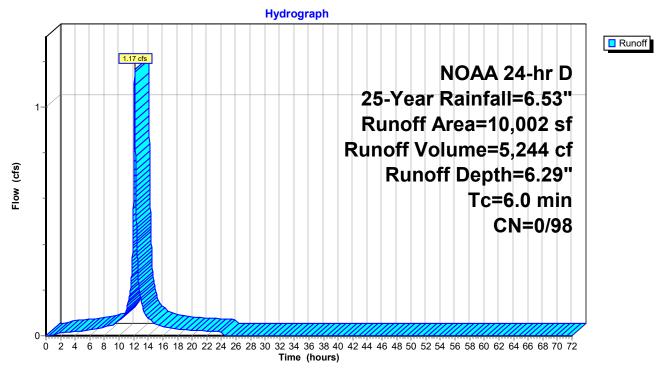
#### Summary for Subcatchment B1Ai: Pr. BAsin Area 1A Imp.

Runoff = 1.17 cfs @ 12.14 hrs, Volume= 5,244 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Description					
10,002	0,002 98 Paved parking, HSG D					
10,002	98 100.00% Impervious Area					
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)					
6.0	Direct Entry,					

#### Subcatchment B1Ai: Pr. BAsin Area 1A Imp.



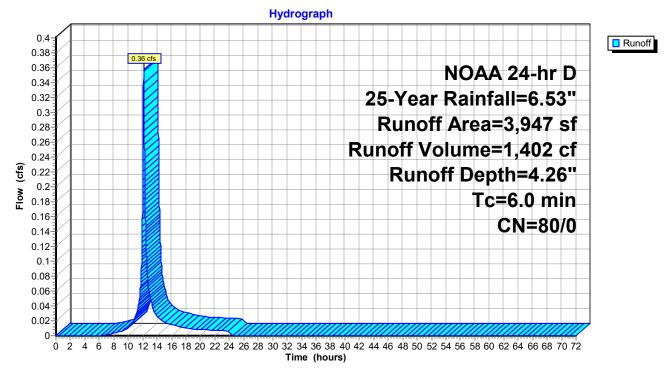
#### Summary for Subcatchment B1Ap: PR. Basin Area 1A Perv.

Runoff = 0.36 cfs @ 12.14 hrs, Volume= 1,402 cf, Depth= 4.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

A	rea (sf)	CN	Description					
	3,947	80	80 >75% Grass cover, Good, HSG D					
	3,947	7 80 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

#### Subcatchment B1Ap: PR. Basin Area 1A Perv.



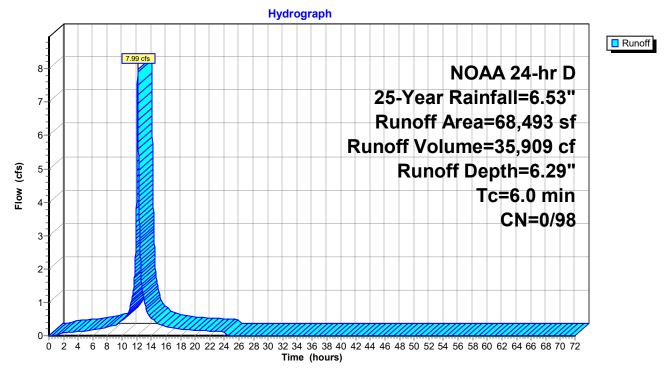
#### Summary for Subcatchment B1Bi: Pr. Basin Area 1B Imp.

Runoff = 7.99 cfs @ 12.14 hrs, Volume= 35,909 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Description					
68,493	193 98 Paved parking, HSG D					
68,493	98 100.00% Impervious Area					
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)					
6.0	Direct Entry,					

#### Subcatchment B1Bi: Pr. Basin Area 1B Imp.



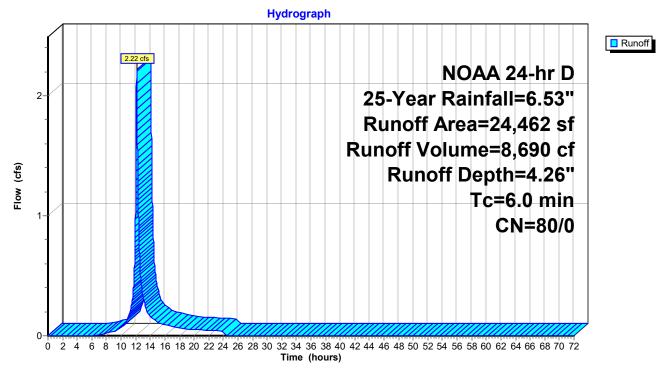
#### Summary for Subcatchment B1Bp: PR. Basin Area 1B Perv.

Runoff = 2.22 cfs @ 12.14 hrs, Volume= 8,690 cf, Depth= 4.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Description	
24,462	80 >75% Grass cover, Good, HSG D	
24,462	80 100.00% Pervious Area	
Tc Length (min) (feet)		
6.0	Direct Entry,	

#### Subcatchment B1Bp: PR. Basin Area 1B Perv.



0-

#### Summary for Subcatchment DA 1Di: Pr. Bypass 1D Imp

Runoff 0.13 cfs @ 12.14 hrs, Volume= 593 cf, Depth= 6.29" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Description	
1,131	98 Paved parking, HSG D	
1,131	98 100.00% Impervious Area	
Tc Length (min) (feet)		
6.0	Direct Entry,	
	Subcatchment DA 1Di: Pr. Bypass 1D Imp	
	Hydrograph	
0.14	0.13 cfs	Runoff
0.13	NOAA 24-hr [	)
0.12		
0.11	25-Year Rainfall=6.53	
0.1	Runoff Area=1,131 s	f
0.09	Runoff Volume=593 c	f
ເຊິ່ງ 0.08 0.07 GL	Runoff Depth=6.29	P.#
<u>8</u> 0.07		
0.06	Tc=6.0 mir	<b>1</b>
0.05	CN=0/98	8
0.04		
0.03		
0.02		
0.01		

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

Flow (cfs)

Runoff Depth=4.16"

Tc=6.0 min

CN=79/0

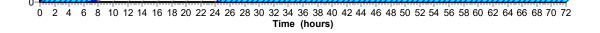
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## Summary for Subcatchment DA 1Dp: Pr. Bypass 1D Per

Runoff = 1.95 cfs @ 12.14 hrs, Volume= 7,617 cf, Depth= 4.16"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Description					
7,982	77 Woods, Good, HSG D					
14,009	80 >75% Grass cover, Good, HSG D					
21,991	79 Weighted Average					
21,991	79 100.00% Pervious Area					
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)					
6.0	Direct Entry,					
	Subcatchment DA 1Dp: Pr. Bypass 1D Per					
2-	NOAA 24-hr D					
-	25-Year Rainfall=6.53"					
-	Runoff Area=21,991 sf					
-	Runoff Volume=7.617 cf					



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## Summary for Subcatchment DW1Bi: Pr. Drywell Area 1B

Runoff = 0.94 cfs @ 12.14 hrs, Volume= 4,228 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf) CN Description							
8,065 98 Paved parking, HSG D							
8,065 98 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.0 <b>Direct Entry</b> ,							
Subcatchment DW1Bi: Pr. Drywell Area 1	В						
Hydrograph							
	Runoff						
	A 24-hr D						
25-Year Rainf	fall=6.53"						
Runoff Area	=8,065 sf						
Runoff Volume	=4,228 cf						
	c=6.0 min						
	CN=0/98						

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

0.06 0.04 0.02

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## Summary for Subcatchment DW2Bi: Pr. Drywell Area 2B

Runoff = 0.39 cfs @ 12.14 hrs, Volume= 1,739 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Are	ea (sf)	CN E	Description					
	3,316 98 Paved parking, HSG D							
	3,316	98 1	00.00% In	npervious A	vrea			
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			
			Subcato	chment D	W2Bi: Pr. Drywell Area 2B			
				Hydro	ograph	_		
0.42 0.4 0.38 0.36 0.34 0.32 0.3 0.3		0.39 cfs			NOAA 24-hr D 25-Year Rainfall=6.53" Runoff Area=3,316 sf Runoff Volume=1,739 cf	Runoff		
0.26 ( <b>sj</b> ) 0.24 0.22 0.22					Runoff Depth=6.29" Tc=6.0 min	-		
E 0.18	1							
0.16	/				<b>CN=0/98</b>			
0.14						-		
0.12 0.1								
0.08	1					_		

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

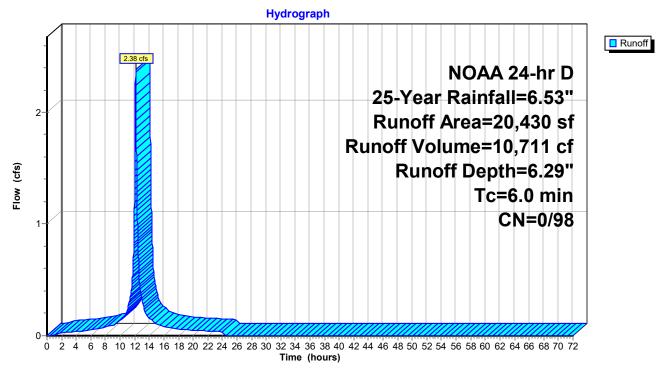
#### Summary for Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.

Runoff = 2.38 cfs @ 12.14 hrs, Volume= 10,711 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Ar	ea (sf)	CN	Description					
2	20,430	98	98 Paved parking, HSG D					
2	20,430		100.00% In	npervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)				
6.0					Direct Entry,			

#### Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.



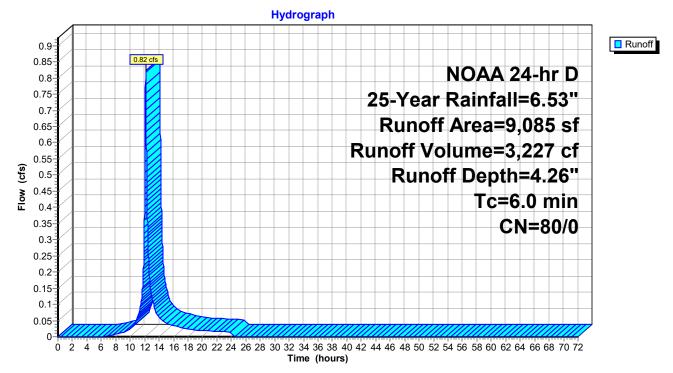
#### Summary for Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.

Runoff = 0.82 cfs @ 12.14 hrs, Volume= 3,227 cf, Depth= 4.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN Desc	N Description		
9,085	80 >75%	80 >75% Grass cover, Good, HSG D		
9,085	80 100.0	80 100.00% Pervious Area		
Tc Length (min) (feet)		elocity Capacity ft/sec) (cfs)	Description	
6.0			Direct Entry,	

#### Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.



#### Summary for Pond B1A: Underground Basin 1A

Inflow Area =	43,464 sf, 70.02% Impervious,	Inflow Depth = 3.44" for 25-Year event
Inflow =	4.40 cfs @ 12.16 hrs, Volume=	12,462 cf
Outflow =	2.65 cfs @ 12.33 hrs, Volume=	12,461 cf, Atten= 40%, Lag= 9.7 min
Primary =	2.65 cfs @ 12.33 hrs, Volume=	12,461 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.65' @ 12.33 hrs Surf.Area= 1,439 sf Storage= 2,477 cf

Plug-Flow detention time= 24.9 min calculated for 12,461 cf (100% of inflow) Center-of-Mass det. time= 24.8 min (798.1 - 773.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	1,318 cf	11.03'W x 130.42'L x 3.52'H Field A
			5,072 cf Overall - 1,778 cf Embedded = 3,295 cf x 40.0% Voids
#2A	19.60'	1,703 cf	Contech ChamberMaxx 2016 x 36 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		3,021 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	5.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	21.00'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=2.65 cfs @ 12.33 hrs HW=21.65' TW=0.00' (Dynamic Tailwater)

**-1=Culvert** (Passes 2.65 cfs of 11.42 cfs potential flow)

**—2=Orifice/Grate** (Orifice Controls 1.00 cfs @ 7.37 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 1.64 cfs @ 2.53 fps)

#### Pond B1A: Underground Basin 1A - Chamber Wizard Field A

# Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

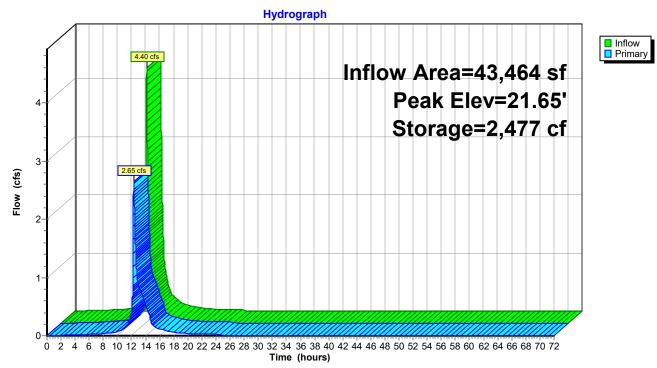
18 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 128.42' Row Length +12.0" End Stone x 2 = 130.42' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

36 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 1,703.2 cf Chamber Storage 36 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 1,777.6 cf Displacement

5,072.2 cf Field - 1,777.6 cf Chambers = 3,294.6 cf Stone x 40.0% Voids = 1,317.8 cf Stone Storage

Chamber Storage + Stone Storage = 3,021.0 cf = 0.069 af Overall Storage Efficiency = 59.6% Overall System Size = 130.42' x 11.03' x 3.52'

36 Chambers 187.9 cy Field 122.0 cy Stone



# Pond B1A: Underground Basin 1A

#### Summary for Pond B1B: Underground Basin 1B

Inflow Area =	104,336 sf, 76.55% Impervious,	Inflow Depth = 5.46" for 25-Year event
Inflow =	11.27 cfs @ 12.14 hrs, Volume=	47,491 cf
Outflow =	4.88 cfs @ 12.39 hrs, Volume=	47,476 cf, Atten= 57%, Lag= 14.9 min
Primary =	4.88 cfs @ 12.39 hrs, Volume=	47,476 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.13' @ 12.39 hrs Surf.Area= 7,780 sf Storage= 11,335 cf

Plug-Flow detention time= 46.5 min calculated for 47,476 cf (100% of inflow) Center-of-Mass det. time= 46.3 min ( 810.3 - 764.0 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	6,625 cf	53.78'W x 144.65'L x 3.52'H Field A
			27,424 cf Overall - 10,861 cf Embedded = 16,563 cf x 40.0% Voids
#2A	19.60'	10,406 cf	Contech ChamberMaxx 2016 x 220 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		17,031 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
			L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	10.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=4.88 cfs @ 12.39 hrs HW=21.13' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 4.88 cfs of 8.94 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 3.33 cfs @ 6.11 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 1.55 cfs @ 2.47 fps)

#### Pond B1B: Underground Basin 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

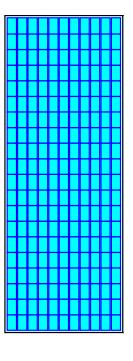
20 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 142.65' Row Length +12.0" End Stone x 2 = 144.65' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

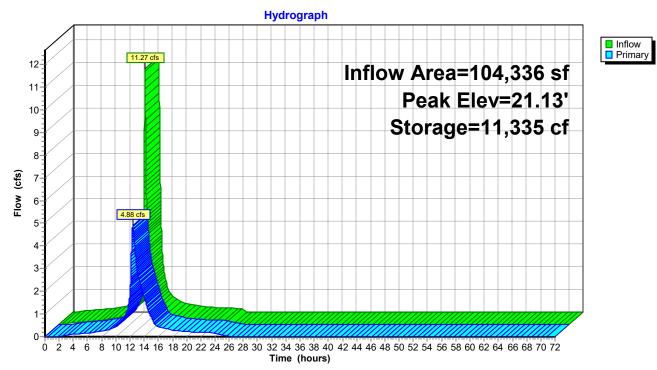
220 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 10,405.7 cf Chamber Storage 220 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 10,860.7 cf Displacement

27,423.7 cf Field - 10,860.7 cf Chambers = 16,563.0 cf Stone x 40.0% Voids = 6,625.2 cf Stone Storage

Chamber Storage + Stone Storage = 17,030.9 cf = 0.391 af Overall Storage Efficiency = 62.1% Overall System Size = 144.65' x 53.78' x 3.52'

220 Chambers 1,015.7 cy Field 613.4 cy Stone





# Pond B1B: Underground Basin 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW1B: Drywell 1B

Inflow Area =	8,065 sf,100.00% Impervious,	Inflow Depth = 6.29" for 25-Year event
Inflow =	0.94 cfs @ 12.14 hrs, Volume=	4,228 cf
Outflow =	0.93 cfs @ 12.15 hrs, Volume=	4,228 cf, Atten= 1%, Lag= 0.8 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,906 cf
Primary =	0.92 cfs @ 12.15 hrs, Volume=	2,322 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 24.35' @ 12.15 hrs Surf.Area= 497 sf Storage= 900 cf

Plug-Flow detention time= 259.8 min calculated for 4,228 cf (100% of inflow) Center-of-Mass det. time= 259.9 min (1,008.5 - 748.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	21.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	22.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	23.90'	15.0" Round Culvert
	-		L= 67.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 23.90' / 21.00' S= 0.0433 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	21.50'	
			Conductivity to Groundwater Elevation = 12.75' Phase-In= 0.01'

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=24.35' (Free Discharge) **2=Exfiltration** (Controls 0.02 cfs)

#### Pond DW1B: Drywell 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

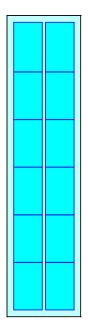
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

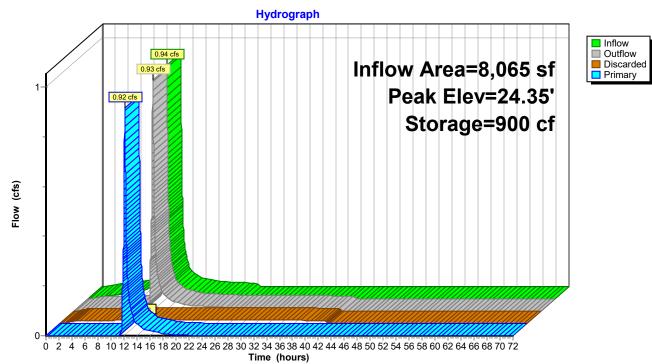
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1B: Drywell 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW2B: Drywell 2B

Inflow Area =	3,316 sf,100.00% Impervious,	Inflow Depth = 6.29" for 25-Year event
Inflow =	0.39 cfs @ 12.14 hrs, Volume=	1,739 cf
Outflow =	0.30 cfs @ 12.22 hrs, Volume=	1,739 cf, Atten= 23%, Lag= 5.0 min
Discarded =	0.01 cfs @ 12.22 hrs, Volume=	1,168 cf
Primary =	0.29 cfs @ 12.22 hrs, Volume=	571 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.99' @ 12.22 hrs Surf.Area= 497 sf Storage= 488 cf

Plug-Flow detention time= 193.5 min calculated for 1,738 cf (100% of inflow) Center-of-Mass det. time= 193.5 min (942.0 - 748.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	21.75'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 21.75' / 21.00' S= 0.0163 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.22 hrs HW=21.99' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

#### Pond DW2B: Drywell 2B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

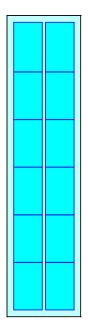
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

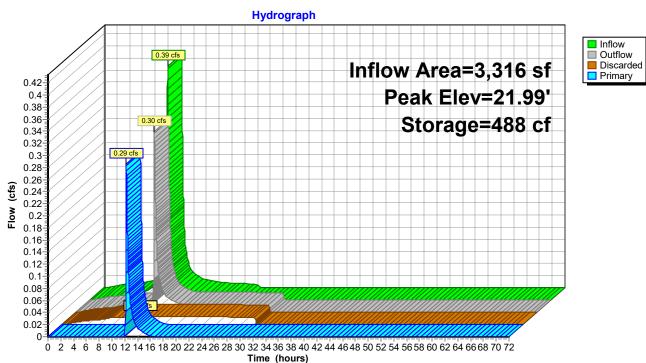
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







Pond DW2B: Drywell 2B

#### Summary for Pond RG1A: Rain Garden 1A

Inflow Area =	29,515 sf, 69.22% Impervious,	Inflow Depth = 5.67" for 25-Year event
Inflow =	3.21 cfs @ 12.14 hrs, Volume=	13,938 cf
Outflow =	3.01 cfs @ 12.17 hrs, Volume=	13,938 cf, Atten= 6%, Lag= 1.9 min
Discarded =	0.07 cfs @ 12.17 hrs, Volume=	8,122 cf
Primary =	2.94 cfs @ 12.17 hrs, Volume=	5,816 cf

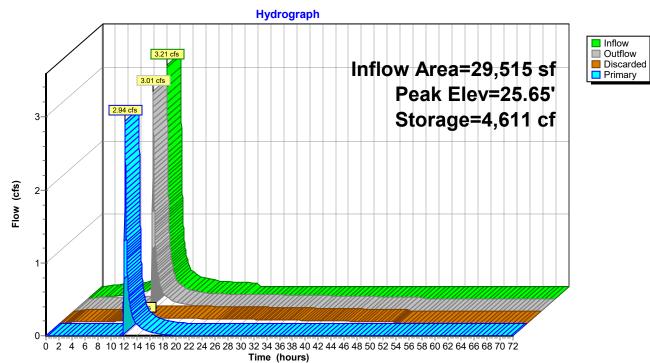
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 25.65' @ 12.17 hrs Surf.Area= 2,631 sf Storage= 4,611 cf

Plug-Flow detention time= 448.9 min calculated for 13,936 cf (100% of inflow) Center-of-Mass det. time= 449.1 min (1,214.7 - 765.6)

Volume	Invert	Avail.S	Storage	Storage Description	ı				
#1	23.00	5	,586 cf	Custom Stage Data	a (Irregular) Listed	below (Recalc)			
	-								
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>			
23.0	00	910	175.0	0	0	910			
24.0	00	1,532	208.0	1,208	1,208	1,934			
25.0	00	2,184	227.0	1,848	3,056	2,627			
26.0	00	2,892	246.0	2,530	5,586	3,380			
Device	Routing	Inve	rt Outle	et Devices					
#1	Primary	22.0	0' <b>15.0</b> '	" Round Culvert					
			L= 2	7.0' RCP, sq.cut en	nd projecting, Ke= (	0.500			
				/ Outlet Invert= 22.0					
			n= 0	.013, Flow Area= 1.3	23 sf				
#2	Device 1 25.50'			" x 48.0" Horiz. Orifi		00			
			Limit	Limited to weir flow at low heads					
#3	#3 Discarded 23.00'		0' <b>1.00</b>	1.000 in/hr Exfiltration over Surface area					
			Cond	ductivity to Groundwa	ater Elevation = 10.	.80' Phase-In= 0.01'			
				,					
Discard	<b>Discorded OutElow</b> Max-0.07 of $(0, 12, 17)$ by $HW = 25.65^{\circ}$ (Free Discharge)								

**Discarded OutFlow** Max=0.07 cfs @ 12.17 hrs HW=25.65' (Free Discharge) **T-3=Exfiltration** (Controls 0.07 cfs)

Primary OutFlow Max=2.94 cfs @ 12.17 hrs HW=25.65' TW=20.67' (Dynamic Tailwater) **1=Culvert** (Passes 2.94 cfs of 10.27 cfs potential flow) **2=Orifice/Grate** (Weir Controls 2.94 cfs @ 1.25 fps)

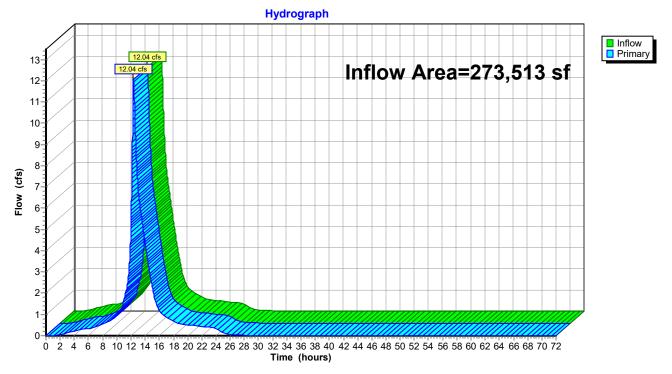


# Pond RG1A: Rain Garden 1A

## Summary for Link 3L: Pr. POA 1

Inflow Are	ea =	273,513 sf, 73.09% Impervious, Inflow Depth = 5.01" for 25-Year event
Inflow	=	12.04 cfs @ 12.35 hrs, Volume= 114,180 cf
Primary	=	12.04 cfs @ 12.35 hrs, Volume= 114,180 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



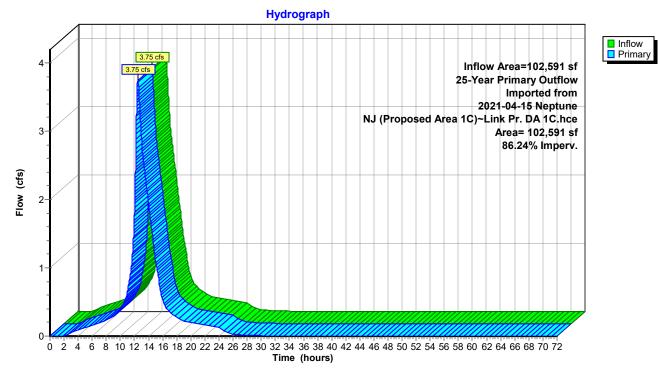
## Link 3L: Pr. POA 1

#### Summary for Link 4L: Pr DA 1C Total

Inflow Area	a =	102,591 sf, 86.24% Impervious, Inflow Depth = 5.38" for	or 25-Year event
Inflow	=	3.75 cfs @ 12.48 hrs, Volume= 46,033 cf	
Primary	=	3.75 cfs @ 12.48 hrs, Volume= 46,033 cf, Atten=	0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

25-Year Primary Outflow Imported from 2021-04-15 Neptune, NJ (Proposed Area 1C)~Link Pr. DA 1C.hce



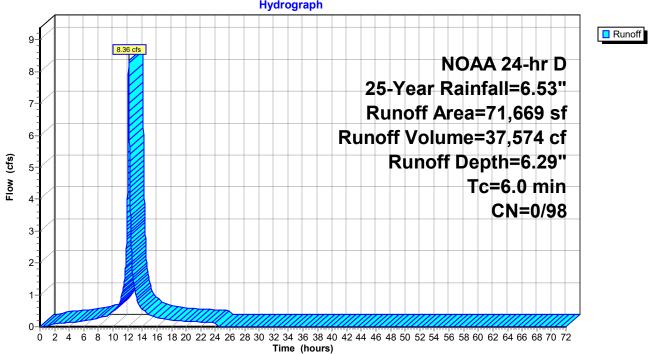
#### Link 4L: Pr DA 1C Total

Page 1

8.36 cfs @ 12.14 hrs, Volume= 37,574 cf, Depth= 6.29" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (	(sf) Cl	CN Description						
71,6	69 9	98 Pa	aved parki	ing, HSG D				
71,6	71,669 98 100.00% Impervious Area							
6.0	6.0 Direct Entry,							
Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.								



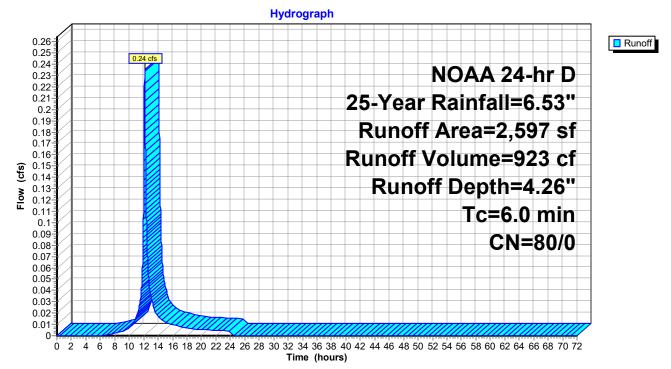
#### Summary for Subcatchment B 1Cp: PR. BASIN Area 1C Perv.

Runoff = 0.24 cfs @ 12.14 hrs, Volume= 923 cf, Depth= 4.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

A	rea (sf)	CN	Description				
	2,597	80	>75% Gras	s cover, Go	lood, HSG D		
	2,597	80	80 100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)			
6.0					Direct Entry,		

#### Subcatchment B 1Cp: PR. BASIN Area 1C Perv.



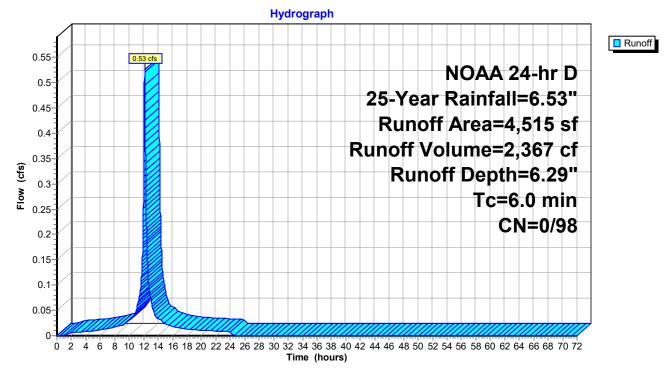
## Summary for Subcatchment DW 1Ci: Pr. Drywell 1C Imp.

Runoff = 0.53 cfs @ 12.14 hrs, Volume= 2,367 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

A	rea (sf)	CN	Description					
	4,515	98	Paved park	ing, HSG D	)			
	4,515	98	98 100.00% Impervious Area					
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

#### Subcatchment DW 1Ci: Pr. Drywell 1C Imp.



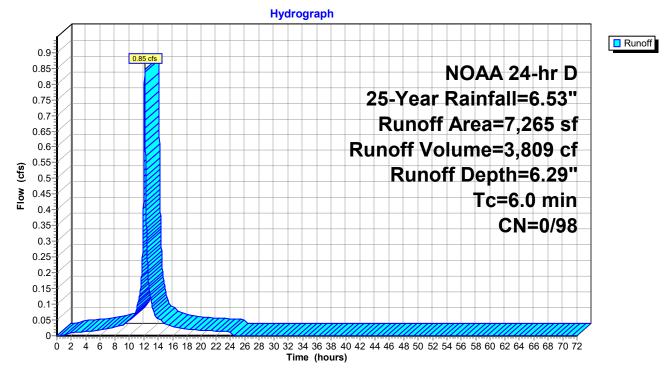
#### Summary for Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.

Page 4

3,809 cf, Depth= 6.29" Runoff = 0.85 cfs @ 12.14 hrs, Volume=

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

#### Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.



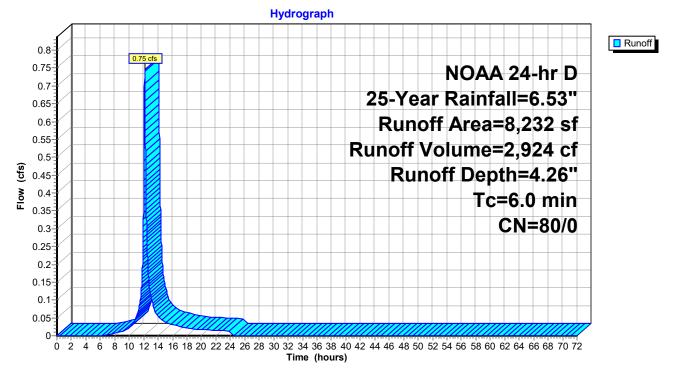
#### Summary for Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.

Runoff = 0.75 cfs @ 12.14 hrs, Volume= 2,924 cf, Depth= 4.26"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area (sf)	CN	Description				
8,232	80	>75% Gras	s cover, Go	Good, HSG D		
8,232	80	80 100.00% Pervious Area				
Tc Lengt (min) (feet		e Velocity ) (ft/sec)	Capacity (cfs)	•		
6.0				Direct Entry,		

#### Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.



#### Summary for Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.

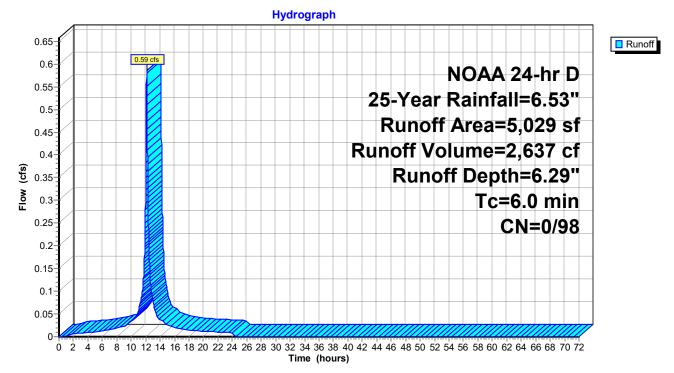
Page 6

0.59 cfs @ 12.14 hrs, Volume= 2,637 cf, Depth= 6.29" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

A	rea (sf)	CN	Description					
	5,029	98	Paved park	ing, HSG D	D			
	5,029	98	98 100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)				
6.0					Direct Entry,			

#### Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.



#### Summary for Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.

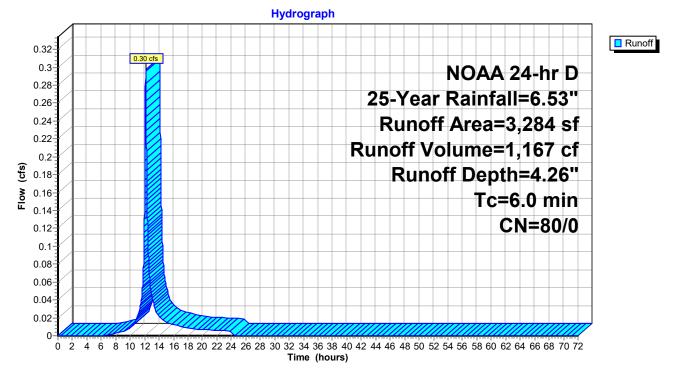
Page 7

0.30 cfs @ 12.14 hrs, Volume= 1,167 cf, Depth= 4.26" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

A	rea (sf)	CN	Description				
	3,284	80	>75% Gras	s cover, Go	Good, HSG D		
	3,284	80	80 100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)			
6.0					Direct Entry,		

#### Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.



#### Summary for Pond B 1C: Underground Basin 1C

Inflow Area =	102,591 sf, 86.24% Impervious,	Inflow Depth = 5.39" for 25-Year event
Inflow =	11.22 cfs @ 12.15 hrs, Volume=	46,060 cf
Outflow =	3.75 cfs @ 12.48 hrs, Volume=	46,033 cf, Atten= 67%, Lag= 19.8 min
Primary =	3.75 cfs @ 12.48 hrs, Volume=	46,033 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 20.88' @ 12.48 hrs Surf.Area= 10,459 sf Storage= 13,311 cf

Plug-Flow detention time= 64.4 min calculated for 46,033 cf (100% of inflow) Center-of-Mass det. time= 64.0 min ( 818.7 - 754.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	8,886 cf	53.78'W x 194.47'L x 3.52'H Field A
			36,868 cf Overall - 14,653 cf Embedded = 22,215 cf x 40.0% Voids
#2A	19.60'	14,040 cf	Contech ChamberMaxx 2016 x 297 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		22,925 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=3.75 cfs @ 12.48 hrs HW=20.88' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 3.75 cfs of 8.15 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 3.07 cfs @ 5.62 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 0.68 cfs @ 1.79 fps)

#### Pond B 1C: Underground Basin 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

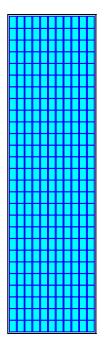
27 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 192.47' Row Length +12.0" End Stone x 2 = 194.47' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

297 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 14,039.6 cf Chamber Storage 297 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 14,653.5 cf Displacement

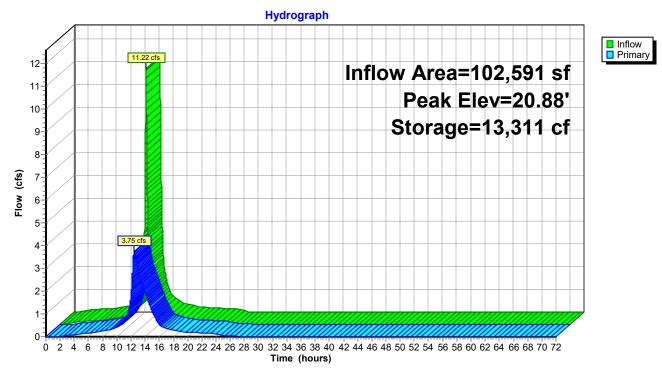
36,868.2 cf Field - 14,653.5 cf Chambers = 22,214.7 cf Stone x 40.0% Voids = 8,885.9 cf Stone Storage

Chamber Storage + Stone Storage = 22,925.5 cf = 0.526 af Overall Storage Efficiency = 62.2% Overall System Size = 194.47' x 53.78' x 3.52'

297 Chambers 1,365.5 cy Field 822.8 cy Stone



00000000000



## Pond B 1C: Underground Basin 1C

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

## Summary for Pond DW1C: Drywell 1C

Inflow Area =	4,515 sf,100.00% Impervious,	Inflow Depth = 6.29" for 25-Year event
Inflow =	0.53 cfs @ 12.14 hrs, Volume=	2,367 cf
Outflow =	0.43 cfs @ 12.21 hrs, Volume=	2,367 cf, Atten= 19%, Lag= 4.2 min
Discarded =	0.01 cfs @ 12.21 hrs, Volume=	1,539 cf
Primary =	0.41 cfs @ 12.21 hrs, Volume=	828 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.70' @ 12.21 hrs Surf.Area= 497 sf Storage= 735 cf

Plug-Flow detention time= 291.4 min calculated for 2,367 cf (100% of inflow) Center-of-Mass det. time= 291.4 min (1,040.0 - 748.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	22.40'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 22.40' / 21.00' S= 0.0304 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.21 hrs HW=22.70' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

## Pond DW1C: Drywell 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

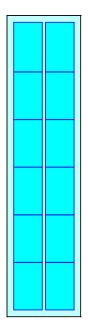
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

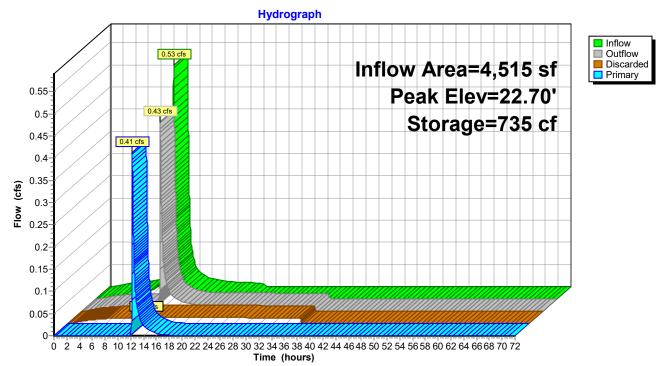
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1C: Drywell 1C

#### Summary for Pond RG 1C: Rain Garden 1C

Inflow Area =	15,497 sf, 46.88% Impervious,	Inflow Depth = 5.21" for 25-Year event
Inflow =	1.59 cfs @ 12.14 hrs, Volume=	6,733 cf
Outflow =	1.58 cfs @ 12.15 hrs, Volume=	6,733 cf, Atten= 1%, Lag= 0.7 min
Discarded =	0.03 cfs @ 12.15 hrs, Volume=	2,260 cf
Primary =	1.55 cfs @ 12.15 hrs, Volume=	4,473 cf

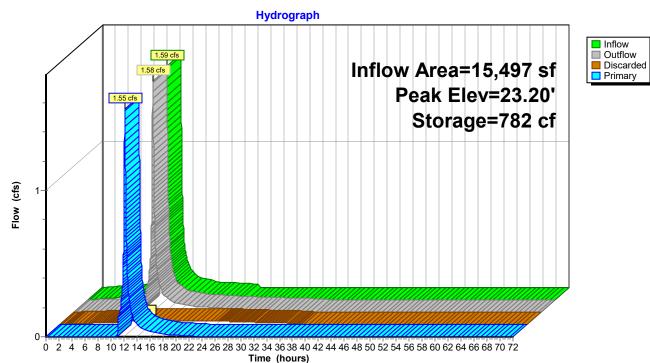
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.20' @ 12.15 hrs Surf.Area= 1,085 sf Storage= 782 cf

Plug-Flow detention time= 125.7 min calculated for 6,733 cf (100% of inflow) Center-of-Mass det. time= 125.7 min (906.2 - 780.5)

Volume	Inver	t Avail.	Storage	Storage Descriptio	n	
#1	22.00	)'	1,964 cf	Custom Stage Dat	t <b>a (Irregular)</b> Listed	below (Recalc)
Elevatio (fee 22.0	et)	Surf.Area (sq-ft) 305	Perim. (feet) 88.0	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	Wet.Area <u>(sq-ft)</u> 305
23.0		922	137.0	586	586	1,190
24.0	00	1,893	201.0	1,379	1,964	2,919
Device	Routing	Inve	ert Outle	et Devices		
#1	Primary	20.4	L= 3 Inlet		nd projecting, Ke= 45' / 20.25' S= 0.00 .23 sf	
#2	Device 1	23.1		" x 48.0" Horiz. Ori	fi <b>ce/Grate</b> C= 0.60 w heads	00
#3	Discardeo	22.0	00' <b>1.00</b>	0 in/hr Exfiltration		.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.03 cfs @ 12.15 hrs HW=23.20' (Free Discharge) **3=Exfiltration** (Controls 0.03 cfs)

**Primary OutFlow** Max=1.55 cfs @ 12.15 hrs HW=23.20' TW=20.38' (Dynamic Tailwater) -1=Culvert (Passes 1.55 cfs of 8.34 cfs potential flow) -2=Orifice/Grate (Weir Controls 1.55 cfs @ 1.01 fps)



## Pond RG 1C: Rain Garden 1C

#### Summary for Pond RG 2C: Rain Garden 2C

Inflow Area =	8,313 sf, 60.50% Impervious,	Inflow Depth = 5.49" for 25-Year event
Inflow =	0.88 cfs @ 12.14 hrs, Volume=	3,803 cf
Outflow =	0.88 cfs @ 12.15 hrs, Volume=	3,803 cf, Atten= 1%, Lag= 0.6 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,541 cf
Primary =	0.86 cfs @ 12.15 hrs, Volume=	2,262 cf

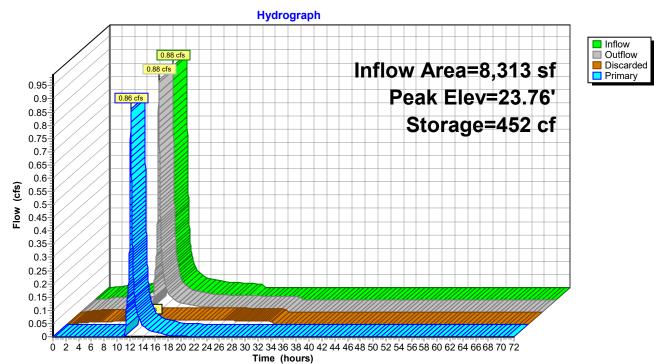
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.76' @ 12.15 hrs Surf.Area= 700 sf Storage= 452 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 115.4 min (886.6 - 771.1)

Volume	Inver	t Avail.S	torage	Storage Description			
#1	23.00	1	842 cf	Custom Stage Data	a (Irregular) Listed I	pelow (Recalc)	
Elevatio (fee 23.0 24.0 24.2	et) 00 00	urf.Area (sq-ft) 489 773 1,429	Perim. (feet) 105.0 138.0 180.0	Inc.Store (cubic-feet) 0 626 217	Cum.Store (cubic-feet) 0 626 842	Wet.Area (sq-ft) 489 1,139 2,202	
Device	Routing	Inver	t Outle	et Devices			
#1	Primary	21.40		" Round Culvert 2.0' RCP, sq.cut end	d projecting Ke= (	500	
			Inlet	/ Outlet Invert= 21.40	0'/21.06' S= 0.00		
#2	Device 1	23.70	' <b>48.0</b>	.013,  Flow Area= 0.7 <b>" x 48.0" Horiz. Orif</b> i	ce/Grate C= 0.60	0	
#3	Discarded	23.00	1.00	ted to weir flow at low 0 in/hr Exfiltration ov ductivity to Groundwa	ver Surface area	טי	

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=23.76' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=0.86 cfs @ 12.15 hrs HW=23.76' TW=20.37' (Dynamic Tailwater) -1=Culvert (Passes 0.86 cfs of 4.43 cfs potential flow) -2=Orifice/Grate (Weir Controls 0.86 cfs @ 0.83 fps)

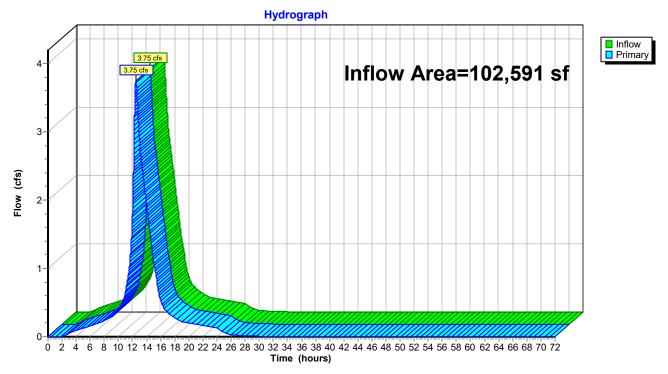


## Pond RG 2C: Rain Garden 2C

## Summary for Link Pr. DA 1C: Pr. DA 1C

Inflow Area	a =	102,591 sf, 86.24% Impervious, Inflow Depth =	5.38"	for 25-Year event
Inflow	=	3.75 cfs @ 12.48 hrs, Volume= 46,033 c	f	
Primary	=	3.75 cfs @ 12.48 hrs, Volume= 46,033 cf	f, Atten	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

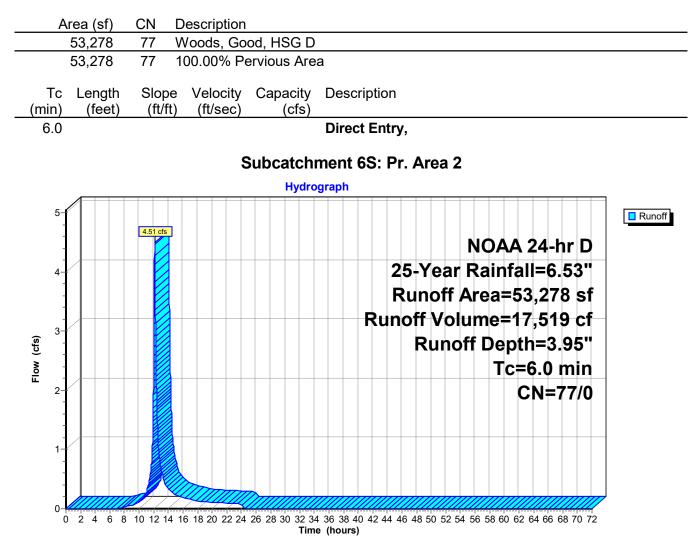


## Link Pr. DA 1C: Pr. DA 1C

#### Summary for Subcatchment 6S: Pr. Area 2

Runoff = 4.51 cfs @ 12.14 hrs, Volume= 17,519 cf, Depth= 3.95"

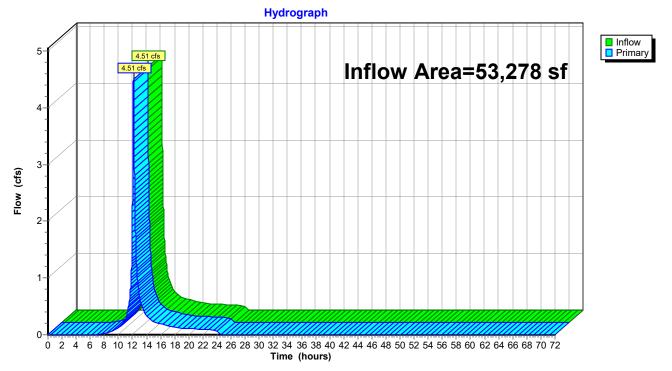
Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"



## Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Are	a =	53,278 sf,	0.00% Impervious,	Inflow Depth =	3.95" for 25-Year event
Inflow	=	4.51 cfs @ 1	12.14 hrs, Volume=	17,519 cf	
Primary	=	4.51 cfs @ 1	12.14 hrs, Volume=	17,519 cf,	, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



# Link 7L: Pr. POA 2 (Reforestation)

# **APPENDIX C-4 I 00-YEAR STORM EVENT HYDROGRAPHS**



## Summary for Subcatchment 1S: Ex. Area 1A Perv.

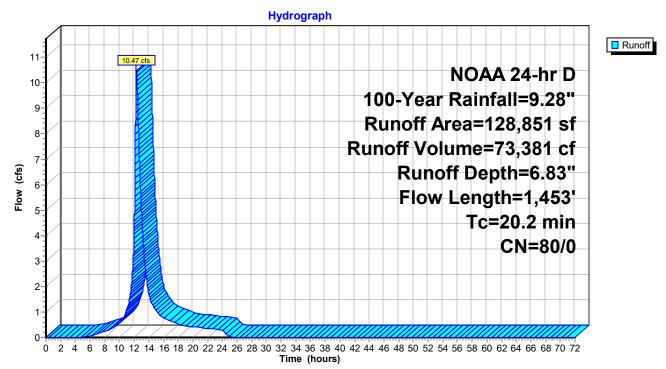
Page 1

Runoff 10.47 cfs @ 12.31 hrs, Volume= 73,381 cf, Depth= 6.83" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

_	Ai	rea (sf)	CN I	Description		
115,266 80 >75% Grass cover, Good, HSG D						
_		13,585	77 \	Noods, Go	od, HSG D	
128,851 80 Weighted Average						
	1	28,851	80 <sup>·</sup>	100.00% Pe	ervious Are	а
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.0	100	0.0600	0.19		Sheet Flow, Sheet Flow
						Grass: Dense n= 0.240 P2= 3.34"
	0.5	75	0.0270	2.65		Shallow Concentrated Flow, Shallow Concentrated
						Unpaved Kv= 16.1 fps
	10.6	1,278		2.00		Direct Entry, Channel Flow
-	20.2	1,453	Total			

#### Subcatchment 1S: Ex. Area 1A Perv.



### Summary for Subcatchment 2S: Ex. Area 1A Imp.

Runoff = 22.60 cfs @ 12.14 hrs, Volume= 102,477 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

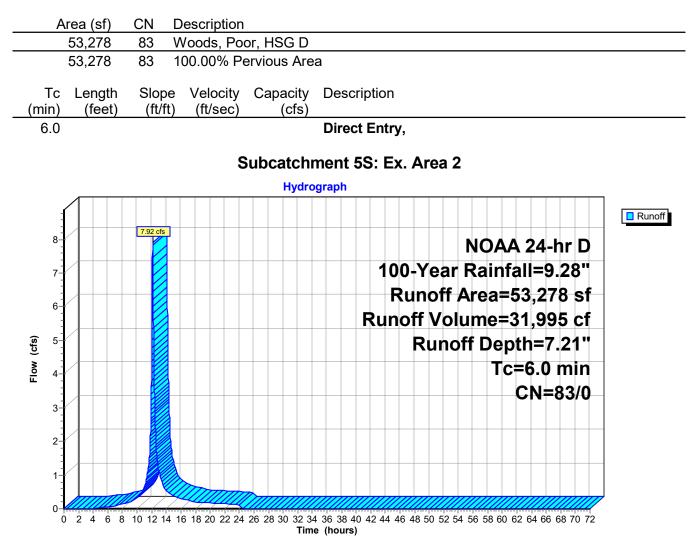
Area (sf)	CN Descriptio	n		
136,039	98 Paved pa	rking, HSG D	)	
136,039	98 100.00%	Impervious A	rea	
Tc Length _(min) (feet)	Slope Velocit (ft/ft) (ft/sec	y Capacity ) (cfs)	Description	
6.0			Direct Entry,	
	Su	bcatchmer	nt 2S: Ex. Area 1A Imp.	
	•••		bgraph	
25-				Runoff
24	22.60 cfs			
22			NOAA 24-hr D	-
21			100-Year Rainfall=9.28"	
19 18			Runoff Area=136,039 sf	-
17			Runoff Volume=102,477 cf	-
<u>الم</u> الم			Runoff Depth=9.04"	-
(cta) 13 13 12 12 12			Tc=6.0 min	-
й 11- 10-			CN=0/98	-

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

#### Summary for Subcatchment 5S: Ex. Area 2

Runoff = 7.92 cfs @ 12.14 hrs, Volume= 31,995 cf, Depth= 7.21"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

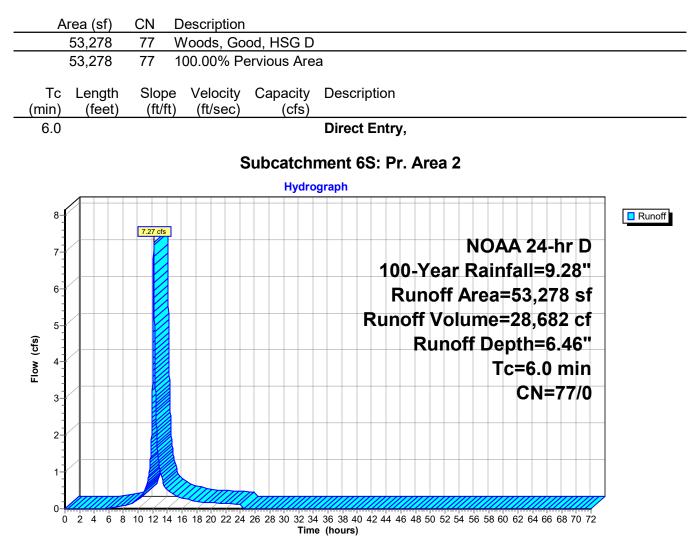


#### Summary for Subcatchment 6S: Pr. Area 2

Page 4

7.27 cfs @ 12.14 hrs, Volume= Runoff = 28,682 cf, Depth= 6.46"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

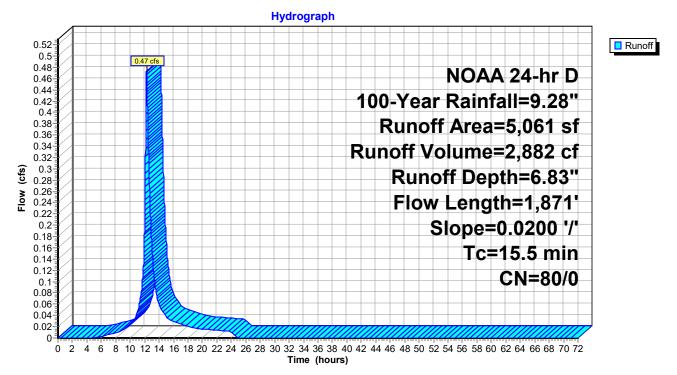


0.47 cfs @ 12.26 hrs, Volume= 2,882 cf, Depth= 6.83" Runoff =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

 A	rea (sf)	CN [	Description		
	5,061	80 >	>75% Gras	s cover, Go	ood, HSG D
	5,061	80 1	100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.2	100	0.0200	1.41		Sheet Flow,
1.1	184	0.0200	2.87		Smooth surfaces n= 0.011 P2= 3.34" <b>Shallow Concentrated Flow,</b> Paved Kv= 20.3 fps
 13.2	1,587		2.00		Direct Entry,
 15.5	1,871	Total			

#### Subcatchment 10S: Ex. Area 1B Perv.



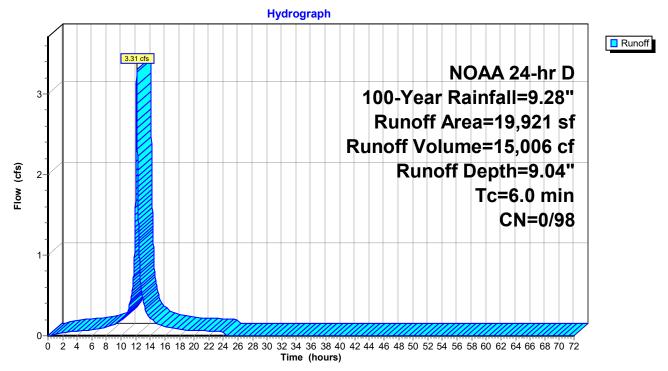
## Summary for Subcatchment 11S: Ex. Area 1B Imp.

Runoff = 3.31 cfs @ 12.14 hrs, Volume= 15,006 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Descript	ion	
19,921	98 Paved p	arking, HSG D	)
19,921	98 100.00%	Impervious A	vrea
Tc Length (min) (feet) 6.0	Slope Veloc (ft/ft) (ft/se	ity Capacity c) (cfs)	Description Direct Entry,

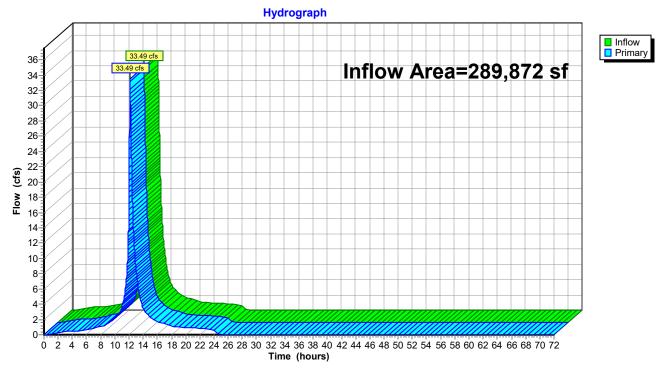
#### Subcatchment 11S: Ex. Area 1B Imp.



## Summary for Link 3L: EX POA 1 (Construction)

Inflow Are	ea =	289,872 sf, 53.80% Impervious, Inflow Depth = 8.02" for 100-Year event
Inflow	=	33.49 cfs @ 12.15 hrs, Volume= 193,747 cf
Primary	=	33.49 cfs @ 12.15 hrs, Volume= 193,747 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

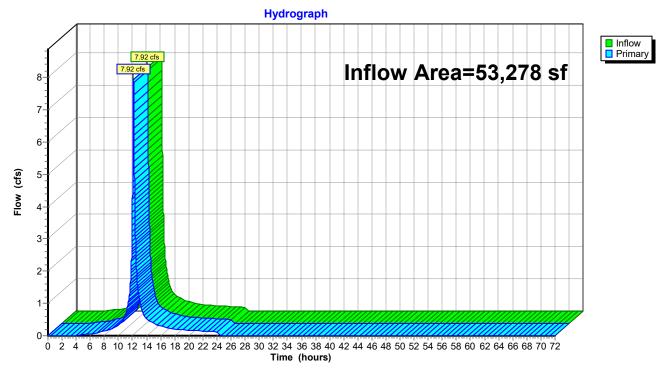


# Link 3L: EX POA 1 (Construction)

## Summary for Link 4L: EX POA 2 (Reforestation)

Inflow Area	ı =	53,278 sf,	0.00% Impervious,	Inflow Depth = 7.21"	for 100-Year event
Inflow	=	7.92 cfs @ 1	12.14 hrs, Volume=	31,995 cf	
Primary	=	7.92 cfs @ 1	12.14 hrs, Volume=	31,995 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

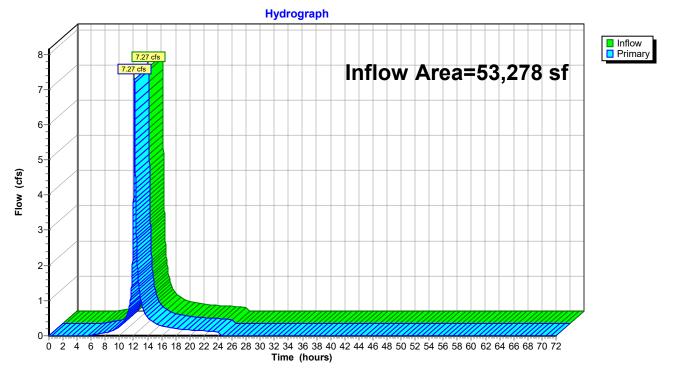


## Link 4L: EX POA 2 (Reforestation)

## Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Area =	53,278 sf,	0.00% Impervious,	Inflow Depth = 6.46"	for 100-Year event
Inflow =	7.27 cfs @ 1	2.14 hrs, Volume=	28,682 cf	
Primary =	7.27 cfs @ 1	2.14 hrs, Volume=	28,682 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Link 7L: Pr. POA 2 (Reforestation)

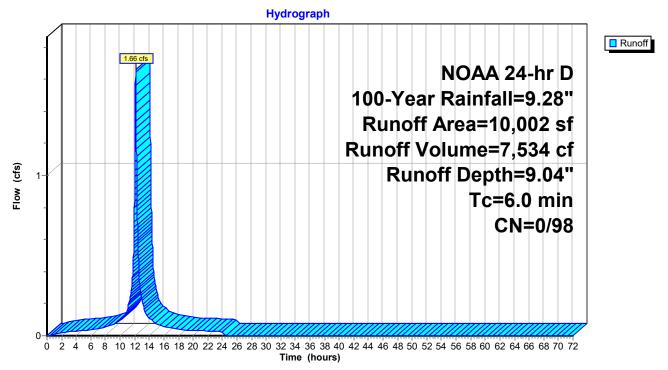
## Summary for Subcatchment B1Ai: Pr. BAsin Area 1A Imp.

Runoff = 1.66 cfs @ 12.14 hrs, Volume= 7,534 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description	
10,002	98 Paved parking, HSG D	
10,002	98 100.00% Impervious Area	
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)	
6.0	Direct Entry,	_

## Subcatchment B1Ai: Pr. BAsin Area 1A Imp.



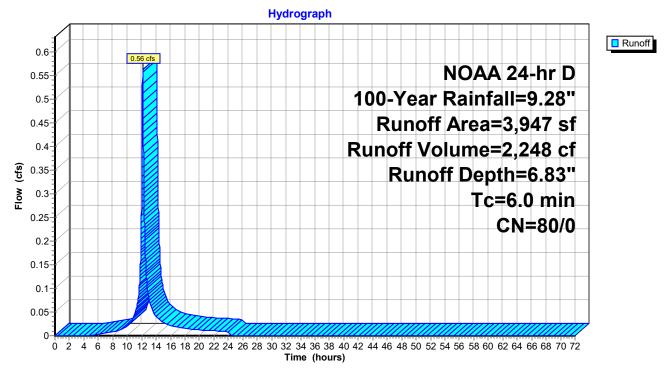
#### Summary for Subcatchment B1Ap: PR. Basin Area 1A Perv.

Runoff = 0.56 cfs @ 12.14 hrs, Volume= 2,248 cf, Depth= 6.83"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description			
	3,947	80	>75% Gras	s cover, Go	ood, HSG D	
	3,947	80	100.00% Pe	ervious Are	а	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
6.0					Direct Entry,	

#### Subcatchment B1Ap: PR. Basin Area 1A Perv.



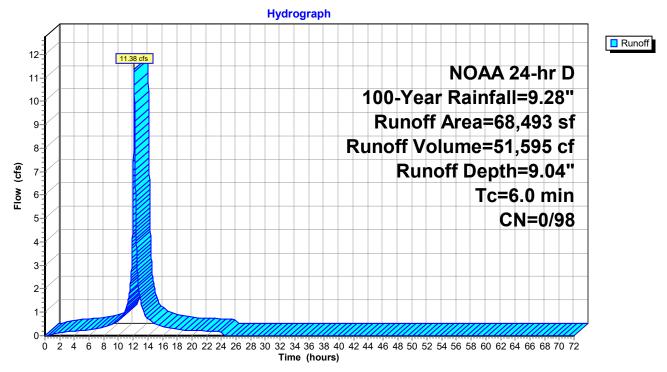
## Summary for Subcatchment B1Bi: Pr. Basin Area 1B Imp.

Runoff = 11.38 cfs @ 12.14 hrs, Volume= 51,595 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Ar	ea (sf)	CN [	Description			
6	58,493	98 F	Paved park	ing, HSG D	D	
6	58,493	98 ´	100.00% In	npervious A	Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	1	
6.0					Direct Entry,	

#### Subcatchment B1Bi: Pr. Basin Area 1B Imp.



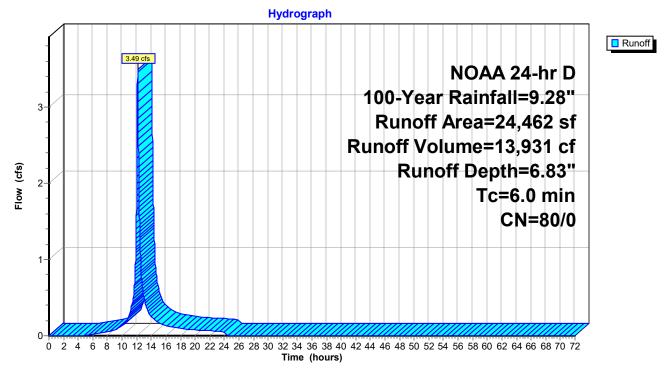
## Summary for Subcatchment B1Bp: PR. Basin Area 1B Perv.

Runoff = 3.49 cfs @ 12.14 hrs, Volume= 13,931 cf, Depth= 6.83"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description				
24,462	80 >75% Grass cover, Good, HSG D	80 >75% Grass cover, Good, HSG D			
24,462	80 100.00% Pervious Area				
Tc Length _(min) (feet)					
6.0	Direct Entry,				

#### Subcatchment B1Bp: PR. Basin Area 1B Perv.



## Summary for Subcatchment DA 1Di: Pr. Bypass 1D Imp

Runoff 0.19 cfs @ 12.14 hrs, Volume= 852 cf, Depth= 9.04" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

1,131	98 Paved p	arking, HSG D	
1,131	98 100.00%	6 Impervious Area	à
Tc Length (min) (feet			escription
6.0		D	irect Entry,
	Subo	catchment DA	1Di: Pr. Bypass 1D Imp
		Hydrogra	••••••
0.21			
0.19	0.19 cfs		NOAA 24-hr D
0.17			100-Year Rainfall=9.28"
0.15			Runoff Area=1,131 sf
0.13			Runoff Volume=852 cf
<del>ق</del> 0.11			Runoff Depth=9.04"
0.09 0.08			Tc=6.0 min
0.07			CN=0/98
0.06			
0.04			
0.02			

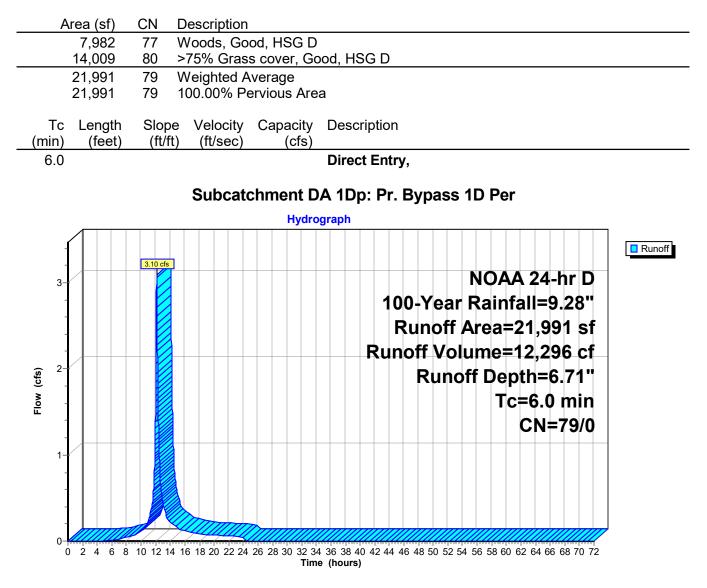
0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

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## Summary for Subcatchment DA 1Dp: Pr. Bypass 1D Per

Runoff = 3.10 cfs @ 12.14 hrs, Volume= 12,296 cf, Depth= 6.71"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"



## Summary for Subcatchment DW1Bi: Pr. Drywell Area 1B

Runoff = 1.34 cfs @ 12.14 hrs, Volume= 6,075 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Aı	rea (sf)	CN Descript			
	8,065		arking, HSG D		
	8,065	98 100.00%	Impervious A	Area	
Tc	Length	Slope Veloc	ity Capacity	Description	
(min)	(feet)	(ft/ft) (ft/se	ec) (cfs)		
6.0				Direct Entry,	
		Subc	atchment D	W1Bi: Pr. Drywell Area 1B	
				ograph	
ſ		1.34 cfs			Runoff
				NOAA 24-hr D	
-				100-Year Rainfall=9.28"	
1-	/			Runoff Area=8,065 sf	
				Runoff Volume=6,075 cf	
Flow (cfs)				Runoff Depth=9.04"	
MO :				Tc=6.0 min	
Ľ -				CN=0/98	
-					
0-4					1
Ó	2 4 6 8	10 12 14 16 18 20 2		4 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 e (hours)	

0.05

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## Summary for Subcatchment DW2Bi: Pr. Drywell Area 2B

Runoff = 0.55 cfs @ 12.14 hrs, Volume= 2,498 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description			
3,316 98 Paved parking, HSG D				
3,316				
Tc Length (min) (feet)				
6.0	Direct Entry,			
	Subcatchment DW2Bi: Pr. Drywell Area 2B			
	Hydrograph			
0.6		Runoff		
0.55	NOAA 24-hr D			
0.5	100-Year Rainfall=9.28"			
0.45	Runoff Area=3,316 sf			
0.4	Runoff Volume=2,498 cf			
ູຊູ 0.35 ວ	Runoff Depth=9.04"			
(g 0.35- у 0.3- 0.3-	Tc=6.0 min			
0.25	CN=0/98			
0.2				
0.15				
0.1				

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

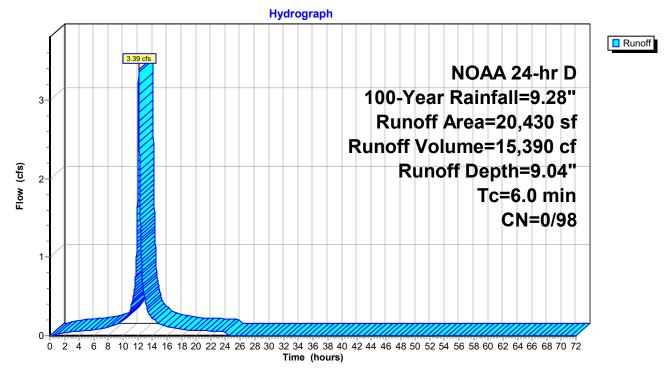
## Summary for Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.

Runoff = 3.39 cfs @ 12.14 hrs, Volume= 15,390 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description				
20,430	98	98 Paved parking, HSG D			
20,430	20,430 98 100.00% Impervious A			Area	
Tc Length (min) (feet)	Slop (ft/1	,	Capacity (cfs)	Description	
6.0				Direct Entry,	

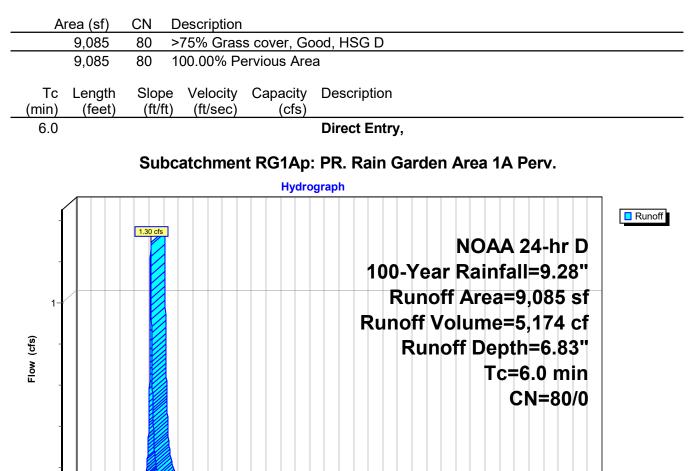
#### Subcatchment RG1Ai: Pr. Rain Garden Area 1A Imp.



#### Summary for Subcatchment RG1Ap: PR. Rain Garden Area 1A Perv.

Runoff = 1.30 cfs @ 12.14 hrs, Volume= 5,174 cf, Depth= 6.83"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"



0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond B1A: Underground Basin 1A

Inflow Area =	43,464 sf, 70.02% Impervious,	Inflow Depth = 6.02" for 100-Year event
Inflow =	6.69 cfs @ 12.15 hrs, Volume=	21,821 cf
Outflow =	6.42 cfs @ 12.18 hrs, Volume=	21,821 cf, Atten= 4%, Lag= 1.6 min
Primary =	6.42 cfs @ 12.18 hrs, Volume=	21,821 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.36' @ 12.18 hrs Surf.Area= 1,439 sf Storage= 2,869 cf

Plug-Flow detention time= 19.9 min calculated for 21,821 cf (100% of inflow) Center-of-Mass det. time= 19.8 min (796.8 - 777.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	1,318 cf	11.03'W x 130.42'L x 3.52'H Field A
			5,072 cf Overall - 1,778 cf Embedded = 3,295 cf x 40.0% Voids
#2A	19.60'	1,703 cf	Contech ChamberMaxx 2016 x 36 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		3,021 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
			L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	5.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	21.00'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=6.42 cfs @ 12.18 hrs HW=22.36' TW=0.00' (Dynamic Tailwater)

**-1=Culvert** (Passes 6.42 cfs of 13.48 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.15 cfs @ 8.41 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 5.27 cfs @ 3.87 fps)

## Pond B1A: Underground Basin 1A - Chamber Wizard Field A

# Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

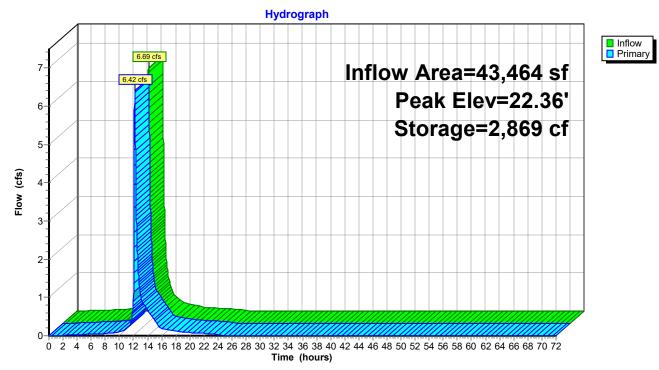
18 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 128.42' Row Length +12.0" End Stone x 2 = 130.42' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

36 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 1,703.2 cf Chamber Storage 36 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 1,777.6 cf Displacement

5,072.2 cf Field - 1,777.6 cf Chambers = 3,294.6 cf Stone x 40.0% Voids = 1,317.8 cf Stone Storage

Chamber Storage + Stone Storage = 3,021.0 cf = 0.069 af Overall Storage Efficiency = 59.6% Overall System Size = 130.42' x 11.03' x 3.52'

36 Chambers 187.9 cy Field 122.0 cy Stone



# Pond B1A: Underground Basin 1A

## Summary for Pond B1B: Underground Basin 1B

Inflow Area =	104,336 sf, 76.55% Impervious,	Inflow Depth = 8.14" for 100-Year event
Inflow =	16.65 cfs @ 12.14 hrs, Volume=	70,816 cf
Outflow =	9.99 cfs @ 12.29 hrs, Volume=	70,800 cf, Atten= 40%, Lag= 9.1 min
Primary =	9.99 cfs @ 12.29 hrs, Volume=	70,800 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.96' @ 12.29 hrs Surf.Area= 7,780 sf Storage= 14,990 cf

Plug-Flow detention time= 40.5 min calculated for 70,790 cf (100% of inflow) Center-of-Mass det. time= 40.6 min ( 800.0 - 759.3 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	6,625 cf	53.78'W x 144.65'L x 3.52'H Field A
			27,424 cf Overall - 10,861 cf Embedded = 16,563 cf x 40.0% Voids
#2A	19.60'	10,406 cf	Contech ChamberMaxx 2016 x 220 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		17,031 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
			L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	10.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=9.98 cfs @ 12.29 hrs HW=21.96' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 9.98 cfs of 12.37 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 4.11 cfs @ 7.53 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 5.88 cfs @ 4.02 fps)

## Pond B1B: Underground Basin 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

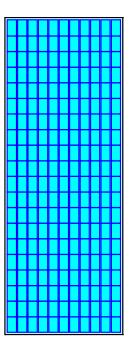
20 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 142.65' Row Length +12.0" End Stone x 2 = 144.65' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

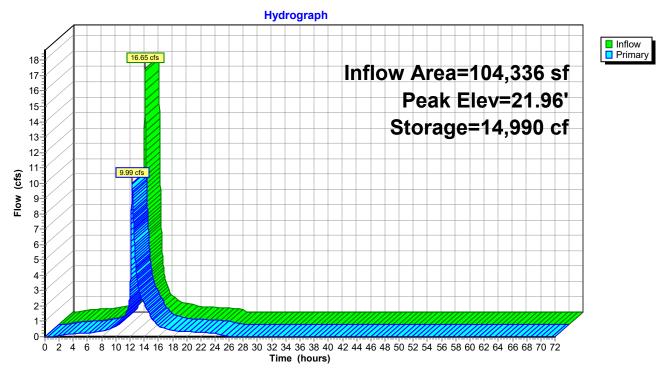
220 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 10,405.7 cf Chamber Storage 220 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 10,860.7 cf Displacement

27,423.7 cf Field - 10,860.7 cf Chambers = 16,563.0 cf Stone x 40.0% Voids = 6,625.2 cf Stone Storage

Chamber Storage + Stone Storage = 17,030.9 cf = 0.391 af Overall Storage Efficiency = 62.1% Overall System Size = 144.65' x 53.78' x 3.52'

220 Chambers 1,015.7 cy Field 613.4 cy Stone





# Pond B1B: Underground Basin 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW1B: Drywell 1B

Inflow Area =	8,065 sf,100.00% Impervious,	Inflow Depth = 9.04" for 100-Year event
Inflow =	1.34 cfs @ 12.14 hrs, Volume=	6,075 cf
Outflow =	1.33 cfs @ 12.15 hrs, Volume=	6,075 cf, Atten= 1%, Lag= 0.7 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,963 cf
Primary =	1.31 cfs @ 12.15 hrs, Volume=	4,112 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 24.45' @ 12.15 hrs Surf.Area= 497 sf Storage= 918 cf

Plug-Flow detention time= 197.7 min calculated for 6,074 cf (100% of inflow) Center-of-Mass det. time= 197.8 min (941.7 - 743.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	21.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	22.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	23.90'	15.0" Round Culvert
	-		L= 67.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 23.90' / 21.00' S= 0.0433 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	21.50'	
			Conductivity to Groundwater Elevation = 12.75' Phase-In= 0.01'

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=24.45' (Free Discharge) **2=Exfiltration** (Controls 0.02 cfs)

Primary OutFlow Max=1.31 cfs @ 12.15 hrs HW=24.45' TW=21.37' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 1.31 cfs @ 2.52 fps)

# Pond DW1B: Drywell 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

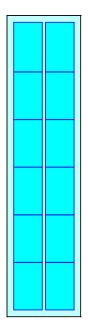
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

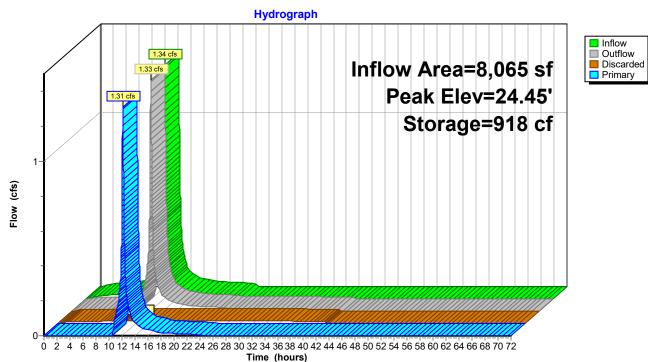
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1B: Drywell 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW2B: Drywell 2B

Inflow Area =	3,316 sf,100.00% Impervious,	Inflow Depth = 9.04" for 100-Year event
Inflow =	0.55 cfs @ 12.14 hrs, Volume=	2,498 cf
Outflow =	0.52 cfs @ 12.17 hrs, Volume=	2,498 cf, Atten= 6%, Lag= 1.8 min
Discarded =	0.01 cfs @ 12.31 hrs, Volume=	1,321 cf
Primary =	0.51 cfs @ 12.17 hrs, Volume=	1,177 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.11' @ 12.31 hrs Surf.Area= 497 sf Storage= 530 cf

Plug-Flow detention time= 163.7 min calculated for 2,498 cf (100% of inflow) Center-of-Mass det. time= 163.8 min (907.6 - 743.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	21.75'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 21.75' / 21.00' S= 0.0163 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.31 hrs HW=22.11' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

# Pond DW2B: Drywell 2B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

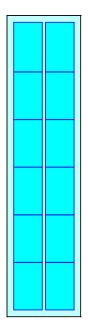
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

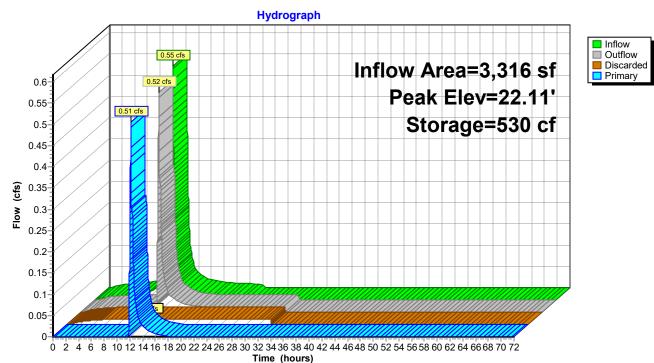
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW2B: Drywell 2B

#### Summary for Pond RG1A: Rain Garden 1A

Inflow Area =	29,515 sf, 69.22% Impervious,	Inflow Depth = 8.36" for 100-Year event
Inflow =	4.69 cfs @ 12.14 hrs, Volume=	20,564 cf
Outflow =	4.57 cfs @ 12.16 hrs, Volume=	20,564 cf, Atten= 3%, Lag= 1.2 min
Discarded =	0.07 cfs @ 12.16 hrs, Volume=	8,525 cf
Primary =	4.50 cfs @ 12.16 hrs, Volume=	12,039 cf

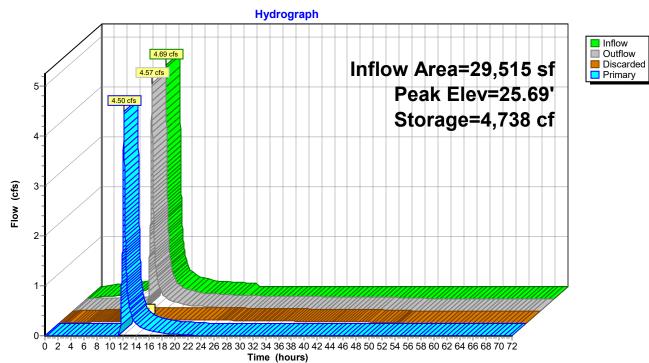
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 25.69' @ 12.16 hrs Surf.Area= 2,665 sf Storage= 4,738 cf

Plug-Flow detention time= 328.3 min calculated for 20,564 cf (100% of inflow) Center-of-Mass det. time= 328.2 min (1,088.1 - 759.8)

Volume	Inver	: Avail.S	Storage	Storage Description	n	
#1	23.00	' 5	5,586 cf	Custom Stage Dat	t <b>a (Irregular)</b> Listed	d below (Recalc)
			<b>_</b> .			
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
23.0	00	910	175.0	0	0	910
24.0	00	1,532	208.0	1,208	1,208	1,934
25.0	00	2,184	227.0	1,848	3,056	2,627
26.0	00	2,892	246.0	2,530	5,586	3,380
Device	Routing	Inve	ert Outle	et Devices		
#1	Primary	22.0	0' <b>15.0</b> '	" Round Culvert		
	,		L= 2	7.0' RCP, sq.cut er	nd projecting, Ke=	• 0.500
				/ Outlet Invert= 22.0	1 7 0	
			n= 0	.013, Flow Area= 1	.23 sf	
#2	Device 1	25.5		" x 48.0" Horiz. Orif		600
			Limit	ed to weir flow at lo	w heads	
#3	Discarded	23.0	0' <b>1.00</b>	0 in/hr Exfiltration of	over Surface area	
			Cond	ductivity to Groundw	ater Elevation = 10	0.80' Phase-In= 0.01'
				,		
Discord	Discourded OutFlow, Max-0.07 of a @ 12.16 bro. HW-25.60' (Free Discourse)					2)

**Discarded OutFlow** Max=0.07 cfs @ 12.16 hrs HW=25.69' (Free Discharge) **T-3=Exfiltration** (Controls 0.07 cfs)

Primary OutFlow Max=4.50 cfs @ 12.16 hrs HW=25.69' TW=22.34' (Dynamic Tailwater) **1=Culvert** (Passes 4.50 cfs of 10.35 cfs potential flow) **2=Orifice/Grate** (Weir Controls 4.50 cfs @ 1.44 fps)

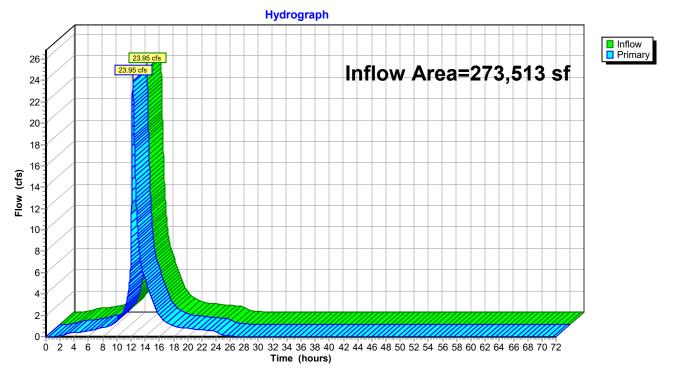


# Pond RG1A: Rain Garden 1A

# Summary for Link 3L: Pr. POA 1

Inflow Are	ea =	273,513 sf, 73.09% Impervious, Inflow Depth = 7.67" for 100-Year event	273,513 sf,	ent
Inflow	=	23.95 cfs @ 12.26 hrs, Volume= 174,722 cf	23.95 cfs @	
Primary	=	23.95 cfs @ 12.26 hrs, Volume= 174,722 cf, Atten= 0%, Lag= 0.0 min	23.95 cfs @	min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



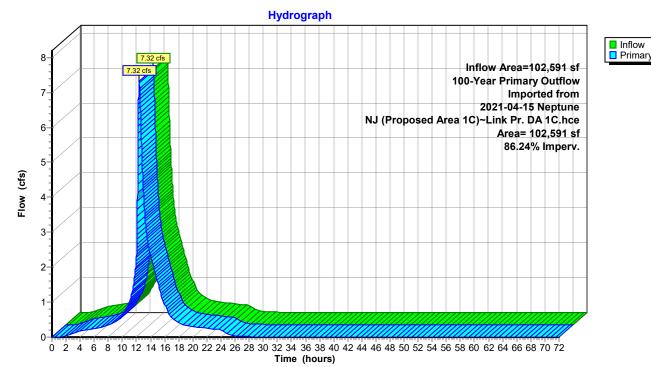
#### Link 3L: Pr. POA 1

# Summary for Link 4L: Pr DA 1C Total

Inflow Area =	102,591 sf, 86.24% Impervious,	Inflow Depth = 8.07" for 100-Year event
Inflow =	7.32 cfs @ 12.38 hrs, Volume=	68,954 cf
Primary =	7.32 cfs @ 12.38 hrs, Volume=	68,954 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

100-Year Primary Outflow Imported from 2021-04-15 Neptune, NJ (Proposed Area 1C)~Link Pr. DA 1C.hce



# Link 4L: Pr DA 1C Total

# Summary for Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.

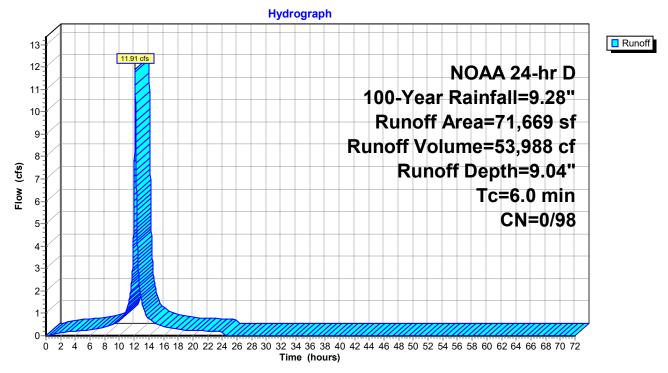
Page 1

Runoff 11.91 cfs @ 12.14 hrs, Volume= 53,988 cf, Depth= 9.04" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description			
71,669	98 Paved parking, HSG D			
71,669	98 100.00% Impervious Area			
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)			
6.0	Direct Entry,			

# Subcatchment B 1Ci: Pr. BASIN Area 1C Imp.

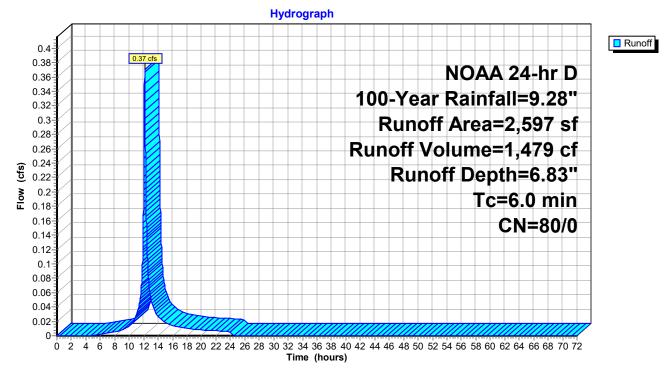


Runoff 0.37 cfs @ 12.14 hrs, Volume= 1,479 cf, Depth= 6.83" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description				
	2,597	80	80 >75% Grass cover, Good, HSG D				
	2,597	80	100.00% Pe	ervious Are	ea		
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)			
6.0					Direct Entry,		

## Subcatchment B 1Cp: PR. BASIN Area 1C Perv.

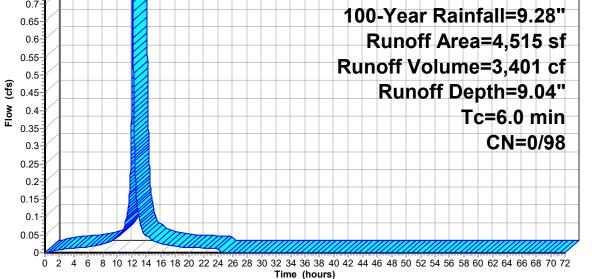


## Summary for Subcatchment DW 1Ci: Pr. Drywell 1C Imp.

Runoff = 0.75 cfs @ 12.14 hrs, Volume= 3,401 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

Area (sf)	CN Description				
4,515	98 Paved parking, HSG D				
4,515	98 100.00% Impervious Area				
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)				
6.0	Direct Entry,				
	Subcatchment DW 1Ci: Pr. Drywell 1C Imp. Hydrograph				
0.8 0.75 0.7	NOAA 24-hr D 100-Year Rainfall=9,28"	Runoff			



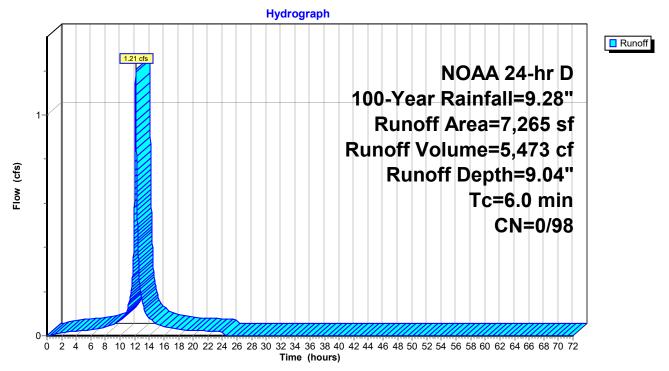
#### Summary for Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.

Runoff = 1.21 cfs @ 12.14 hrs, Volume= 5,473 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description			
	7,265	98	98 Paved parking, HSG D			
	7,265	98	100.00% In	npervious A	Area	
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)		
6.0					Direct Entry,	

#### Subcatchment RG 1Ci: Pr. Rain Garden Area 1C Imp.



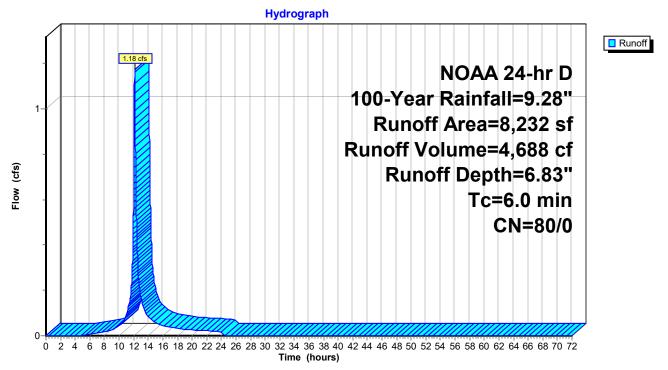
## Summary for Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.

Runoff = 1.18 cfs @ 12.14 hrs, Volume= 4,688 cf, Depth= 6.83"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description			
	8,232	80	80 >75% Grass cover, Good, HSG D			
	8,232	80	100.00% Pe	ervious Are	ea	
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)		
6.0					Direct Entry,	

## Subcatchment RG 1Cp: PR. Rain GardenArea 1C Perv.



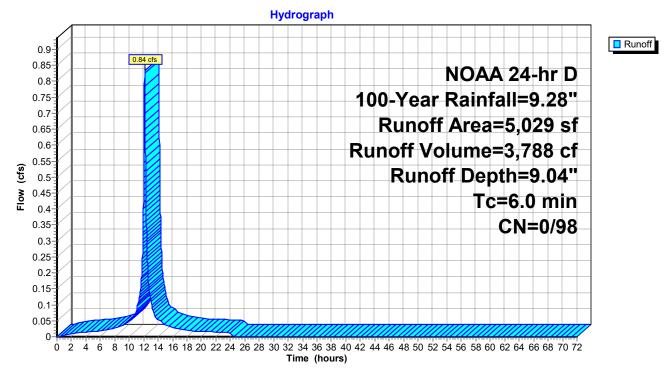
#### Summary for Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.

Runoff = 0.84 cfs @ 12.14 hrs, Volume= 3,788 cf, Depth= 9.04"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description			
	5,029	98	98 Paved parking, HSG D			
	5,029	98	100.00% Im	npervious A	Area	
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•	
6.0					Direct Entry,	

#### Subcatchment RG 2Ci: Pr. Rain Garden Area 2C Imp.



#### Summary for Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.

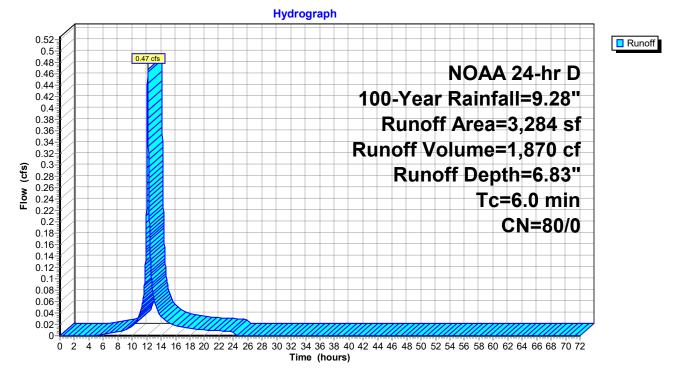
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Runoff 0.47 cfs @ 12.14 hrs, Volume= 1,870 cf, Depth= 6.83" =

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"

A	rea (sf)	CN	Description			
	3,284	80	) >75% Grass cover, Good, HSG D			
	3,284	80	100.00% Pe	ervious Are	ea	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0					Direct Entry,	

#### Subcatchment RG 2Cp: PR. Rain Garden Area 2C Perv.



#### Summary for Pond B 1C: Underground Basin 1C

Inflow Area =	=	102,591 sf, 86.249	% Impervious, I	Inflow Depth =	8.07" for	100-Year event
Inflow =		16.56 cfs @ 12.14 h	nrs, Volume=	68,980 cf		
Outflow =		7.32 cfs @ 12.38 h	nrs, Volume=	68,954 cf	, Atten= 5	6%, Lag= 14.3 min
Primary =		7.32 cfs @ 12.38 h	nrs, Volume=	68,954 cf		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.55' @ 12.38 hrs Surf.Area= 10,459 sf Storage= 18,325 cf

Plug-Flow detention time= 57.0 min calculated for 68,944 cf (100% of inflow) Center-of-Mass det. time= 57.0 min ( 810.0 - 752.9 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	8,886 cf	53.78'W x 194.47'L x 3.52'H Field A
			36,868 cf Overall - 14,653 cf Embedded = 22,215 cf x 40.0% Voids
#2A	19.60'	14,040 cf	Contech ChamberMaxx 2016 x 297 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		22,925 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=7.32 cfs @ 12.38 hrs HW=21.55' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 7.32 cfs of 11.09 cfs potential flow)

**—2=Orifice/Grate** (Orifice Controls 3.74 cfs @ 6.87 fps)

-3=Broad-Crested Rectangular Weir (Weir Controls 3.57 cfs @ 3.40 fps)

#### Pond B 1C: Underground Basin 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

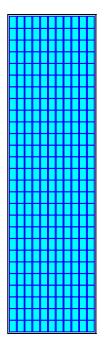
27 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 192.47' Row Length +12.0" End Stone x 2 = 194.47' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

297 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 14,039.6 cf Chamber Storage 297 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 14,653.5 cf Displacement

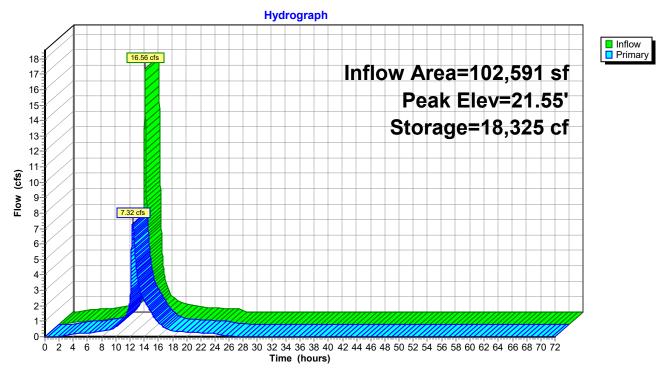
36,868.2 cf Field - 14,653.5 cf Chambers = 22,214.7 cf Stone x 40.0% Voids = 8,885.9 cf Stone Storage

Chamber Storage + Stone Storage = 22,925.5 cf = 0.526 af Overall Storage Efficiency = 62.2% Overall System Size = 194.47' x 53.78' x 3.52'

297 Chambers 1,365.5 cy Field 822.8 cy Stone



00000000000



# Pond B 1C: Underground Basin 1C

# Summary for Pond DW1C: Drywell 1C

Inflow Area =	4,515 sf,100.00% Impervious,	Inflow Depth = 9.04" for 100-Year event
Inflow =	0.75 cfs @ 12.14 hrs, Volume=	3,401 cf
Outflow =	0.72 cfs @ 12.16 hrs, Volume=	3,401 cf, Atten= 4%, Lag= 1.4 min
Discarded =	0.01 cfs @ 12.16 hrs, Volume=	1,675 cf
Primary =	0.71 cfs @ 12.16 hrs, Volume=	1,726 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 22.79' @ 12.16 hrs Surf.Area= 497 sf Storage= 766 cf

Plug-Flow detention time= 234.6 min calculated for 3,401 cf (100% of inflow) Center-of-Mass det. time= 234.7 min (978.5 - 743.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	22.40'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 22.40' / 21.00' S= 0.0304 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 12.16 hrs HW=22.79' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

# Pond DW1C: Drywell 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

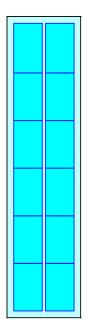
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

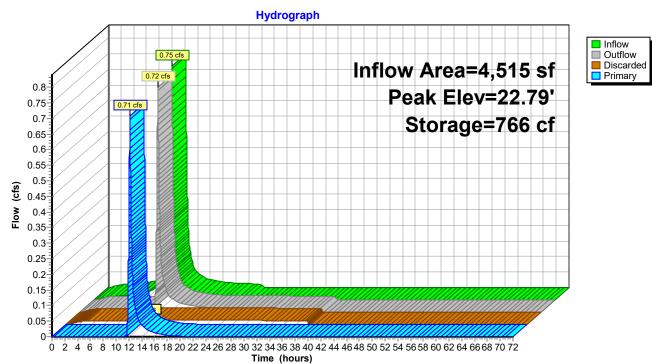
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







# Pond DW1C: Drywell 1C

#### Summary for Pond RG 1C: Rain Garden 1C

Inflow Area =	15,497 sf, 46.88% Impervious,	Inflow Depth = 7.87" for 100-Year event
Inflow =	2.38 cfs @ 12.14 hrs, Volume=	10,161 cf
Outflow =	2.36 cfs @ 12.15 hrs, Volume=	10,161 cf, Atten= 1%, Lag= 0.7 min
Discarded =	0.03 cfs @ 12.15 hrs, Volume=	2,397 cf
Primary =	2.34 cfs @ 12.15 hrs, Volume=	7,764 cf

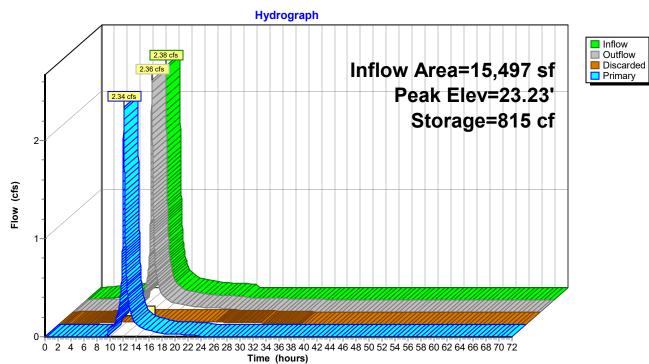
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.23' @ 12.15 hrs Surf.Area= 1,111 sf Storage= 815 cf

Plug-Flow detention time= 91.2 min calculated for 10,159 cf (100% of inflow) Center-of-Mass det. time= 91.3 min (864.5 - 773.2)

Volume	Inver	t Avail.	.Storage	Storage Description	on	
#1	22.00	)'	1,964 cf	Custom Stage Da	<b>ata (Irregular)</b> Liste	ed below (Recalc)
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
22.0	00	305	88.0	0	0	305
23.0	00	922	137.0	586	586	1,190
24.0	00	1,893	201.0	1,379	1,964	2,919
<u>Device</u> #1 #2 #3	Routing Primary Device 1	20.	45' <b>15.0</b> L= 3 Inlet n= 0 10' <b>48.0</b> Limit	.013, Flow Area= <b>" x 48.0" Horiz. Or</b> ted to weir flow at lo	.45' / 20.25' S= 0 1.23 sf <b>ifice/Grate</b> C= 0 ow heads	.0054 '/' Cc= 0.900 .600
#3	Discarded	22.		<b>0 in/hr Exfiltration</b> ductivity to Ground		-

**Discarded OutFlow** Max=0.03 cfs @ 12.15 hrs HW=23.23' (Free Discharge) **3=Exfiltration** (Controls 0.03 cfs)

**Primary OutFlow** Max=2.34 cfs @ 12.15 hrs HW=23.23' TW=20.97' (Dynamic Tailwater) -1=Culvert (Passes 2.34 cfs of 8.42 cfs potential flow) -2=Orifice/Grate (Weir Controls 2.34 cfs @ 1.16 fps)



# Pond RG 1C: Rain Garden 1C

#### Summary for Pond RG 2C: Rain Garden 2C

Inflow Area =	8,313 sf, 60.50% Impervious,	Inflow Depth = 8.17" for 100-Year event
Inflow =	1.30 cfs @ 12.14 hrs, Volume=	5,659 cf
Outflow =	1.30 cfs @ 12.15 hrs, Volume=	5,659 cf, Atten= 0%, Lag= 0.5 min
Discarded =	0.02 cfs @ 12.15 hrs, Volume=	1,635 cf
Primary =	1.28 cfs @ 12.15 hrs, Volume=	4,024 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.78' @ 12.15 hrs Surf.Area= 706 sf Storage= 466 cf

Plug-Flow detention time= 86.9 min calculated for 5,658 cf (100% of inflow) Center-of-Mass det. time= 87.0 min (851.8 - 764.8)

Volume	Inver	t Avail.S	Storage	Storage Descriptio	n		
#1	23.00	1	842 cf	Custom Stage Dat	<b>ta (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee 23.0 24.0 24.2	20 20 20	urf.Area (sq-ft) 489 773 1,429	Perim. (feet) 105.0 138.0 180.0	Inc.Store (cubic-feet) 0 626 217	Cum.Store (cubic-feet) 0 626 842	Wet.Area (sq-ft) 489 1,139 2,202	
Device	Routing	Inve	rt Outle	et Devices			
#1	Primary	21.4	21.40' <b>12.0'' Round Culvert</b> L= 62.0' RCP, sq.cut end projecting, Ke= 0.500				
		Inlet / Outlet Invert= 21.40' / 21.06' S= 0.0055 '/' Cc= 0.900					
#2	Device 1	23.7	0' <b>48.0</b>	.013, Flow Area= 0 <b>" x 48.0" Horiz. Ori</b>	fice/Grate C= 0.60	00	
#3	Discarded	23.0	0' <b>1.00</b>	Limited to weir flow at low heads <b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 6.50'			

**Discarded OutFlow** Max=0.02 cfs @ 12.15 hrs HW=23.78' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=1.28 cfs @ 12.15 hrs HW=23.78' TW=20.96' (Dynamic Tailwater) -1=Culvert (Passes 1.28 cfs of 4.46 cfs potential flow) -2=Orifice/Grate (Weir Controls 1.28 cfs @ 0.95 fps)

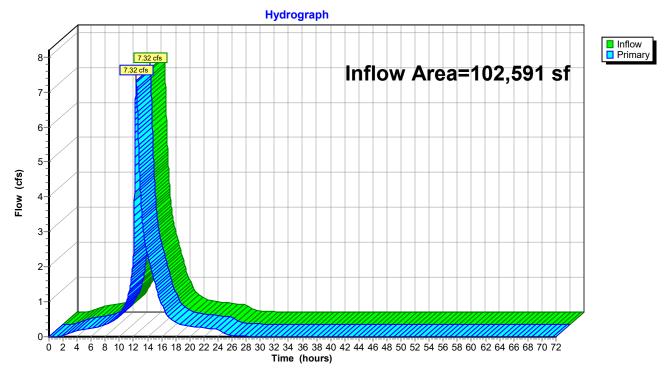
Time (hours)

# Pond RG 2C: Rain Garden 2C

# Summary for Link Pr. DA 1C: Pr. DA 1C

Inflow Area	=	102,591 sf,	86.24% Impervious	Inflow Depth = 8.07	for 100-Year event
Inflow =	=	7.32 cfs @	12.38 hrs, Volume=	68,954 cf	
Primary =	=	7.32 cfs @	12.38 hrs, Volume=	68,954 cf, Att	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



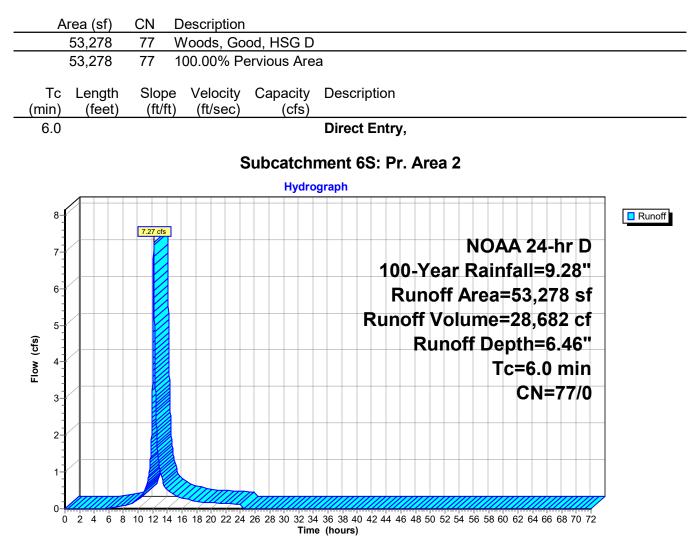
# Link Pr. DA 1C: Pr. DA 1C

#### Summary for Subcatchment 6S: Pr. Area 2

Page 1

7.27 cfs @ 12.14 hrs, Volume= Runoff = 28,682 cf, Depth= 6.46"

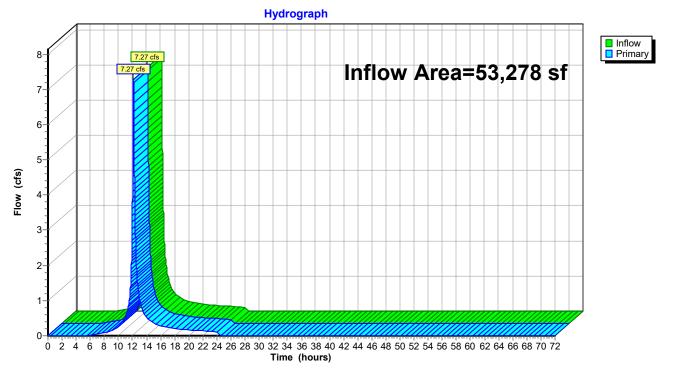
Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=9.28"



# Summary for Link 7L: Pr. POA 2 (Reforestation)

Inflow Area =	53,278 sf,	0.00% Impervious,	Inflow Depth = 6.46"	for 100-Year event
Inflow =	7.27 cfs @ 1	2.14 hrs, Volume=	28,682 cf	
Primary =	7.27 cfs @ 1	2.14 hrs, Volume=	28,682 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



# Link 7L: Pr. POA 2 (Reforestation)

# APPENDIX C-5 BMP DISCHARGE & STORAGE TABLES



# Hydrograph for Pond B 1C: Underground Basin 1C

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	0	19.10	0.00
0.20 0.40	0.00 0.00	0 0	19.10 19.10	0.00 0.00
0.40	0.00	3	19.10	0.00
0.80	0.04	21	19.11	0.00
1.00 1.20	0.06 0.08	57 109	19.11 19.13	0.00 0.00
1.40	0.10	171	19.14	0.01
1.60 1.80	0.11 0.13	241 315	19.16 19.18	0.01 0.02
2.00	0.10	389	19.19	0.02
2.20 2.40	0.15 0.16	462 532	19.21 19.23	0.05
2.40	0.16	596	19.23	0.06 0.08
2.80	0.17	654	19.26	0.10
3.00 3.20	0.18 0.19	706 753	19.27 19.28	0.11 0.13
3.40	0.19	794	19.29	0.14
3.60 3.80	0.20 0.20	830 862	19.30 19.31	0.15 0.16
4.00	0.21	890	19.31	0.17
4.20 4.40	0.21 0.22	915 937	19.32 19.32	0.18 0.19
4.40	0.22	957	19.32	0.19
4.80	0.23	975	19.33	0.21
5.00 5.20	0.23 0.24	992 1,007	19.34 19.34	0.21 0.22
5.40	0.24	1,021	19.34	0.22
5.60 5.80	0.25 0.25	1,035 1,048	19.35 19.35	0.23 0.24
6.00	0.26	1,060	19.35	0.24
6.20 6.40	0.27 0.28	1,073 1,090	19.36 19.36	0.25 0.25
6.60	0.29	1,112	19.30	0.25
6.80	0.31	1,136	19.37	0.27
7.00 7.20	0.32 0.34	1,162 1,190	19.38 19.38	0.29 0.30
7.40	0.35	1,219	19.39	0.31
7.60 7.80	0.37 0.38	1,248 1,277	19.40 19.41	0.33 0.34
8.00	0.40	1,306	19.41	0.36
8.20 8.40	0.41 0.43	1,336 1,365	19.42 19.43	0.37 0.38
8.60	0.44	1,393	19.43	0.40
8.80 9.00	0.45 0.47	1,422 1,450	19.44 19.45	0.41 0.43
9.20	0.47	1,482	19.45	0.45
9.40	0.55	1,530	19.47	0.47
9.60 9.80	0.60 0.73	1,590 1,693	19.48 19.50	0.51 0.57
10.00	0.79	1,805	19.53	0.64
10.20	0.89	1,914	19.56	0.71

# Hydrograph for Pond B 1C: Underground Basin 1C (continued)

<b>_</b> .		<b>e</b> /	_	<b>_</b> .
Time	Inflow	Storage	Elevation	Primary
<u>(hours)</u> 10.40	(cfs) 0.97	(cubic-feet)	(feet) 19.59	<u>(cfs)</u> 0.79
10.40	1.06	2,046 2,177	19.59	0.85
10.80	1.00	2,381	19.63	0.91
11.00	1.50	2,688	19.66	1.00
11.20	1.83	3,111	19.71	1.13
11.40	2.24	3,694	19.77	1.31
11.60	2.92	4,462	19.86	1.54
11.80	4.24	5,748	20.00	1.82
12.00	8.95	8,599	20.32	2.35
12.20	14.11	15,957	21.22	5.39
12.40	6.83	18,309	21.55	7.30
12.60	3.95	17,095	21.37	6.28
12.80	2.77	15,400	21.15	4.98
13.00	2.24	13,954	20.96	4.08
13.20	1.82	12,704	20.80	3.46
13.40	1.53	11,586	20.67	3.01
13.60 13.80	1.28 1.12	10,550 9,541	20.55 20.43	2.69 2.51
14.00	1.04	8,569	20.43	2.35
14.20	0.97	7,659	20.32	2.19
14.40	0.90	6,812	20.12	2.03
14.60	0.83	6,027	20.03	1.88
14.80	0.76	5,303	19.95	1.73
15.00	0.69	4,635	19.87	1.58
15.20	0.63	4,030	19.81	1.42
15.40	0.60	3,512	19.75	1.26
15.60	0.58	3,082	19.71	1.13
15.80	0.56	2,726	19.67	1.01
16.00	0.54	2,427	19.64	0.92
16.20 16.40	0.52 0.51	2,175	19.61 19.57	0.85 0.74
16.60	0.48	1,969 1,827	19.57	0.65
16.80	0.40	1,724	19.51	0.59
17.00	0.45	1,647	19.49	0.54
17.20	0.43	1,585	19.48	0.51
17.40	0.41	1,533	19.47	0.48
17.60	0.39	1,486	19.46	0.45
17.80	0.37	1,443	19.45	0.43
18.00	0.35	1,403	19.44	0.41
18.20	0.33	1,364	19.43	0.38
18.40	0.32	1,330	19.42	0.37
18.60	0.32	1,303	19.41	0.35
18.80	0.31	1,281	19.41	0.34
19.00 19.20	0.31 0.30	1,262 1,246	19.40 19.40	0.33 0.33
19.20	0.30	1,240	19.40	0.32
19.60	0.29	1,219	19.39	0.31
19.80	0.29	1,213	19.39	0.31
20.00	0.29	1,195	19.39	0.30
20.20	0.28	1,184	19.38	0.30
20.40	0.28	1,173	19.38	0.29
20.60	0.27	1,162	19.38	0.29

#### Hydrograph for Pond B 1C: Underground Basin 1C (continued)

Time	Inflow	Storage	Elevation	Primary
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
20.80 21.00	0.27 0.26	1,152 1,141	19.38 19.37	0.28 0.28
21.00	0.20	1,141	19.37	0.28
21.20	0.20	1,130	19.37	0.27
21.40	0.25	1,109	19.37	0.26
21.80	0.23	1,098	19.36	0.26
22.00	0.24	1,087	19.36	0.25
22.20	0.23	1,077	19.36	0.25
22.40	0.23	1,066	19.35	0.24
22.60	0.22	1,055	19.35	0.24
22.80	0.22	1,044	19.35	0.23
23.00	0.21	1,033	19.35	0.23
23.20	0.21	1,022	19.34	0.22
23.40	0.20	1,011	19.34	0.22
23.60	0.20	1,000	19.34	0.22
23.80	0.19	989	19.34	0.21
24.00	0.19	978	19.33	0.21
24.20	0.04	921	19.32	0.18
24.40 24.60	0.01 0.00	817 724	19.30 19.27	0.15 0.12
24.00	0.00	649	19.27	0.12
25.00	0.00	588	19.20	0.08
25.20	0.00	537	19.24	0.06
25.40	0.00	494	19.22	0.06
25.60	0.00	457	19.21	0.05
25.80	0.00	425	19.20	0.04
26.00	0.00	397	19.19	0.04
26.20	0.00	373	19.19	0.03
26.40	0.00	351	19.18	0.03
26.60	0.00	332	19.18	0.03
26.80	0.00	315	19.18	0.02
27.00	0.00	299	19.17	0.02
27.20 27.40	0.00 0.00	285 272	19.17 19.17	0.02 0.02
27.60	0.00	260	19.17	0.02
27.80	0.00	249	19.16	0.02
28.00	0.00	239	19.16	0.01
28.20	0.00	230	19.16	0.01
28.40	0.00	222	19.15	0.01
28.60	0.00	214	19.15	0.01
28.80	0.00	206	19.15	0.01
29.00	0.00	199	19.15	0.01
29.20	0.00	193	19.15	0.01
29.40	0.00	187	19.14	0.01
29.60	0.00	181	19.14	0.01
29.80	0.00	176	19.14	0.01
30.00 30.20	0.00 0.00	171 166	19.14 19.14	0.01 0.01
30.20	0.00	162	19.14	0.01
30.60	0.00	157	19.14	0.01
30.80	0.00	153	19.14	0.01
31.00	0.00	149	19.14	0.01

#### Hydrograph for Pond B1A: Underground Basin 1A

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	0	19.10	0.00
0.20 0.40	0.00 0.00	0 0	19.10 19.10	0.00 0.00
0.40	0.00	0	19.10	0.00
0.80	0.01	3	19.11	0.00
1.00	0.01	8	19.11	0.00
1.20	0.01 0.01	14	19.12	0.00
1.40 1.60	0.01	22 28	19.14 19.15	0.00 0.01
1.80	0.02	34	19.16	0.01
2.00	0.02	39	19.17	0.01
2.20	0.02	43 47	19.18	0.02
2.40 2.60	0.02 0.02	47 49	19.18 19.19	0.02 0.02
2.80	0.02	51	19.19	0.02
3.00	0.02	53	19.19	0.02
3.20 3.40	0.03	54 55	19.19 19.20	0.02 0.03
3.40 3.60	0.03 0.03	57	19.20	0.03
3.80	0.03	58	19.20	0.03
4.00	0.03	58	19.20	0.03
4.20	0.03 0.03	59	19.20	0.03 0.03
4.40 4.60	0.03	60 61	19.20 19.21	0.03
4.80	0.03	62	19.21	0.03
5.00	0.03	63	19.21	0.03
5.20	0.03	64	19.21 19.21	0.03
5.40 5.60	0.04 0.04	65 66	19.21	0.03 0.04
5.80	0.04	67	19.22	0.04
6.00	0.04	68	19.22	0.04
6.20 6.40	0.04 0.04	69 71	19.22 19.22	0.04 0.04
6.60	0.04	71	19.22	0.04
6.80	0.05	75	19.23	0.04
7.00	0.05	77	19.23	0.05
7.20	0.05	79	19.24	0.05
7.40 7.60	0.05 0.06	81 83	19.24 19.24	0.05 0.05
7.80	0.06	85	19.25	0.06
8.00	0.06	88	19.25	0.06
8.20 8.40	0.07 0.07	90 92	19.26 19.26	0.06 0.07
8.60	0.07	92 94	19.20	0.07
8.80	0.07	96	19.27	0.07
9.00	0.08	98	19.27	0.07
9.20 9.40	0.08 0.09	101 105	19.27 19.28	0.08 0.08
9.40 9.60	0.09	105	19.20	0.08
9.80	0.11	116	19.30	0.10
10.00	0.12	122	19.31	0.11
10.20	0.13	128	19.32	0.12

#### Hydrograph for Pond B1A: Underground Basin 1A (continued)

Time	lu fla	Character	Elevation	
Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
10.40	0.14	133	19.33	0.13
10.60	0.15	139	19.34	0.14
10.80	0.18	151	19.36	0.16
11.00 11.20	0.21 0.26	167 187	19.39 19.43	0.19 0.22
11.40	0.31	215	19.47	0.27
11.60	0.41	255	19.54	0.32
11.80	1.64	699	19.95	0.52
12.00 12.20	3.41 6.02	1,871 2,858	21.01 22.34	0.86 6.30
12.40	2.95	2,550	21.77	3.26
12.60	1.68	2,333	21.49	1.99
12.80	1.14	2,143	21.28	1.35
13.00	0.91	2,017	21.16	1.07
13.20 13.40	0.73 0.61	1,907 1,773	21.05 20.92	0.89 0.83
13.60	0.50	1,591	20.74	0.79
13.80	0.43	1,378	20.55	0.73
14.00	0.40	1,170	20.36	0.67
14.20 14.40	0.37 0.34	979 809	20.19 20.04	0.62 0.56
14.60	0.34	656	19.91	0.50
14.80	0.28	520	19.79	0.46
15.00	0.25	400	19.69	0.41
15.20 15.40	0.23 0.21	296 223	19.61 19.49	0.36 0.28
15.40 15.60	0.21	223 191	19.49	0.28
15.80	0.19	178	19.41	0.21
16.00	0.19	171	19.40	0.19
16.20	0.18	166	19.39	0.18
16.40 16.60	0.17 0.16	161 157	19.38 19.37	0.18 0.17
16.80	0.15	152	19.36	0.16
17.00	0.15	148	19.36	0.15
17.20	0.14	143	19.35	0.14
17.40 17.60	0.13 0.12	138 133	19.34 19.33	0.14 0.13
17.80	0.12	133	19.33	0.13
18.00	0.11	124	19.31	0.11
18.20	0.10	119	19.31	0.10
18.40	0.09	115	19.30 19.29	0.10
18.60 18.80	0.09 0.09	112 110	19.29	0.09 0.09
19.00	0.09	109	19.29	0.09
19.20	0.08	107	19.29	0.09
19.40	0.08	106	19.28	0.08
19.60 19.80	0.08 0.08	105 103	19.28 19.28	0.08 0.08
20.00	0.08	103	19.28	0.08
20.20	0.07	100	19.27	0.08
20.40	0.07	99	19.27	0.07
20.60	0.07	97	19.27	0.07

#### Hydrograph for Pond B1A: Underground Basin 1A (continued)

Time	Inflow	Storage	Elevation	Primary
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
20.80	0.07	96	19.27	0.07
21.00	0.07	94	19.26	0.07
21.20	0.06	93	19.26	0.07
21.40	0.06	91	19.26	0.06
21.60	0.06	90	19.26	0.06
21.80	0.06	88	19.25	0.06
22.00	0.06	87	19.25	0.06
22.20	0.05 0.05	85 84	19.25	0.06
22.40 22.60	0.05	82	19.25 19.24	0.05 0.05
22.80	0.05	80	19.24	0.05
23.00	0.05	79	19.24	0.05
23.20	0.04	77	19.23	0.05
23.40	0.04	75	19.23	0.05
23.60	0.04	73	19.23	0.04
23.80	0.04	72	19.22	0.04
24.00	0.04	70	19.22	0.04
24.20	0.01	60	19.20	0.03
24.40	0.00	46	19.18	0.02
24.60	0.00	36	19.16	0.01
24.80	0.00	30	19.15	0.01
25.00	0.00	25	19.14	0.01
25.20 25.40	0.00 0.00	22 19	19.14 19.13	0.00 0.00
25.40	0.00	19	19.13	0.00
25.80	0.00	15	19.13	0.00
26.00	0.00	14	19.12	0.00
26.20	0.00	13	19.12	0.00
26.40	0.00	12	19.12	0.00
26.60	0.00	11	19.12	0.00
26.80	0.00	10	19.12	0.00
27.00	0.00	10	19.12	0.00
27.20	0.00	9	19.12	0.00
27.40	0.00	9	19.12	0.00
27.60	0.00	8	19.11	0.00
27.80 28.00	0.00	8 7	19.11 19.11	0.00
28.00	0.00 0.00	7	19.11	0.00 0.00
28.40	0.00	7	19.11	0.00
28.60	0.00	7	19.11	0.00
28.80	0.00	6	19.11	0.00
29.00	0.00	6	19.11	0.00
29.20	0.00	6	19.11	0.00
29.40	0.00	6	19.11	0.00
29.60	0.00	5	19.11	0.00
29.80	0.00	5	19.11	0.00
30.00	0.00	5	19.11	0.00
30.20	0.00	5	19.11	0.00
30.40	0.00	5	19.11	0.00
30.60 30.80	0.00 0.00	5 5	19.11 19.11	0.00 0.00
30.80	0.00	5	19.11	0.00
51.00	0.00	4	19.11	0.00

#### Hydrograph for Pond B1B: Underground Basin 1B

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	Ó	19.10	0.00
0.20 0.40	0.00 0.00	0 0	19.10 19.10	0.00 0.00
0.40	0.00	3	19.10	0.00
0.80	0.04	20	19.11	0.00
1.00 1.20	0.06 0.08	55 103	19.12 19.13	0.00 0.00
1.40	0.10	161	19.15	0.01
1.60	0.11	223	19.17	0.02
1.80 2.00	0.12 0.13	286 347	19.19 19.21	0.03 0.05
2.20	0.14	404	19.23	0.07
2.40 2.60	0.15 0.16	455 499	19.25 19.26	0.08 0.10
2.80	0.16	537	19.20	0.10
3.00	0.17	570	19.28	0.13
3.20 3.40	0.18 0.18	598 621	19.29 19.30	0.14 0.15
3.60	0.19	642	19.31	0.16
3.80 4.00	0.19 0.20	660 675	19.31 19.32	0.17 0.18
4.00	0.20	689	19.32	0.18
4.40	0.21	702	19.33	0.19
4.60 4.80	0.22 0.22	715 728	19.33 19.33	0.20 0.21
5.00	0.23	740	19.34	0.21
5.20 5.40	0.24 0.24	752 764	19.34 19.35	0.22 0.23
5.60	0.24	704	19.35	0.23
5.80	0.25	786	19.35	0.24
6.00 6.20	0.26 0.27	797 810	19.36 19.36	0.25 0.25
6.40	0.29	826	19.37	0.26
6.60	0.30 0.32	846	19.37 19.38	0.27
6.80 7.00	0.32	868 892	19.30	0.29 0.30
7.20	0.35	918	19.39	0.32
7.40 7.60	0.37 0.39	943 969	19.40 19.41	0.34 0.35
7.80	0.41	995	19.42	0.37
8.00	0.42	1,021	19.43	0.39
8.20 8.40	0.44 0.46	1,047 1,072	19.44 19.44	0.41 0.43
8.60	0.48	1,097	19.45	0.44
8.80 9.00	0.50 0.52	1,122 1,147	19.46 19.47	0.46 0.48
9.20	0.56	1,177	19.48	0.50
9.40 9.60	0.61 0.67	1,221	19.49 19.51	0.54 0.58
9.80 9.80	0.87	1,278 1,342	19.51	0.58
10.00	0.79	1,409	19.55	0.69
10.20	0.85	1,478	19.57	0.75

#### Hydrograph for Pond B1B: Underground Basin 1B (continued)

			_	5.
Time	Inflow (ofa)	Storage	Elevation	Primary
(hours) 10.40	(cfs) 0.91	(cubic-feet) 1,547	(feet) 19.60	<u>(cfs)</u> 0.81
10.40	1.00	1,629	19.60	0.85
10.80	1.00	1,801	19.64	0.92
11.00	1.52	2,099	19.68	1.04
11.20	1.86	2,494	19.74	1.21
11.40	2.27	3,027	19.81	1.43
11.60	2.97	3,729	19.92	1.67
11.80	4.28	4,944	20.10	2.00
12.00	9.03	7,662	20.52	2.63
12.20	14.07	14,126	21.66	7.96
12.40	6.90	14,529	21.79	8.81
12.60	4.00	12,890	21.41	6.49
12.80	2.82	11,245	21.11	4.80
13.00 13.20	2.28 1.86	9,998	20.90	3.82
13.20	1.57	8,970 8,064	20.73 20.58	3.20 2.78
13.60	1.32	7,205	20.30	2.53
13.80	1.16	6,334	20.31	2.34
14.00	1.08	5,523	20.19	2.15
14.20	1.01	4,792	20.08	1.96
14.40	0.94	4,143	19.98	1.78
14.60	0.87	3,567	19.89	1.62
14.80	0.80	3,062	19.82	1.44
15.00	0.73	2,637	19.76	1.27
15.20	0.67	2,279	19.70	1.12
15.40	0.64	1,987	19.66	1.00
15.60 15.80	0.62	1,756 1,571	19.63	0.90 0.83
16.00	0.60 0.58	1,438	19.60 19.56	0.83
16.20	0.56	1,356	19.54	0.65
16.40	0.54	1,301	19.52	0.60
16.60	0.52	1,260	19.50	0.57
16.80	0.50	1,226	19.49	0.54
17.00	0.48	1,197	19.48	0.52
17.20	0.46	1,169	19.48	0.50
17.40	0.44	1,142	19.47	0.48
17.60	0.42	1,116	19.46	0.46
17.80	0.40	1,089	19.45	0.44
18.00	0.38	1,063	19.44	0.42
18.20	0.36	1,037	19.43	0.40
18.40 18.60	0.36 0.35	1,014 997	19.43 19.42	0.38 0.37
18.80	0.35	983	19.42	0.36
19.00	0.34	973	19.41	0.36
19.20	0.34	963	19.41	0.35
19.40	0.33	954	19.41	0.34
19.60	0.33	946	19.40	0.34
19.80	0.32	939	19.40	0.33
20.00	0.32	931	19.40	0.33
20.20	0.31	924	19.40	0.32
20.40	0.31	916	19.39	0.32
20.60	0.30	909	19.39	0.31

#### Hydrograph for Pond B1B: Underground Basin 1B (continued)

Time	Inflow	Storage	Elevation	Primary
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
20.80	0.30	902	19.39	0.31
21.00	0.29	894	19.39	0.30
21.20 21.40	0.29 0.28	887 879	19.38 19.38	0.30 0.29
21.60	0.28	872	19.38	0.29
21.80	0.27	864	19.38	0.29
22.00	0.27	857	19.38	0.28
22.20	0.27	849	19.37	0.28
22.40 22.60	0.26 0.26	841 833	19.37 19.37	0.27 0.27
22.80	0.25	826	19.37	0.26
23.00	0.25	818	19.36	0.26
23.20	0.24	810	19.36	0.25
23.40	0.24	802	19.36	0.25
23.60 23.80	0.23 0.23	794 786	19.36 19.35	0.24 0.24
23.80	0.23	780	19.35	0.24
24.20	0.06	720	19.33	0.20
24.40	0.01	613	19.30	0.15
24.60	0.00	522	19.27	0.11
24.80 25.00	0.00 0.00	454 401	19.25 19.23	0.08 0.07
25.00 25.20	0.00	358	19.23	0.07
25.40	0.00	324	19.20	0.04
25.60	0.00	296	19.19	0.04
25.80	0.00	272	19.19	0.03
26.00	0.00	251 234	19.18	0.03
26.20 26.40	0.00 0.00	234 218	19.18 19.17	0.02 0.02
26.60	0.00	205	19.17	0.02
26.80	0.00	193	19.16	0.02
27.00	0.00	182	19.16	0.01
27.20 27.40	0.00	173 164	19.16 19.15	0.01 0.01
27.60	0.00 0.00	104	19.15	0.01
27.80	0.00	149	19.15	0.01
28.00	0.00	143	19.15	0.01
28.20	0.00	137	19.14	0.01
28.40 28.60	0.00 0.00	132 127	19.14 19.14	0.01 0.01
28.80	0.00	127	19.14	0.01
29.00	0.00	117	19.14	0.01
29.20	0.00	113	19.14	0.01
29.40	0.00	110	19.14	0.01
29.60	0.00	106	19.13	0.00
29.80 30.00	0.00 0.00	103 100	19.13 19.13	0.00 0.00
30.20	0.00	97	19.13	0.00
30.40	0.00	94	19.13	0.00
30.60	0.00	91	19.13	0.00
30.80	0.00	89 87	19.13	0.00
31.00	0.00	87	19.13	0.00

# **APPENDIX C-6** C.R. 16 25-YEAR STORM EVENT HYDROGRAPH



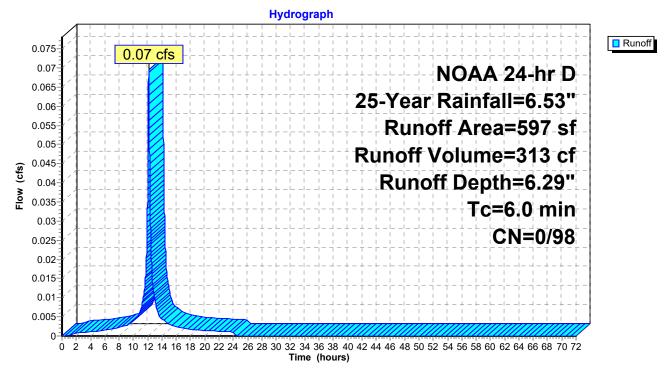
#### Summary for Subcatchment 14S: Pr. Area R.O.W. Imp.

Runoff = 0.07 cfs @ 12.14 hrs, Volume= 313 cf, Depth= 6.29"

Runoff by SCS TR-20 method, UH=Delmarva, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.53"

Area	(sf)	CN	Description		
5	597	98 Paved parking, HSG A			
Ę	597	98	100.00% In	npervious A	Area
	ngth feet)	Slope (ft/ft)		Capacity (cfs)	
6.0					Direct Entry,

#### Subcatchment 14S: Pr. Area R.O.W. Imp.

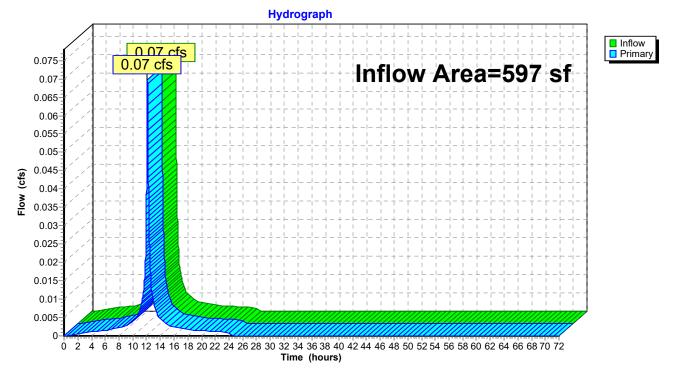


#### Summary for Link 13L: Pr. POA R.O.W.

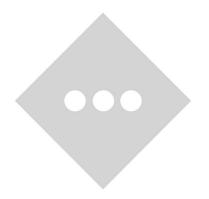
Inflow Area	a =	597 sf,100.00% Impervious,	Inflow Depth = 6.29" for 25-Year event
Inflow	=	0.07 cfs @ 12.14 hrs, Volume=	313 cf
Primary	=	0.07 cfs @ 12.14 hrs, Volume=	313 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

#### Link 13L: Pr. POA R.O.W.



# APPENDIX C-7 BMP WQ Event Hydrographs



#### Summary for Pond B 1C: Underground Basin 1C

Inflow Area =	102,591 sf, 86.24% Impervious,	Inflow Depth = 0.73" for WQ Storm event
Inflow =	3.93 cfs @ 1.12 hrs, Volume=	6,216 cf
Outflow =	1.24 cfs @ 1.40 hrs, Volume=	6,197 cf, Atten= 68%, Lag= 16.4 min
Primary =	1.24 cfs @ 1.40 hrs, Volume=	6,197 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 19.75' @ 1.40 hrs Surf.Area= 10,459 sf Storage= 3,466 cf

Plug-Flow detention time= 76.2 min calculated for 6,196 cf (100% of inflow) Center-of-Mass det. time= 76.5 min (150.5 - 74.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	8,886 cf	53.78'W x 194.47'L x 3.52'H Field A
			36,868 cf Overall - 14,653 cf Embedded = 22,215 cf x 40.0% Voids
#2A	19.60'	14,040 cf	Contech ChamberMaxx 2016 x 297 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		22,925 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

**Primary OutFlow** Max=1.24 cfs @ 1.40 hrs HW=19.75' TW=0.00' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.24 cfs of 1.72 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 1.24 cfs @ 2.74 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond B 1C: Underground Basin 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

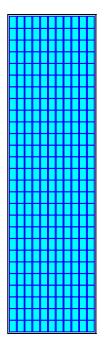
27 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 192.47' Row Length +12.0" End Stone x 2 = 194.47' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

297 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 14,039.6 cf Chamber Storage 297 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 14,653.5 cf Displacement

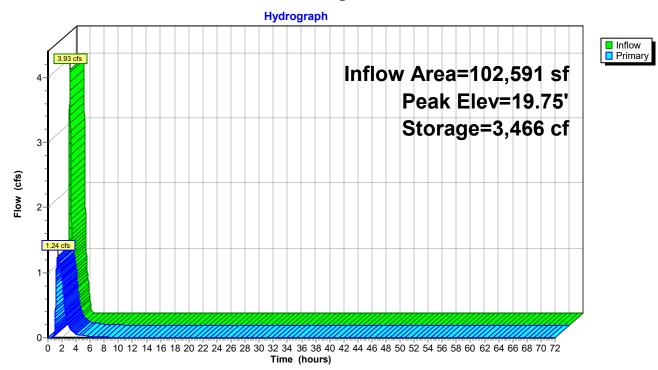
36,868.2 cf Field - 14,653.5 cf Chambers = 22,214.7 cf Stone x 40.0% Voids = 8,885.9 cf Stone Storage

Chamber Storage + Stone Storage = 22,925.5 cf = 0.526 af Overall Storage Efficiency = 62.2% Overall System Size = 194.47' x 53.78' x 3.52'

297 Chambers 1,365.5 cy Field 822.8 cy Stone



00000000000



## Pond B 1C: Underground Basin 1C

#### Summary for Pond DW1C: Drywell 1C

Inflow Area =	4,515 sf,100.00% Impervious,	Inflow Depth = 1.03" for WQ Storm event
Inflow =	0.25 cfs @ 1.12 hrs, Volume=	389 cf
Outflow =	0.01 cfs @ 2.03 hrs, Volume=	389 cf, Atten= 95%, Lag= 54.3 min
Discarded =	0.01 cfs @ 2.03 hrs, Volume=	389 cf
Primary =	0.00 cfs $\overline{@}$ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.55' @ 2.03 hrs Surf.Area= 497 sf Storage= 319 cf

Plug-Flow detention time= 221.8 min calculated for 389 cf (100% of inflow) Center-of-Mass det. time= 221.8 min (295.7 - 73.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	22.40'	15.0" Round Culvert
	-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 22.40' / 21.00' S= 0.0304 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	20.50'	1.000 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 2.03 hrs HW=21.55' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=20.50' TW=19.10' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.00 cfs)

#### Pond DW1C: Drywell 1C - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

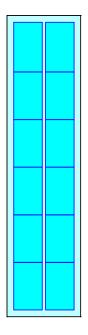
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

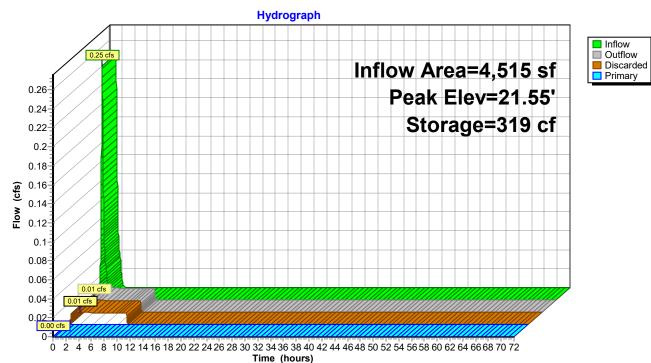
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







## Pond DW1C: Drywell 1C

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#### Summary for Pond RG 1C: Rain Garden 1C

Inflow Area =	15,497 sf, 46.8	8% Impervious,	Inflow Depth = 0.58"	for WQ Storm event
Inflow =	0.45 cfs @ 1.13	3 hrs, Volume=	745 cf	
Outflow =	0.02 cfs @ 2.07	hrs, Volume=	745 cf, Atter	n= 95%, Lag= 56.4 min
Discarded =	0.02 cfs @ 2.07	hrs, Volume=	745 cf	
Primary =	0.00 cfs @ 0.00	) hrs, Volume=	0 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.06' @ 2.07 hrs Surf.Area= 972 sf Storage= 644 cf

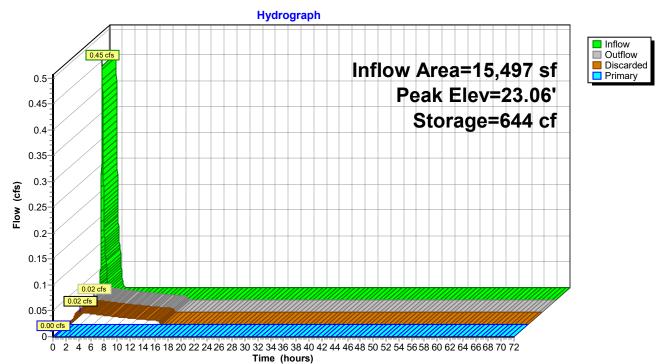
Plug-Flow detention time= 307.7 min calculated for 745 cf (100% of inflow) Center-of-Mass det. time= 307.8 min (383.7 - 75.9)

Volume	Inver	t Avail.	Storage	Storage Description	n	
#1	22.00	)'	1,964 cf	Custom Stage Da	<b>ta (Irregular)</b> Liste	d below (Recalc)
Elevatio (fee 22.0 23.0 24.0	et) )0 )0	Surf.Area (sq-ft) 305 922 1,893	Perim. (feet) 88.0 137.0 201.0	Inc.Store (cubic-feet) 0 586 1,379	Cum.Store (cubic-feet) 0 586 1,964	Wet.Area (sq-ft) 305 1,190 2,919
Device	Routing	Inv	ert Outle	et Devices		
#1	Primary	20.4		" Round Culvert 7.0' RCP, sq.cut e	nd projecting Ke	= 0.500
			Inlet	/ Outlet Invert= 20.4 .013, Flow Area= 1	45' / 20.25' S= 0.0	
#2	Device 1	23.	10' <b>48.0</b> '	" x 48.0" Horiz. Ori	fice/Grate C= 0.0	600
#3	Discarded	22.0	00' <b>1.00</b>	ed to weir flow at lo <b>0 in/hr Exfiltration</b> ductivity to Groundv	over Surface area	

**Discarded OutFlow** Max=0.02 cfs @ 2.07 hrs HW=23.06' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=22.00' TW=19.10' (Dynamic Tailwater)

**1=Culvert** (Passes 0.00 cfs of 4.93 cfs potential flow) **2=Orifice/Grate** (Controls 0.00 cfs)



## Pond RG 1C: Rain Garden 1C

#### Summary for Pond RG 2C: Rain Garden 2C

Inflow Area =	8,313 sf, 60.50% Impervio	ous, Inflow Depth = 0.69"	for WQ Storm event
Inflow =	0.30 cfs @ 1.13 hrs, Volum	e= 481 cf	
Outflow =	0.02 cfs @ 2.05 hrs, Volum	e= 481 cf, Atten=	= 95%, Lag= 55.4 min
Discarded =	0.02 cfs @ 2.05 hrs, Volum	e= 481 cf	
Primary =	0.00 cfs @ 0.00 hrs, Volum	e= 0 cf	

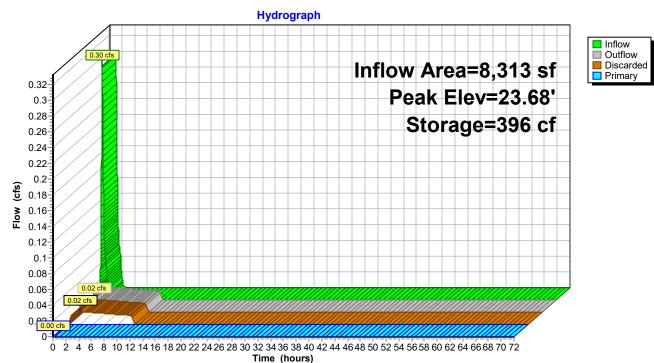
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.68' @ 2.05 hrs Surf.Area= 676 sf Storage= 396 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 233.6 min ( 308.8 - 75.2 )

Inve	rt Avail	.Storage	Storage Description	on		
23.0	0'	842 cf	Custom Stage Da	<b>ta (Irregular)</b> Listed	l below (Recalc)	
on 5 et) 00 00 20	(sq-ft) 489 773	Perim. (feet) 105.0 138.0 180.0	Inc.Store (cubic-feet) 0 626 217	Cum.Store (cubic-feet) 0 626 842	Wet.Area (sq-ft) 489 1,139 2,202	
Routing			et Devices		, -	
Primary	21.	L= 6 Inlet	2.0' RCP, sq.cut e / Outlet Invert= 21.	40' / 21.06' S= 0.0		
Device 1	23.	70' <b>48.0</b>	" x 48.0" Horiz. Ori	fice/Grate C= 0.6	00	
Discarde	d 23.	00' 1.00	0 in/hr Exfiltration	over Surface area	50'	
	23.0 on s et) 00 20 Routing Primary Device 1	23.00'         on       Surf.Area         et)       (sq-ft)         00       489         00       773         20       1,429         Routing       Inv         Primary       21.         Device 1       23.	23.00'         842 cf           on         Surf.Area         Perim.           et)         (sq-ft)         (feet)           00         489         105.0           00         773         138.0           20         1,429         180.0           Routing         Invert         Outlet           Primary         21.40'         12.0           L= 6         Inlet         n= 0           Device 1         23.70'         48.0           Limit         Discarded         23.00'         1.00	23.00'         842 cf         Custom Stage Da           on         Surf.Area         Perim.         Inc.Store           et)         (sq-ft)         (feet)         (cubic-feet)           00         489         105.0         0           00         773         138.0         626           20         1,429         180.0         217           Routing         Invert         Outlet Devices           Primary         21.40'         12.0" Round Culvert           L= 62.0'         RCP, sq.cut et           Inlet / Outlet Invert= 21.         n= 0.013, Flow Area= 0           Device 1         23.70'         48.0" x 48.0" Horiz. Ori           Limited to weir flow at low         23.00'         1.000 in/hr Exfiltration	23.00'842 cfCustom Stage Data (Irregular) ListedonSurf.AreaPerim.Inc.StoreCum.Storeet)(sq-ft)(feet)(cubic-feet)(cubic-feet)00489105.00000773138.0626626201,429180.0217842RoutingInvertOutlet DevicesPrimary21.40'12.0" Round CulvertL= 62.0'RCP, sq.cut end projecting, Ke=Inlet / Outlet Invert=21.40' / 21.06'S= 0.0n= 0.013, Flow Area=0.79 sfDevice 123.70'48.0" x 48.0" Horiz. Orifice/GrateC= 0.6Limited to weir flow at low heads23.00'1.000 in/hr Exfiltration over Surface area	23.00'       842 cf       Custom Stage Data (Irregular) Listed below (Recalc)         on       Surf.Area       Perim.       Inc.Store       Cum.Store       Wet.Area         et)       (sq-ft)       (feet)       (cubic-feet)       (cubic-feet)       (sq-ft)         00       489       105.0       0       0       489         00       773       138.0       626       626       1,139         20       1,429       180.0       217       842       2,202         Routing         Invert       Outlet Devices         Primary       21.40'       12.0" Round Culvert         L= 62.0'       RCP, sq.cut end projecting, Ke= 0.500       Inlet / Outlet Invert= 21.40' / 21.06' S= 0.0055 '/' Cc= 0.900         Device 1       23.70'       48.0" K 48.0" Horiz. Orifice/Grate       C= 0.600         Limited to weir flow at low heads       C= 0.600       Limited to weir flow at low heads

**Discarded OutFlow** Max=0.02 cfs @ 2.05 hrs HW=23.68' (Free Discharge) **3=Exfiltration** (Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=23.00' TW=19.10' (Dynamic Tailwater) **1=Culvert** (Passes 0.00 cfs of 3.29 cfs potential flow) **2=Orifice/Grate** (Controls 0.00 cfs)



#### Pond RG 2C: Rain Garden 2C

#### Summary for Pond B1A: Underground Basin 1A

Inflow Area =	43,464 sf,	70.02% Impervious,	Inflow Depth = 0.25" for WQ Storm event
Inflow =	0.57 cfs @	1.12 hrs, Volume=	919 cf
Outflow =	0.36 cfs @	1.26 hrs, Volume=	919 cf, Atten= 37%, Lag= 8.2 min
Primary =	0.36 cfs @	1.26 hrs, Volume=	919 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 19.61' @ 1.26 hrs Surf.Area= 1,439 sf Storage= 296 cf

Plug-Flow detention time= 23.2 min calculated for 919 cf (100% of inflow) Center-of-Mass det. time= 23.1 min ( 97.8 - 74.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	1,318 cf	11.03'W x 130.42'L x 3.52'H Field A
			5,072 cf Overall - 1,778 cf Embedded = 3,295 cf x 40.0% Voids
#2A	19.60'	1,703 cf	Contech ChamberMaxx 2016 x 36 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		3,021 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	5.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	21.00'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.36 cfs @ 1.26 hrs HW=19.61' TW=0.00' (Dynamic Tailwater)

**-1=Culvert** (Passes 0.36 cfs of 1.11 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 0.36 cfs @ 2.63 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond B1A: Underground Basin 1A - Chamber Wizard Field A

## Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

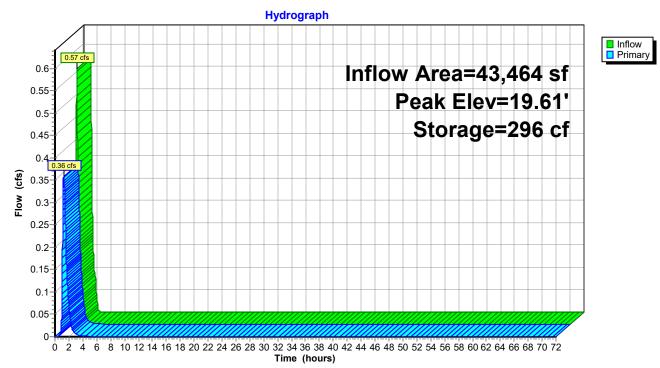
18 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 128.42' Row Length +12.0" End Stone x 2 = 130.42' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

36 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 1,703.2 cf Chamber Storage 36 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 1,777.6 cf Displacement

5,072.2 cf Field - 1,777.6 cf Chambers = 3,294.6 cf Stone x 40.0% Voids = 1,317.8 cf Stone Storage

Chamber Storage + Stone Storage = 3,021.0 cf = 0.069 af Overall Storage Efficiency = 59.6% Overall System Size = 130.42' x 11.03' x 3.52'

36 Chambers 187.9 cy Field 122.0 cy Stone



## Pond B1A: Underground Basin 1A

#### Summary for Pond B1B: Underground Basin 1B

Inflow Area =	104,336 sf, 76.55% Impervious,	Inflow Depth = 0.72" for WQ Storm event
Inflow =	3.90 cfs @ 1.12 hrs, Volume=	6,258 cf
Outflow =	1.46 cfs @ 1.37 hrs, Volume=	6,247 cf, Atten= 62%, Lag= 14.6 min
Primary =	1.46 cfs @ 1.37 hrs, Volume=	6,247 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 19.83' @ 1.37 hrs Surf.Area= 7,780 sf Storage= 3,119 cf

Plug-Flow detention time= 53.0 min calculated for 6,246 cf (100% of inflow) Center-of-Mass det. time= 53.5 min (128.1 - 74.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	19.10'	6,625 cf	53.78'W x 144.65'L x 3.52'H Field A
			27,424 cf Overall - 10,861 cf Embedded = 16,563 cf x 40.0% Voids
#2A	19.60'	10,406 cf	Contech ChamberMaxx 2016 x 220 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 11 rows
		17,031 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	19.10'	18.0" Round Culvert
	-		L= 25.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 19.10' / 18.85' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf
#2	Device 1	19.10'	10.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	20.50'	1.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=1.46 cfs @ 1.37 hrs HW=19.83' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 1.46 cfs of 2.10 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 1.46 cfs @ 2.90 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Pond B1B: Underground Basin 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 11 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

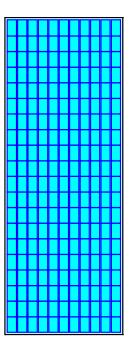
20 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 142.65' Row Length +12.0" End Stone x 2 = 144.65' Base Length 11 Rows x 51.4" Wide + 5.6" Spacing x 10 + 12.0" Side Stone x 2 = 53.78' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

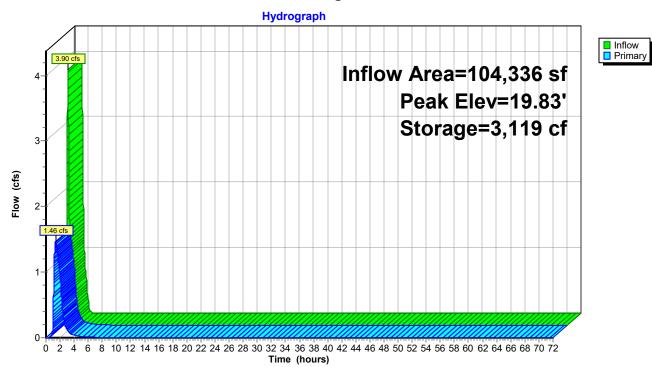
220 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 11 Rows = 10,405.7 cf Chamber Storage 220 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 11 Rows = 10,860.7 cf Displacement

27,423.7 cf Field - 10,860.7 cf Chambers = 16,563.0 cf Stone x 40.0% Voids = 6,625.2 cf Stone Storage

Chamber Storage + Stone Storage = 17,030.9 cf = 0.391 af Overall Storage Efficiency = 62.1% Overall System Size = 144.65' x 53.78' x 3.52'

220 Chambers 1,015.7 cy Field 613.4 cy Stone





## Pond B1B: Underground Basin 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW1B: Drywell 1B

Inflow Area =	8,065 sf,100.00% Impervious,	Inflow Depth = 1.03" for WQ Storm event
Inflow =	0.44 cfs @ 1.12 hrs, Volume=	695 cf
Outflow =	0.01 cfs @ 2.10 hrs, Volume=	695 cf, Atten= 97%, Lag= 58.5 min
Discarded =	0.01 cfs @ 2.10 hrs, Volume=	695 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 23.35' @ 2.10 hrs Surf.Area= 497 sf Storage= 616 cf

Plug-Flow detention time= 397.3 min calculated for 695 cf (100% of inflow) Center-of-Mass det. time= 397.3 min (471.3 - 73.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	21.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	22.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	23.90'	15.0" Round Culvert
	-		L= 67.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 23.90' / 21.00' S= 0.0433 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Discarded	21.50'	
			Conductivity to Groundwater Elevation = 12.75' Phase-In= 0.01'

**Discarded OutFlow** Max=0.01 cfs @ 2.10 hrs HW=23.35' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=21.50' TW=19.10' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.00 cfs)

#### Pond DW1B: Drywell 1B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

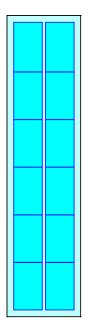
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

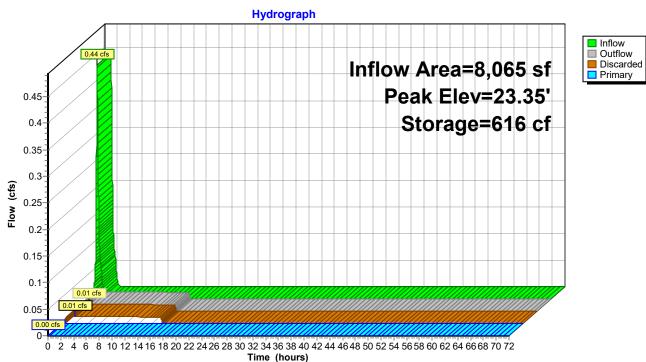
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone







## Pond DW1B: Drywell 1B

2021-04-15 Neptune, NJ (Proposed 1)

#### Prepared by {enter your company name here} HydroCAD® 10.00-25 s/n 10626 © 2019 HydroCAD Software Solutions LLC

#### Summary for Pond DW2B: Drywell 2B

Inflow Area =	3,316 sf,100.00% Impervious,	Inflow Depth = 1.03" for WQ Storm event
Inflow =	0.18 cfs @ 1.12 hrs, Volume=	286 cf
Outflow =	0.01 cfs @ 1.93 hrs, Volume=	286 cf, Atten= 93%, Lag= 48.3 min
Discarded =	0.01 cfs @ 1.93 hrs, Volume=	286 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 21.30' @ 1.93 hrs Surf.Area= 497 sf Storage= 220 cf

Plug-Flow detention time= 157.2 min calculated for 286 cf (100% of inflow) Center-of-Mass det. time= 157.2 min (231.1 - 73.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	20.50'	462 cf	11.03'W x 45.02'L x 3.52'H Field A
			1,751 cf Overall - 595 cf Embedded = 1,155 cf x 40.0% Voids
#2A	21.00'	571 cf	Contech ChamberMaxx 2016 x 12 Inside #1
			Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf
			Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf
			Row Length Adjustment= +0.32' x 6.63 sf x 2 rows
		1.033 cf	Total Available Storage

1,033 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Routing	Invert	Outlet Devices
Primary	21.75'	15.0" Round Culvert
-		L= 46.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 21.75' / 21.00' S= 0.0163 '/' Cc= 0.900
		n= 0.013, Flow Area= 1.23 sf
Discarded	20.50'	1.000 in/hr Exfiltration over Surface area
		Conductivity to Groundwater Elevation = 10.54' Phase-In= 0.01'
	Primary	Primary 21.75'

**Discarded OutFlow** Max=0.01 cfs @ 1.93 hrs HW=21.30' (Free Discharge) **2=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=20.50' TW=19.10' (Dynamic Tailwater) ↓ 1=Culvert (Controls 0.00 cfs)

#### Pond DW2B: Drywell 2B - Chamber Wizard Field A

Chamber Model = Contech ChamberMaxx 2016 (Contech® ChamberMaxx® capped at 47.2cf for air pocket)

Inside= 49.6"W x 25.2"H => 6.63 sf x 7.12'L = 47.2 cf Outside= 49.6"W x 30.0"H => 6.92 sf x 7.12'L = 49.3 cf Row Length Adjustment= +0.32' x 6.63 sf x 2 rows

51.4" Wide + 5.6" Spacing = 57.0" C-C Row Spacing

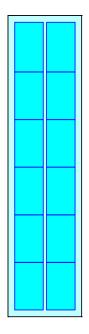
6 Chambers/Row x 7.12' Long +0.32' Row Adjustment = 43.02' Row Length +12.0" End Stone x 2 = 45.02' Base Length 2 Rows x 51.4" Wide + 5.6" Spacing x 1 + 12.0" Side Stone x 2 = 11.03' Base Width 6.0" Base + 30.3" Chamber Height + 6.0" Cover = 3.52' Field Height

12 Chambers x 47.2 cf +0.32' Row Adjustment x 6.63 sf x 2 Rows = 570.5 cf Chamber Storage 12 Chambers x 49.3 cf +0.32' Row Adjustment x 6.92 sf x 2 Rows = 595.5 cf Displacement

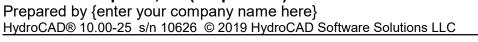
1,750.8 cf Field - 595.5 cf Chambers = 1,155.3 cf Stone x 40.0% Voids = 462.1 cf Stone Storage

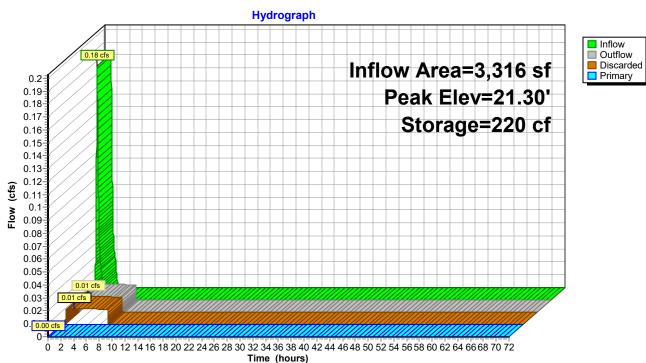
Chamber Storage + Stone Storage = 1,032.7 cf = 0.024 af Overall Storage Efficiency = 59.0%Overall System Size = 45.02' x 11.03' x 3.52'

12 Chambers 64.8 cy Field 42.8 cy Stone









## Pond DW2B: Drywell 2B

#### Summary for Pond RG1A: Rain Garden 1A

Inflow Area =	29,515 sf,	69.22% Impervious,	Inflow Depth = 0.77" for WQ Storm event
Inflow =	1.18 cfs @	1.12 hrs, Volume=	1,892 cf
Outflow =	0.04 cfs @	2.09 hrs, Volume=	1,892 cf, Atten= 96%, Lag= 58.2 min
Discarded =	0.04 cfs @	2.09 hrs, Volume=	1,892 cf
Primary =	0.00 cfs @	0.00 hrs, Volume=	0 cf

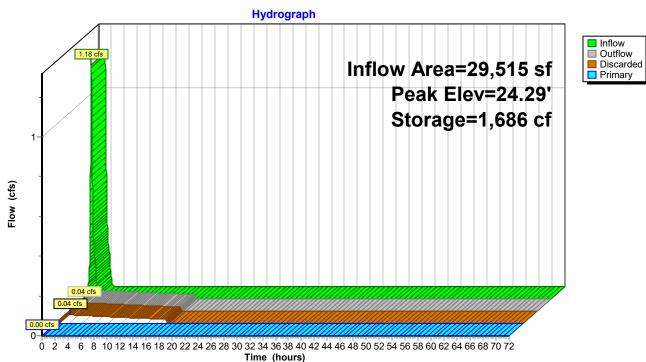
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 24.29' @ 2.09 hrs Surf.Area= 1,712 sf Storage= 1,686 cf

Plug-Flow detention time= 404.0 min calculated for 1,892 cf (100% of inflow) Center-of-Mass det. time= 404.0 min ( 478.8 - 74.8 )

Volume	Inver	t Avail.S	Storage	Storage Description	n	
#1	23.00	' 5	5,586 cf	Custom Stage Da	<b>ta (Irregular)</b> Listed	d below (Recalc)
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)
23.0	00	910	175.0	0	0	910
24.0	00	1,532	208.0	1,208	1,208	1,934
25.0	00	2,184	227.0	1,848	3,056	2,627
26.0	00	2,892	246.0	2,530	5,586	3,380
Device	Routing	Inve	ert Outle	et Devices		
#1	Primary	22.0		<b>" Round Culvert</b> 7.0' RCP, sq.cut e	and projecting Ke-	0.500
				/ Outlet Invert= 22.		
				.013, Flow Area= 1		
#2	Device 1	25.5		" x 48.0" Horiz. Ori		800
			Limit	ted to weir flow at lo	ow heads	
#3	Discarded	23.0	0' <b>1.00</b>	0 in/hr Exfiltration	over Surface area	
			Con	ductivity to Groundw	vater Elevation = 10	0.80' Phase-In= 0.01'

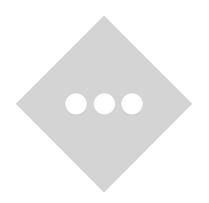
**Discarded OutFlow** Max=0.04 cfs @ 2.09 hrs HW=24.29' (Free Discharge) **3=Exfiltration** (Controls 0.04 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=23.00' TW=19.10' (Dynamic Tailwater) **1=Culvert** (Passes 0.00 cfs of 3.58 cfs potential flow) **2=Orifice/Grate** (Controls 0.00 cfs)

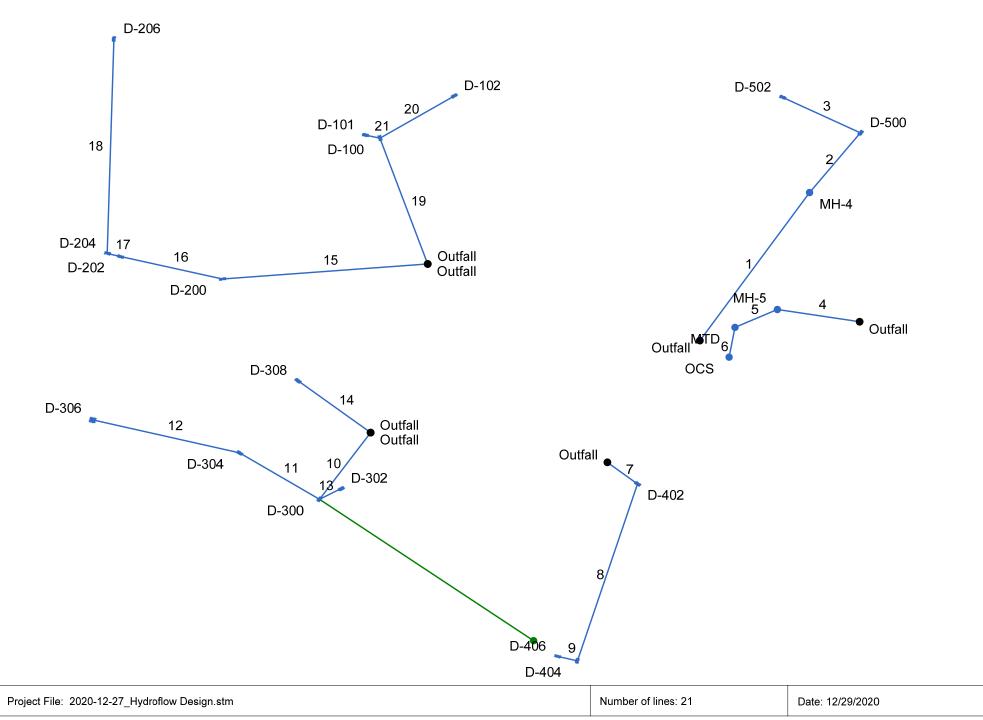


## Pond RG1A: Rain Garden 1A

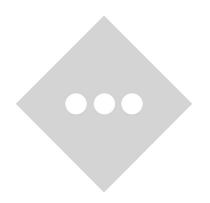
# **APPENDIX C-8** HydraFlow Routing Diagram



# Hydraflow Storm Sewers Extension for Autodesk® Civil 3D® Plan



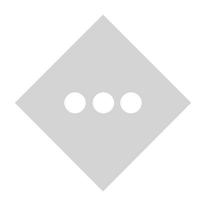
# **APPENDIX C-9** Pipe Conveyance Summary

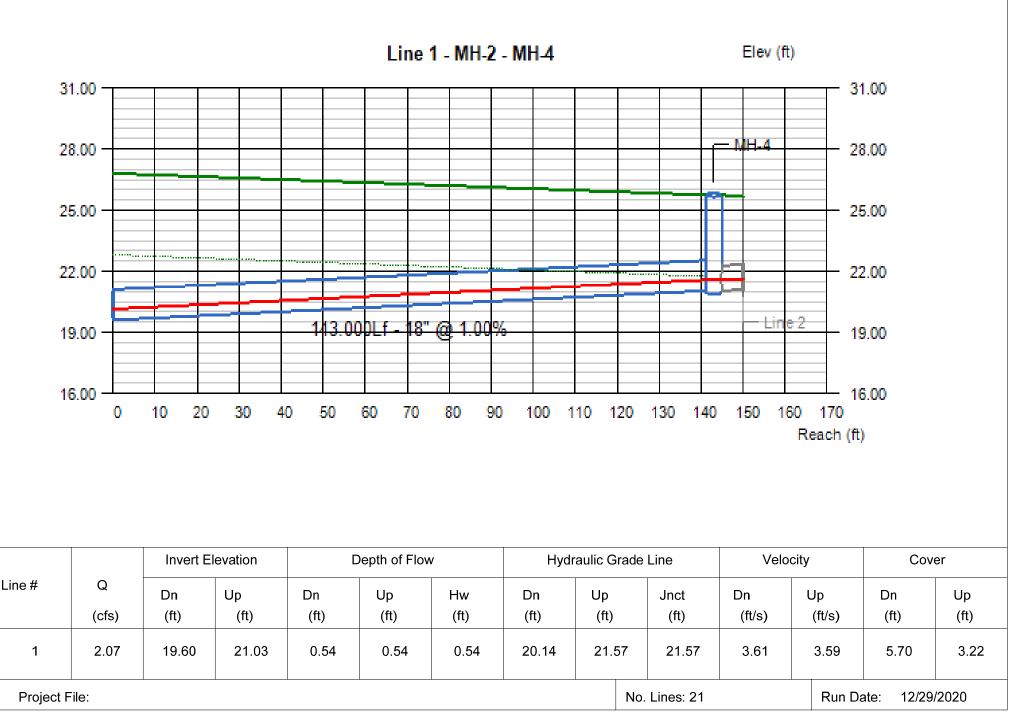


SED

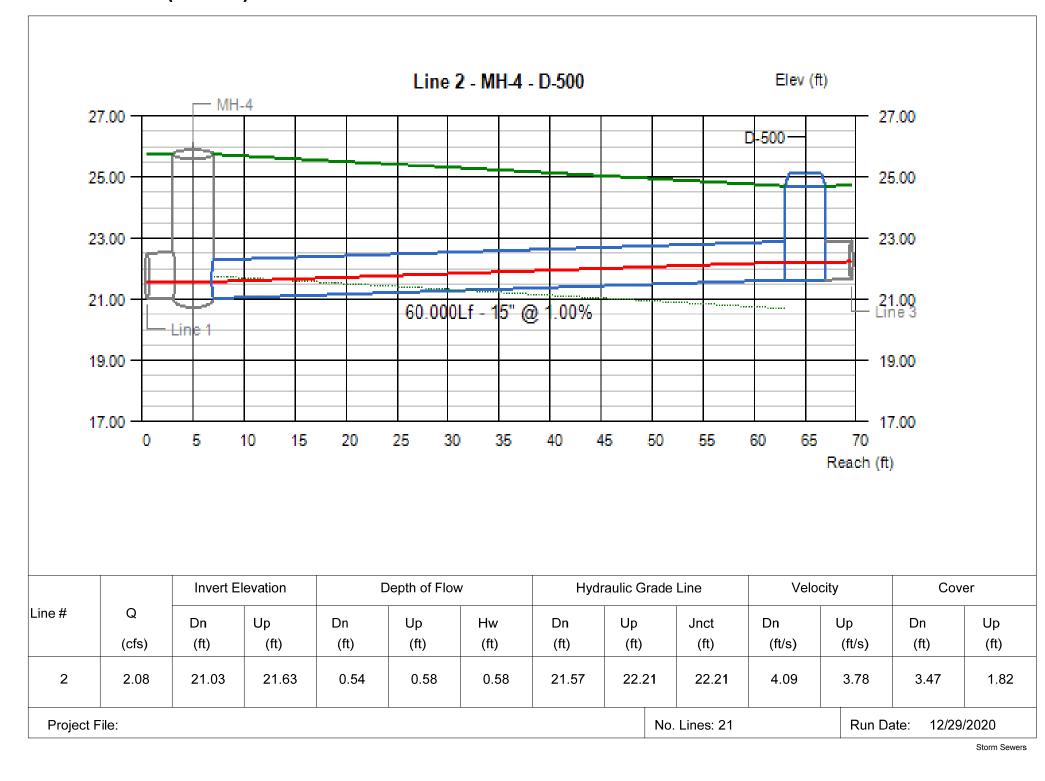
Line No.	Line ID	Gnd/Rim El Dn	Gnd/Rim El Up	Invert Dn	Invert Up	Line Size	Line Length	Flow Rate	Vel Dn	Capac Full	HGL Dn	HGL Up	Drng Area	Runoff Coeff	Тс	i Inlet	n-val Pipe	i Sys
		(ft)	(ft)	(ft)	(ft)	(in)	(ft)	(cfs)	(ft/s)	(cfs)	(ft)	(ft)	(ac)	(C)	(min)	(in/hr)		(in/hr)
1	MH-2 - MH-4	26.80	25.75	19.60	21.03	18	143.000	2.07	3.61	12.41	20.14	21.57	0.00	0.00	10.6	0.00	0.011	6.33
2	MH-4 - D-500	25.75	24.70	21.03	21.63	15	60.000	2.08	4.09	7.63	21.57	22.21	0.16	0.94	10.4	6.47	0.011	6.38
3	D-500 - D502	24.70	25.25	21.63	22.23	15	60.000	1.14	2.06	7.63	22.21	22.65 j	0.22	0.80	10.0	6.47	0.011	6.47
4	OF - MH-5	14.75	25.75	14.95	15.53	18	56.000	9.96	5.64	12.63	16.44	16.75 j	0.00	0.00	10.1	0.00	0.011	0.00
5	MH-5 - MTD	25.75	26.80	15.53	15.85	18	32.000	9.96	6.49	12.41	16.75	17.07	0.00	0.00	10.1	0.00	0.011	0.00
6	MTD - OCS	26.80	26.25	18.85	19.10	18	25.000	9.96	7.81	12.41	19.87	20.32	0.00	0.00	10.0	0.00	0.011	0.00
7	D-400 - D-402	24.20	24.40	19.60	19.80	18	27.000	1.66	1.03	10.68	20.89	20.28	0.00	0.00	10.8	0.00	0.011	6.30
8	D-402 - D-404	24.40	23.75	19.80	20.95	15	152.000	1.70	3.87	6.64	20.28	21.47	0.14	0.97	10.1	6.47	0.011	6.44
9	D-404 - D-406	23.75	24.25	20.95	21.06	15	14.000	0.82	1.72	6.76	21.47	21.42 j	0.13	0.98	10.0	6.47	0.011	6.47
10	MH-3 - D-300	25.40	23.30	19.60	19.92	15	65.000	5.32	5.32	5.35	20.55	20.96	0.11	0.98	12.2	6.47	0.011	6.01
11	D-300 - D-304	23.30	24.20	19.92	20.25	15	66.000	1.97	1.61	5.40	21.79	21.84	0.12	0.97	11.5	6.47	0.011	6.14
12	D-304 - D-306	24.20	24.65	20.25	20.76	15	102.000	1.32	1.08	5.40	21.86	21.89	0.23	0.89	10.0	6.47	0.011	6.47
13	D-302 - D-300	23.30	22.50	19.92	20.00	15	17.000	2.95	2.40	5.24	21.79	21.82	0.48	0.95	10.0	6.47	0.011	6.47
14	D-308 - MH-3	25.40	24.95	19.60	21.23	15	65.000	1.17	3.12	12.09	20.03	21.66 j	0.19	0.95	10.0	6.47	0.011	6.47
15	MH-1 - D-200	27.00	24.65	19.60	20.98	15	138.000	5.30	5.41	7.63	20.53	21.91	0.31	0.94	11.9	6.47	0.011	6.08
16	D-200 - D-202	24.65	25.05	20.98	21.69	15	71.000	3.56	3.62	7.63	21.91	22.45 j	0.21	0.97	11.6	6.47	0.011	6.14
17	D-202 - D-204	25.05	24.90	21.69	21.79	15	9.000	2.31	2.95	8.04	22.45	22.40 j	0.28	0.96	11.5	6.47	0.011	6.15
18	D-204 - D-206	24.90	26.95	21.79	23.13	15	178.000	0.69	1.17	6.62	22.40	23.45 j	0.12	0.89	10.0	6.47	0.011	6.47
19	MH-1 - D-100	27.00	26.65	21.10	22.83	15	109.000	4.57	5.01	9.61	21.97	23.70 j	0.18	0.98	10.7	6.47	0.011	6.31
20	D-100 - D-102	26.65	27.35	22.83	24.05	15	61.000	0.38	0.42	10.79	23.70	24.29 j	0.07	0.84	10.0	6.47	0.011	6.47
21	D-101 - D-100	26.65	26.10	22.83	23.03	15	10.000	3.16	3.49	10.79	23.70	23.75 j	0.52	0.94	10.0	6.47	0.011	6.47
Project	File: 2020-12-27_Hydroflow De	sign.stm								Numbe	er of lines:	21		Date	: 12/29/	2020	L	
NOTES	NOTES: Intensity = 182.59 / (Inlet time + 19.10) ^ 0.99 Return period = 25 Yrs. ; ** Critical depth																	

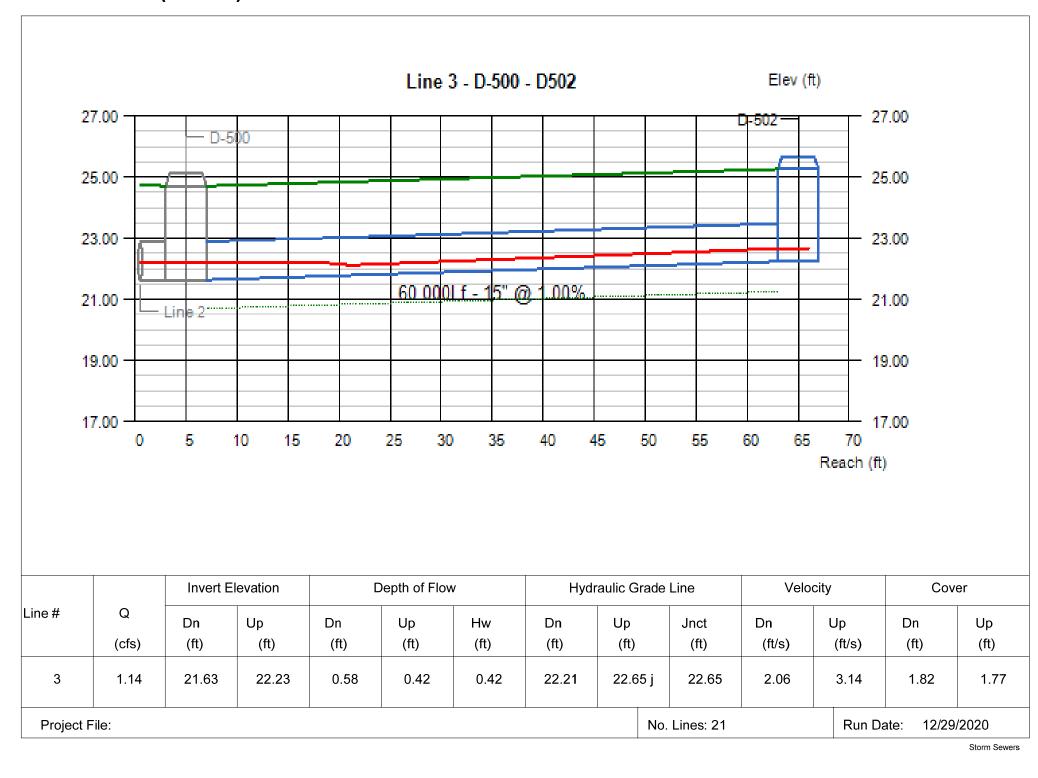
# APPENDIX C-10 Pipe Profiles



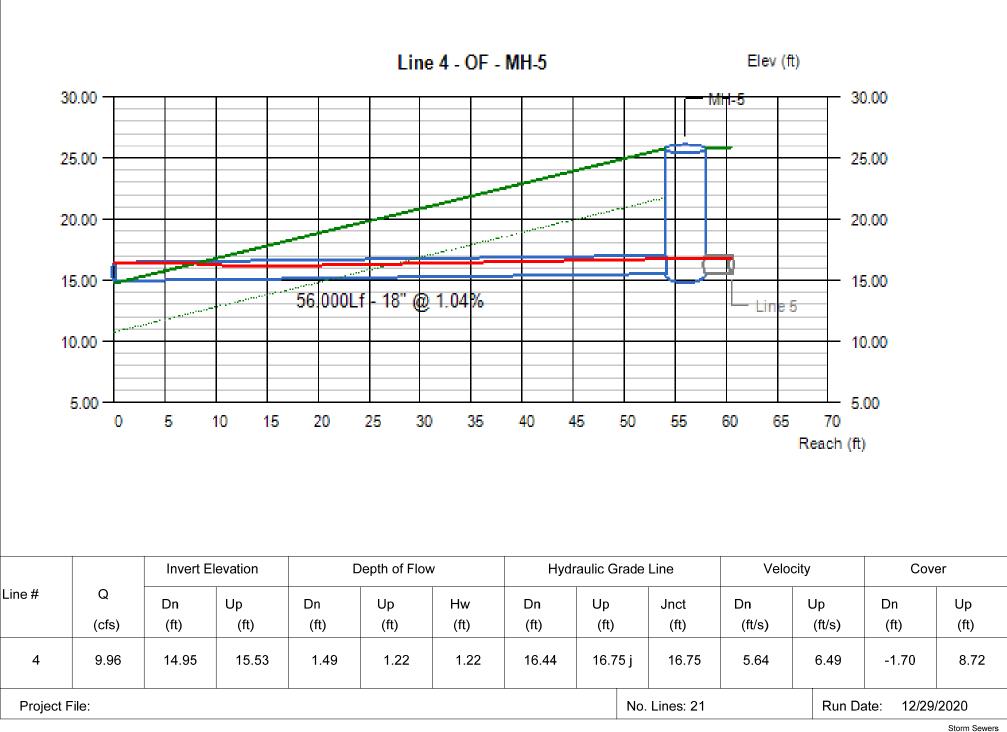


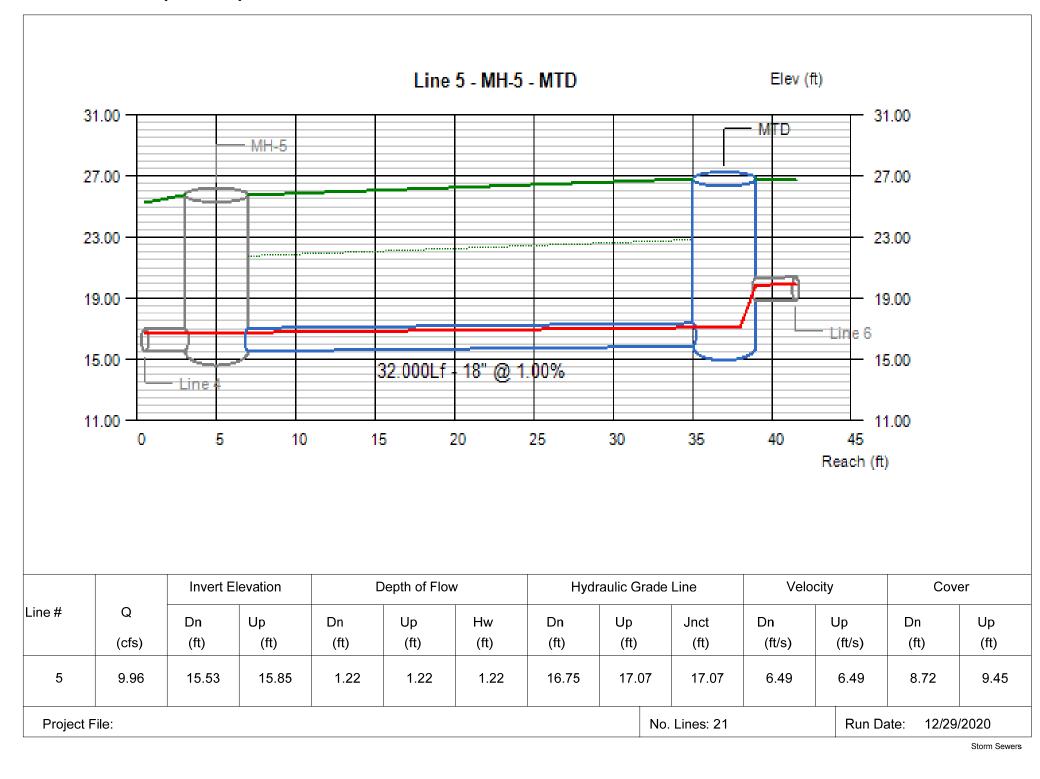
Storm Sewers

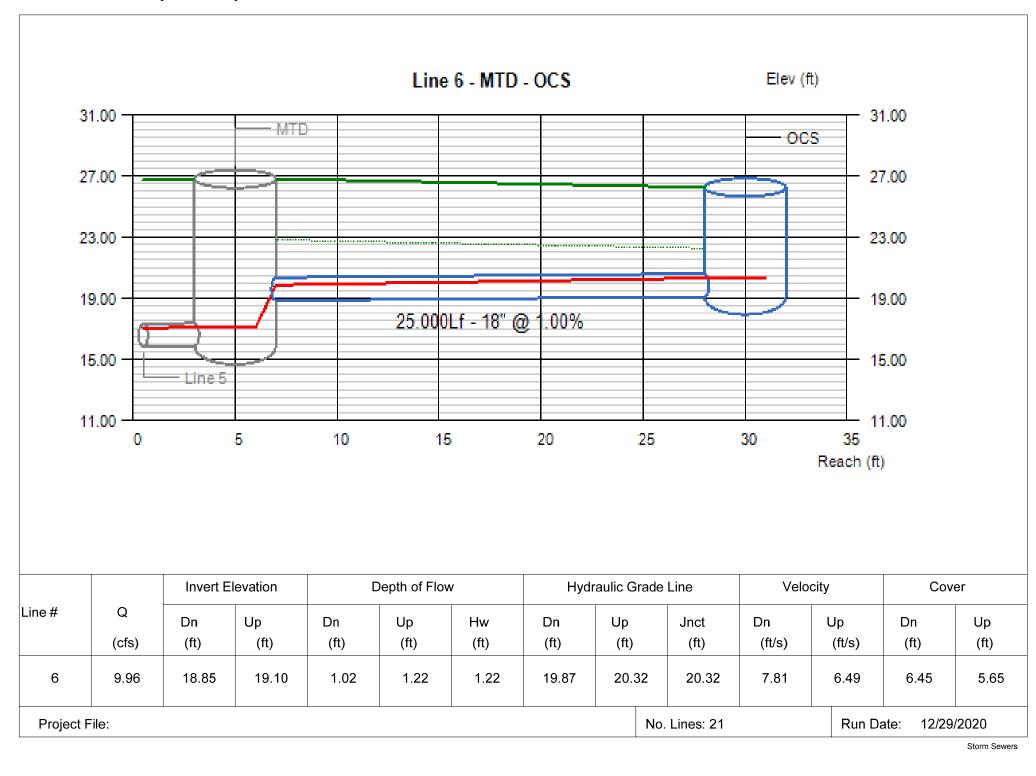


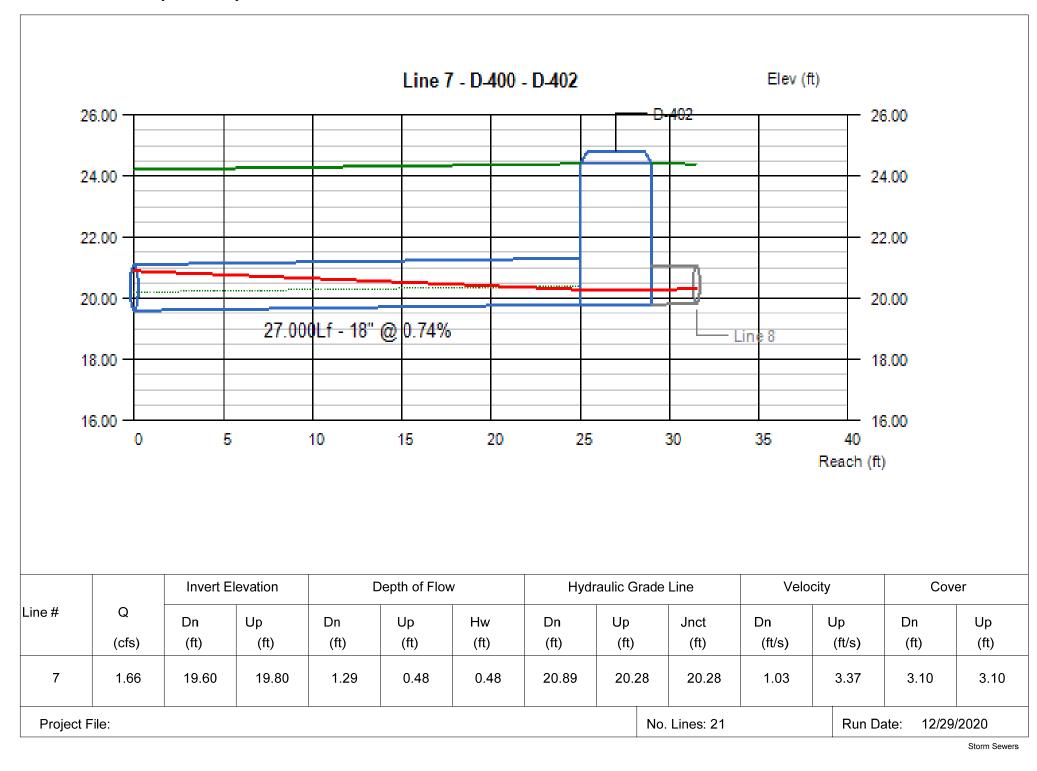


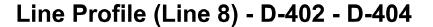
Line Profile (Line 4) - OF - MH-5

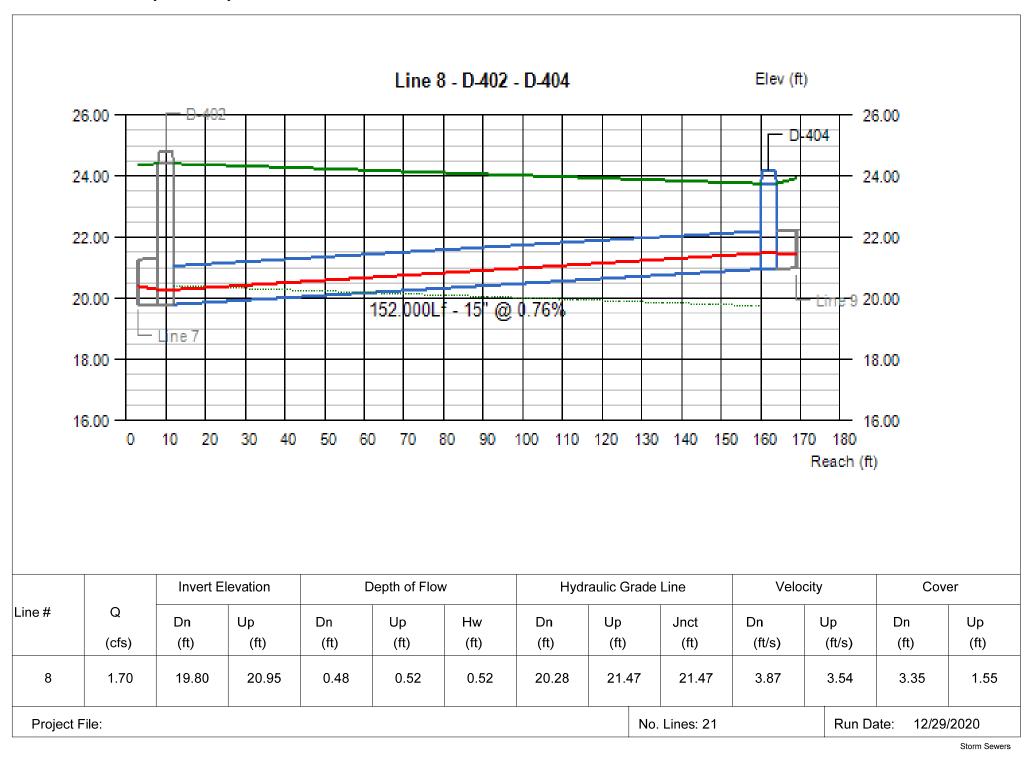


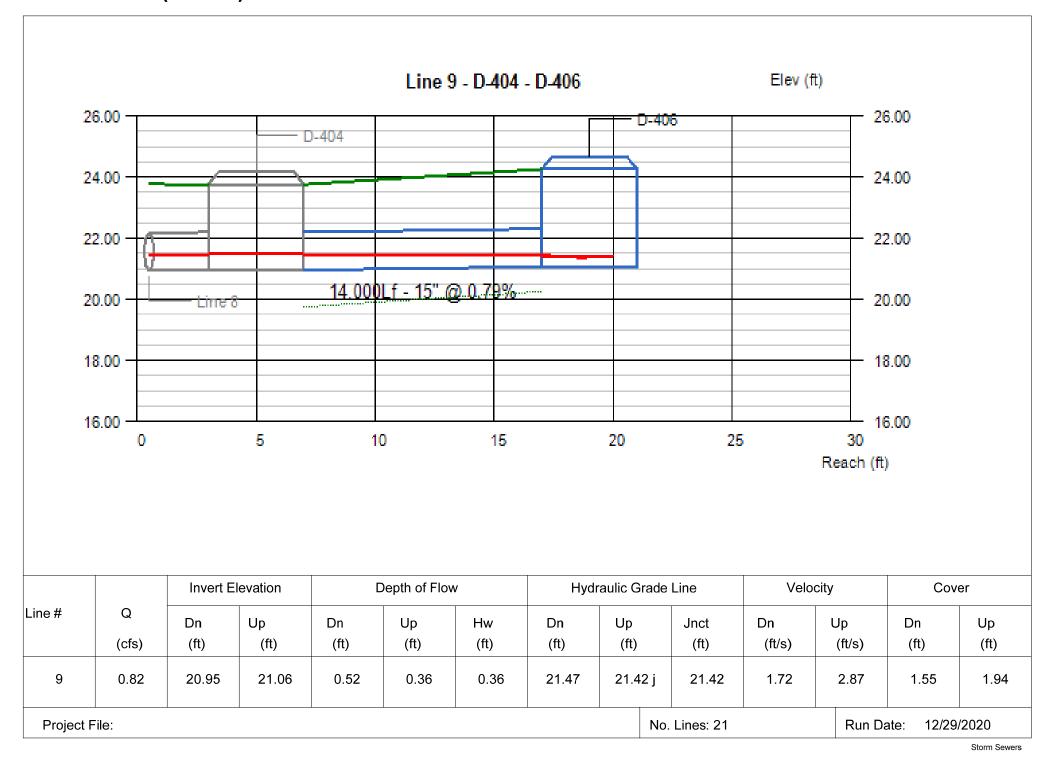


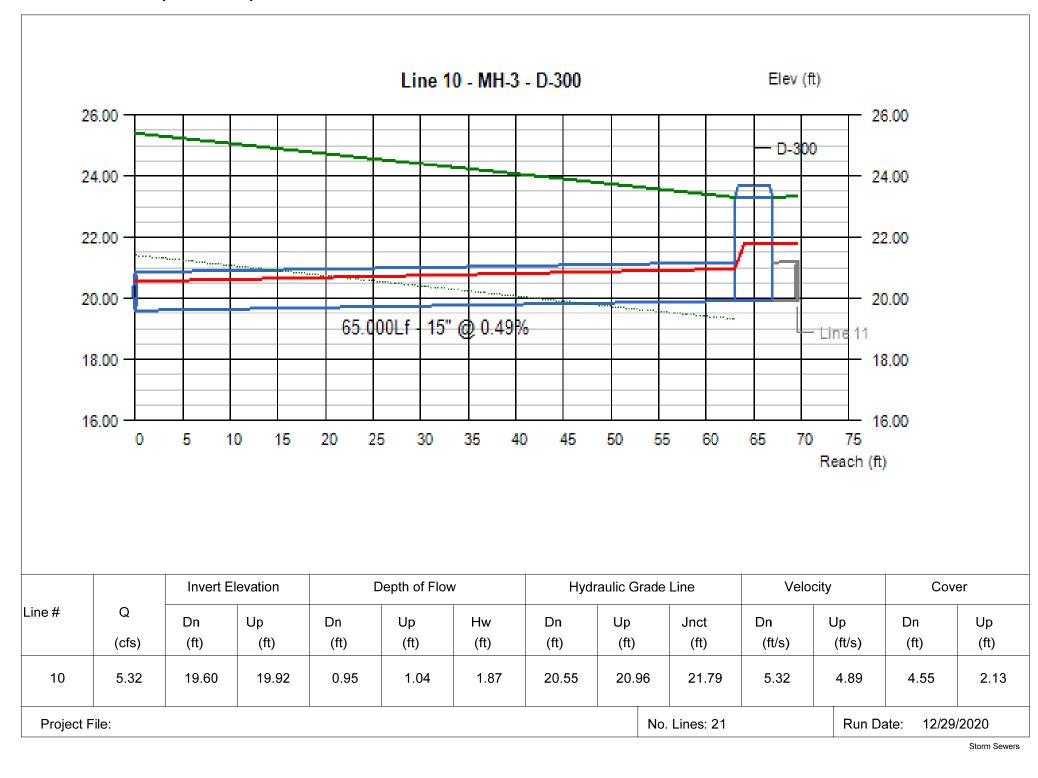


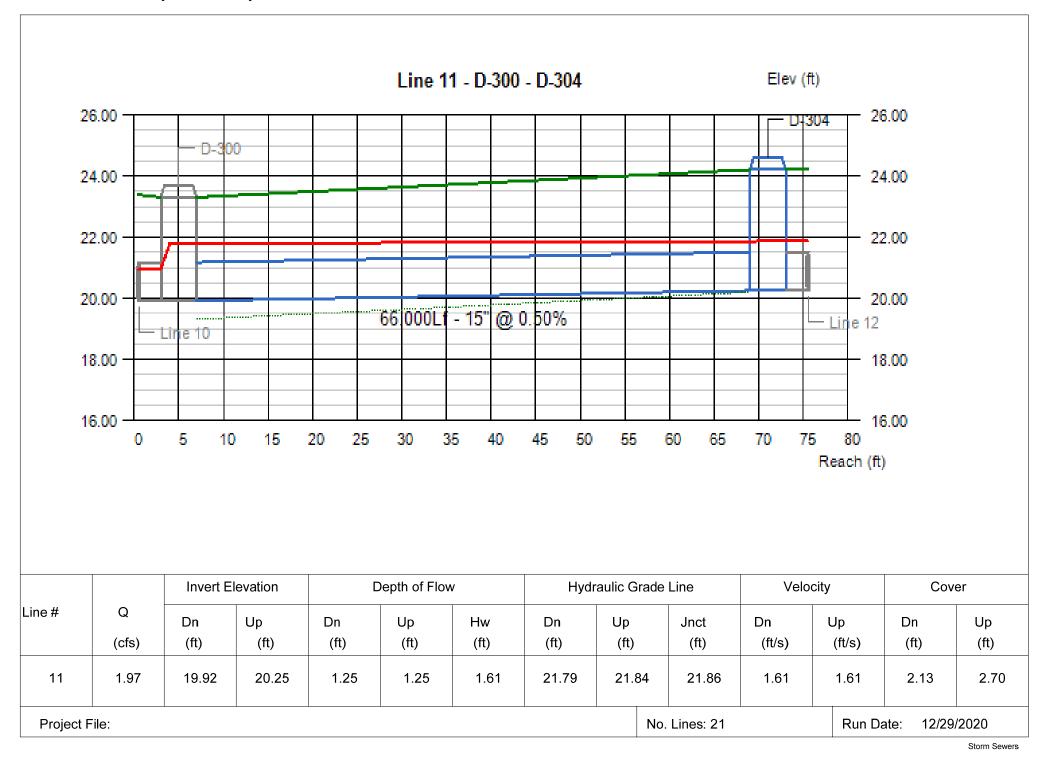


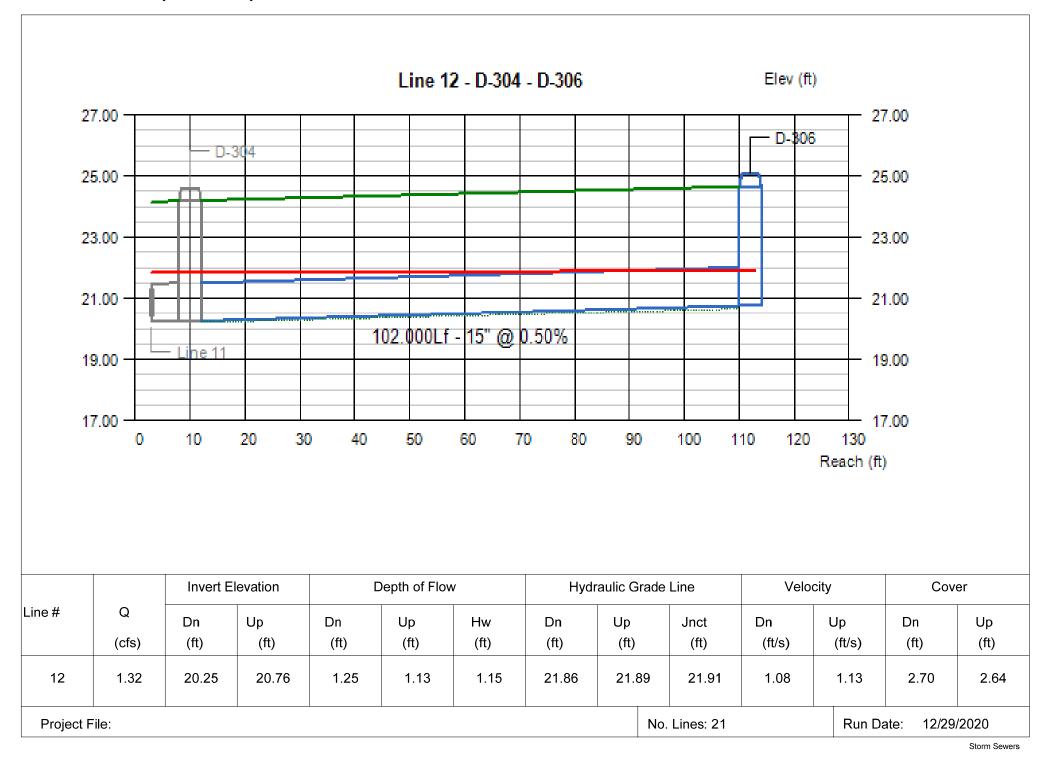


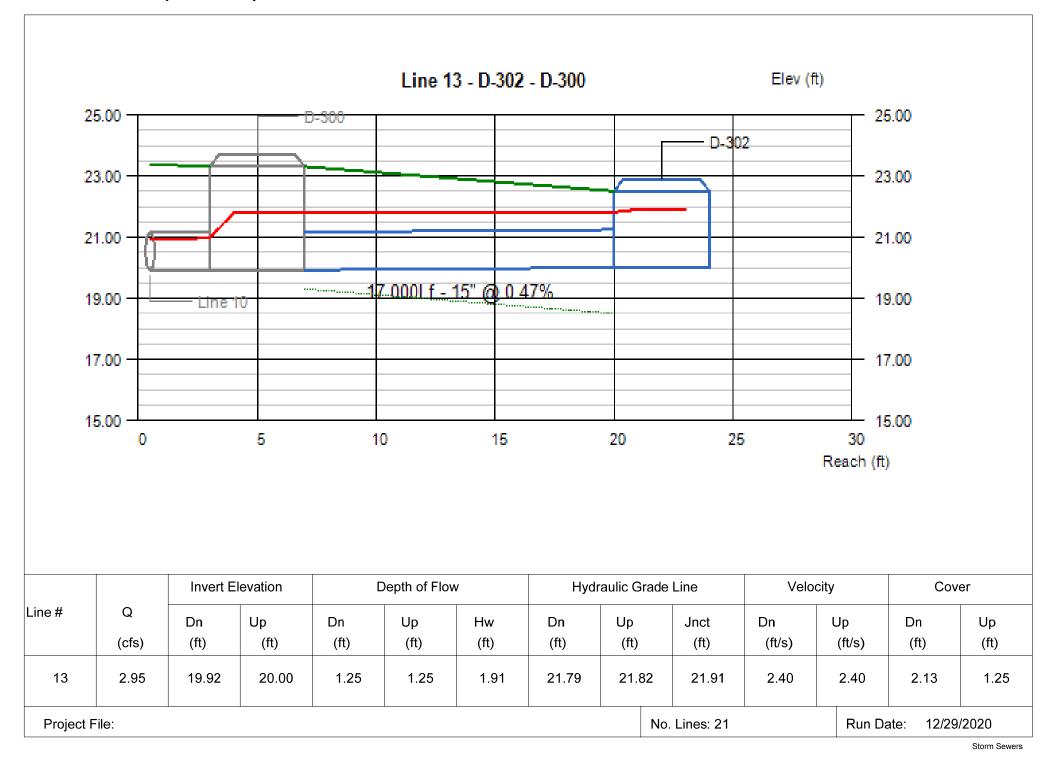


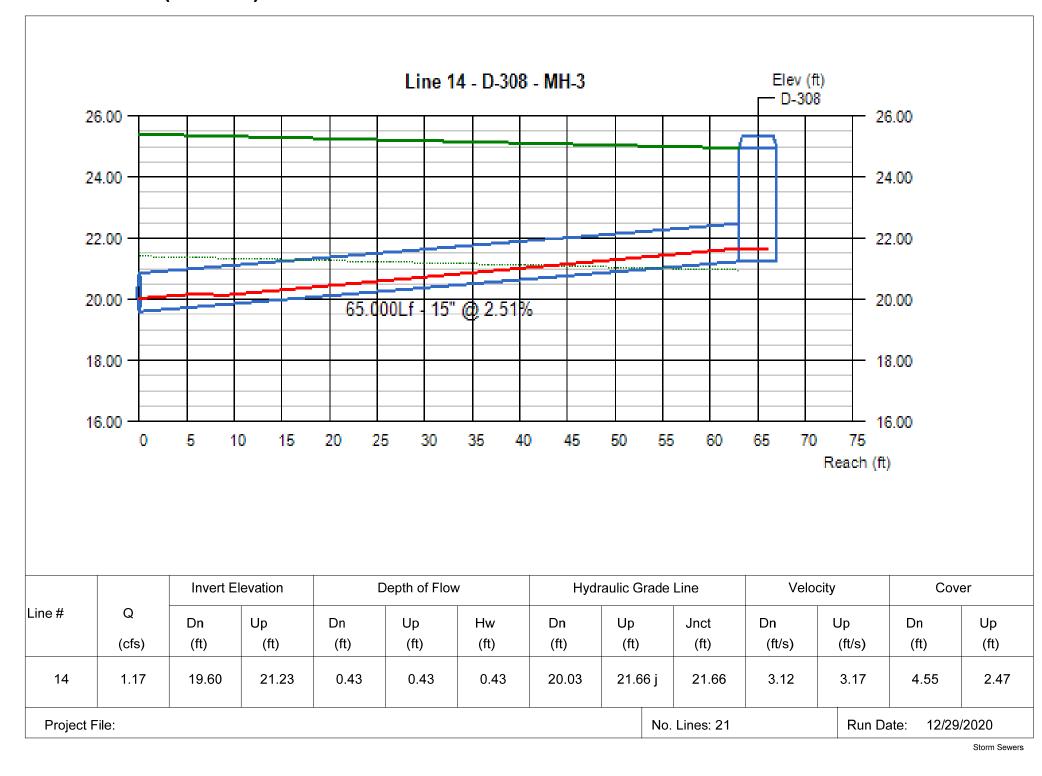


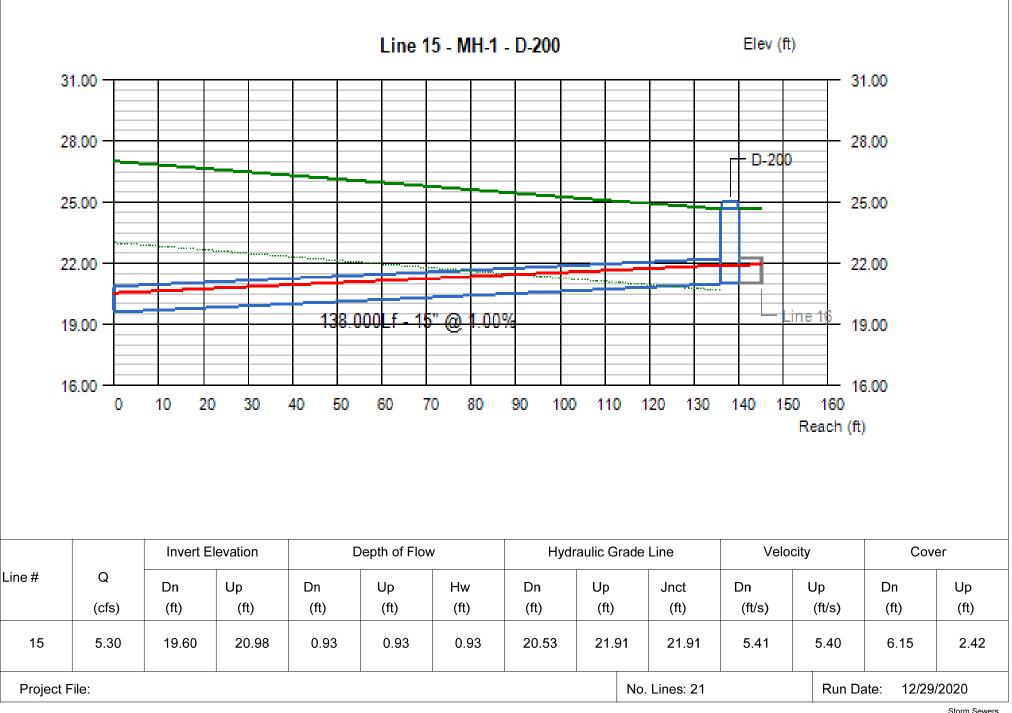




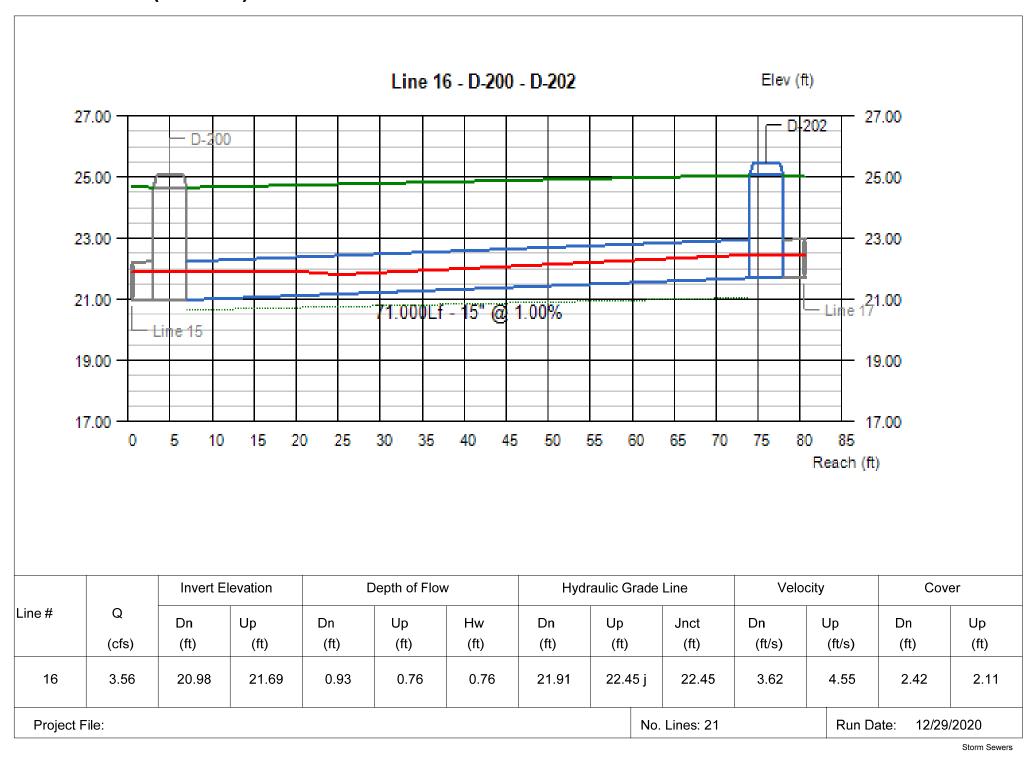


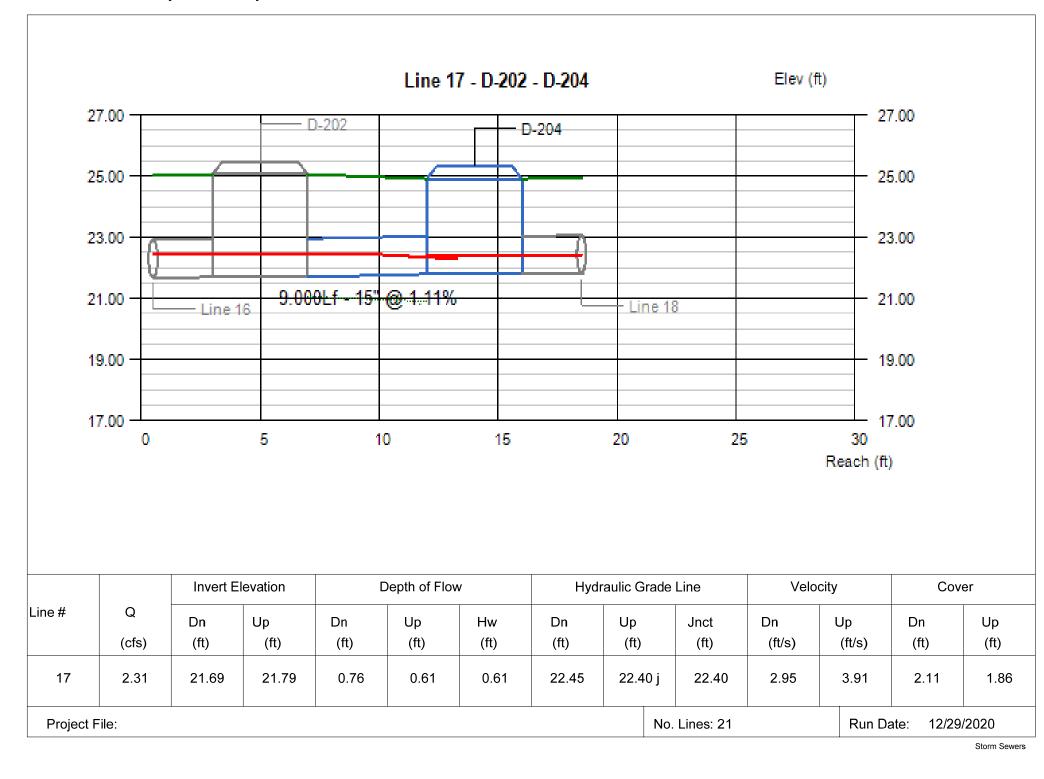


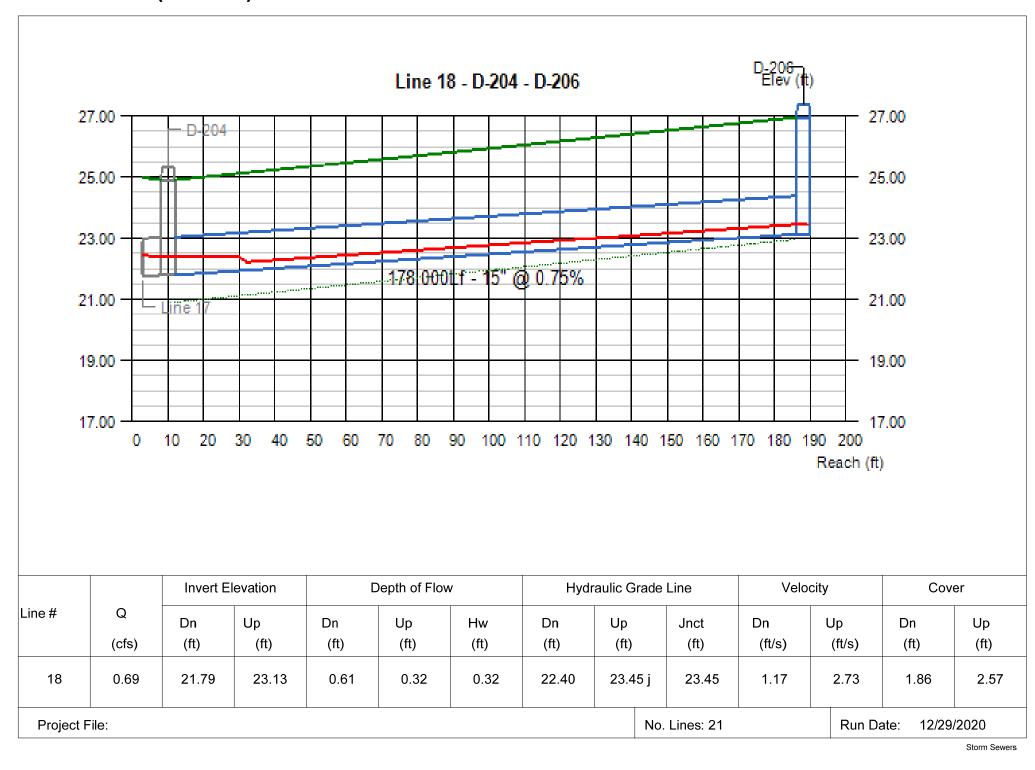


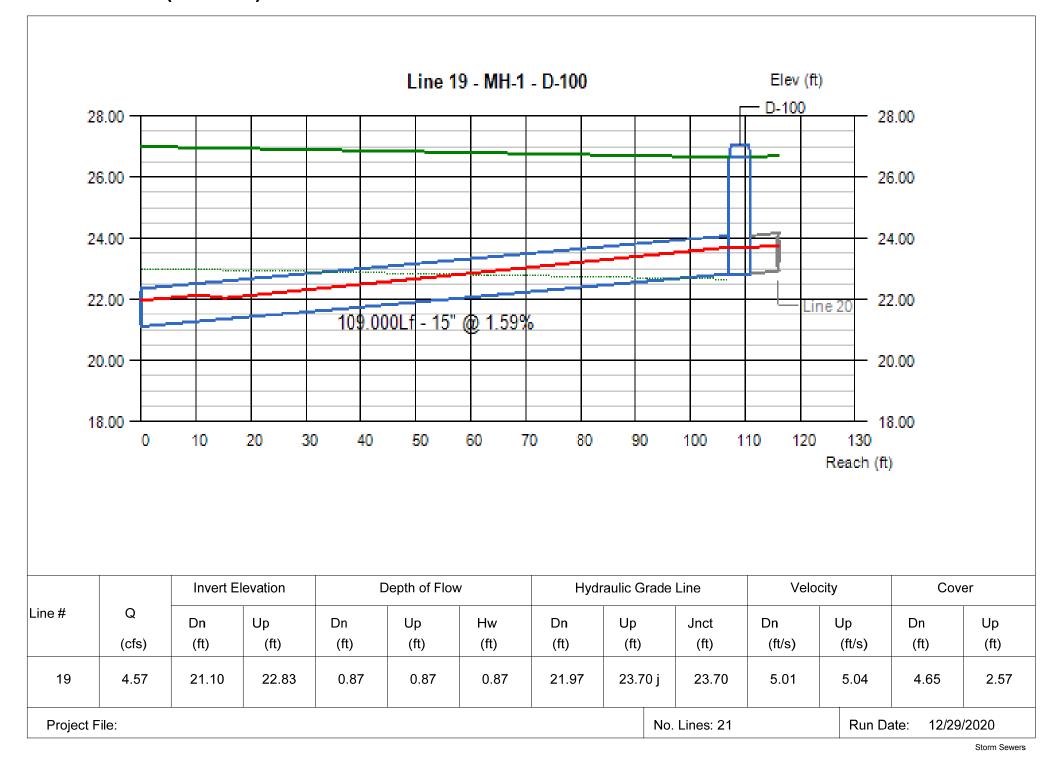


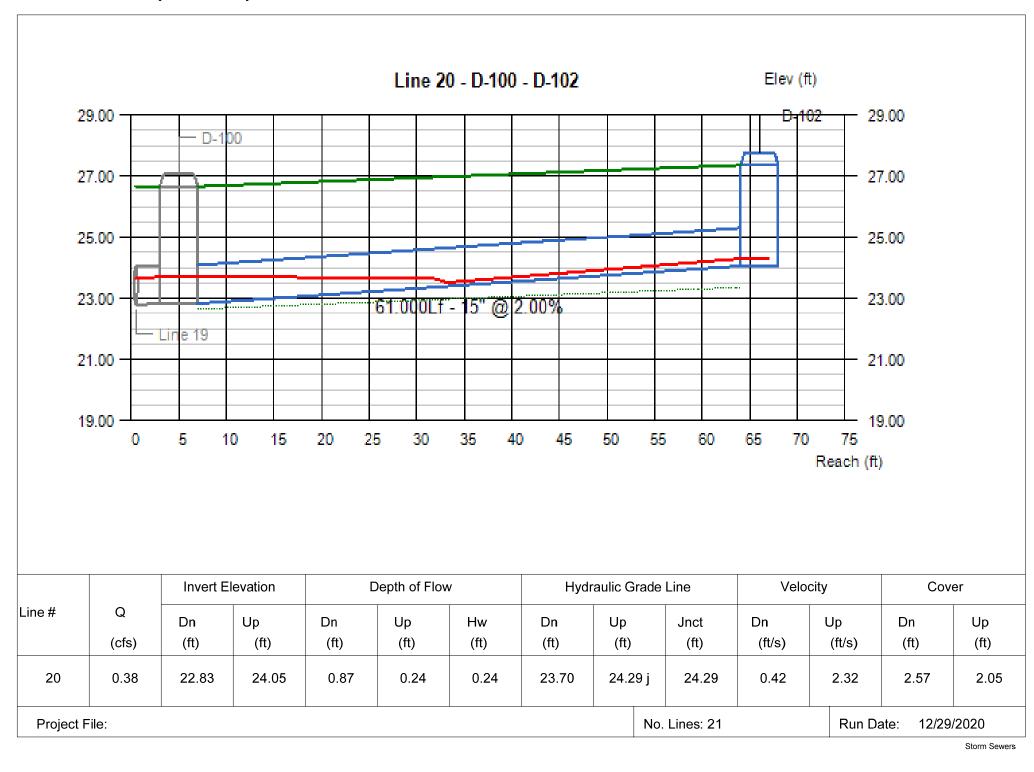
Storm Sewers

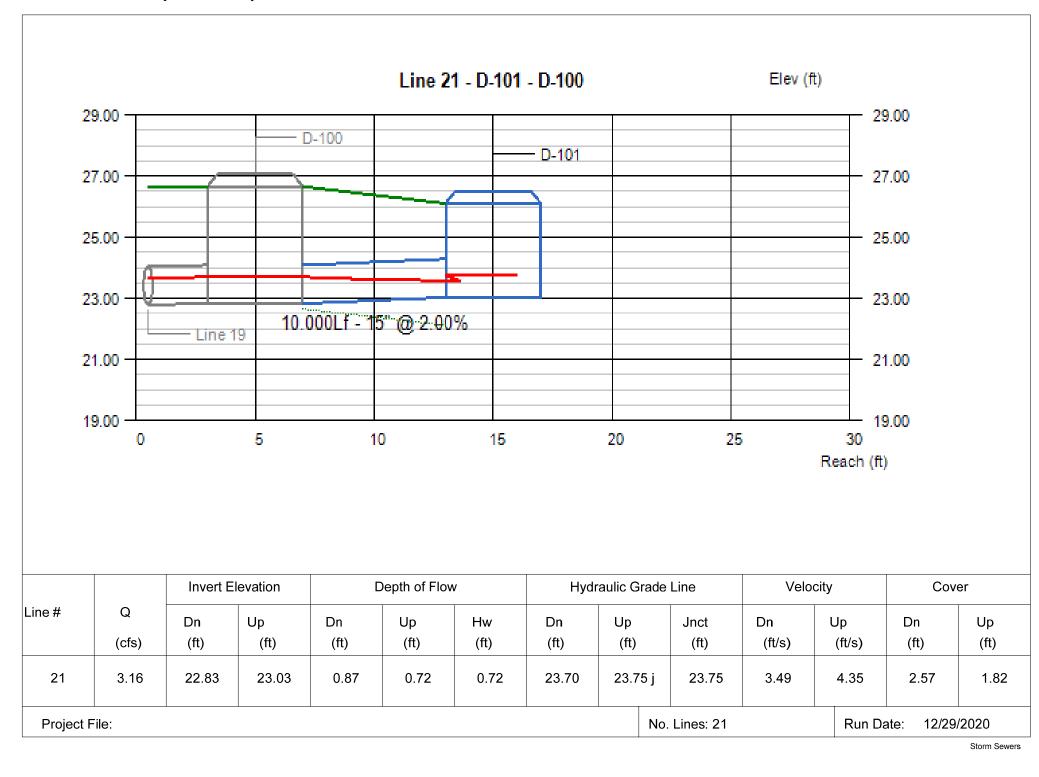




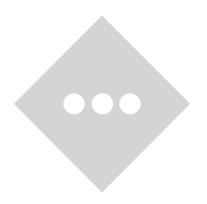








# APPENDIX D SUBSURFACE STORMWATER INVESTIGATION RESULTS





# Report of Infiltration Evaluation

# M & M at Neptune, LLC

Block 701, Lot 1 Township of Neptune, Monmouth County, New Jersey

May 20, 2019

Prepared For

Mr. Devon McDonough, PE, LSIT EP Design Services, LLC 2901 Hamilton Boulevard South Plainfield, NJ 07080

Prepared By

Maser Consulting P.A. Corporate Headquarters 331 Newman Springs Road, Suite 203 Red Bank, NJ 07701 732.383.1950

Michael Camiralo ==

Michael Carnivale, III, P.E. Senior Project Manager, Geotechnical Services Professional Engineer New Jersey License No. 45357

MC Project No. 19000475A





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# **FIGURES**

Site Location Map	. Figure No. 1
Exploration Location Plan	. Figure No. 2

## **APPENDICES**

APPENDIX A	Test Pit Logs
APPENDIX B	Tube Permeameter Test Results



## **1.0 INTRODUCTION**

This report presents the results of our geotechnical evaluation performed at the project site with respect to the proposed stormwater management areas and infiltration rates for use in design. A multi-use facility is planned within the currently abandoned property between NJ Route 35 and Asbury Avenue in the Township of Neptune, Monmouth County, New Jersey (Block 701, Lot 1 on the Township of Neptune Tax Maps).

Infiltration rate recommendations provided in this report are based on review of published data, accepted engineering practice, and field observations. Maser Consulting P.A. has evaluated the subsurface conditions at the site, and provides an evaluation of potential infiltration rates for soils encountered at depth within the area of the proposed stormwater management systems and design seasonal high water levels.

### 2.0 SITE DESCRIPTION

The project site is located on NJ Route 35 at the intersection with Asbury Avenue in Neptune, New Jersey (Figure No. 1) and referred to as Block 701, Lot 1 on the Township of Neptune Tax Maps. The subject site currently holds an abandoned building and associated parking lot at its center, a landscaped lawn on the west end, and an overgrown section that borders wetlands on the eastern end. The site is bounded by residential properties to the north, wetlands to the east, and commercial properties to the west and south. The proposed development consists of a proposed restaurant, retail and convenience store with gasoline service.

### **3.0 SCOPE OF SERVICES**

To evaluate the subsurface soil and groundwater conditions within the influence of the proposed stormwater management areas and to subsequently provide consultation regarding anticipated subsurface infiltration rates and estimated seasonal high-water levels (ESHWL) for design, we performed the following scope of services:





- a) Provided full-time technical observation of the excavation contractor, provided by the Client, to excavate test pits for exploration of subsurface soil and groundwater conditions within the proposed stormwater management areas;
- b) Obtained representative soil samples encountered within the zone of influence of the proposed stormwater basin construction;
- c) Evaluated the field data and prepared test pit logs showing the types of soils observed, depths to encountered groundwater, and depths to estimated seasonal high groundwater;
- d) Performed Tube Permeameter tests to evaluate infiltration rates for the subgrade soils in accordance with BMP-E methods; and
- e) Provided this *Report of Infiltration Evaluation* that reviews potential soil infiltration rates for design and groundwater considerations for the proposed basin requirements.

### 4.0 SUBSURFACE EXPLORATION

The subsurface conditions, for the purpose of infiltration evaluation, were explored on April 14, 2019 through the excavation of a total of 7 test pits, labeled TP-1 through TP-7. The test pits were advanced to termination depths between 11 and 12.5 feet below ground surface (bgs) by Edgewood Properties using a CAT 322C trackhoe.

Representatives from Maser Consulting's Geotechnical Department observed the test pit excavations. Soils encountered in the test pits were classified in the field in accordance with N.J.A.C. 7:9A, Subchapter 5.3, Terminology Required for Soil Logs. Representative soil samples of strata encountered were collected and returned to Maser Consulting's Red Bank laboratory facilities for further evaluation and analyses. Details pertaining to the subsurface conditions encountered are presented on the Test Pit Logs in Appendix A.

The depth of groundwater was measured from the ground surface to the point of observed seepage or consistent soil moisture. Groundwater was encountered within all test pits at depths that ranged from approximately 4.5 feet to 11.5 feet bgs. It should be noted that fluctuation in groundwater levels can occur due to several factors, including variations in precipitation, seasonal changes, and site development activities, which can alter surface water drainage paths. It should also be noted



that test pits TP-1 through TP-4 were located near a wetlands boundary on the eastern side of the property.

The subsurface strata were also evaluated with respect to mottling and soil staining to determine if seasonal high groundwater levels extended into the test pit depths. Staining and mottling within a soil stratum can indicate seasonal high-water level fluctuations, but is also found along wormholes, as a result of prior farming practices, or as an indication of geologic depositional factors. Please refer to Table 1 for a summary of depths to the groundwater table and to the estimated seasonal high-water level (ESHWL).

TABLE 1 DEPTH TO GWT AND ESHWL SUMMARY								
Test PitApprox.Test BoringGround SurfaIDElev. (ft)		Depth to Groundwater Water Table, GWT (in)	Depth to Estimated Seasonal High-Water Level, ESHWL (in)					
TP-1	22.33	138	6 (Perched)					
TP-2	20.72	102	8 (Perched)					
TP-3	21.05	54	47					
TP-4	17.50	132	16 (Perched)					
TP-5	21.54	132	20 (Perched)					
TP-6	21.82	132	6 (Perched)					
TP-7	23.25	126	16 (Perched)					

### 5.0 SUBSURFACE CONDITIONS

The surface cover in the test pits was a brown to dark brown sandy loam layer of topsoil that ranged from 2 to 19 inches in thickness. Test pits TP-1 through TP-4 had layers of varying topsoil thickness and inconsistent boundaries indicating that potential fill exists at the surface layer.

Underlying the surface cover was primarily a brownish yellow to light gray sandy loam to loam that extended to depths ranging from 24 to 108 inches bgs. Under this stratum was a gray clay loam layer in all test pits, except for test pit TP-4 which terminated in a gray sand. This gray sand stratum was encountered under the clay loam layer in the remaining test pits and was very moist to wet in all cases.



As indicated on the test pit logs and in Table 1 on the previous page, indicators of seasonal high water levels (SHWL), in particular, mottling, and light to moderate seepage was encountered at shallow depths within the test pits. Based on our observations in the field, it appears that this is a result of perched conditions due to the gray clay loam layer underlying the soil layers where the mottling and seepage was encountered and not indicative of the true groundwater table which was encountered at depths ranging from 102 to 132 inches below the existing grade. The noted exception was test pit TP-3, where the soil was saturated at a depth of 54 inches below existing grade and may be the result of its proximately to the wetland boundary.

### 6.0 SOIL INFILTRATION EVALUATION

Selected soil samples were tested by the Maser Consulting Geotechnical Laboratory in Red Bank, New Jersey. The testing consisted of 10 Tube Permeameter Tests performed to estimate the infiltration rate of groundwater through the soils at depth. Tube Permeameter testing was performed in accordance with N.J.A.C. 7:9A-6.2 and New Jersey Stormwater Best Management Practices Manual, Appendix E (BMP-E) requirements. The soil samples were selected based on review of test pit logs by design personnel, the proposed infiltration depths, and comparison to other strata encountered at each test pit location. The tube samples were collected from the soils directly by inserting the sample tube into the ground and retrieving the tube by excavating the soils surrounding it.

Infiltration test results are summarized in Table 2 and Tube Permeameter test results are presented in Appendix B.



TABLE 2 TUBE PERMEAMETER TEST SUMMARY								
Test Pit ID	Approx. Ground Surface Elev. (ft)	Depth below Existing Grade (in)	Infiltration Rate (in/hr)					
TP-1	22.33	21	0.12 / 0.00					
TP-2	20.72	<u>8</u> 24	0.00 / 0.00 0.00 / 0.00					
TP-3	21.05	19	0.00 / 0.00					
11-5	21.05	47	0.00 / 0.13					
TP-4	17.50	20	0.00 / 0.00					
TP-5	21.54	15	0.00 / 0.00					
1P-5	21.54	53	0.79 / 1.25					
TP-6	21.82	18	0.00 / 0.00					
TP-7	23.25	24	0.00 / 0.00					

### 7.0 **DISCUSSION**

According to the Natural Resources Conservation Service (NRCS) Web Soil Survey, the soils at the project site are classified as *Elkton loam*, (*EkaAr*), classified as Hydrologic Soil Group C/D, and *Evesboro-Urban land complex (EvuB)*, classified as Hydrologic Soil Group A. Based on the findings of our field exploration, the project site is underlain by several layers of fine-grained soils (loam, sandy loam, and clay loam) with low infiltration characteristics which also result in perched conditions at shallow depths. In accordance with the Addendum to Appendix E of the NJ Stormwater BMP Manual, areas where the SHWL is encountered within the upper 24 inches of the soil profile should be classified as Hydrologic Soil Group (HSG) D. For the purposes of subject property's HSG classification, our professional opinion is that perched water conditions can be considered a restrictive horizon similar to the SHWL, as both of these features will restrict the vertical movement of water. Based on this criteria, we recommend that consideration be given to classifying the property as having HSG D which can result in not having the requirement for groundwater recharge. Additional test pits may be required to meet the NJDEP requirements for HSG reclassification and would be subject to agency review and approval.

In lieu of HSG reclassification, a limited soil exchange program where the fine-grained soils can be removed and replaced with coarse-grained soils (K3 or better soil) which ties into the granular layer encountered near the groundwater level, can be performed.



Excavated soils with high silt and clay contents are unsuitable for use as structural fill throughout the site. Soils containing significant quantities of organic materials may need to be removed from the site and disposed in a manner consistent with local, state, and federal regulations. Stripped topsoil and any cohesive materials may be used to raise site grades in lawn areas but may be difficult to re-handle and place in a manner that will minimize post-construction subsidence. During periods of inclement weather, placing and compaction difficulties will also occur since the materials, in general, will be moisture sensitive. Granular materials encountered during site earthwork operations should be segregated for reuse as general fills for this project.

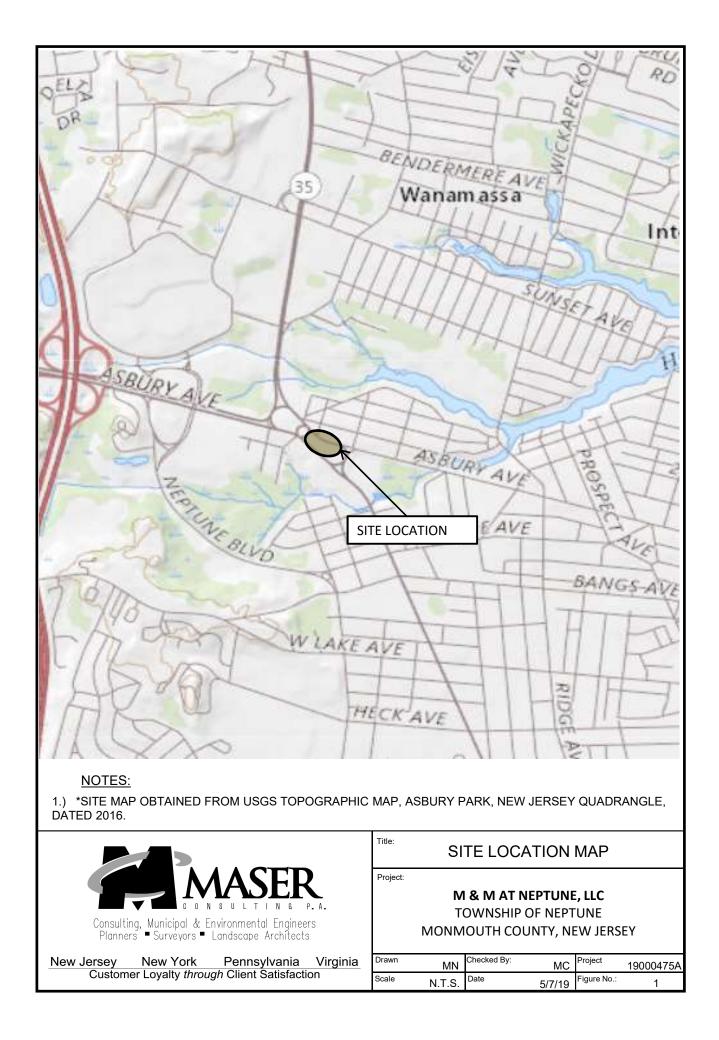
### 8.0 CLOSING

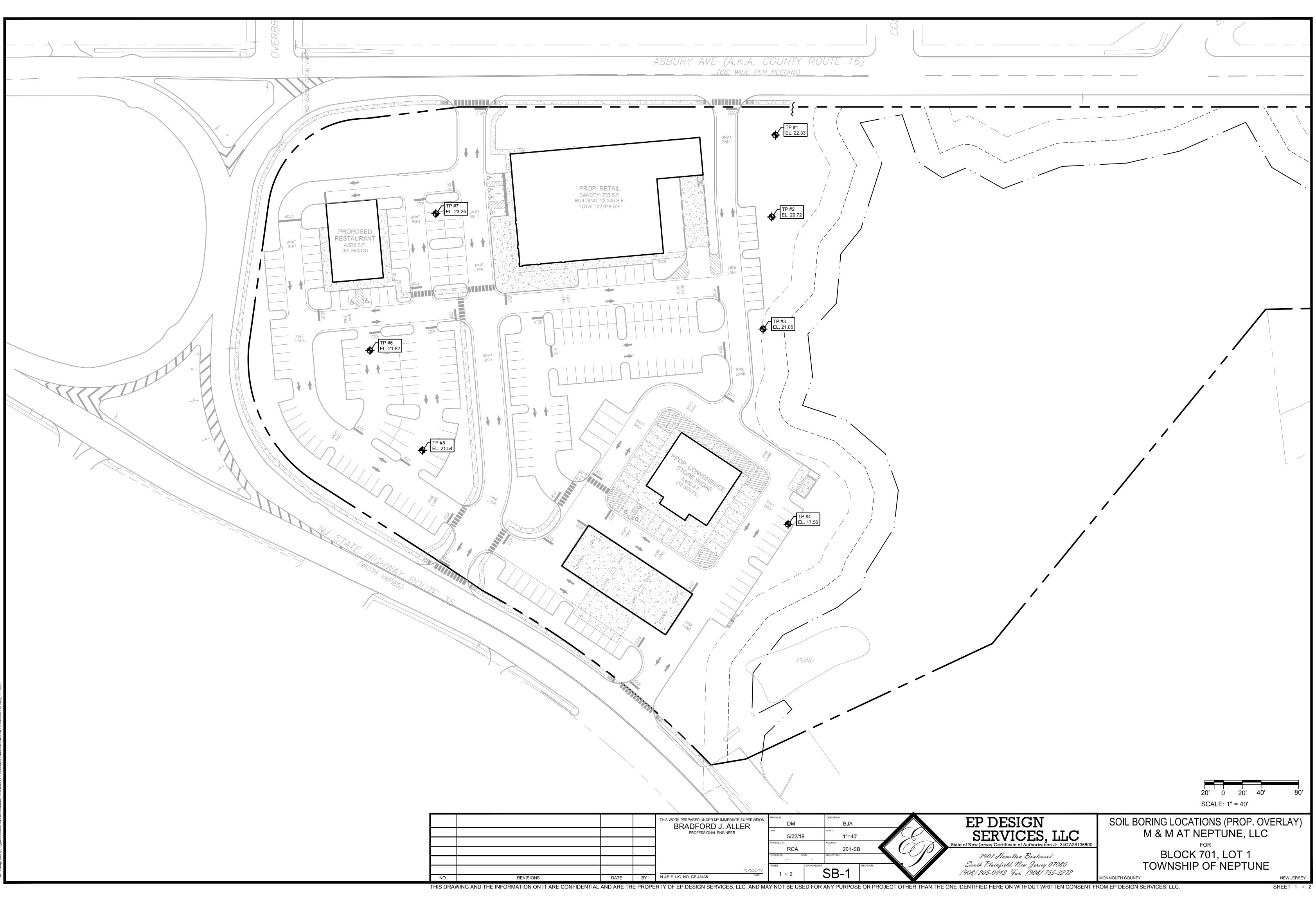
Successful construction of the project will require competent field observation of the construction operations. Earthwork, including clearing and grubbing, subgrade identification, grading, and fill placement should be observed by a competent individual familiar with the recommendations contained herein. We are available to perform construction observation services, if requested.

The recommendations contained herein are contingent upon the actual field conditions being consistent with those encountered during our field exploration. Should any variation in the anticipated conditions be encountered or site regrading be proposed, Maser Consulting P.A. should be notified immediately to determine what impact the changed conditions may have upon the presented recommendations.

### 9.0 LIMITATIONS

Services performed by Maser Consulting P.A. during this project have been conducted in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing in the same locality under similar conditions. No other representation, expressed or implied, and no warranty or guarantee is included or intended in the services provided. This is not an Environmental Assessment.





			THIS WORK PREPARED UNDER MY IMMEDIATE SUPERVISION BRADFORD J. ALLER	DM		BJA	
			PROFESSIONAL ENGINEER	DATE: 5/22/1		scale: 1"=40'	
				APPROVED BY:		ACAD NO.:	
				RCA	AGE: P	201-SB	
			5/22/19	SHEET 1 of 2	DRAWING NO.		REVISION
REVISIONS	DATE	BY	N.J.P.E. LIC. NO. GE 43435	I 0F Z			



## **APPENDIX A**

## **TEST PIT LOGS**

Planners = Su P Loo	cation: <u>N</u>		s Road 01 50 0 naserconsulting.com _C	DATE E	EXCAVATED: ELEVATION:	
DEPTH (ft)	DEPTH (in)		DESCRIPTIC	N		REMARKS
- 0 - - - -	$ \begin{array}{c}                                     $	(10YR 4/3) Brown Friable. Frequent (10YR 6/6) Brown Subangular Blocky, (10YR 7/1) Light Friable. (7.5 YR Coarse, Distinct M	Roots. (To iish Yellow S Friable. (N Gray f Sand 5/8) Strong	psoil, Moist). andy Loam. Aoist). y Loam. Subang Brown Many,	jular Block	4" 6" 2y, 46"
5 —	- 60	(10YR 3/2) Very I Firm. Micaceous.		Brown Clay Loar Very Moist with D		ve,
10—	108 	(7.5YR 6/1) Gray	Sand (Wat	}		<u>138"</u>
 15—	156 —	END OF MODERATE	TEST PIT AT	150 INCHES T 28 INCHES		
-	192 204 216 228					
<u> </u>	240		T			
		R:       DEPTH (ft.)       DATE         ed $\checkmark$ 11.5       4/17/19         s.) $\checkmark$ 11.5       4/17/19	ESTIMATED DEP	TH TO SEASONAL HIGH G		6 Inches (Perched)
	letion (>24 hrs	—		TEST	PIT	No. <u>TP-1</u>

Planners = Si P Lo	cation: <u></u>	331 Nev Suite 20 Suite 20 Red Bar Phone ( ental Engineers Fax (73	nk, N.J. 07701 732) 383-1950 32) 383-1990 geotech@maserconsulting.com ne, LLC		-/17/19 20.72 roperties
DEPTH (ft)	DEPTH (in)		DESCRIPT	ΓΙΟΝ	REMARKS
- 0 - - - 5 - - - - -	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Subangular B (Topsoil, Mois (10YR 6/4) Friable. (Mo (10YR 7/2) Friable. Mico Fine, Distinct (10YR 3/2) Firm. Comm	Blocky, Friable. St). Light Yellowish B ist). Light Gray f Sa aceous. (7.5YR Mottles Through Nottles Through Very Dark Grayis	dy Loam. 10% Gravel. Frequent Roots. Brown Loam. Subangular Blocky <u>8</u> ndy Loam. Subangular Blocky, 5/8) Strong Brown Common, nout. (Moist, Seepage at 20") <sub>24</sub> , sh Brown Clay Loam. Massive, Light Gray Loamy Sand Becoming Wet at 102").	_
10—	120 			126	"
  15    	102	END	Gray Sand. Si		
	GROUNDWATE	( )	ATE ESTIMATED D	DEPTH TO SEASONAL HIGH GROUNDWATER: <u>8</u>	Inches (Perched
At Co		s.) $\underline{\underline{V}}$ 8.5 4/1		TEST PIT No	D. <u>TP−2</u>

	pject: $\underline{M}$	RED BANK OFFICE331 Newman Springs Road Suite 203 Red Bank, N.J. 07701 Phone (732) 383-1950 Fax (732) 383-1990 E-mail - geotech@maserco1 & M At Neptune, LLC1 eptune, Monmouth, NJ9000475A	DATE EXCAVATED:	4/17/19 21.05 Properties
(ft)	DEPTH (in)	D	ESCRIPTION	REMARKS
- 0 - - - 5 - - - - - - - - - - -	$\begin{array}{c} & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & &$	Subangular Blocky, Fr (10YR 5/6) Yellowish Subangular Blocky, Fr (10YR 2/2) Very Darl Friable. (10YR 7/1) Many Roots. (Possib (10YR 7/1) Light Gro Friable. (7.5YR 5/8) Medium to Fine, Disti Wet at 54").	Brown Sandy Loam. 10% Gravel. Table. (Possible Fill, Moist). 2 k Brown Loam. Subangular Blocky, Light Gray f Loamy Sand Partings. The Fill, Moist, Seepage at 47"). 4 ay f Sandy Loam. Subangular Blocky, Strong Brown Common, nct Mottles. (Moist Becoming	9" 9" 7"
20	132	SEEPAG	ST PIT AT 144 INCHES SE AT 47 INCHES	
Firs	letion (0 hrs	$\begin{array}{c c} ed & \underline{\nabla} & \underline{4.5} & \underline{4/17/19} \\ \hline s.) & \underline{\Psi} & \underline{4.5} & \underline{4/17/19} \end{array}$	STIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 4	7 Inches

Planners = St P LO4	cation: <u>N</u>	RED BANK OFFICE 331 Newman Springs Suite 203 Red Bank, N.J. 0770 Phone (732) 383-195 Fax (732) 383-1990 E-mail - geotech@ma & M At Neptune, LL eptune, Monmouth, Nu 9000475A	IESI PII N DATE EXCAVATED: SURFACE ELEVATION: C EXCAVATED BY: Edgewood F	4/17/19 17.5 Properties
DEPTH (ft)	DEPTH (in)		DESCRIPTION	REMARKS
- 0 - - - 5 - - - - - - - - -	$ \begin{array}{c}                                     $	Friable. 5% Grave (10YR 5/6) Yellowi Subangular Blocky, (10YR 7/1) Light Friable. (7.5YR 5/ Distinct Mottles Thr	Brown Sandy Loam. Subangular Blocky, I. (Topsoil, Moist). 19 ish Brown Loamy Sand. Friable. (Fill, Moist). 16 Gray f Sandy Loam. Subangular Blocky, /6) Strong Brown Many, Coarse, roughout. (Moist, Seepage at 47"). 66 f Sandy Loam. Subangular Blocky, st).	)" 
-		(7.5YR 6/1) Gray	13 Sand. Single Grain, Loose. (Wet).	<u>32"</u>
 15—	156  168   180   192		TEST PIT AT 150 INCHES PAGE AT 47 INCHES	
  20	204 — — — — — — — — — — — — — — — — — — —			
At Co		$ \underbrace{ \nabla}_{\text{ad}} = \underbrace{ 11.0 }_{11.0} \underbrace{ \frac{4/17/19}{4/17/19} }_{4/17/19} $	ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 16 TEST PIT N	

Planners = St	cation: <u>N</u>	RED BANK OFFICE         331 Newman Springs         Suite 203         nntal Engineers         pe Architects         1 & M At Neptune, LLu         Jeptune, Monmouth, NL         9000475A	1 60 aserconsulting.com C	DATE	EXCAVATED: ELEVATION:	
DEPTH (ft)	DEPTH (in)		DESCRIPTION			REMARKS
- 0 - - - 5 -	$ \begin{array}{c}                                     $	(10YR 4/3) Brown (Topsoil, Possible F (10YR 4/3) Brown (10YR 6/2) Light I Partings. (Possible (10YR 5/1) Gray f (Possible Fill, Moist	Fill, Moist). Loam. Subo Brownish Gray E Fill, Moist). Sandy Loam t, Seepage fro	angular Blocky, v Stratified f Lo . Subangular l om 20" to 31")	amy Sand Blocky, Fric	2" <u>18"</u> 1ble. <u>31"</u>
 10	84 96 108 108 120 132 132 144	(10YR 4/2) Dark ( Very Moist with De (7.5YR 6/1) Gray	pth).		(Wet).	132"
 15 	156 168 168 1 192 204 216 1 216 1 1 1 1 1 1 1 1 1 1 1 1 1		TEST PIT AT 1 FROM 20 TO			
 20	228 					
At Co		ed $\underline{\nabla}$ <u>11.0</u> <u>4/17/19</u> s.) $\underline{\nabla}$ <u>11.0</u> <u>4/17/19</u>	ESTIMATED DEPTH			20 Inches (Perched)

Planners = Si P Lo	cation: <u>N</u>	RED BANK OFFICE         331 Newman Spring:         Suite 203         Red Bank, N.J. 0770         Phone (732) 383-1990         pe Architects         1 & M At Neptune, LL         leptune, Monmouth, N.         9000475A	Test pit     Test pit     And test pit       Date     Excavated     4/       Date     Surface     21	/17/19 .82 perties
DEPTH (ft)	DEPTH (in)		DESCRIPTION	REMARKS
- 0 -	6 <mark>3-</mark> 12	(10YR 4/3) Brown Frequent Roots. (	Sandy Loam. Subangular Blocky, Friable. (Topsoil, Moist). <sub>6"</sub>	
_	24 —	Friable. (7.5YR 5,	Gray f Sandy Loam. Subangular Blocky, /8) Strong Brown Many, Coarse, (Moist, Seepage from 37" to 56"). 42"	
_ 5 —	48		42	
_	72 84 96		Dark Grayish Brown Clay Loam. Massive, . (Moist to Very Moist with Depth).	
 10	108 —		<u>132"</u> Dark Grayish Brown Sand.	
_	144 —	Single Grain, Loose END OF <sup>-</sup>	e. (Wet). TEST PIT AT 144 INCHES	
_ 15—	- 168	SELPAGE	FROM 37 TO 56 INCHES	
	192 —			
_	216			
	GROUNDWATE		ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 6 Inc	hes (Perched)
At Co		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	TEST PIT No.	. <u>TP-6</u>

		RED BANK OFFICE 331 Newman Springs Suite 203		
	unicipal & Environmen urveyors = Landscap	Red Bank, N.J. 0770 N & P.A. Phone (732) 383-1950 tal Engineers Fax (732) 383-1990	DATE EXCAVATED: $\frac{4/17/19}{2}$	
	, —	<u>&amp; M At Neptune, LL</u> eptune, Monmouth, N	<u>C</u> EXCAVATED BY: Edgewood Properties	
		9000475A	INSPECTED BY: Megan Nugent	
DEPTH (ft)	DEPTH (in)		DESCRIPTION	s
- 0 -	6 <u>9</u> 12	(10YR 4/3) Brown (Topsoil, Fill, Moist)		
_	24	(10YR 3/3) Dark E (Possible Fill, Moist	Brown Loam. Subangular Blocky, Friable. t). <sub>16"</sub>	
	- 36	, , , 5	Gray Loam. Subangular Blocky, Friable. g Brown Many, Coarse, Distinct Mottles. om 28" to 66").	
5 —	- 60 	(10YR 3/1) Very E (Very Moist).	<u>66"</u> Dark Gray Clay Loam. Massive, Firm.	
_	96			
10—	120 	(10YR 3/1) Very [ (Wet).	Dark Gray Sand. Single Grained, Loose.	
_	144 —		TEST PIT AT 132 INCHES FROM 28 TO 66 INCHES	
_	168			
15—	180			
-	192			
-	216			
-	228			
<u>     20    </u>	GROUNDWATER	R: DEPTH (ft.) DATE	ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 16 Inches (Perch	ned)
At Co			TEST PIT No. TP-7	



## **APPENDIX B**

### **TUBE PERMEAMETER TEST RESULTS**

One River Centre - Building Two 331 Newman Springs Road, Red Bank, NJ 07701 Tel: 732.383.1950 = Fax: 732.383.1984 www.maserconsulting.com

#### TUBE PERMEAMETER TEST DATA

	101		IESI DATA			
Project Name:	M & M At Neptune		Project I	Number:	19000475A	
Block:	701		Municip	ality:	Neptune	
Lot:	1		County:		Monmouth	
Test Number:	1		Date Co	llected:	4/17/2019	
Material Tested	d: TP-1		Replicat	e:	A	
Depth of Sample	e: 21"		Sample	Type:	Undisturbed	
1.	Sample Dimensions	Radius = Length =	1.905 3.375	cm in		
2.	Measurements	Tube Weight =	365.85	g		
	tube #: F-6	Total Weight = Soil Weight =	672.20 306.35	g g		
	Volume = Le Volume =	ngth * 2.54 cm/inch * 97.68	π * Radius²			
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.14	olume			
	Height of Wat	ter Level above Rim o	f Test Basin (inches	)		
	At beginning			,		
	At end of					
3.	Test Data					
_	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length	(min)	∆ Height	(in)
a.	0:00:00	1:00	60.0		0.125	
b. c.	0:00:00 0:00:00	1:00 1:00	60.0 60.0		0.125 0.125	
d.	0:00:00	1:00 av	= 60.0 = 60.0		0.125 0.125	
4.	Permeability Calcula		= 60 min/hr * r²/R² * l	_ (in)/T (n	nin) * In (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.12	in/hr = Soil Pe	rmeability Class	K0		
5.	Any <b>Defects</b> in Sam	ple: No				
t	hereby certify that the i hat falsification of data i and is subject to penaltie	s a violation of the wa	ter polution Control	is true an Act (NJS	nd accurate. I am A 59:10A-1 et seo	aware <sub>1</sub> .)
Signature o	of Professional Engineer				License #4	45357
5	5	Micha	ael Carnivale, III, P.E			

One River Centre - Building Two 331 Newman Springs Road, Red Bank, NJ 07701 Tel: 732.383.1950 = Fax: 732.383.1984 www.maserconsulting.com

#### **TUBE PERMEAMETER TEST DATA**

	TOBE		ESIDATA					
Project Name:	M & M At Neptune		Project Number	: 19000475A				
Block:	701		Municipality:	Neptune				
Lot:	1		County:	Monmouth				
Test Number:	1		Date Collected:	4/17/2019				
Material Teste	d: TP-1		Replicate:	В				
Depth of Sampl	e: 21"		Sample Type:	Undisturbed				
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.250 in					
2.	Measurements	Tube Weight =	354.06 g					
	tube #: B-100	Total Weight = Soil Weight =	666.08 g 312.02 g					
	Volume = Leng Volume =	th * 2.54 cm/inch * π 94.07	* Radius²					
	Bulk Density = Bulk Density =	Soil Weight / Vol 3.32	lume					
	Height of Water	Level above Rim of	Test Basin (inches)					
	At beginning of At end of int							
3.	Test Data							
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)				
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00 av =	60.0 60.0 60.0 60.0 = 60.0	0.000 0.000 0.000 0.000 0.000				
4.	Permeability Calculation	on K (in/hr) =	: 60 min/hr * r²/R² * L (in)/T (	min) * In (H <sub>1</sub> /H <sub>2</sub> )				
	K = 0.00	in/hr = Soil Perr	neability Class K0					
5.	Any <b>Defects</b> in Sample	e: No						
	I hereby certify that the info that falsification of data is a and is subject to penalties	a violation of the wate	er polution Control Act (NJ	nd accurate. I am aware SA 59:10A-1 et seq.)				
Signature	Signature of Professional Engineer License #45357 Michael Carnivale, III, P.E.							

One River Centre - Building Two 331 Newman Springs Road, Red Bank, NJ 07701 Tel: 732.383.1950 = Fax: 732.383.1984 www.maserconsulting.com

#### **TUBE PERMEAMETER TEST DATA**

	TOBE		ESIDATA					
Project Name:	M & M At Neptune		Project Numbe	r: 19000475A				
Block:	701		Municipality:	Neptune				
Lot:	1		County:	Monmouth				
Test Number:	1		Date Collected	: 4/17/2019				
Material Teste	d: TP-2		Replicate:	А				
Depth of Sampl	e: 8"		Sample Type:	Undisturbed				
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.000 in					
2.	Measurements	Tube Weight =	357.77 g					
	tube #: JM-10	Total Weight = Soil Weight =	649.97 g 292.2 g					
	Volume = Leng Volume =	th * 2.54 cm/inch * π 86.83	r * Radius²					
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.37	lume					
	Height of Water	Level above Rim of	Test Basin (inches)					
	At beginning of At end of int	interval: 3.62	5 H <sub>1</sub>					
3.	Test Data							
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)				
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00 av =	60.0 60.0 60.0 60.0 = 60.0	0.000 0.000 0.000 0.000 0.000				
4.	Permeability Calculati	on K (in/hr) =	= 60 min/hr * r²/R² * L (in)/T	(min) * ln (H <sub>1</sub> /H <sub>2</sub> )				
	K = 0.00	in/hr = Soil Perr	meability Class Ki	)				
5.	Any <b>Defects</b> in Sample	e: No						
	I hereby certify that the info that falsification of data is a and is subject to penalties	a violation of the wat	er polution Control Act (N.	and accurate. I am aware JSA 59:10A-1 et seq.)				
Signature	Signature of Professional Engineer License #45357 Michael Carnivale, III, P.E.							

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#### **TUBE PERMEAMETER TEST DATA**

		DE l'ERMEAMETER	IEOI DATA				
Project Name:	M & M At Neptune		Project I	Number:	19000475A		
Block:	701		Municip	ality:	Neptune		
Lot:	1		County:		Monmouth		
Test Number:	1		Date Co	llected:	4/17/2019		
Material Teste	d: TP-2		Replicat	e:	В		
Depth of Sampl	e: 8"		Sample	Type:	Undisturbed		
1.	Sample Dimensions	Radius = Length =	1.905 3.125	cm in			
2.	Measurements	Tube Weight =	353.09	g			
	tube #: M	Total Weight = -7 Soil Weight =	677.20 324.11	g g			
	Volume = L Volume =	ength * 2.54 cm/inch * 90.45	π * Radius²				
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.58	olume				
	Height of W	ater Level above Rim o	f Test Basin (inches	)			
	At beginnin	g of interval: 3.75 f interval: 3.75	50 H <sub>1</sub>	,			
3.	Test Data						
-	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length	(min)	∆ Height	(in)	
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00 av	60.0 60.0 60.0 60.0 = 60.0		$\begin{array}{c} 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\end{array}$		
4.	Permeability Calcu	lation K (in/hr)	= 60 min/hr * r²/R² * l	_ (in)/T (r	nin) * In (H <sub>1</sub> /H <sub>2</sub> )		
	K = 0.00	in/hr = Soil Pe	meability Class	K0			
5.	Any <b>Defects</b> in Sar	nple: No					
	I hereby certify that the that falsification of data and is subject to penalt	is a violation of the wa	ter polution Control				
Signature of Professional Engineer License #45357 Michael Carnivale, III, P.E.							

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#### **TUBE PERMEAMETER TEST DATA**

		TUBE	FERINE			А			
Project Name:	M & M At Nept	une				Project	t Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	/:	Monmouth	
Test Number:	1					Date C	collected:	4/17/2019	
Material Teste	d: TP-2					Replica	ate:	А	
Depth of Sampl	e: 24"					Sample	e Type:	Undisturbed	
1.	Sample Dimens	ions	Radius Length		1.905 3.250		cm in		
2.	Measurement	s	Tube W		360		g		
	tube #:	M-1	Total W Soil We		654 294		g g		
	Volume = Volume =	Lengt		cm/inch * π 94.07	* Radius	2			
	Bulk Density = Bulk Density =		Soil V	/eight / Vol 3.13	ume				
	Height	of Water	Level abo	ove Rim of <sup>·</sup>	Test Bas	in (inche	s)		
	-	nning of		3.500		` H₁	,		
	At e	nd of inte	erval:	3.500		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>		Time End	, T <sub>2</sub>	Test	Length	(min)	∆ Height	(in)
a. b.	0:00:00 0:00:00		1:00 1:00		6	0.0 0.0		0.000	
c. d.	0:00:00 0:00:00		1:00 1:00	av =	6	60.0 60.0 60.0		$0.000 \\ 0.000 \\ 0.000$	
4.	Permeability C	alculatio	on	K (in/hr) =	60 min/h	<b>)r *</b> r²/R² *	L (in)/T (n	nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.00	i	n/hr =	Soil Pern	neability	Class	K0		
5.	Any <b>Defects</b> ir	sample	:	No					
	l hereby certify tha that falsification of and is subject to p	data is a	violation	of the wate	r polutio	n Contro	n is true ar I Act (NJS	nd accurate. Ta A 59:10A-1 et	am aware seq.)
Signature	of Professional Eng	nineer						license	e #45357
Signature		J. 1001		Michae	l Carniva	ale III P	F		5 #40001

Michael Carnivale, III, P.E.

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#### TUBE PERMEAMETER TEST DATA

	100		LOI DATA	
Project Name:	M & M At Neptune		Project Numb	er: 19000475A
Block:	701		Municipality:	Neptune
Lot:	1		County:	Monmouth
Test Number:	1		Date Collecte	d: 4/17/2019
Material Tested	d: TP-2		Replicate:	В
Depth of Sample	e: 24"		Sample Type	Undisturbed
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.250 in	
2.	Measurements	Tube Weight =	351.88 g	
	tube #: BC-6	Total Weight = Soil Weight =	642.78 g 290.9 g	
	Volume = Len Volume =	gth * 2.54 cm/inch * т 94.07	r * Radius²	
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.09	lume	
	Height of Wate	r Level above Rim of	Test Basin (inches)	
	At beginning o			
	At end of in			
3.	Test Data			
-	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)
a.	0:00:00	1:00	60.0	0.000
b. c.	0:00:00 0:00:00	1:00 1:00	60.0 60.0	0.000 0.000
d.	0:00:00	1:00 av :	60.0 = 60.0	0.000 0.000
4.	Permeability Calculat	ion K (in/hr) =	= 60 min/hr * r²/R² * L (in)/	Γ (min) * In (H <sub>1</sub> /H <sub>2</sub> )
	K = 0.00	in/hr = Soil Peri	meability Class	60
5.	Any <b>Defects</b> in Samp	le: No		
t	hereby certify that the in that falsification of data is and is subject to penalties	a violation of the wat	er polution Control Act (N	and accurate. I am aware IJSA 59:10A-1 et seq.)
Signature o	of Professional Engineer			License #45357
2.3.144.00		Micha	el Carnivale, III, P.E.	

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#### **TUBE PERMEAMETER TEST DATA**

		IODL				~			
Project Name:	M & M At Nepti	une				Project	Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	<i>'</i> :	Monmouth	
Test Number:	1					Date C	ollected:	4/17/2019	
Material Tested	d: TP-3					Replica	ate:	A	
Depth of Sample	e: 19"					Sample	e Type:	Undisturbed	
1.	Sample Dimens	ons	Radius : Length :		1.905 3.125		cm in		
2.	Measurement	s	Tube We		366.		g		
	tube #:	M-3	Total We		690. 323.		g g		
	Volume = Volume =	Lengt		n/inch * π <sup>·</sup> 0.45	* Radius <sup>;</sup>	2			
	Bulk Density = Bulk Density =		Soil We	eight / Volu 3.57	ime				
	Height o	of Water I	Level abov	/e Rim of T	est Basi	n (inche	s)		
	_	nning of i		3.875		H₁			
	At ei	nd of inte	rval:	3.875	I	H <sub>2</sub>			
3.	Test Data								
_	Time Begin, T <sub>1</sub>	_	Гime End,	T <sub>2</sub>	Test	Length	(min)	∆ Height	_ (in)
a.	0:00:00		1:00			0.0		0.000	
b. c.	0:00:00 0:00:00		1:00 1:00		6	0.0 0.0		0.000 0.000	
d.	0:00:00		1:00	av =		0.0 0.0		0.000 0.000	
4.	Permeability C	alculatio	n	K (in/hr) =	60 min/h	<b>f *</b> r <sup>2</sup> /R <sup>2</sup> *	L (in)/T (n	nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.00	i	n/hr =	Soil Perm	eability (	Class	K0		
5.	Any <b>Defects</b> in	Sample	:	No					
1	l hereby certify tha that falsification of and is subject to pe	data is a	violation of	of the wate	r polutior	Contro			
Signature o	of Professional Eng	jineer						License	e #45357

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#### **TUBE PERMEAMETER TEST DATA**

		TUDE			SIDA	A			
Project Name:	M & M At Neptur	ne				Project	Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	/:	Monmouth	
Test Number:	1					Date C	ollected:	4/17/2019	
Material Teste	d: TP-3					Replica	ate:	В	
Depth of Sample	e: 19"					Sample	e Type:	Undisturbed	
1.	Sample Dimensio	ns	Radius Length		1.905 3.000		cm in		
2.	Measurements		Tube W		356		g		
	tube #:	M-4	Total W Soil We		669 312		g g		
	Volume = Volume =	Lengt		m/inch * π 86.83	* Radius	2			
	Bulk Density = Bulk Density =		Soil W	eight / Volu 3.60	ume				
	Height of	Water I	_evel abo	ve Rim of <sup>-</sup>	Fest Bas	in (inche	s)		
	At beginr			3.750		H <sub>1</sub>	-,		
		d of inte		3.625		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>		Time End	, T <sub>2</sub>	Test	Length	(min)	Δ Height	_ (in)
a.	0:00:00		1:00			0.0		0.125	
b. c.	0:00:00 0:00:00		1:00 1:00		6	60.0 60.0		0.125 0.125	
d.	0:00:00		1:00	av =		60.0 60.0		0.125 0.125	
4.	Permeability Cal	culatio	n	. ,			L (in)/T (n	nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.10	i	n/hr =	Soil Perm	neability	Class	K0		
5.	Any <b>Defects</b> in S	Sample	:	No					
1	l hereby certify that t that falsification of da and is subject to per	ata is a	violation	of the wate	r polutio	n Contro	n is true ar I Act (NJS	nd accurate. I A 59:10A-1 et	am aware seq.)
Signature o	of Professional Engir	neer						License	e #45357
5 -				Michae	l Carniva	ale, III, P	.E.		

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#### **TUBE PERMEAMETER TEST DATA**

	IUBE		ESIDATA	
Project Name:	M & M At Neptune		Project Number	: 19000475A
Block:	701		Municipality:	Neptune
Lot:	1		County:	Monmouth
Test Number:	1		Date Collected:	4/17/2019
Material Teste	d: TP-3		Replicate:	А
Depth of Sample	e: 47"		Sample Type:	Undisturbed
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.250 in	
2.	Measurements	Tube Weight =	358.17 g	
	tube #: BM-69	Total Weight = Soil Weight =	683.15 g 324.98 g	
	Volume = Leng Volume =	gth * 2.54 cm/inch * τ 94.07	τ * Radius²	
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.45	lume	
	Height of Water	r Level above Rim of	Test Basin (inches)	
	At beginning of	f interval: 3.50	0 Н1	
	At end of int	terval: 3.50	0 H <sub>2</sub>	
3.	Test Data			
-	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)
a.	0:00:00	1:00	60.0	0.000
b. c.	0:00:00 0:00:00	1:00 1:00	60.0 60.0	0.000 0.000
d.	0:00:00	1:00 av :	= 60.0 = 60.0	0.000 0.000
4.	Permeability Calculati	on K (in/br) -	= 60 min/hr * r²/R² * L (in)/T	(min) * In (1, 4, 1)
4.	K = 0		meability Class K0	. ,
	K - 0			
5.	Any <b>Defects</b> in Sample	e: No		
1		a violation of the wat	o of this application is true a er polution Control Act (NJ AC 7:14-8.	
Signature o	of Professional Engineer			License #45357
5 -	5	Micha	el Carnivale, III, P.E.	

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#### **TUBE PERMEAMETER TEST DATA**

		TODE							
Project Name:	M & M At Nept	une				Project	Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	:	Monmouth	
Test Number:	1					Date C	ollected:	4/17/2019	
Material Teste	d: TP-3					Replica	ate:	В	
Depth of Sample	e: 47"					Sample	e Type:	Undisturbed	
1.	Sample Dimensi	ons	Radius Length		1.905 3.625		cm in		
2.	Measurement	s	Tube W		359		g		
	tube #:	M-2	Total W Soil We		682 323		g g		
	Volume = Volume =	Lengt		cm/inch * π 104.92	* Radius	2			
	Bulk Density = Bulk Density =		Soil V	Veight / Vol 3.08	ume				
	Height o	of Water	Level abo	ove Rim of	Test Bas	in (inche	s)		
	_	nning of i		3.625		Ϋ́Η1	,		
	At ei	nd of inte	erval:	3.500		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>		Time End	l <u>, </u> T <sub>2</sub>	Test	Length	(min)	∆ Height	_ (in)
a.	0:00:00		1:00			0.0		0.125	
b. c.	0:00:00 0:00:00		1:00 1:00		6	60.0 60.0		0.125 0.125	
d.	0:00:00		1:00	av =		60.0 60.0		0.125 0.125	
4.	Permeability C	alculatic	on	K (in/hr) =	60 min/h	<b>)f *</b> r²/R² *	L (in)/T (n	nin) * In (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.13	i	n/hr =	Soil Pern	neability	Class	K0		
5.	Any <b>Defects</b> in	Sample	:	No					
	l hereby certify tha that falsification of and is subject to pe	data is a	violation	of the wate	r polutio	n Contro	is true ar I Act (NJS	nd accurate. Ta A 59:10A-1 et	am aware seq.)
Signatura	of Professional Eng	lineer						Licono	e #45357
Signature	มากายเธรรเบทสา ENg	jii ieei		Michae	l Carniva	ale III P	F		5 #40007

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#### **TUBE PERMEAMETER TEST DATA**

		IUDE			SIDAI	A			
Project Name:	M & M At Neptur	ne				Project	Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	/:	Monmouth	
Test Number:	1					Date C	ollected:	4/17/2019	
Material Tested	d: TP-4					Replica	ate:	А	
Depth of Sample	e: 20"					Sample	e Type:	Undisturbed	
1.	Sample Dimensio	ns	Radius Length		1.905 3.250		cm in		
2.	Measurements		Tube W		362		g		
	tube #:	M-5	Total W Soil We		697 335		g g		
	Volume = Volume =	Lengtl		m/inch * π 94.07	* Radius	2			
	Bulk Density = Bulk Density =		Soil W	/eight / Volı 3.57	ume				
	Height of	Water I	_evel abo	ve Rim of <sup>-</sup>	Fest Bas	in (inche	s)		
	At beginr			4.000		H <sub>1</sub>	-,		
		l of inte		4.000		H <sub>2</sub>			
3.	Test Data								
_	Time Begin, T <sub>1</sub>		Fime End	, T <sub>2</sub>	Test	Length	(min)	Δ Height	_ (in)
a.	0:00:00		1:00			0.0		0.000	
b. c.	0:00:00 0:00:00		1:00 1:00		6	0.0 0.0		0.000 0.000	
d.	0:00:00		1:00	av =		0.0 0.0		0.000 0.000	
4.	Permeability Cal							nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.00	i	n/hr =	Soil Perm	eability (	Class	K0		
5.	Any <b>Defects</b> in S	Sample:	:	No					
t	l hereby certify that t that falsification of da and is subject to per	ata is a	violation	of the wate	r polutio	n Contro	n is true ar I Act (NJS	nd accurate. Ta A 59:10A-1 et	am aware seq.)
Signature o	of Professional Engir	neer						License	e #45357
J				Michae	l Carniva	ale, III, P	.E.		

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#### **TUBE PERMEAMETER TEST DATA**

		IUDE				~			
Project Name:	M & M At Neptu	ne				Projec	t Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	/:	Monmouth	
Test Number:	1					Date C	collected:	4/17/2019	
Material Teste	d: TP-4					Replic	ate:	В	
Depth of Sample	e: 20"					Sampl	e Type:	Undisturbed	
1.	Sample Dimensi	ons	Radius Length		1.905 3.125		cm in		
2.	Measurement	S	Tube W		357		g		
	tube #:	M-6	Total W Soil We		687. 329.		g g		
	Volume = Volume =	Lengt		m/inch * π 90.45	* Radius	2			
	Bulk Density = Bulk Density =		Soil W	/eight / Volu 3.64	ume				
	Height o	f Water I	Level abo	ve Rim of <sup>-</sup>	Fest Basi	in (inche	s)		
	At begir			3.875		H <sub>1</sub>			
	At er	d of inte	rval:	3.875		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>		Time End	, T <sub>2</sub>	Test	Length	(min)	∆ Height	(in)
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00		1:00 1:00 1:00 1:00		6 6 6	0.0 0.0 0.0 0.0		0.000 0.000 0.000 0.000	
				av =	6	0.0		0.000	
4.	Permeability Ca	lculatio	'n	K (in/hr) =	60 min/h	1 <b>r *</b> r²/R² *	L (in)/T (n	nin) * In (H <sub>1</sub> /H <sub>2</sub> )	1
	K = 0.00	i	n/hr =	Soil Perm	eability (	Class	K0		
5.	Any <b>Defects</b> in	Sample	:	No					
	l hereby certify that that falsification of c and is subject to pe	lata is a	violation	of the wate	r polutio	n Contro	n is true ar I Act (NJS	nd accurate. I A 59:10A-1 et	am aware seq.)
Signature	of Professional Eng	ineer						Licens	e #45357
Signature	an Tolessional Elly			Michae	l Carniva	ale III P	F		ις <del>πη</del> ουσι

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#### **TUBE PERMEAMETER TEST DATA**

	TOBE		IEST DATA	
Project Name:	M & M At Neptune		Project Numbe	r: 19000475A
Block:	701		Municipality:	Neptune
Lot:	1		County:	Monmouth
Test Number:	1		Date Collected	: 4/17/2019
Material Teste	d: TP-5		Replicate:	A
Depth of Sampl	e: 15"		Sample Type:	Undisturbed
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.500 in	
2.	Measurements	Tube Weight =	349.96 g	
	tube #: AC-5	Total Weight = Soil Weight =	666.40 g 316.44 g	
	Volume = Leng Volume =	th * 2.54 cm/inch * ז 101.30	т * Radius²	
	Bulk Density = Bulk Density =	Soil Weight / Vo 3.12	lume	
	Height of Water	r Level above Rim of	f Test Basin (inches)	
	At beginning of At end of in	f interval: 3.62	5 Н1	
3.	Test Data			
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00 av	60.0 60.0 60.0 60.0 = 60.0	0.000 0.000 0.000 0.000 0.000
4.	Permeability Calculati	i <b>on</b> K (in/hr) :	= 60 min/hr * r²/R² * L (in)/T	(min) * ln (H <sub>1</sub> /H <sub>2</sub> )
	K = 0.00	in/hr = Soil Per	meability Class K(	)
5.	Any <b>Defects</b> in Sampl	e: No		
	I hereby certify that the inf that falsification of data is and is subject to penalties	a violation of the wat	ter polution Control Act (N.	
Signature	of Professional Engineer	Micha	el Carnivale, III, P.E.	License #45357

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#### **TUBE PERMEAMETER TEST DATA**

	101					
Project Name:	M & M At Neptune		Proj	ect Number:	19000475A	
Block:	701		Mur	icipality:	Neptune	
Lot:	1		Cou	nty:	Monmouth	
Test Number:	1		Date	e Collected:	4/17/2019	
Material Teste	d: TP-5		Rep	licate:	В	
Depth of Samp	le: 15"		San	nple Type:	Undisturbed	
1.	Sample Dimensions	Radius = Length =	1.905 3.375	cm in		
2.	Measurements	Tube Weight =	346.53	g		
	tube #: M-8	Total Weight = Soil Weight =	654.51 307.98	g g		
	Volume = Lee Volume =	ngth * 2.54 cm/inch * 97.68	π * Radius²			
	Bulk Density = Bulk Density =	Soil Weight / V 3.15	/olume			
	Height of Wat	er Level above Rim	of Test Basin (inc	hes)		
	At beginning At end of i					
3.	Test Data					
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Leng	th_ (min)	∆ Height	(in)
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00	$ \begin{array}{rcl} 60.0 \\ 60.0 \\ 60.0 \\ 60.0 \\ 60.0 \\ y = 60.0 \end{array} $		0.000 0.000 0.000 0.000 0.000	
4.	Permeability Calcula	ition K (in/hr)	) = 60 min/hr * r²/F	ռ² * L (in)/T (r	min) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.00	in/hr = Soil Pe	ermeability Class	K0		
5.	Any <b>Defects</b> in Sam	ple: No				
6.	I hereby certify that the in that falsification of data is and is subject to penaltie	s a violation of the wa	ater polution Con	tion is true ar trol Act (NJS	nd accurate. Ta A 59:10A-1 et s	am aware seq.)
Signature	of Professional Engineer	Mich	ael Carnivale, III	, P.E.	License	#45357

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#### **TUBE PERMEAMETER TEST DATA**

		TUDE	FERINEA			~			
Project Name:	M & M At Nept	une				Project N	lumber:	19000475A	
Block:	701					Municipa	lity:	Neptune	
Lot:	1					County:		Monmouth	
Test Number:	1					Date Col	lected:	4/17/2019	
Material Teste	d: TP-5					Replicate	e:	А	
Depth of Sampl	e: 53"					Sample <sup>-</sup>	Гуре:	Undisturbed	
1.	Sample Dimens	ions	Radius Length		1.905 3.000		cm in		
2.	Measurement	s	Tube W		358.		g		
	tube #:	M-9	Total W Soil We		590. 232.		g g		
	Volume = Volume =	Lengt		m/inch * π 86.83	* Radius <sup>;</sup>	2			
	Bulk Density = Bulk Density =		Soil W	/eight / Volı 2.68	ume				
	Height	of Water	Level abo	ve Rim of <sup>-</sup>	Test Basi	n (inches)			
	-	nning of		3.875		Η <sub>1</sub>			
		nd of inte		3.625		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>		Time End	, T <sub>2</sub>	Test	Length (	min)	∆ Height	_ (in)
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00		15:00 15:15 15:22 15:21	av =	1 1 1	5.0 5.3 5.4 5.4 5.3		0.250 0.250 0.250 0.250 0.250	
4.	Permeability C	alculatio	on	K (in/hr) =	60 min/h	r * r²/R² * L	(in)/T (n	nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	
	K = 0.79	i	in/hr =	Soil Perm	neability (	Class	K2		
5.	Any <b>Defects</b> ir	sample	:	No					
	l hereby certify tha that falsification of and is subject to p	data is a	violation	of the wate	r polutior	Control A	s true ar Act (NJS	nd accurate. Ta A 59:10A-1 et	am aware seq.)
Signaturo	of Professional Eng	nineer						License	e #45357
Signature	or inforessional Eng	JUICEI		Michae	l Carniva	le III PF			5 #40001

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#### TUBE PERMEAMETER TEST DATA

		IODL				~			
Project Name:	M & M At Nept	une				Project	Number:	19000475A	
Block:	701					Municip	ality:	Neptune	
Lot:	1					County:		Monmouth	
Test Number:	1					Date Co	ollected:	4/17/2019	
Material Tested	d: TP-5					Replica	te:	В	
Depth of Sample	e: 53"					Sample	Type:	Undisturbed	
1.	Sample Dimens	ons	Radius Length		1.905 3.000		cm in		
2.	Measurement	s	Tube W		358.		g		
	tube #:	M-10	Total W Soil We		602. 243.		g g		
	Volume = Volume =	Lengt		m/inch * π 86.83	* Radius	2			
	Bulk Density = Bulk Density =		Soil W	/eight / Volu 2.81	ıme				
	Height o	of Water I	Level abo	ve Rim of ⊺	rest Basi	in (inches	;)		
	_	nning of i		3.750		Ϋ́Η1	,		
	At er	nd of inte	rval:	3.375		H <sub>2</sub>			
3.	Test Data								
-	Time Begin, T <sub>1</sub>	-	Time End	, T <sub>2</sub>	Test	Length	(min)	<u>Δ</u> Height	(in)
a.	0:00:00		15:00			5.0		0.375	
b. c.	0:00:00 0:00:00		15:12 15:18		1	5.2 5.3		0.375 0.375	
d.	0:00:00		15:15	av =		5.3 5.2		0.375 0.375	
4.	Permeability C	alculatio	'n	K (in/hr) =	60 min/h	r * r <sup>2</sup> /R <sup>2</sup> *	l (in)/T (r	nin) * ln (H <sub>1</sub> /H <sub>2</sub> )	1
	K = 1.25		n/hr =	Soil Perm			K2		
	-				,				
5.	Any <b>Defects</b> in	Sample	:	No					
t	hereby certify tha hat falsification of and is subject to pe	data is a	violation	of the wate	r polution	n Control			
Signaturo	of Professional Eng	lineer						Licone	e #45357
Signature	ท 1 ายเธรรเยแลเ ธิกยุ	JILICEI		Michae	l Carniva	ale III PI	=		<del>c #4</del> 0001

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#### TUBE PERMEAMETER TEST DATA

	TOBE		ESIDATA	
Project Name:	M & M At Neptune		Project Number:	19000475A
Block:	701		Municipality:	Neptune
Lot:	1		County:	Monmouth
Test Number:	1		Date Collected:	4/17/2019
Material Tested	l: TP-6		Replicate:	A
Depth of Sample	e: 18"		Sample Type:	Undisturbed
1.	Sample Dimensions	Radius = Length =	1.905 cm 3.375 in	
2.	Measurements	Tube Weight =	357.34 g	
	tube #: BM-65	Total Weight = Soil Weight =	713.85 g 356.51 g	
	Volume = Lengt Volume =	h * 2.54 cm/inch * π 97.68	* Radius²	
	Bulk Density = Bulk Density =	Soil Weight / Vol 3.65	ume	
	Height of Water	Level above Rim of	Test Basin (inches)	
	At beginning of			
	At end of inte			
3.	Test Data			
_	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Length (min)	$\Delta$ Height (in)
a.	0:00:00	1:00	60.0	0.000
b. c.	0:00:00 0:00:00	1:00 1:00	60.0 60.0	0.000 0.000
d.	0:00:00	1:00 av =	60.0 = 60.0	0.000 0.000
4.	Permeability Calculation		60 min/hr * r²/R² * L (in)/T (	min) * In (H <sub>1</sub> /H <sub>2</sub> )
	K = 0.00	in/hr = Soil Pern	neability Class K0	
5.	Any <b>Defects</b> in Sample	:: No		
t	hereby certify that the info hat falsification of data is a and is subject to penalties a	violation of the wate	of this application is true a er polution Control Act (NJS C 7:14-8.	nd accurate. I am aware SA 59:10A-1 et seq.)
Signature o	of Professional Engineer			License #45357
5	5	Michae	el Carnivale, III, P.E.	

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#### **TUBE PERMEAMETER TEST DATA**

Project Name:	M & M At Neptune		Pro	ject Number:	19000475A					
Block:	701		Mu	nicipality:	Neptune					
Lot:	1		Cou	unty:	Monmouth					
Test Number:	1		Dat	e Collected:	4/17/2019					
Material Teste	d: TP-6		Rep	olicate:	В					
Depth of Sampl	e: 18"		Sar	nple Type:	Undisturbed					
1.	Sample Dimensions	Radius = Length =	1.905 3.375	cm in						
2.	Measurements	Tube Weight =	355.68							
	tube #: AC-20	Total Weight = Soil Weight =	712.84 357.16	g g						
	Volume =Length * 2.54 cm/inch * π * Radius²Volume =97.68									
Bulk Density = Soil Weight / Volume Bulk Density = 3.66										
	Height of Water	r Level above Rim of	Test Basin (in	ches)						
	At beginning of At end of int									
3.	Test Data									
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Leng	<u>ıth</u> (min)	∆ Height	(in)				
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00 av	60.0 60.0 60.0 60.0 = 60.0		0.000 0.000 0.000 0.000 0.000					
4. <b>Permeability Calculation</b> K (in/hr) = 60 min/hr * $r^2/R^2$ * L (in)/T (min) * ln (H <sub>1</sub> /H <sub>2</sub> )										
	K = 0.00	in/hr = Soil Per	meability Class	6 K0						
5.	Any <b>Defects</b> in Sampl	e: No								
	6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water polution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.									
Signature of Professional Engineer License #45357 License #45357										

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#### **TUBE PERMEAMETER TEST DATA**

Project Name:	M & M At Neptune		F	Project Number:	19000475A					
Block:	701		Ν	Municipality:	Neptune					
Lot:	1		C	County:	Monmouth					
Test Number:	1		C	Date Collected:	4/17/2019					
Material Teste	d: TP-7		F	Replicate:	A					
Depth of Samp	e: 24"		S	Sample Type:	Undisturbed					
1.	Sample Dimensions	Radius = Length =	1.905 3.375	cm in						
2.	Measurements	Tube Weight =	368.57							
	tube #: B-8	Total Weight = Soil Weight =	712.14 343.57							
	Volume = Length * 2.54 cm/inch * π * Radius² Volume = 97.68									
	Bulk Density = Soil Weight / Volume Bulk Density = 3.52									
	Height of Wat	er Level above Rim	of Test Basin	(inches)						
	At beginning At end of i	of interval: 3.8	375 H <sub>1</sub> 375 H <sub>2</sub>							
3.	Test Data									
	Time Begin, T <sub>1</sub>	Time End, T <sub>2</sub>	Test Le	ength_ (min)	∆ Height	(in)				
a. b. c. d.	0:00:00 0:00:00 0:00:00 0:00:00	1:00 1:00 1:00 1:00	60. 60. 60. 60. v = 60.	0 0 0	0.000 0.000 0.000 0.000 0.000					
4.	4. <b>Permeability Calculation</b> K (in/hr) = 60 min/hr * $r^2/R^2$ * L (in)/T (min) * ln (H <sub>1</sub> /H <sub>2</sub> )									
	K = 0.00	in/hr = Soil P	ermeability Cla	ass K0						
5.	Any <b>Defects</b> in Sam	ple: No								
6.	6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water polution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.									
Signature of Professional Engineer License #45357 License #45357										

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#### **TUBE PERMEAMETER TEST DATA**

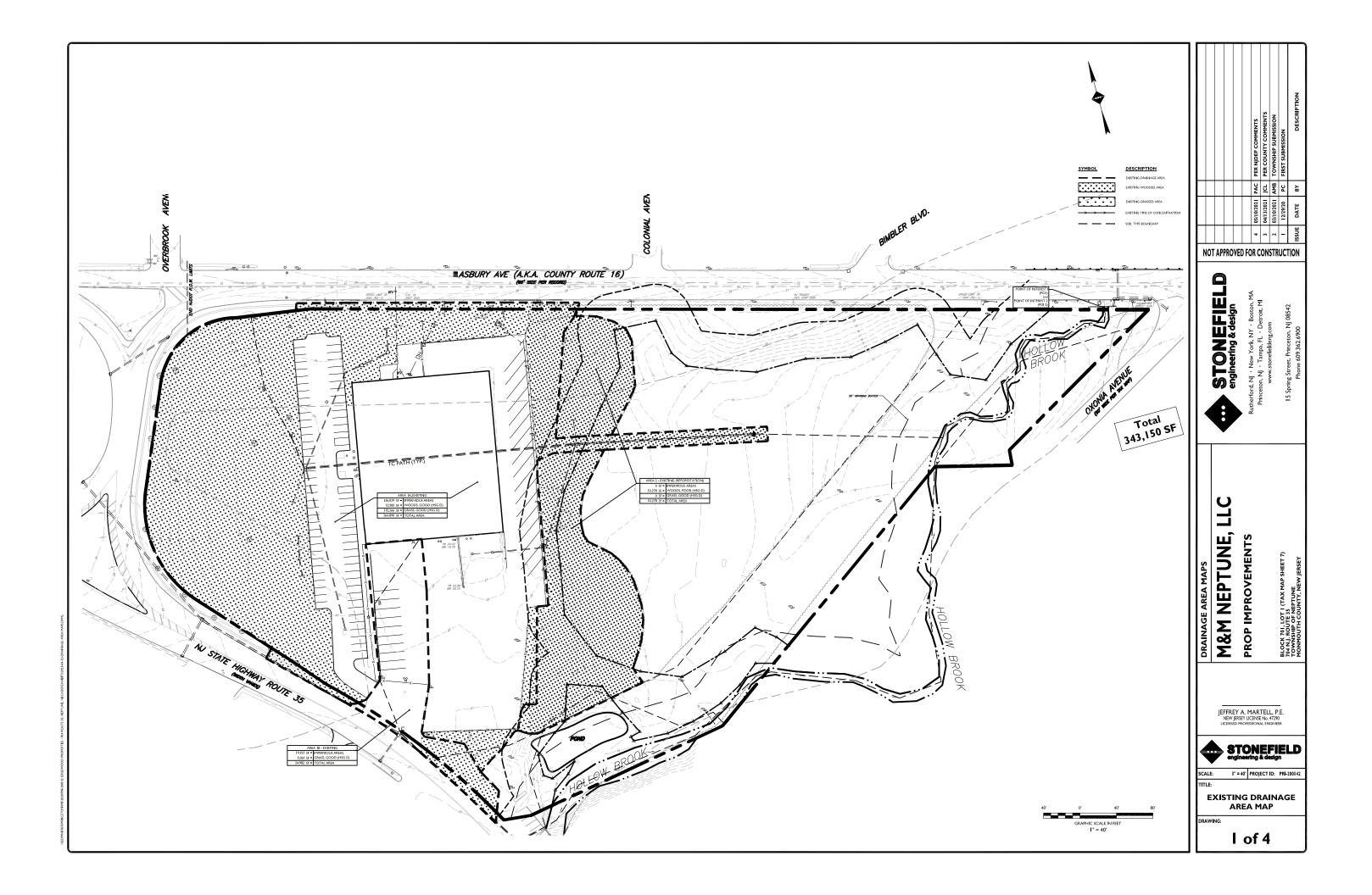
		IODE				~			
Project Name:	M & M At Nept	une				Projec	t Number:	19000475A	
Block:	701					Munici	pality:	Neptune	
Lot:	1					County	/:	Monmouth	
Test Number:	1					Date C	collected:	4/17/2019	
Material Testee	d: TP-7					Replica	ate:	В	
Depth of Sample	e: 24"					Sample	e Type:	Undisturbed	
1.	Sample Dimens			1.905 3.125					
2.	Measurements		Tube Weight =		356				
	tube #:	M-11	Total Weight = Soil Weight =		688.36 332.24		g g		
	Volume = Volume =	Lengt		m/inch * π 90.45	* Radius	2			
Bulk Density =Soil Weight / VolumeBulk Density =3.67									
	Height	of Water	Level abo	ove Rim of <sup>-</sup>	Гest Bas	in (inche	s)		
Height of Water Level above Rim of Test Basin (inches) At beginning of interval: 3.750 H <sub>1</sub>									
	At e	nd of inte	rval:	3.750		H <sub>2</sub>			
3.	Test Data								
_	Time Begin, T <sub>1</sub>		Time End	, T <sub>2</sub>	Test	Length	(min)	∆ Height	(in)
a.	0:00:00		1:00			0.0		0.000	
b. c.	0:00:00 0:00:00		1:00 1:00		6	0.0 0.0		0.000 0.000	
d.	0:00:00		1:00	av =		0.0 0.0		0.000 0.000	
4.	Permeability C	alculatio	'n	K (in/br) =	60 min/h	$r * r^{2}/P^{2} *$	· l (in)/T (n	nin) * In (ப /ப )	
т.	K = 0.00 in/hr =			K (in/hr) = 60 min/hr * $r^2/R^2$ * L (in)/T (min) * ln (H <sub>1</sub> /H <sub>2</sub> ) Soil Permeability Class K0					
					loability	Cidoo			
5.	Any <b>Defects</b> in	Sample	:	No					
f	l hereby certify tha that falsification of and is subject to p	data is a	violation	of the wate	r polutio	n Contro	n is true ar I Act (NJS	nd accurate. I A 59:10A-1 et	am aware seq.)
Signaturo	of Professional Eng	nineer						License	e #45357
Signature	มา าบเธรรเบทสเ Eli	JUICEI		Michae	l Carniva	ale III P	F		5 #40007

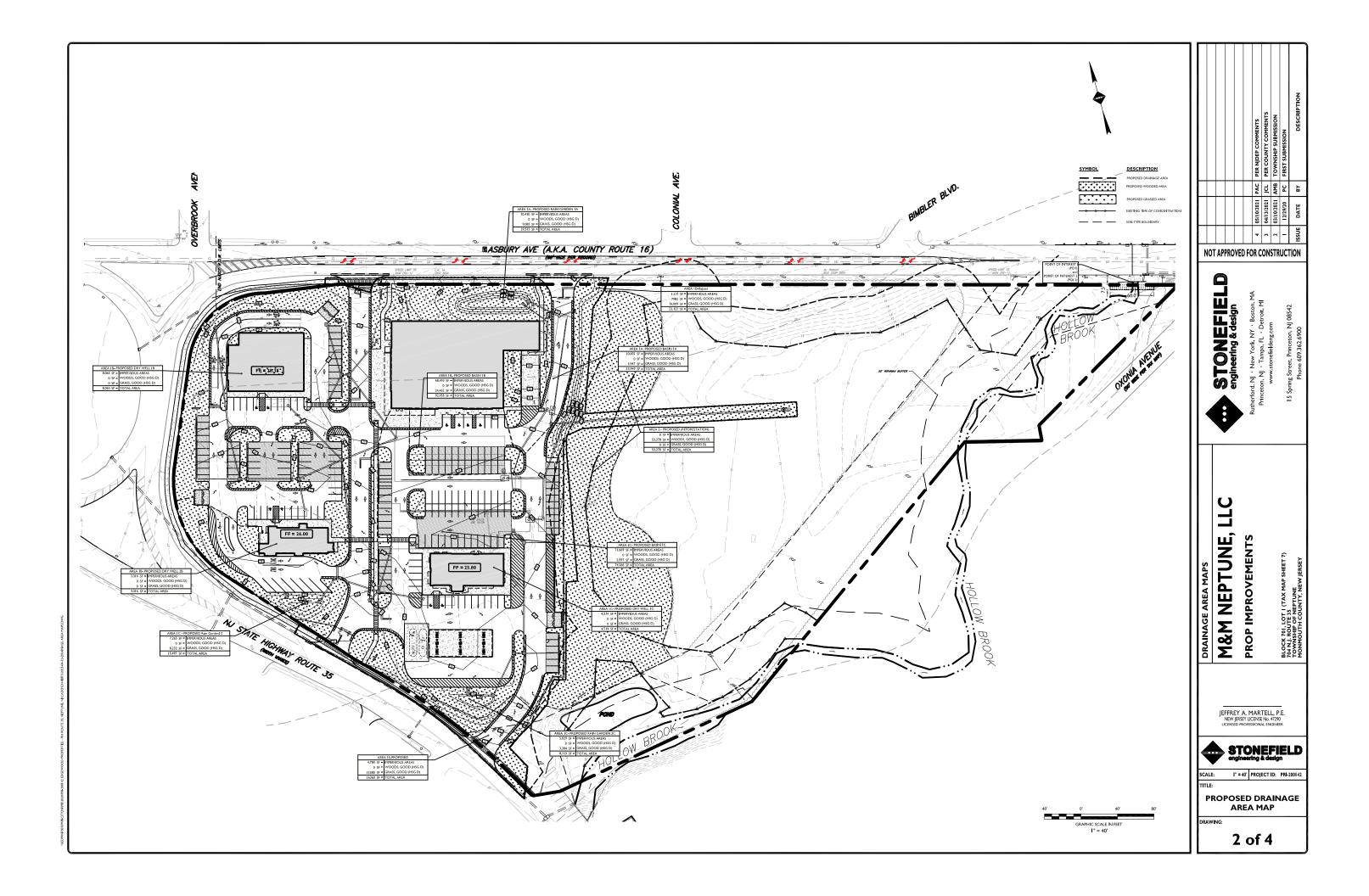
## APPENDIX E DRAINAGE AREA MAPS

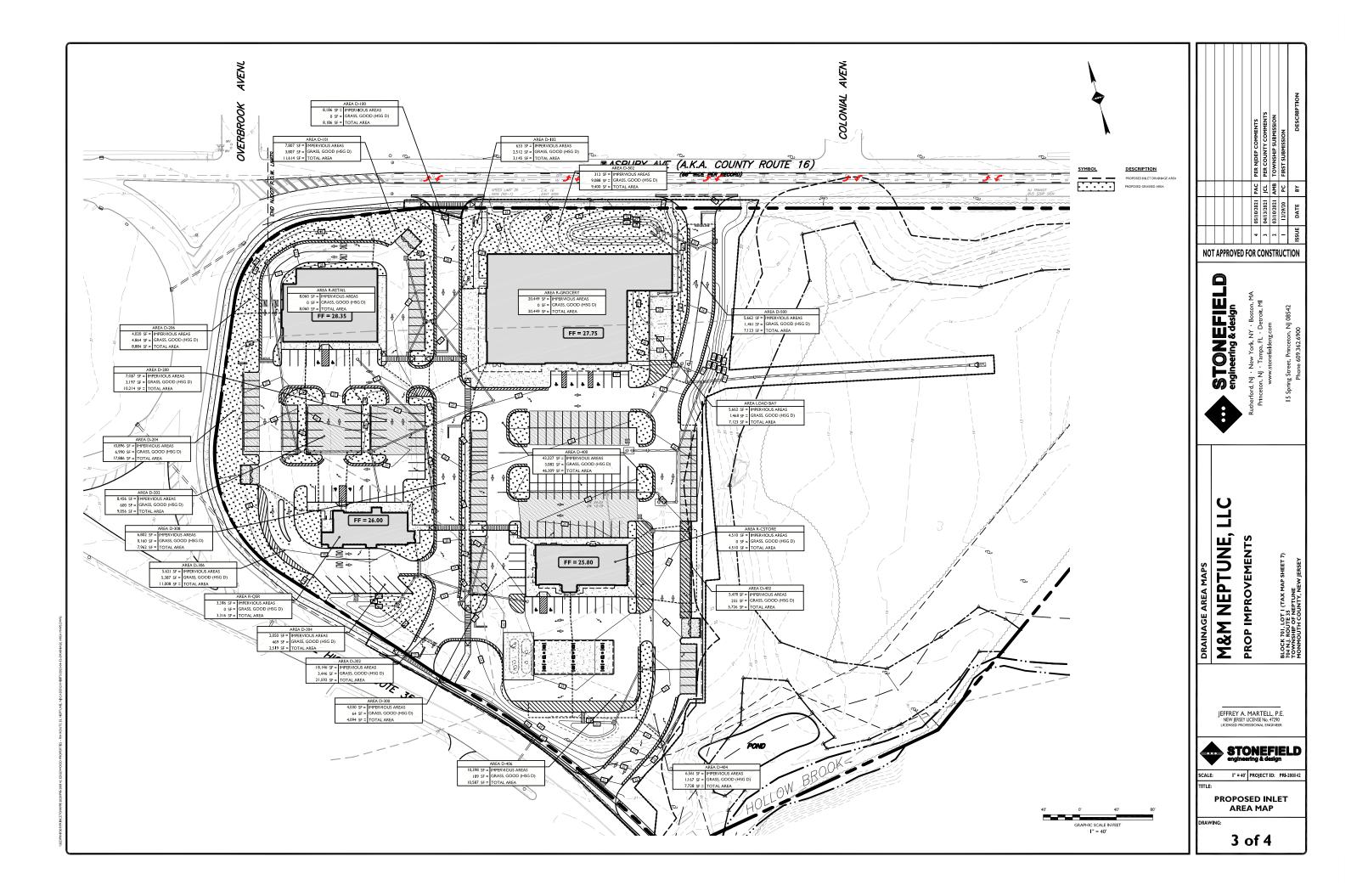
**INVENTORY** 

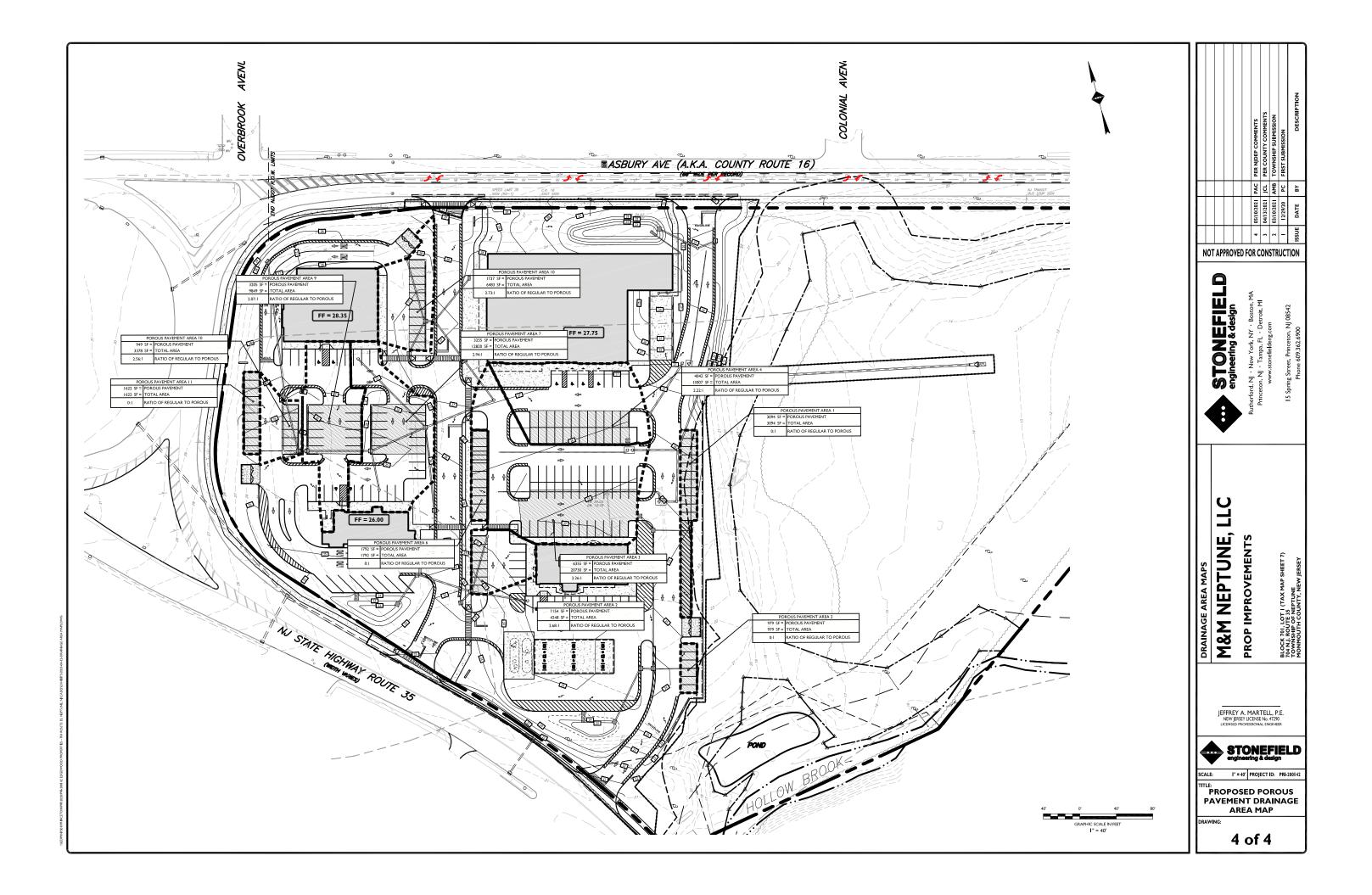
Existing Drainage Area Map Proposed Drainage Area Map Proposed Inlet Drainage Area Map Proposed Porous Pavement Drainage Area Map Drainage Area to Deal Lake

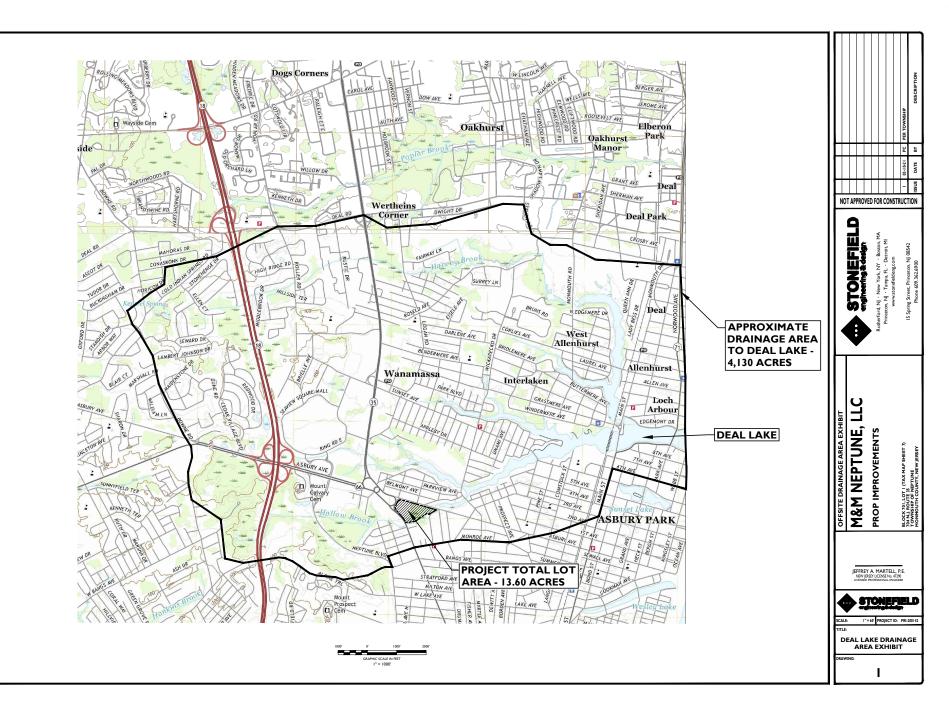












# APPENDIX F MANUFACTURED TREATMENT DEVICE SPECIFICATIONS





# Mixed Use (MTD 1A)

Neptune, NJ 4/30/21

## Information Provided by Engineer:

- Required TSS removal rate = 80%
- Water quality flow rate = 0.36 cfs
- 25-YR peak flow rate = 2.65 cfs
- Motor vehicle impervious area = 0.23 acres
- Presiding agency = NJDEP

# StormFilter Information and Cartridge Data:

The Stormwater Management StormFilter<sup>®</sup> is a passive, siphon-actuated, flow-through stormwater filtration system consisting of a precast concrete structure that houses rechargeable, media-filled filter cartridges. The StormFilter works by passing stormwater through the media-filled cartridges, which trap particulates and adsorb pollutants such as dissolved metals, nutrients, and hydrocarbons. The StormFilter has received final certification from the NJDEP for 80% TSS removal as a stand-alone treatment system.

- StormFilter cartridge filter media = Perlite
- StormFilter cartridge media height = 27 inches (nominal)
- StormFilter cartridge surface area = 10.61 square feet (nominal)
- StormFilter cartridge specific treatment flow rate = 2.12 gallons/minute per square foot (nominal)
- StormFilter cartridge treatment flow = 22.5 gpm
- Hydraulic head required: 3.05 feet (with 27 inch cartridge)
- Minimum physical drop between inlet and outlet pipe = 6 inches

## **Design Summary:**

The StormFilter is sized based on the NJDEP certification, which lists an approved treatment flow rate and maximum impervious acreage limit per cartridge in Table 1. The number of cartridges required based on the impervious drainage area is compared with the number of cartridges required based on the treatment flow rate; the larger number of cartridges governs the sizing.

The StormFilter for this site was sized to provide **8 cartridges** in order to meet the hydraulic load requirement (calculations shown below). To house this number of cartridges, Contech Engineered Solutions recommends a 6'x12' precast Peak Diversion StormFilter.

$$N_{cartridges \ hyd.load} = \frac{Q_{treat} \ X \ 449 \ gpm/_{cfs}}{Q_{cartridge}} = \frac{0.36 \ cfs \ X \ 449 \ gpm/_{cfs}}{22.5 \ gpm/_{cartridge}} = 7.18 \Rightarrow (8) \ 27" \ Cartridges$$

 $N_{cartridges\ mass\ load} = \frac{Area_{site}}{Max\ Area_{cartridge}} = \frac{0.23\ acre}{0.136\ acres/_{cartridge}} = 1.69 \Rightarrow (2)\ 27"\ Cartridges$ 



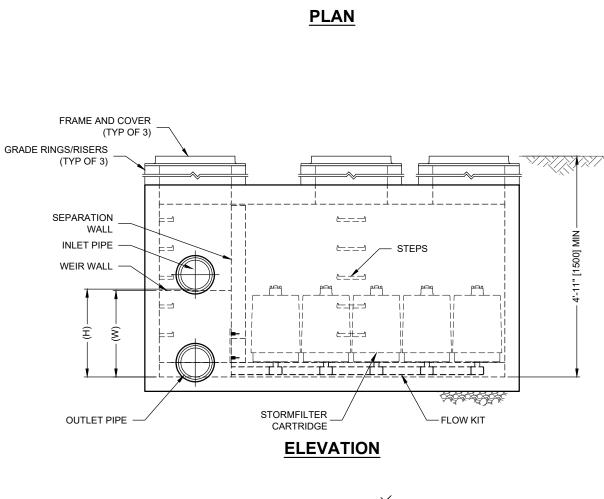
### Maintenance:

Maintenance of Stormwater best management practices is required per the New Jersey Administrative Code 7:8-5.8. Recommendations for maintenance are included in chapters 8 & 9 of the New Jersey Stormwater Best Management Practices Manual. To comply with requirements, CONTECH offers a network of Preferred Service Providers that have the capability to perform all necessary inspections, compliance reporting and cleaning services. CONTECH recommends inspecting the system annually and maintaining the system at the recommendation of the annual inspection. Full maintenance is typically required every 24-36 months. Disposal of material should be handled in accordance with local regulations. Please contact CONTECH's Maintenance Department for all questions regarding maintenance at (503) 258-3157 or visit our website at www.conteches.com/maintenance.

Thank you for the opportunity to present this information to you and your client. If you have any questions, please call me at (443-457-1529).

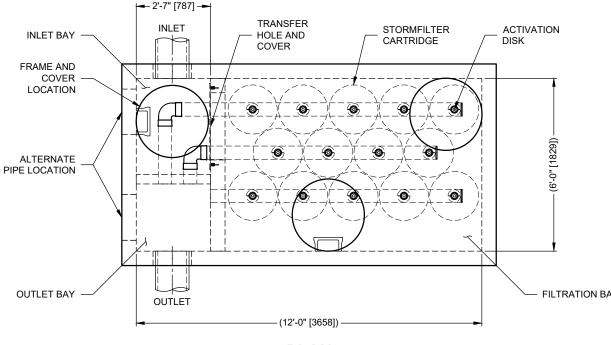
Sincerely,

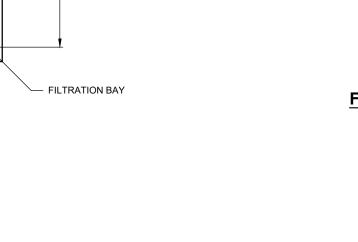
Taylor Murdock Contech Engineered Solutions LLC

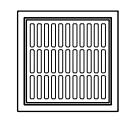


The Stormwater Manage StormFilter\*

THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: 5,322,629,5,524,576,5,707,527,5,985,157,6,027,639,6649,048; RELATED FOREIGN PATENTS, OR OTHER PATENTS PENDING.







FRAME AND GRATE

(24" SQUARE) (NOT TO SCALE)



(30" ROUND)

(NOT TO SCALE)

PERFORMANCE SPECIFICATION FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF CLEANING. RADIAL MEDIA DEPTH SHALL BE 7" [178]. FILTER MEDIA CONTACT TIME SHALL BE AT LEAST 37 SECONDS. SPECIFIC FLOW RATE SHALL BE 2 GPM/SF [1.36 L/s/m<sup>2</sup>] (MAXIMUM). SPECIFIC FLOW RATE IS THE MEASURE OF THE FLOW (GPM) DIVIDED BY THE MEDIA SURFACE CONTACT AREA (SF). MEDIA VOLUMETRIC FLOW RATE SHALL BE 6 GPM/CF [13.39 L/s/m<sup>3</sup>] OF MEDIA (MAXIMUM).

#### GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE
- 2. DIMENSIONS MARKED WITH ( ) ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
- 3. ALTERNATE DIMENSIONS ARE IN MILLIMETERS [mm] UNLESS NOTED OTHERWISE.
- 4. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH REPRESENTATIVE. www.ContechES.com
- 5. STORMFILTER WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 6. STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' 10' [3048] AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.

#### INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL SECTIONS AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH OUTLET PIPE INVERT WITH OUTLET BAY FLOOR
- E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION-RELATED EROSION RUNOFF.



- CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD A 6' x 12' [1829 x 3658] PEAK DIVERSION STYLE STORMFILTER IS SHOWN WITH THE MAXIMUM NUMBER OF CARTRIDGES (14) AND IS AVAILABLE IN A LEFT INLET (AS SHOWN) OR A RIGHT INLET CONFIGURATION
- ALL BARTS AND INTERNAL ASSEMBLY PROVIDED BY CONTECHLINI ESS NOTED OTHERWISE

• ALL PARTS AND INTERINAL ASSEMBLT FROVIDED BT CONTECTIONEESS NOTED OTTER WISE									
CARTRIDGE SIZE (in. [mm])	27 [686]			18 [457]			LOW DROP		
RECOMMENDED HYDRAULIC DROP (H) (ft. [mm])	3.05 [930]			2.3 [701]			1.8 [549]		
HEIGHT OF WEIR (W) (ft. [mm])	3.00 [914]		2.25 [686]			1.75 [533]			
SPECIFIC FLOW RATE (gpm/sf [L/s/m <sup>2</sup> ])	2 [1.36]	1.67* [1.13]*	1 [0.68]	2 [1.36]	1.67* [1.13]*	1 [0.68]	2 [1.36]	1.67* [1.13]*	1 [0.68]
CARTRIDGE FLOW RATE (gpm [L/s])	22.5 [1.42]	18.79 [1.19]	11.25 [0.71]	15 [0.95]	12.53 [0.79]	7.5 [0.47]	10 [0.63]	8.35 [0.53]	5 [0.32]

\* 1.67 gpm/sf [1.13 L/s/m<sup>2</sup>] SPECIFIC FLOW RATE IS APPROVED WITH PHOSPHOSORB<sup>®</sup> (PSORB) MEDIA ONLY

# **STORMFILTER DESIGN NOTES**

STORMFILTER TREATMENT CAPACITY VARIES BY CARTRIDGE COUNT AND LOCALLY APPROVED SURFACE AREA SPECIFIC FLOW RATE. PEAK

SITE SPECIFIC DATA REQUIREMENTS						
	NLQ0					
WATER QUALITY F	LOW RATE (	cfs [L/s])				
PEAK FLOW RATE		[])				
RETURN PERIOD O		W (yrs)				
CARTRIDGE FLOW		,				
CARTRIDGE SIZE (27, 18, LOW DROP (LD))						
MEDIA TYPE (PERLITE, ZPG, PSORB)						
NUMBER OF CARTI	RIDGES REC	UIRED				
FILTER BAY RIM EL	EVATION					
PIPE DATA:	INVERT	MATERIAL	DIAMETER			
INLET PIPE 1						
INLET PIPE 2						
OUTLET PIPE						
NOTES/SPECIAL REQUIREMENTS:						

B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMFILTER STRUCTURE.

# SFPD0612 (6' x 12') PEAK DIVERSION STORMFILTER STANDARD DETAIL



# Mixed Use (MTD 1B)

Neptune, NJ 4/30/21

## Information Provided by Engineer:

- Required TSS removal rate = 80%
- Water quality flow rate = 1.46 cfs
- 25-YR peak flow rate = 4.88 cfs
- Motor vehicle impervious area = 1.57 acres
- Presiding agency = NJDEP

# StormFilter Information and Cartridge Data:

The Stormwater Management StormFilter<sup>®</sup> is a passive, siphon-actuated, flow-through stormwater filtration system consisting of a precast concrete structure that houses rechargeable, media-filled filter cartridges. The StormFilter works by passing stormwater through the media-filled cartridges, which trap particulates and adsorb pollutants such as dissolved metals, nutrients, and hydrocarbons. The StormFilter has received final certification from the NJDEP for 80% TSS removal as a stand-alone treatment system.

- StormFilter cartridge filter media = Perlite
- StormFilter cartridge media height = 27 inches (nominal)
- StormFilter cartridge surface area = 10.61 square feet (nominal)
- StormFilter cartridge specific treatment flow rate = 2.12 gallons/minute per square foot (nominal)
- StormFilter cartridge treatment flow = 22.5 gpm
- Hydraulic head required: 3.05 feet (with 27 inch cartridge)
- Minimum physical drop between inlet and outlet pipe = 6 inches

## **Design Summary:**

The StormFilter is sized based on the NJDEP certification, which lists an approved treatment flow rate and maximum impervious acreage limit per cartridge in Table 1. The number of cartridges required based on the impervious drainage area is compared with the number of cartridges required based on the treatment flow rate; the larger number of cartridges governs the sizing.

The StormFilter for this site was sized to provide **30** cartridges in order to meet the hydraulic load requirement (calculations shown below). To house this number of cartridges, Contech Engineered Solutions recommends an 8'x20' precast Peak Diversion StormFilter.

$$N_{cartridges \ hyd.load} = \frac{Q_{treat} \ X \ 449 \ gpm/_{cfs}}{Q_{cartridge}} = \frac{1.46 \ cfs \ X \ 449 \ gpm/_{cfs}}{22.5 \ gpm/_{cartridge}} = 29.14 \Rightarrow (30) \ 27" \ Cartridges$$

 $N_{cartridges \ mass \ load} = \frac{Area_{site}}{Max \ Area_{cartridge}} = \frac{1.57 \ acre}{0.136 \ acres/_{cartridge}} = 11.54 \Rightarrow (12) \ 27" \ Cartridges$ 



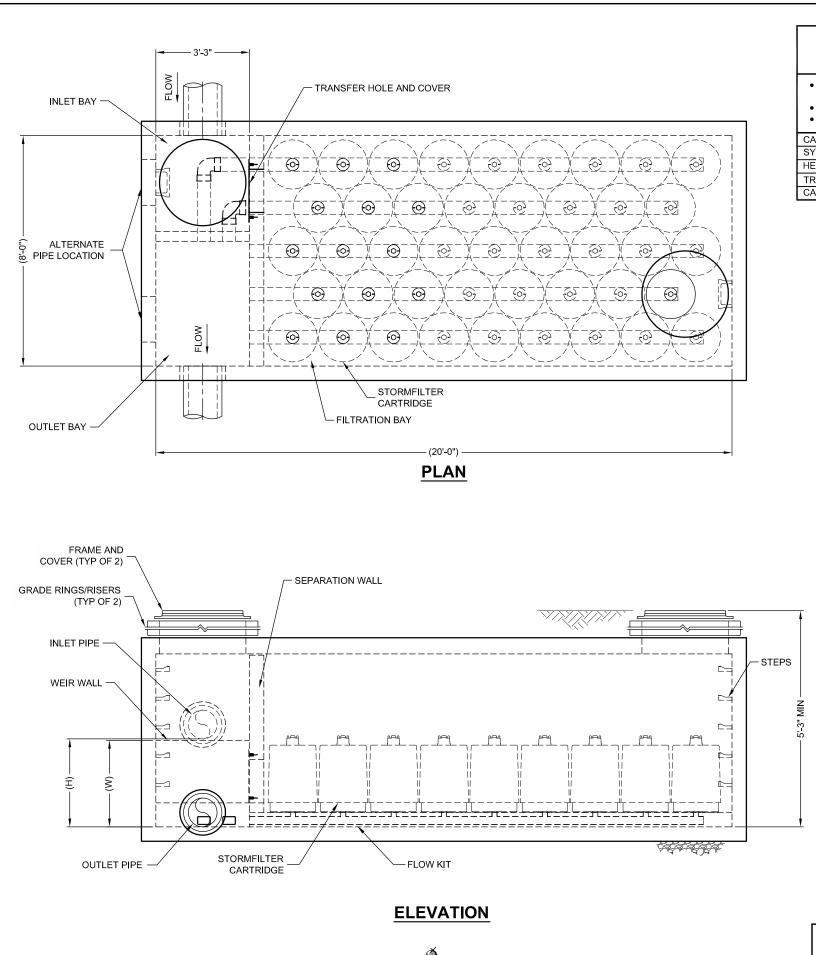
### Maintenance:

Maintenance of Stormwater best management practices is required per the New Jersey Administrative Code 7:8-5.8. Recommendations for maintenance are included in chapters 8 & 9 of the New Jersey Stormwater Best Management Practices Manual. To comply with requirements, CONTECH offers a network of Preferred Service Providers that have the capability to perform all necessary inspections, compliance reporting and cleaning services. CONTECH recommends inspecting the system annually and maintaining the system at the recommendation of the annual inspection. Full maintenance is typically required every 24-36 months. Disposal of material should be handled in accordance with local regulations. Please contact CONTECH's Maintenance Department for all questions regarding maintenance at (503) 258-3157 or visit our website at www.conteches.com/maintenance.

Thank you for the opportunity to present this information to you and your client. If you have any questions, please call me at (443-457-1529).

Sincerely,

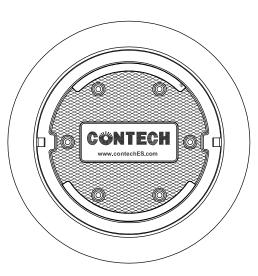
Taylor Murdock Contech Engineered Solutions LLC



StormFilter\*

- SPECIFIC FLOW RATE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD.
- ALL PARTS AND INTERNAL ASSEMBLY PROVIDED BY CONTECH UNLESS OTHERWISE NOTED.

CARTRIDGE HEIGHT	27"		18"		LOW DROP	
SYSTEM HYDRAULIC DROP (H - REQ'D. MIN.)	3.05'		2.3'		1.8'	
HEIGHT OF WEIR (W)	3.00'		2.25'		1.75'	
TREATMENT BY MEDIA SURFACE AREA	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>
CARTRIDGE FLOW RATE (gpm)	22.5	11.25	15	7.5	10	5



FRAME AND COVER (DIAMETER VARIES)

N.T.S.

### PERFORMANCE SPECIFICATION

FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF CLEANING. RADIAL MEDIA DEPTH SHALL BE 7-INCHES. FILTER MEDIA CONTACT TIME SHALL BE AT LEAST 37 SECONDS. SPECIFIC FLOW RATE SHALL BE 2 GPM/SF (MAXIMUM). SPECIFIC FLOW RATE IS THE MEASURE OF THE FLOW (GPM) DIVIDED BY THE MEDIA SURFACE CONTACT AREA (SF). MEDIA VOLUMETRIC FLOW RATE SHALL BE 6 GPM/CF OF MEDIA (MAXIMUM).

### GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. DIMENSIONS MARKED WITH ( ) ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
- REPRESENTATIVE. www.contechES.com
- THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.

### INSTALLATION NOTES

- SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- STRUCTURE (LIFTING CLUTCHES PROVIDED).
- C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL SECTIONS AND ASSEMBLE STRUCTURE.



# STORMFILTER DESIGN TABLE

• THE 8' x 20' PEAK DIVERSION STORMFILTER TREATMENT CAPACITY VARIES BY CARTRIDGE COUNT AND LOCALLY APPROVED SURFACE AREA • THE PEAK DIVERSION STORMFILTER IS AVAILABLE IN A LEFT INLET (AS SHOWN) OR RIGHT INLET CONFIGURATION.

SITE SPECIFIC							
DATA REQUIREMENTS							
STRUCTURE ID *							
WATER QUALITY	WATER QUALITY FLOW RATE (cfs) *						
PEAK FLOW RATE	E (cfs)				*		
RETURN PERIOD	OF PEAK F	LO۱	N (yrs)		*		
# OF CARTRIDGES REQUIRED *							
CARTRIDGE FLOW RATE *							
MEDIA TYPE (CSF, PERLITE, ZPG) *							
PIPE DATA: I.E. MATERIAL DIAMETER							
INLET PIPE	۱.L. *						
OUTLET PIPE	*		*		*		
INLET BAY RIM EI	LEVATION				*		
FILTER BAY RIM I	ELEVATION				*		
ANTI-FLOTATION BALLAST WIDTH HEIGHT							
NOTES/SPECIAL REQUIREMENTS:							

3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH

4. STORMFILTER WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN 5. STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' - 5' AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.

A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND

CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMFILTER

D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH OUTLET PIPE INVERT WITH OUTLET BAY FLOOR. E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION-RELATED EROSION RUNOFF. F. CONTRACTOR TO REMOVE THE TRANSFER HOLE COVER WHEN THE SYSTEM IS BROUGHT ONLINE.

# THE STORMWATER MANAGEMENT STORMFILTER 8' x 20' PEAK DIVERSION STORMFILTER STANDARD DETAIL



# Mixed Use (MTD 1C)

Neptune, NJ 4/30/21

## Information Provided by Engineer:

- Required TSS removal rate = 80%
- Water quality flow rate = 1.24 cfs
- 25-YR peak flow rate = 3.75 cfs
- Motor vehicle impervious area = 1.65 acres
- Presiding agency = NJDEP

# StormFilter Information and Cartridge Data:

The Stormwater Management StormFilter<sup>®</sup> is a passive, siphon-actuated, flow-through stormwater filtration system consisting of a precast concrete structure that houses rechargeable, media-filled filter cartridges. The StormFilter works by passing stormwater through the media-filled cartridges, which trap particulates and adsorb pollutants such as dissolved metals, nutrients, and hydrocarbons. The StormFilter has received final certification from the NJDEP for 80% TSS removal as a stand-alone treatment system.

- StormFilter cartridge filter media = Perlite
- StormFilter cartridge media height = 27 inches (nominal)
- StormFilter cartridge surface area = 10.61 square feet (nominal)
- StormFilter cartridge specific treatment flow rate = 2.12 gallons/minute per square foot (nominal)
- StormFilter cartridge treatment flow = 22.5 gpm
- Hydraulic head required: 3.05 feet (with 27 inch cartridge)
- Minimum physical drop between inlet and outlet pipe = 6 inches

## **Design Summary:**

The StormFilter is sized based on the NJDEP certification, which lists an approved treatment flow rate and maximum impervious acreage limit per cartridge in Table 1. The number of cartridges required based on the impervious drainage area is compared with the number of cartridges required based on the treatment flow rate; the larger number of cartridges governs the sizing.

The StormFilter for this site was sized to provide **25 cartridges** in order to meet the hydraulic load requirement (calculations shown below). To house this number of cartridges, Contech Engineered Solutions recommends an 8'x18' precast Peak Diversion StormFilter.

$$N_{cartridges\ hyd.load} = \frac{Q_{treat}\ X\ 449\ gpm/_{cfs}}{Q_{cartridge}} = \frac{1.24\ cfs\ X\ 449\ gpm/_{cfs}}{22.5\ gpm/_{cartridge}} = 24.74 \Rightarrow (25)\ 27"\ Cartridges$$

 $N_{cartridges \ mass \ load} = \frac{Area_{site}}{Max \ Area_{cartridge}} = \frac{1.65 \ acre}{0.136 \ acres/cartridge} = 12.13 \Rightarrow (13) \ 27" \ Cartridges$ 



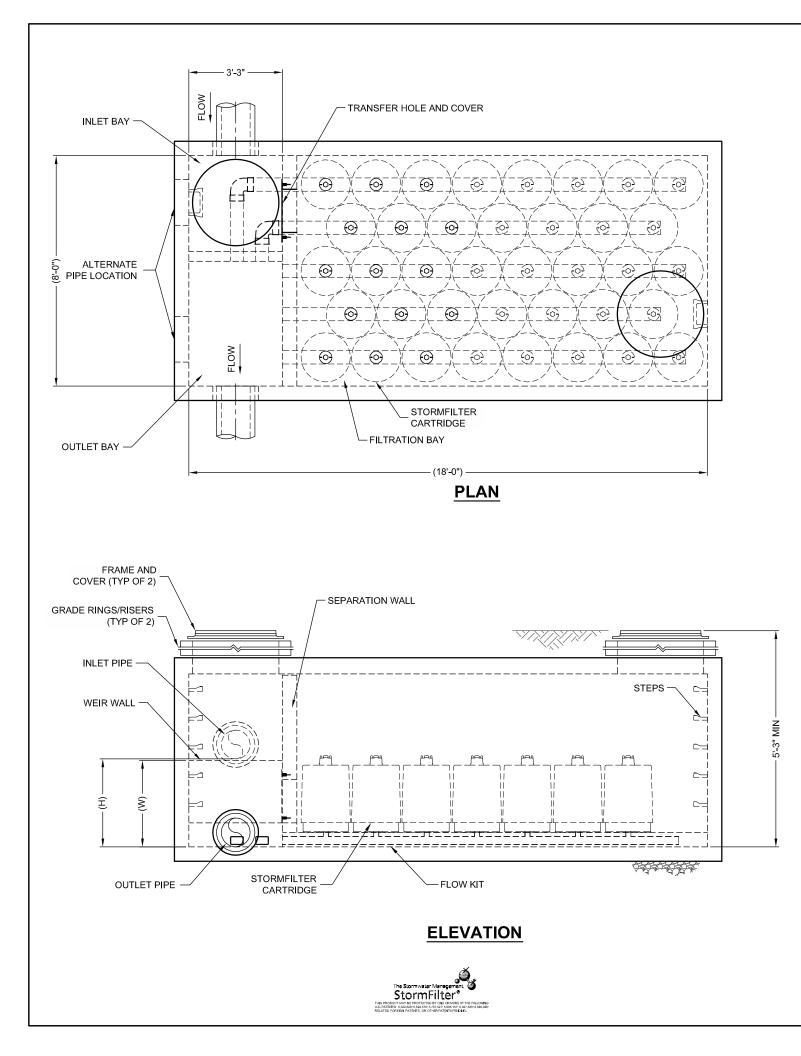
### Maintenance:

Maintenance of Stormwater best management practices is required per the New Jersey Administrative Code 7:8-5.8. Recommendations for maintenance are included in chapters 8 & 9 of the New Jersey Stormwater Best Management Practices Manual. To comply with requirements, CONTECH offers a network of Preferred Service Providers that have the capability to perform all necessary inspections, compliance reporting and cleaning services. CONTECH recommends inspecting the system annually and maintaining the system at the recommendation of the annual inspection. Full maintenance is typically required every 24-36 months. Disposal of material should be handled in accordance with local regulations. Please contact CONTECH's Maintenance Department for all questions regarding maintenance at (503) 258-3157 or visit our website at www.conteches.com/maintenance.

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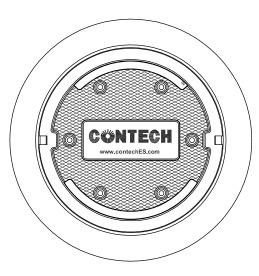
Taylor Murdock Contech Engineered Solutions LLC



# STORMFILTER DESIGN TABLE

- SPECIFIC FLOW RATE. PEAK CONVEYANCE CAPACITY TO BE DETERMINED BY ENGINEER OF RECORD.
- ALL PARTS AND INTERNAL ASSEMBLY PROVIDED BY CONTECH UNLESS OTHERWISE NOTED.

CARTRIDGE HEIGHT	27"		18"		LOW DROP	
SYSTEM HYDRAULIC DROP (H - REQ'D. MIN.)	3.05'		2.3'		1.8'	
HEIGHT OF WEIR (W)	3.00'		2.25'		1.75'	
TREATMENT BY MEDIA SURFACE AREA	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>	2 gpm/ft <sup>2</sup>	1 gpm/ft <sup>2</sup>
CARTRIDGE FLOW RATE (gpm)	22.5	11.25	15	7.5	10	5



FRAME AND COVER (DIAMETER VARIES)

N.T.S.

#### PERFORMANCE SPECIFICATION

FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF CLEANING. RADIAL MEDIA DEPTH SHALL BE 7-INCHES. FILTER MEDIA CONTACT TIME SHALL BE AT LEAST 37 SECONDS. SPECIFIC FLOW RATE SHALL BE 2 GPM/SF (MAXIMUM). SPECIFIC FLOW RATE IS THE MEASURE OF THE FLOW (GPM) DIVIDED BY THE MEDIA SURFACE CONTACT AREA (SF). MEDIA VOLUMETRIC FLOW RATE SHALL BE 6 GPM/CF OF MEDIA (MAXIMUM).

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• THE 8' x 18' PEAK DIVERSION STORMFILTER TREATMENT CAPACITY VARIES BY CARTRIDGE COUNT AND LOCALLY APPROVED SURFACE AREA • THE PEAK DIVERSION STORMFILTER IS AVAILABLE IN A LEFT INLET (AS SHOWN) OR RIGHT INLET CONFIGURATION.

SITE SPECIFIC							
DATA REQUIREMENTS							
STRUCTURE ID *							
WATER QUALITY	FLOW RATI	E (c	fs)		*		
PEAK FLOW RATE	E (cfs)				*		
RETURN PERIOD	OF PEAK F	LOV	V (yrs)		*		
# OF CARTRIDGE	S REQUIRE	D			*		
CARTRIDGE FLOW RATE *							
MEDIA TYPE (CSF, PERLITE, ZPG) *							
PIPE DATA:	I.E.	Ν	MATERIAL DIAMETER				
INLET PIPE	*		* *				
OUTLET PIPE	*		* *				
INLET BAY RIM EI	LEVATION				*		
FILTER BAY RIM E	ELEVATION				*		
ANTI-FLOTATION	BALLAST		WIDTH	$\perp$	HEIGHT		
	* *						
NOTES/SPECIAL REQUIREMENTS:							

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# THE STORMWATER MANAGEMENT STORMFILTER 8' x 18' PEAK DIVERSION STORMFILTER STANDARD DETAIL



# State of New Jersey

DEPARTMENT OF ENVIRONMENTAL PROTECTION Bureau of Nonpoint Pollution Control Division of Water Quality Mail Code 401-02B Post Office Box 420 Trenton, New Jersey 08625-0420 609-633-7021 Fax: 609-777-0432 http://www.state.nj.us/dep/dwq/bnpc\_home.htm

BOB MARTIN Commissioner

December 14, 2016

Derek M. Berg Director - Stormwater Regulatory Management - East Contech Engineered Solutions LLC 71 US Route 1, Suite F Scarborough, ME 04074

Re: MTD Laboratory Certification Stormwater Management StormFilter® (StormFilter) by Contech Engineered Solutions LLC Off-line Installation

# TSS Removal Rate 80%

Dear Mr. Berg:

The Stormwater Management rules under N.J.A.C. 7:8-5.5(b) and 5.7(c) allow the use of manufactured treatment devices (MTDs) for compliance with the design and performance standards at N.J.A.C. 7:8-5 if the pollutant removal rates have been verified by the New Jersey Corporation for Advanced Technology (NJCAT) and have been certified by the New Jersey Department of Environmental Protection (NJDEP). Contech Engineered Solutions LLC has requested a Laboratory Certification for the StormFilter System.

This project falls under the "Procedure for Obtaining Verification of a Stormwater Manufactured Treatment Device from New Jersey Corporation for Advanced Technology" dated January 25, 2013. The applicable protocol is the "New Jersey Department of Environmental Protection Laboratory Protocol to Assess Total Suspended Solids Removal by a Filtration Manufactured Treatment Device" dated January 25, 2013.

NJCAT verification documents submitted to the NJDEP indicate that the requirements of the aforementioned protocol have been met or exceeded. The NJCAT letter also included a recommended certification TSS removal rate and the required maintenance plan. The NJCAT Verification Report with the Verification Appendix for this device is published online at <u>http://www.njcat.org/verificationprocess/technology-verification-database.html</u>.

CHRIS CHRISTIE Governor

KIM GUADAGNO Lt. Governor The NJDEP certifies the use of the StormFilter System by Contech Engineered Solutions LLC at a TSS removal rate of 80%, when designed, operated and maintained in accordance with the information provided in the Verification Appendix and subject to the following conditions:

- The maximum treatment flow rate (MTFR) for the manufactured treatment device (MTD) is calculated using the New Jersey Water Quality Design Storm (1.25 inches in 2 hrs) in N.J.A.C. 7:8-5.5. The MTFR is calculated based on a verified loading rate of 2.12 gpm/sf of effective filtration treatment area.
- 2. The StormFilter System shall be installed using the same configuration as the unit tested by NJCAT, and sized in accordance with the criteria specified in item 6 below.
- 3. This device cannot be used in series with another MTD or a media filter (such as a sand filter), to achieve an enhanced removal rate for total suspended solids (TSS) removal under N.J.A.C. 7:8-5.5.
- 4. Additional design criteria for MTDs can be found in Chapter 9.6 of the New Jersey Stormwater Best Management Practices (NJ Stormwater BMP) Manual which can be found on-line at <u>www.njstormwater.org</u>.
- 5. The maintenance plan for a site using this device shall incorporate, at a minimum, the maintenance requirements for the StormFilter, which is attached to this document. However, it is recommended to review the maintenance website at <a href="http://www.conteches.com/DesktopModules/Bring2mind/DMX/Download.aspx?EntryId=2813">http://www.conteches.com/DesktopModules/Bring2mind/DMX/Download.aspx?EntryId=2813</a> & PortalId=0&DownloadMethod=attachment for any changes to the maintenance requirements.
- 6. Sizing Requirements:

The example below demonstrates the sizing procedure for a StormFilter System.

Example: A 0.25 acre impervious site is to be treated to 80% TSS removal using a StormFilter System. The impervious site runoff (Q) based on the New Jersey Water Quality Design Storm was determined to be 0.79 cfs or 354.58 gpm.

The calculation of the minimum number of cartridges for use in the StormFilter System is based upon both the MTFR and the maximum inflow drainage area. It is necessary to calculate the required cartridges using both methods and to rely on the method that results in the highest minimum number of cartridges determined by the two methods.

Inflow Drainage Area Evaluation:

The drainage area to the StormFilter System in this example is 0.25 acres. Based upon the information in Table 1 below, the following minimum number of cartridges are required in a StormFilter System to treat the impervious area without exceeding the maximum drainage area:

- 1. Five (5) 12" cartridges,
- 2. Three (3) 18" cartridges, or
- 3. Two (2) 27" cartridges

# Maximum Treatment Flow Rate (MTFR) Evaluation:

The site runoff (Q) was determined based on the following: time of concentration = 10 minutes i=3.2 in/hr (page 5-8, Fig. 5-3 of the NJ Stormwater BMP Manual) c=0.99 (runoff coefficient for impervious) Q=ciA=0.99x3.2x0.25=0.79 cfs=0.79x448.83 gpm=354.58 gpm

Based on a flow rate of 354.58 gpm, the following minimum number of cartridges are required in a StormFilter System to treat the impervious area without exceeding the MTFR:

- 1. Thirty-six (36) 12" cartridges,
- 2. Twenty-four (24) 18" cartridges, or
- 3. Sixteen (16) 27" cartridges

The MTFR Evaluation results will be used since that method results in the higher minimum number of cartridges determined by the two methods.

The sizing table corresponding to the available system models are noted below:

TABLE 1 STORMFILTER CARTRIDGE HEIGHTS AND NEW JERSEY TREATMENT CAPACITIES

StormFilter Cartridge Heights and New Jersey Treatment Capacities								
StormFilter Cartridge Height	Filtration Surface Area (sq.ft)	MTFR <sup>1</sup> (GPM)	Mass Capture Capacity (lbs)	Maximum Allowable Inflow Area <sup>2</sup> (acres)				
Low Drop (12")	4.71	10	36.3	0.061				
18"	7.07	15	54.5	0.09				
27"	10.61	22.5	81.8	0.136				

Notes:

1. MTFR calculated based on 4.72x10-3 cfs/sf (2.12 gpm/sf) of effective filtration treatment area.

2. Based upon the equation found in the NJDEP Filter Protocol Maximum Inflow Drainage Area (acres) = weight of TSS before 10% loss in MTFR (lbs)/600 lbs/acre of drainage area annually.

Be advised a detailed maintenance plan is mandatory for any project with a Stormwater BMP subject to the Stormwater Management Rules, N.J.A.C. 7:8. The plan must include all of the items identified in Stormwater Management Rules, N.J.A.C. 7:8-5.8. Such items include, but are not limited to, the list of

indication of problems in the system, and training of maintenance personnel. Additional information can be found in Chapter 8: Maintenance and Retrofit of Stormwater Management Measures.

If you have any questions regarding the above information, please contact Shashi Nayak of my office at (609) 633-7021.

Sincerely,

James J. Murphy, Chief Bureau of Nonpoint Pollution Control

Attachment: Maintenance Plan

cc: Chron File

Richard Magee, NJCAT Vince Mazzei, NJDEP - DLUR Ravi Patraju, NJDEP - BES Gabriel Mahon, NJDEP - BNPC Shashi Nayak, NJDEP - BNPC

